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## ▶ To cite this version:

Jiayu Wang, Stéphanie Deboeuf, Arnaud Antkowiak, Sébastien Neukirch. Constrained Euler Buckling: the von Karman approximation. 2024. hal-04839683

## HAL Id: hal-04839683 https://hal.science/hal-04839683v1

Preprint submitted on 16 Dec 2024

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## Constrained Euler Buckling: the von Karman approximation

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#### Abstract

We consider the classical problem of the buckling of a planar elastica inside a rectangular cavity. We compute the equilibrium solutions analytically in the (von Karman) small deflection approximation. We list the different equilibrium states and their domain of validity in terms of the imposed horizontal  $\Delta$  and vertical *H* displacements. We compute the horizontal *P* and the vertical *F* applied forces and show how they increase and scale when the compaction ratio  $\sqrt{\Delta}/H$  is increased. Finally, we introduce an approximate response state, where the system adopts a periodic configuration with a noninteger number of repeated folds. This solution represents an average response of the system and brings information on its global behaviour.

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Keywords: elastic rods, bifurcation diagram, contact, packing

## 1 1. Introduction

The buckling of elastic beams is a classical subject, especially in the planar case (Euler, 1744), (Thomson 3 and Tait, 1883, section 611, p. 148), (Goss, 2009), 4 (Levien, 2009, Chap. 5). Analytical solutions are known 5 for simple (Love, 1944; Bigoni, 2012) and less simple 6 (Djondjorov et al., 2011) cases. The global buckling 7 behavior is nevertheless often studied numerically, and 8 the goal is to achieve a thorough understanding of the 9 phase diagram of the problem (Domokos, 1994) which 10 in some cases may be cluttered (Holmes et al., 1999, 11 Fig. 7), (Domokos and Healey, 2005, Fig. 3), (Hen-12 derson and Neukirch, 2004, Fig. 16), (Coleman and 13 Swigon, 2004, Fig. 4). The equilibrium solutions may 14 be computed either by direct resolution of the equi-15 librium equations (boundary value problem - strong 16 form approach), finite element method (weak form ap-17 proach), or minimization of the elastic energy (Char-18 rondière et al., 2024). 19

The writhing and coiling of elastic rods in cavities has numerous applications. Examples comprise the helical buckling of tubings in the drilling industry (Lubinski and Althouse, 1962; Miller et al., 2015), the interaction of endoscopes and arteries in vascular surgery, the stuff box crimpling in the textile industry (Hearle,

Preprint submitted to Int. J. Sol. Struct.

2014), DNA viral capsids (Vetter et al., 2014), spider threads in liquid drops (Elettro et al., 2016); see other examples in Judah and Givli (2024). However, the buckling of elastic beams in cavities brings the complication of inequality constraints (Villaggio, 1979) (arising from the contact condition) and their non-classical features in the bifurcation diagram (for example, corner points (Schulz and Pellegrino, 2000)). Their numerical treatment involves more sophisticated approaches, among which are linear and nonlinear complementary problems (Daviet et al., 2011), interior point methods (Wachter and Biegler, 2006), or barrier potentials (Manning and Bulman, 2005).

The present study considers the simpler case of a planar beam buckled in a rectangular cavity. To our knowledge, this problem was first addressed by Chateau and Nguyen (1991), then Adan et al. (1994) considered the case where the constraining surface has imperfections, Domokos et al. (1997) studied pinned-pinned boundary conditions, and Pocheau and Roman (2004) unveiled the presence of multiple solutions and their self-similar aspect, as well as tilted solutions (Roman and Pocheau, 1999, 2002). The effect of the thickness of the beam was studied by Chai (1998), while Tzokova (2020) combined Abagus FEM and experiments on beams and plates, and Katz and Givli (2015) considered the case of springy walls, see also (Judah and Givli, 2024). Finally, the influence of shear deformations in the section of the beam has been studied by Bosten et al. (2023) where this contact problem is used as a benchmark scenario to

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56 test finite element simulations.

Here we use what is generally called the von Kar-57 man approximation (Woinowsky-Krieger, 1950; Erin-58 gen, 1952; Thomas, 2025). In this approximation, the 59 moment balance is linearized, and a von Kármán-type 60 axial strain measure, first introduced for the buckling 61 of elastic plates (von Kármán, 1907; Eisley, 1964), is 62 used. This model has been widely used in the litera-63 ture and has shown its efficiency in computing approxi-64 mate solutions for the equilibrium (Bazant and Cedolin, 65 2010, Section 1.9), (Neukirch et al., 2021) and dynam-66 ics (Lacarbonara and Yabuno, 2006; Pandey et al., 2014; 67 Thomas et al., 2016) of elastic rods in the weakly non-68 linear regime. It turns out that Kirchhoff himself intro-69 duced this approximated model in his book, see Kirch-70 hoff (1876, Eq. (16), p. 441). We show in this paper that 71 constrained Euler buckling can be studied and partially 72 understood with this von-Karman kinematics approxi-73 mation; see also (Chai, 1998; Judah and Givli, 2024). 74

75 Paper contributions

In the case of small vertical (H) and horizontal ( $\Delta$ ) displacements:

We present an analytical study of the Euler buck ling problem with contact constraints.

• We give closed-form formulas for the vertical Fand horizontal P forces and the number of folds as functions of H and  $\Delta$ , and identify the scaling  $F H \sim P \Delta$ .

• We show that a reduced kinematic loading parameter  $\sqrt{\Delta}/H$  can be used to rationalize the bifurcation curves.

 We introduce a cellular model, where the number of folds is a noninteger, that provides an all-in-one view of the response of the system.

## 90 2. Problem setup

#### 91 2.1. The planar Elastica

We consider an inextensible, unshearable, elastic beam buckled inside a planar, rectangular cavity. The beam has total length *L*, bending rigidity *YI*, and is horizontally clamped at both ends. The shape of the beam is studied parametrically as (x(s), y(s)), where *s* is the arc length along the beam. We introduce the angle  $\theta(s)$ between the tangent to the beam and the horizontal axis  $e_x$ , see Figure 1. As the beam is considered inextensible and unshearable, we have

$$x'(s) = \cos \theta(s)$$
,  $y'(s) = \sin \theta(s)$  (1)

where  $(\cdot)' = d(\cdot)/ds$ . We consider clamped boundary conditions

$$x(0) = 0$$
  $y(0) = 0$   $\theta(0) = 0$  (2a)

$$x(L) = L(1 - \Delta)$$
  $y(L) = 0$   $\theta(L) = 0$  (2b)

where  $\Delta$  is the (dimensionless) end-shortening, ranging from  $\Delta = 0$  at buckling to  $\Delta = 1$  when the two ends meet and the elastica adopts a Lemniscate-like shape (Goss, 2009). Additionally, as the beam is constrained to stay in a cavity of height *H*, we have the inequality constraint

$$\forall s, \ 0 \le y(s) \le H \tag{3}$$

The elastic deformation energy only involves the curvature  $\kappa(s)$ :

$$E_{\kappa} = \int_0^L \frac{1}{2} YI \kappa^2(s) \,\mathrm{d}s \text{ where } \kappa(s) = \theta'(s) \quad (4)$$

This quadratic deformation energy is associated with the linear constitutive relation relating the internal moment m(s) and the curvature

$$m(s) = YI\kappa(s) \tag{5}$$

We look for equilibrium solutions of the system, that is stationary points of this energy under constraints (2) and (3). The equilibrium condition on the internal moment reads

$$m'(s) = n_x(s) \sin \theta(s) - n_y(s) \cos \theta(s)$$
(6)

As we study the frictionless case, the horizontal component  $n_x(s)$  of the internal force is uniform across the system and we note  $P = -n_x(s)$ . The vertical component  $n_y(s)$  is uniform in each free rod section and jumps every time the rod contacts the lower or the upper wall. Please also note that the moment m(s) and hence the curvature  $\kappa(s)$  do not experience any jump at contact points and hence are continuous along the entire rod (Bigoni, 2012). We will note *F* the total (vertical) force applied by the upper wall down to the rod, see Figure 1. In this frictionless-contact case, the Hamiltonian invariant (Dichmann et al., 1996; Kehrbaum and Maddocks, 1997)

$$I_{\rm nv} = \frac{1}{2} YI \kappa^2(s) - P \cos \theta(s) + n_y(s) \sin \theta(s)$$
(7)

takes the same value in the entire system.

#### 2.2. The von Karman approximation

In the limit where the rod is only slightly bent, the deflection  $\theta(s)$  stays small and we use the first terms in

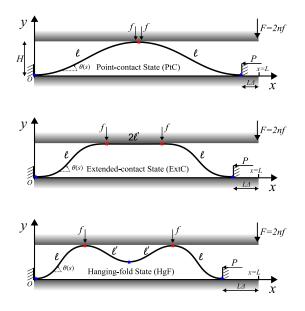


Figure 1: Planar elastica bent in a cavity. The left end is horizontally clamped at the origin, while the right end is horizontally clamped on a block constrained to slide along the horizontal axis. The elastica 106 has a total length L, and the angle  $\theta$  is defined between the tangent to the beam and the horizontal axis. An external force is applied to the 107 system at s = L, and we note its horizontal component -P.

the Taylor expansion of the sin and cos functions. The equilibrium equations (1), (5) and (6) simplify to

$$x'(s) = 1 - (1/2)\theta^2(s)$$
 (8a) 113

$$y'(s) = \theta(s) \tag{8b}$$
<sup>114</sup>

$$YI \theta''(s) = -P \theta(s) - n_y(s)$$
 (8c) <sup>115</sup>

Note that the only nonlinear remaining term is in (8a), 94 see Thomas (2025) for more details. 95

#### 2.3. Non-dimensionalization 96

In this equilibrium problem, we introduce nondimensionalized quantities

$$\hat{s} = s/L, \quad \hat{x} = x/L, \quad \hat{y} = y/L, \quad \hat{H} = H/L, \quad \hat{\theta} = \theta$$
(9a)

$$\hat{P} = \frac{PL^2}{YI}, \quad \hat{F} = \frac{FL^2}{YI}, \quad \hat{m} = \frac{mL}{YI}, \quad \hat{\kappa} = \kappa L$$
(9b)

Note that it boils down to choosing L as length unit and 97  $YI/L^2$  as unit force, as can be done in most equilib-98 rium problems of elastic rods. Please also note the non-99 dimensionalized version of the equations can be readily 100 obtained by formally setting L = 1 and YI = 1 in the 101 equations of Sections 2.1 and 2.2. 102

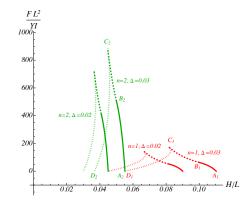


Figure 2: The total vertical force F applied on the rod from the upper wall as a function of the height of the enclosing cavity. For  $\Delta$  = 0.02 and 0.03 and n = 1 and n = 2. Legend: Point-Contact state (continuous line), Extended-Contact state (dashed line, only k = 1), and Hanging-Fold state (dotted line).

Anticipating the results of section 4, we plot in Figure 2 a typical bifurcation diagram where the vertical force  $\hat{F} = \frac{\hat{F}L^2}{YI}$  is plotted as a function of the height  $\hat{H} = H/L$ of the cavity. We see that even if non-dimensionalized quantities have been used to draw the diagram, the different curves still depend on both the Atlas number n (to be defined in Section 4) and the end shortening  $\Delta$ . See also Figure 3 for the dual experiment where the endshortening  $\Delta$  is varied while keeping the height  $\hat{H}$  fixed. In this experiment, we find a dependence of the curves on both *n* and  $\hat{H}$ . One of the goal of the present paper is to introduce rescaled quantities on the axes that induce partial collapses of the curves of Figures 2 and 3. Moreover, we will show that the important loading measure is  $\sqrt{\Delta}/\hat{H}$ .

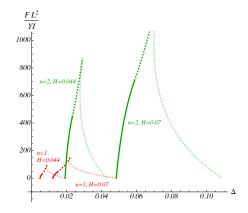


Figure 3: The total vertical force F applied on the rod from the upper wall as a function of the end-shortening  $\Delta$ . For H = 0.044 and 0.07 and n = 1 and n = 2. Legend: Point-Contact state (continuous line), Extended-Contact state (dashed line, only k = 1), and Hanging-Fold state (dotted line).

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To keep notations simple, we will drop the hats for the non-dimensionalized quantities in the remainder of the paper.

#### **3.** The Arch solution

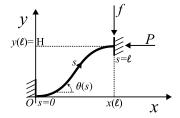


Figure 4: The arch solution is the building block with which we compute the Point-Contact, Extended-Contact, and Hanging-Fold states presented in Section 4, as well as the cellular model presented in Section 5. This solution is in contact with both the lower and upper walls, has length  $\ell$ , and inner force  $n_x = -P$  and  $n_y = -f$ .

We will describe the different solution types in Section 4. They are all based on the following fundamental equilibrium solution, which we denote as the 'Arch' solution, see Figure 4 and (Chai, 1998; Judah and Givli, 2024). This solution has boundary conditions (2a) at s = 0. Its length is  $\ell$  and the boundary conditions at  $s = \ell$  are

$$y(\ell) = H$$
 and  $\theta(\ell) = 0$  (10)

Hence the boundary value problem to solve is system (8) with  $n_y(s) \equiv -f$  together with (2a) and (10). The solution is

$$m(s) = \frac{f}{\sqrt{P}} \left( \sin \sqrt{P}s - \tan \frac{\sqrt{P}\ell}{2} \cos \sqrt{P}s \right)$$
(11a)

$$\theta(s) = \frac{f}{P} \left( 1 - \cos \sqrt{Ps} - \tan \frac{\sqrt{P\ell}}{2} \sin \sqrt{Ps} \right) \quad (11b)$$

$$y(s) = \frac{f}{P\sqrt{P}} \left( \sqrt{Ps} - \sin\sqrt{Ps} + \tan\frac{\sqrt{P\ell}}{2} \left[ \cos\sqrt{Ps} - 1 \right] \right]$$
(11c)

where  $\pi \leq \sqrt{P\ell} \leq 2\pi$ . In the following, we will need the two geometrical quantities *H* and  $x(\ell)$ , as well as the

energy  $E_k$ , which are here computed as

$$H = y(\ell) = \frac{f}{P\sqrt{P}} \left( \sqrt{P}\ell - 2\tan\frac{\sqrt{P}\ell}{2} \right)$$
(12a)  
$$x(\ell) = \int_0^\ell 1 - (1/2)\,\theta^2(s)\,\mathrm{d}s$$
$$= \ell - f^2 \frac{\sqrt{P}\ell \left[ 2 + \cos\sqrt{P}\ell \right] - 3\sin\sqrt{P}\ell}{2P^2\sqrt{P}\left(\cos\sqrt{P}\ell + 1\right)}$$
(12b)  
$$E_\kappa = \frac{1}{2} \int_0^\ell m^2(s)\,\mathrm{d}s = \frac{f^2}{2P\sqrt{P}} \frac{\sqrt{P}\ell - \sin\sqrt{P}\ell}{1 + \cos\sqrt{P}\ell}$$

Please note that, by symmetry, the moment at s = 0 and  $s = \ell$  have the same magnitude and opposite sign

$$m(\ell) = -m(0) = \frac{f}{\sqrt{P}} \tan \frac{\sqrt{P}\ell}{2}$$
(13)

(12c)

### 4. The different equilibrium states

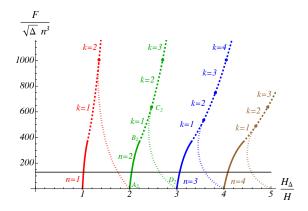


Figure 5: Vertical force *F* plotted as a function of the compaction ratio  $H_{\Delta}/H$ . This plot does not depend on the value of  $\Delta$ . The threshold  $H_{\Delta}$  is defined in Eq. (14). The black line corresponds to the continuous *n* model presented in Section 5. Legend: Point-Contact state (continuous line), Extended-Contact state (dashed line), and Hanging-Fold state (dotted line). We show Extended-Contact states where the flat region is fragmented into *k* pieces. For n = 2, we show the limit points between the different states.

In this section, we describe three different fundamental states the system visits as either *H* is lowered or  $\Delta$  is increased (Domokos et al., 1997; Roman and Pocheau, 2002). For large values of *H* or small values of  $\Delta$ , the elastica buckles when the horizontal force *P* reaches the threshold  $P = 4\pi^2$  (Euler, 1744). Contact with the upper wall does not happen in this first regime where, in the present von Karman approximation, *P* stays constant

 $P = 4\pi^2$  while *H* is lowered or  $\Delta$  increased. Eventually, contact first happens when *H* and  $\Delta$  are such that (Bigoni, 2012; Neukirch et al., 2021) 141

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We define the *Atlas* number *n* as half the number of rod segments spreading out from y = 0 to y = H (i.e. touching both walls), such as the Arch of Section 3. In the following section, we first describe the solution for n = 1 and then generalize formulas for any *n* value. This Atlas number has been called the number of folds in previous works.

## 129 4.1. Point-Contact states

We first describe configurations in which the elastica
contacts the upper wall only at isolated point(s), see Figure 1. We call these solutions *Point-Contact states*.

When n = 1, we have  $\ell = 1/2$ , the total force from the upper wall *F* takes the value F = 2f and the end shortening is  $\Delta = 1 - 2x(\ell)$ . From the Arch solution, Eqs. (12) and (13), we have

$$H = \frac{F}{4P\sqrt{P}} \left(\sqrt{P} - 4\tan\frac{\sqrt{P}}{4}\right)$$
(15a)

$$\Delta = F^2 \frac{\sqrt{P} \left[ 2 + \cos(\sqrt{P}/2) \right] - 6\sin(\sqrt{P}/2)}{8P^2 \sqrt{P} \left[ \cos(\sqrt{P}/2) + 1 \right]}$$
(15b)

$$m(\ell) = -m(0) = \frac{F}{2\sqrt{P}} \tan \frac{\sqrt{P}}{4}$$
(15c)

This branch of solutions starts at point  $A_1$  (see Figure 149 2) when  $P = 4\pi^2$ , F = 0,  $H = H_{\Delta}$  and ends with 150 point  $B_1$  when  $P = 16\pi^2$ ,  $F = 3^{-1/2} 64 \pi^2 \sqrt{\Delta}$ , and 151  $H = 3^{-1/2} \sqrt{\Delta}$ .

These formulas generalize directly to the case with <sup>153</sup>  $n \ge 1$  by setting  $\ell = 1/(2n)$ , F = 2nf, and  $\Delta = ^{154}$  $1 - 2nx(\ell)$ . Using Eqs. (12) and (13) from the Arch <sup>155</sup> solution, we find <sup>156</sup>

$$H_{\rm u} = \frac{F_{\rm u}}{4P_{\rm u}\sqrt{P_{\rm u}}} \left(\sqrt{P_{\rm u}} - 4\tan\frac{\sqrt{P_{\rm u}}}{4}\right) \tag{16a}$$

$$\Delta = F_{u}^{2} \frac{\sqrt{P_{u}} \left[ 2 + \cos(\sqrt{P_{u}}/2) \right] - 6\sin(\sqrt{P_{u}}/2)}{8P_{u}^{2} \sqrt{P_{u}} \left[ \cos(\sqrt{P_{u}}/2) + 1 \right]}$$
(16b)

$$\frac{m(\ell)}{n} = -\frac{m(0)}{n} = \frac{F_{\rm u}}{2\sqrt{P_{\rm u}}} \tan \frac{\sqrt{P_{\rm u}}}{4}$$
(16c)

with  $H_u = nH$ ,  $P_u = P/n^2$ , and  $F_u = F/n^3$ . These branches of solutions each start at point  $A_n$  when  $P_u =$   $4\pi^2$ ,  $F_u = 0$ ,  $H_u = H_{\Delta}$  and end at point  $B_n$  when  $P_u = 16\pi^2$ ,  $F_u = 3^{-1/2} 64 \pi^2 \sqrt{\Delta}$ , and  $H_u = 3^{-1/2} \sqrt{\Delta}$ . Using (16a) et (16b), one can write a single equation for  $P_u$  having the form  $\Delta/H_u^2 = \phi(P_u)$ . For  $4\pi^2 \le P_u \le 16\pi^2$ , the function  $\phi(\cdot)$  is monotonously increasing, taking values from  $(\pi/2)^2$  to 3, which means that, for all *n*, there is always a unique Point-Contact solution.

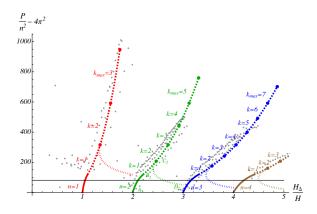


Figure 6: Horizontal force *P* plotted as a function of the compaction ratio  $H_{\Delta}/H$ . This plot does not depend on the value of  $\Delta$ . The contact threshold  $H_{\Delta}$  is defined in Eq. (14). The black line corresponds to the continuous *n* model presented in Section 5. Legend: Point-Contact state (continuous line), Extended-Contact state (dashed line), and Hanging-Fold state (dotted line). We show Extended-Contact states where the flat region is fragmented into *k* pieces. For n = 2, we show the limit points between the different states. Gray dots correspond to experimental data from Figure 5 of Deboeuf et al. (2024).

For this point-contact state, the vertical force F and the horizontal force P are plotted in plain line against the height H of the cavity for different values of the end shortening  $\Delta$  in Figures 5 and 6. We remark that, contrary to Figure 2, the rescaling with  $H_{\Delta}$  induces a collapse of the bifurcation curves, which now do not depend on the value of  $\Delta$ . Furthermore, we show in Figures 7 and 8 (continuous line) that, using the Atlas number n, the Point-Contact branches all collapse on a single curve, for any  $\Delta$  and any n. We also plot in Figures 6 and 8 the experimental data from Figure 5 of Deboeuf et al. (2024), where  $\Delta < 0.16$ ,  $0.042 \le H \le 0.085$ , and  $1 \le n \le 4$ . In these data, friction tends to increase the measured horizontal force P which otherwise is in good agreement with the present theory.

As the first contact occurs, when  $H = H_{\Delta}$ , the vertical force *F* increases from F = 0. This is repeated for each *n* as we have F = 0 each time  $nH = H_u = H_{\Delta}$ . At this point, we compute the slope of the function F = F(H)by performing a Taylor expansion of Eqs. (16a) and (16b) for  $H_u$  near  $H_{\Delta}$  and find

$$F_{\rm u} = \frac{16\,\pi^4}{10 - \pi^2} \,\left[H_{\Delta} - H_{\rm u}\right] + \dots \tag{17}$$

The large value of the prefactor (nearly  $1.2 \cdot 10^4$ ) shows 161

that the vertical force steeply increases as one com-162

presses the system vertically. 163

> At  $P_{\rm u} = 16\pi^2$ , when the Point-Contact branch ends, the deflection angle at the inflexion point  $s = \ell/2$  takes the value

$$\theta_{\max} = \frac{F_u}{P_u} = \frac{4}{\sqrt{3}} \sqrt{\Delta}$$
(18)

This maximum value yields an upper bound for  $\Delta$  if we 164

require  $\theta_{\text{max}}$  to stay small, e.g.  $\theta_{\text{max}} < 0.5$  implies  $\Delta <$ 165 179 0.06. 166

#### 4.2. Extented-Contact states 167

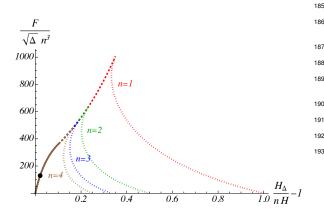


Figure 7: Vertical force F plotted as a function of the rescaled quantity  $H_{\Delta}/(nH) - 1$ . For the Point-Contact and Extended-Contact states the plot does not depend on the Atlas number n. The entire plot does not depend on the value of  $\Delta$ . The single black point at (0.02, 130) corresponds to the response of the cellular model. Legend: Point-Contact state (continuous line), Extended-Contact state (dashed line, only k = 1), and Hanging-Fold state (dotted line).

In Figure 2, as the vertical force F is increased above 168 point *B*, the system exhibits a region where the contact 169 with the upper (or lower) wall is flat; We call these so-170 lutions Extended-Contact states, see Figure 1. For these 171 solutions, both the deflection angle  $\theta(s)$  and the moment 172 m(s) at the start of the flat region vanishes, and conse-173 quently the Hamiltonian invariant  $I_{nv} = -P$ , see Eq. (7). 174 It has been explained that the total length  $2\ell'$  of flat con-175 tact may be distributed within several sections of the 176 system with no change of its energy  $E_{\kappa}$  (Pocheau and 177 Roman, 2004). 178

When n = 1, we have  $2\ell + 2\ell' = 1$ . The value of  $\ell$  is fixed through the condition  $m(\ell) = 0$  which yields  $\ell = 2\pi / \sqrt{P}$ , see (13). Using the Arch solution (12) with

F = 2f, we find

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$$H = \frac{\pi F}{P\sqrt{P}} \tag{19a}$$

$$\Delta = 1 - [2x(\ell) + 2\ell'] = \frac{3\pi}{4} \frac{F^2}{P^2 \sqrt{P}}$$
(19b)

$$m(\ell) = m(0) = 0$$
 (19c)

This branch of solutions starts at point  $B_1$  (see Figure 2) when  $\ell' = 0$ ,  $P = 16\pi^2$ ,  $F = 3^{-1/2} 64 \pi^2 \sqrt{\Delta}$ , and  $H = 3^{-1/2} \sqrt{\Delta}$  and ends when the flat region buckles, that is when  $P\ell'^2 = \pi^2$  (buckling threshold of a clamped-clamped beam of length  $2\ell'$ ). As we do not know whether the flat region stands in one piece or is fragmented in k pieces, we can only give lower and upper bounds on the end of the branch, as follows.

If the flat region stands in one piece, k = 1, the branch ends at its lower bound, point  $C_1$ , when  $\ell' = 1/6$ , P = $36\pi^2$ ,  $F = 72\sqrt{2}\pi^2\sqrt{\Delta}$ , and  $H = (\sqrt{2}/3)\sqrt{\Delta}$ .

In this n = 1 case, the flat region can be, at most, fragmented into three pieces. This yields the upper bound for the end of the branch with  $\ell' = 3/10$ ,  $P = 100\pi^2$ ,  $F = 100 \sqrt{40/3} \pi^2 \sqrt{\Delta}$ , and  $H = \sqrt{2/15} \sqrt{\Delta}$ .

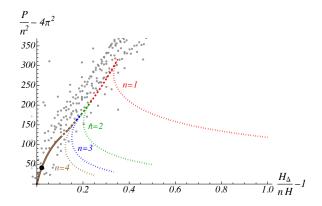


Figure 8: Horizontal force P plotted as a function of the rescaled compaction ratio  $H_{\Delta}/H$ . For the Point-Contact and Extended-Contact states the plot does not depend on the Atlas number n. The entire plot does not depend on the value of  $\Delta$ . The single black point at (0.02, 41) corresponds to the response of the cellular model. Legend: Point-Contact state (continuous line), Extended-Contact state (dashed line, only k = 1), and Hanging-Fold state (dotted line). Gray dots correspond to experimental data from Figure 5 of Deboeuf et al. (2024).

When  $n \ge 1$ , we have  $2n\ell + 2\ell' = 1$ , F = 2nf, and the condition  $m(\ell) = 0$  still yields  $\ell = 2\pi/\sqrt{P}$ . As in Section 4.1, formulas (19) are directly generalized to

$$H_{\rm u} = \frac{\pi F_{\rm u}}{P_{\rm u} \sqrt{P_{\rm u}}} \tag{20a}$$

$$\Delta = 1 - [2nx(\ell) + 2\ell'] = \frac{3\pi}{4} \frac{F_{\rm u}^2}{P_{\rm u}^2 \sqrt{P_{\rm u}}}$$
(20b)

$$m(\ell) = m(0) = 0$$
 (20c)

This rescaling of the Extended-Contact solution for any *n* and any  $\Delta$  is illustrated in Figures 7 and 8 (dashed line) where we see that all the  $(n \ge 1)$  Extended-Contact solution branches collapse. This branch of solutions starts at point  $B_n$  when  $\ell' = 0$ ,  $P_u = 16\pi^2$ ,  $F_{\rm u} = 3^{-1/2} \, 64 \, \pi^2 \, \sqrt{\Delta}$ , and  $H_{\rm u} = 3^{-1/2} \, \sqrt{\Delta}$  and ends when <sup>211</sup> the flat region buckles. As the flat region could be frag- 212 mented into k pieces (with k = 1 to k = 2n + 1), the <sup>213</sup> branch of solutions ends when

$$\ell' = k/(4n+2k)$$
 and  $\ell = 1/(2n+k)$  (21a)

$$P_{\rm u} = 4\pi^2 \left(\frac{2n+k}{n}\right)^2 \tag{21b}$$

$$F_{\rm u} = 8\pi^2 \ \sqrt{\Delta} \ \sqrt{2/3} \left(\frac{2n+k}{n}\right)^{5/2}$$
 (21c)

$$H_{\rm u} = \sqrt{\Delta} \sqrt{2/3} \sqrt{\frac{n}{2n+k}}$$
(21d)

which means that for large n and with a flat region 194 fragmented into a maximum number of pieces (k = k)195 2n + 1) the vertical force can take values up to  $F \simeq$ 196  $2000 n^3 \sqrt{\Delta} \simeq 140 \Delta^2/H^3$ , and the horizontal force val-197 ues up to  $P \simeq 600 n^2 \simeq 100 \Delta/H^2$ . 198

#### 4.3. Hanging-fold states 199

Along the Extended-Contact solution branch, when 200 the flat region is in one piece, k = 1, a new branch bifur-<sup>215</sup> 201 cates as the compression force P exceeds the buckling 216 202 threshold given in (21b). A hanging fold is created, re- 217 203 placing the flat region, see Figure 1. We call these con- 218 204 figurations Hanging-Fold states. Please note that a con- 219 205 figuration having a fold and a flat region is not possible, 220 206 as it would imply two different values of the Hamilto- 221 207 nian invariant  $I_{nv}$ : the invariant value for solutions with 222 208 a flat region is  $I_{nv} = -P$  and the invariant value for so- 223 209 lutions with a fold is  $I_{nv} = -P + (1/2)\kappa^2(\ell) > -P$ . 224 210

When n = 1, we have  $2\ell + 2\ell' = 1$  where the value 225 of  $\ell'$  is fixed by Eq. (A.1) in Appendix A. We have 226 F = 2f and  $\Delta = 1 - 2 [x(\ell) + x(\ell')]$  where  $x(\ell)$  is given 227 by Eq. (12b) and  $x(\ell')$  by Eqs.(A.2) and (A.3) in Ap-228 pendix A. Further using (12a) for the height H, we find 229

$$H = \frac{F}{2P\sqrt{P}} \left( \sqrt{P}\ell - 2\tan\frac{\sqrt{P}\ell}{2} \right)$$
(22a)  
$$\Delta = F^2 \left\{ \frac{\sqrt{P}\ell \left[ 2 + \cos\sqrt{P}\ell \right] - 3\sin\sqrt{P}\ell}{4P^2\sqrt{P}\left(\cos\sqrt{P}\ell + 1\right)} + \frac{1/2 - \ell}{8P^2} \frac{1 - \cos\sqrt{P}\ell}{1 + \cos\sqrt{P}\ell} \right\}$$
(22b)  
$$P = \left( -\frac{\pi}{2} \right)^2$$
(22c)

(22C)  $r = \left(\frac{1}{2 - \ell}\right)$ 

This branch starts at point  $C_1$  (see Figure 2) when  $\ell =$  $1/3, \ell' = 1/6, P = 36\pi^2, F = 72\sqrt{2}\pi^2\sqrt{\Delta}, \text{ and } H =$  $(\sqrt{2}/3)\sqrt{\Delta}$  and ends at point  $D_1$  when  $\ell = 1/4$ ,  $\ell' =$ 1/4,  $P = 16\pi^2$ , F = 0, and  $H = H_{\Delta}/2$ .

When  $n \ge 1$ , we have  $2n\ell + 2\ell' = 1$  where the value of  $\ell'$  is still given by Eq. (A.1) in Appendix A. We have F = 2nf and  $\Delta = 1 - 2 [nx(\ell) + x(\ell')]$  where  $x(\ell)$  is given by Eq. (12b) and  $x(\ell')$  by Eqs. (A.2) and (A.3) in Appendix A. As in Section 4.1 and 4.2, we use  $H_{\rm u} = n H$ ,  $P_{\rm u} = P/n^2$ , and  $F_{\rm u} = F/n^3$  to write the generalization of Eqs. (22) to any  $n \ge 1$  value

$$H_{\rm u} = \frac{F_{\rm u}}{2P_{\rm u}\sqrt{P_{\rm u}}} \left(\sqrt{P_{\rm u}}\ell_{\rm u} - 2\tan\frac{\sqrt{P_{\rm u}}\ell_{\rm u}}{2}\right)$$
(23a)  
$$\Delta = F_{\rm u}^2 \left\{\frac{\sqrt{P_{\rm u}}\ell_{\rm u}\left[2 + \cos\sqrt{P_{\rm u}}\ell_{\rm u}\right] - 3\sin\sqrt{P_{\rm u}}\ell_{\rm u}}{4P_{\rm u}^2\sqrt{P_{\rm u}}\left(\cos\sqrt{P_{\rm u}}\ell_{\rm u} + 1\right)} + \frac{1/2 - \ell_{\rm u}}{8P_{\rm u}^2}\frac{1 - \cos\sqrt{P_{\rm u}}\ell_{\rm u}}{1 + \cos\sqrt{P_{\rm u}}\ell_{\rm u}}\right\}$$
(23b)

$$P_{\rm u} = \frac{1}{n^2} \left( \frac{\pi}{1/2 - \ell_{\rm u}} \right)^2 \tag{23c}$$

where we have introduced  $\ell_u = n\ell$ . The explicit presence of the Atlas number n in Eq. (23c) implies that, contrary to what was found in Sections (4.1) and (4.2), there is no for-all-*n* collapse of the solution branches for the Hanging-Fold states, as can be seen in Figures 7 and 8. The  $n \ge 1$  branches start when  $\ell = 1/(1 + 2n)$ ,  $\ell' = 1/(2+4n), P = 4\pi^2(1+2n)^2, F = 8\pi^2 \sqrt{\Delta} \sqrt{2/3} (1+2n)^2$  $(2n)^{5/2} \sqrt{n}$ , and  $H = \sqrt{2\Delta} / \sqrt{3n(1+2n)}$  and ends when  $\ell = \ell' = 1/(2+2n), P = 4\pi^2(1+n)^2, F = 0$ , and  $H = H_{\Delta}/(1+n).$ 

We recall that the description of the sequence of the different states we have made in this section 4 holds both when H is constant and  $\Delta$  increased or when  $\Delta$  is constant and H decreased: The important quantity defining the state of the system is the compaction ratio  $H_{\Delta}/H$ .

# 5. The continuous *n* approximation: The cellular model

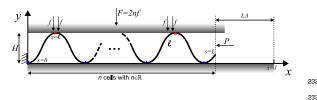


Figure 9: In the cellular model, the system is the repetition of the Arch solution with  $L = 2n\ell$ . As the Atlas number *n* takes non-integer values, the boundary condition at s = L is unsatisfied. The importance of this defect decreases as *n* gets large and is in fact already minor for small *n* values.

As the compaction ratio  $H_{\Delta}/H = 2\sqrt{\Delta}/(\pi H)$  is increased, the system repeatedly visits the Point-Contact state with an increasing Atlas number *n*. When *n* becomes large, the precise boundary conditions at the rod extremities become less important, that is the integer character of *n* is no longer critical and one could approximate the system's response with a model where *n* is a non-integer quantity, see Figure 9. We look for the equilibrium of such a cellular system by minimizing the total potential energy

$$E_{\text{tot}} = 2nE_{\kappa} + 2nx(\ell)P + y(\ell)F \qquad (24)$$

where we work in the dead loading case (i.e. for prescribed *P* and *F*) as it yields simpler calculations than the (equivalent) rigid loading case (i.e. for prescribed  $\Delta$ and *H*). We use Eqs. (12) with  $2n\ell = 1$  and F = 2nf, and find

$$E_{\text{tot}}(z) = P + \frac{F^2}{2P^2} z \left[ \mu_{\kappa}(z) - \mu_{\chi}(z) + \mu_{\chi}(z) \right] \quad (25a)^{244}$$

with 
$$\mu_{\kappa}(z) = \frac{z - \sin z}{1 + \cos z}$$
 (25b)

$$\mu_x(z) = \frac{z (2 + \cos z) - 3 \sin z}{1 + \cos z}$$
(25c)

$$\mu_{y}(z) = 2z - 4 \frac{\sin z}{1 + \cos z}$$
(25d)

with  $z = \sqrt{P} \ell$ . The minimum is reached for  $z = z_e$  such that  $z_e = \tan z_e$ , that is  $z_e \simeq 4.5$ . For this solution, we

have  $\mu_e = \mu_x(z_e) = \mu_y(z_e) = 2\mu_k(z_e) \simeq 14$  and

$$P = \frac{z_e \,\mu_e}{2} \,\frac{\Delta}{H^2} \simeq 31 \,\frac{\Delta}{H^2} \tag{26a}$$

$$F = \frac{z_e \,\mu_e}{2} \frac{\Delta^2}{H^3} \simeq 31 \frac{\Delta^2}{H^3} \tag{26b}$$

$$n = \sqrt{\frac{\mu_e}{8z_e}} \frac{\sqrt{\Delta}}{H} \simeq 0.62 \frac{\sqrt{\Delta}}{H}$$
(26c)

$$2nE_{\kappa} = \frac{z_e \,\mu_e}{4} \,\frac{\Delta^2}{H^2} \simeq 16 \frac{\Delta^2}{H^2} \tag{26d}$$

Eq. (26c) is to be compared to Eq. (2.19) of Tzokova (2020) Phd dissertation, which also introduced a continuous approach based on the vertical force *F* peaks. The total curvature energy of the cellular system,  $2nE_{\kappa}$ , is plotted in Figure 10 and compared to the total curvature energy of the different states introduced in Section 4. In this Figure 10, we see that the cellular model is approximately a convexification of the energy of the 'exact' system: Non-integer *n* values of the cellular model appear as approximate phase transitions between the integer-*n* states of the 'exact' system.

Using the scalings introduced in Section 4, we can rewrite these quantities as

$$P_{\rm u} = \frac{P}{n^2} = 4z_e^2 \simeq 81 \text{ and } F_{\rm u} = \frac{F}{n^3} = \frac{8\sqrt{2} z_e^{5/2}}{\sqrt{\mu_e}} \sqrt{\Delta} \simeq 130 \sqrt{\Delta}$$
(27)

(See Appendix B for the different other ways to express these quantities). These quantities are plotted in Figures 5, 6, 7, 8, and 10 as black lines. Another important quantity is the maximum value of the deflection angle  $\theta$ which happens at the inflection point  $s = \ell/2$ . In this continuous *n* model, we have

$$\theta_{\max} = \frac{\cos(z_e/2) - 1}{\cos(z_e/2)} \frac{F_u}{P_u} \simeq 1.3 \frac{F_u}{P_u} \simeq 2.1 \sqrt{\Delta}$$
 (28)

which near the value found for the Point-Contact state, see Eq. (18), and therefore yields the same limit  $\Delta < 0.06$  to stay in the small angle,  $\theta_{\text{max}} < 0.5$ , regime.

In Figure 11 we plot the vertical force *F* as a function of the horizontal force *P*, with properly rescaled axes. As explained earlier, for Point-Contact and Extended-Contact states this (*P*, *F*) diagram does not depend on the values of  $\Delta$  or *n*. Moreover, we see that the relation is almost linear, with a slope value approximately given by the cellular model:  $\frac{130}{81-4\pi^2} \simeq 3.1$ . Additionally, we plot the numerical results of the fully nonlinear system (1)-(6) for Point-Contact and Extended-Contact solutions to measure how much the nonlinearities cause a deviation from the virtually linear (*P*, *F*) relation, see

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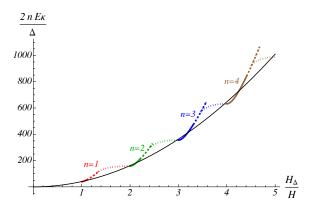


Figure 10: Curvature energy of the system as a function of the compaction ratio  $H_{\Delta}/H$ . Point-Contact states (continuous line), Extended-Contact states (dashed line, only k = 1), and Hanging-Fold states (dotted line) are shown for n = 1 to n = 4. The black line corresponds to the continuous-n model, Eq. (26d), and is approximately a convexification of the energy of the system.

gray dots in Figure 11 and (Wang et al., 2025). This linear relation implies that

$$FH \sim P\Delta$$
 (29)

271 which means that the work done by the horizontal and 246 vertical loads are comparable. 247

#### 6. Global phase diagram 248

In a typical experiment, one imposes the value of  $\Delta$ 249 and H and observes the state of the system. In Fig-250 278 ure 12, we plot the limit of the ranges of the exis-251 279 tence of the different states for n = 1 and n = 2 and 252 280 see that a clear arrangement materializes, linked to the 253 compaction ratio  $H_{\Delta}/H$  (for simplicity reasons, we only 282 254 consider Extended-Contact states with their flat region 283 255 in k = 1 piece). Nevertheless, as shown in Figure 13, 284 256 where *n H* is plotted as a function of  $\sqrt{\Delta}$ , this arrange-257 ment is not conserved as n is increased: point  $A_{n+1}$ 258 286 (of slope  $\frac{2}{\pi} \frac{n}{n+1}$ ) moves relatively to point  $C_n$  (of slope 259 287  $\int \frac{2n}{2n+1} / \sqrt{3}$  and crosses it as  $n \simeq 7.1$ . At a larger n260 289 value ( $n \simeq 9.7$ ), the same point  $A_{n+1}$  even crosses point 261  $B_n$  (of slope  $1/\sqrt{3}$ ). And at  $n \simeq 16.8$  it is point  $A_{n+2}$  that <sup>290</sup> 262 crosses  $C_n$ . Eventually, for any fixed integer *j*, when 263  $n \to \infty$  the arrangement is such that  $A_{n+j}$  is reached be-264 291 fore  $B_n$  (in terms of compaction ratio  $H_{\Delta}/H$  values). In 265 contrast, the points  $B_n$  and  $C_n$  keep their relative posi- 292 266 tioning:  $B_n > C_n > B_{n+1} > C_{n+1} > \ldots > B_{n+j} > C_{n+j}$ , 267 293 for any *n* and *j*. Overall, we conclude that for n < 8 (and 294 268 under the assumption k = 1), we will observe sequen- 295 269 tially the three states: Point-Contact, Extended-Contact, 296 270

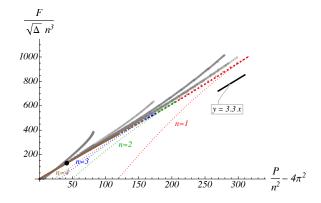


Figure 11: Evolution of the horizontal P and the vertical F forces as H is decreased or  $\Delta$  is increased. Point-Contact state (continuous line), Extended-Contact state (dashed line, only k = 1), and Hanging-Fold state (dotted line). For the Point-Contact and Extended-Contact states the plot does not depend on the Atlas number n. The single black point at (41, 130) corresponds to the response of the cellular model. The black line, with slope 3.3, is drawn as a guide to the eye to show the roughly linear behavior of the Point-Contact and Extended-Contact states. Gray dots correspond to numerical solutions of the nonlinear system (1)-(6) where only Point-Contact and Extended-Contact states are shown, a total of 1298 points with  $H \in (0.02, 0.15)$  and  $\Delta \in (0.006, 0.4).$ 

and finally Hanging-Fold, before switching to the n + 1mode. In contrast, for  $n \ge 8$ , we will not observe the Hanging-Fold state and will switch directly form the  $n^{th}$ Extended-Contact state to the  $n+1^{th}$  Point-Contact state. Note that all these arrangements depend on the value of k taken for the number of fragmented pieces of the flat region in the Extended-Contact state (here k = 1), with  $k_{max} = 2n + 1$ . In Figure 14 we plot a high *n* part of the bifurcation diagram, with k = 3, and show the behavior of the system will exhibit multistability and hysteresis. We consider we have loaded the system by increasing  $H_{\Delta}/H$  up to the point  $U_0$  lying at the end of the k = 3 segment of the  $25^{th}$  Extended-Contact branch. Upon increasing  $H_{\Delta}/H$  further, and if k does not in-

crease, the system will jump on the point  $U_1$  on the  $26^{th}$ Point-Contact branch. We then start unloading. The system will reach point  $U_2$  on the 25<sup>th</sup> Hanging-Fold branch and, upon further decrease of  $H_{\Delta}/H$ , will jump toward on of the points  $U_3$ ,  $U_4$ , or  $U_5$ . Resuming the loading will close the hysteresis loop.

#### 7. Discussion and Conclusion

We have detailed the different equilibrium configurations a planar beam may adopt when buckled in a rectangular cavity (Section 4). We have shown that they are all based on an Arch solution that spans from the lower to the upper wall (Section 3). For each solution type,

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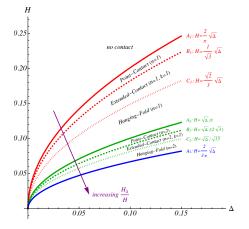


Figure 12: An attempt to draw a global bifurcation diagram for the Constrained Euler Buckling problem. Here, we give the state of the system for given values of H and  $\Delta$ , and for Atlas numbers n = 1 and n = 2. A clear arrangement of the different solutions appears, but this arrangement does not hold for any n, as is shown in Figure 13.

we have given their range of existence in terms of the 297 imposed horizontal ( $\Delta$ ) and vertical (H) displacements. 298 We have explained how the global system is a repetition 299 of the same solution and have introduced the Atlas num-300 ber n which counts the repeats. We have used rescaled 301 quantities to illustrate how the vertical (F) and horizon-302 tal (P) applied forces vary as functions of the imposed 303 displacements H and  $\Delta$ , and we have shown that the 304 state of the system during loading is entirely character-305 ized by the compaction ratio  $H_{\Delta}/H = (2/\pi) \sqrt{\Delta}/H$ . Fi-306 nally, we have introduced a cellular model that yields 307 an averaged response of the system as the loads are in-308 creased. The cellular model also clearly illustrates why 309 it is much harder to compress and squash a beam inside 310 a cavity than in the unconstrained case, see Figure 15. 311

All the above results are based on the von Karman 312 kinematic approximation and are then only valid as long 313 as H and  $\Delta$  are small enough. As H > 0.2 and/or  $\Delta >$ 314 0.1, the system enters the full nonlinear regime and, for 315 example, the exact collapse seen in Figure 7 and 8 is no 316 longer valid, Ying-Yang solutions may appear, and the 317 forces F and P (together with the number n of repeats) 318 will reach maximum values before decreasing (Deboeuf 319 et al., 2024). The global bifurcation diagram, Figures 320 12 and 13, gets more complicated and depends on the 321 loading history due to the presence of large hysteresis. 322 These features will be the subject of a subsequent report. 323

#### 324 Acknowledgments

It is a pleasure to thank Paul Grandgeorge, Eytan Katzav, and Benoit Roman for numerous discussions.

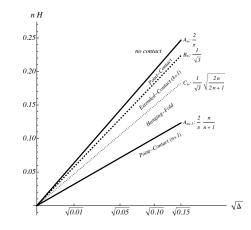


Figure 13: The relative positioning of the curves in Figure 12 does not hold for any *n*. The curves  $B_n$  and  $C_n$  keep their relative positioning, but the curves  $A_{n+j}$  all eventually cross  $B_n$  and  $C_n$ . For example,  $A_{n+1}$  crosses  $C_n$  at  $n \approx 7.1$  and  $B_n$  at  $n \approx 9.7$ .

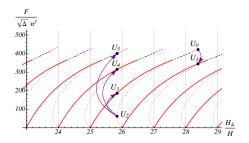


Figure 14: Hysteresis and multistability at large Atlas number. Starting on point  $U_1$  and decreasing the compaction ratio  $H_{\Delta}/H = (2/\pi) \sqrt{\Delta}/H$  will cause the system to visit point  $U_2$  then jump on either points  $U_3$ ,  $U_4$ , or  $U_5$ . Then increasing the compaction ratio will bring the system to  $U_0$  where it will jump on  $U_1$  to close a hysteresis loop. Point-Contact states are plotted as continuous red lines, Extended-Contact states with k = 1 as dashed green lines, Extended-Contact states with k = 2 as dashed blue lines, Extended-Contact states with k = 3 as dashed magenta lines, and Hanging-Fold states as dotted brown lines.

# Appendix A. The planar elastica solution in the hanging-fold state

We here compute some basic quantities attached to the Hanging-fold state. In this state, part of the system does not touch both walls. The rod equilibrium in this part of the system is then a planar elastica solution, see Figure A.16, with no vertical component in its inner force. We therefore have to integrate system (8) with  $n_y(s) \equiv 0$ , and the boundary conditions (2a) together with  $\theta(\ell') = 0$ . The solution of  $\theta''(s) = -P\theta(s)$  with  $\theta(0) = 0$  is  $\theta(s) = c \sin \sqrt{Ps}$ . Enforcing  $\theta(\ell') = 0$  yields

$$\sqrt{P\,\ell'} = \pi \tag{A.1}$$

Additionally, the continuity of the bending moment at the contact point (s = 0 in Figure A.16) implies that

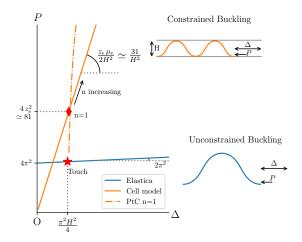


Figure 15: Buckling an elastica inside a tunnel is much harder than in the unconstrained setup. The horizontal force needed to compress 335 an elastic rod increases much more rapidly in the constrained case 336 (orange) than in the unconstrained case (blue). Note that we have 337 considered the fully nonlinear version of the equilibrium equations 338 (1), (2), (4), (5), (6) for the unconstrained case, where  $P(\Delta) = 4\pi^2 +$ 339  $2\pi^2 \Delta + \dots$  instead of  $P(\Delta) = 4\pi^2$  in the von Karman approximation, 340 see for example Bazant and Cedolin (2010, Fig. 1.26a). 341

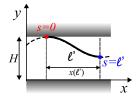


Figure A.16: . The part of the system of the Hanging-fold state where the rod does not touch both walls. The rod equilibrium is a planar elastica solution with an unknown length  $2\ell'$  and an internal force 354  $n_x = -P$  and  $n_y = 0$ .

 $\theta'(s = 0) = c\sqrt{P}$  equals the value given in Eq. (13). 358 This continuity constraint yields

$$c = \frac{f}{P} \tan \frac{\sqrt{P\ell}}{2} \tag{A.2}$$

Finally, we can compute the horizontal extent of the solution as

$$x(\ell') = \int_0^{\ell'} 1 - (1/2) \,\theta^2(s) \,\mathrm{d}s = \ell' \left( 1 - \frac{c^2}{4} \right) \quad (A.3)^{368}_{369}_{370}_{371}$$

#### Appendix B. The cellular model formula in all vari-329 373 ables 330

The solution of the cellular model (Section 5) may be 331 expressed using different pairs of variables. We list in 332 Table B.1 the different possibilities. 333

	Р	F	$2nE_{\kappa}$	_
$(\Delta, H)$	$31\frac{\Delta}{H^2}$	$31\frac{\Delta^2}{H^3}$	$16\frac{\Delta^2}{H^2}$	$n = 0.62 \frac{\sqrt{\Delta}}{H}$
$(\Delta, n)$	81 <i>n</i> <sup>2</sup>	$130 n^3 \sqrt{\Delta}$	$40 \Delta n^2$	$H = 0.62 \frac{\sqrt{\Delta}}{n}$
(H, n)	81 <i>n</i> <sup>2</sup>	$208 Hn^4$	$104 H^2 n^4$	$\Delta = 2.6 H^2 n^2$

Table B.1: Formulas for the forces P and F, the bending energy  $2nE_{\kappa}$ , the end shortening  $\Delta$ , the height *H*, and the Atlas number *n* for the cellular model, as functions of different pairs of variables.

### References

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- Adan, N., Sheinman, I., Altus, E., 1994. Post-buckling behavior of beams under contact constraints. Journal Applied Mechanics 61, 764-772.
- Bazant, Z.P., Cedolin, L., 2010. Stability of Structures: Elastic, Inelastic, Fracture, and Damage Theories. World Science Publishing
- Bigoni, D., 2012. Nonlinear Solids Mechanics. Cambridge University Press.
- Bosten, A., Denoël, V., Cosimo, A., Linn, J., Brüls, O., 2023. A beam contact benchmark with analytic solution. ZAMM - Journal of Applied Mathematics and Mechanics 103, e202200151. doi:10. 1002/zamm.202200151.
- Chai, H., 1998. The post-buckling response of a bi-laterally constrained column. J. Mech. Phys. Solids 46, 1155-1181. doi:10. 1016/S0022-5096(98)00004-0.
- Charrondière, R., Neukirch, S., Bertails-Descoubes, F., 2024. MERCI: Mixed Curvature-Based Elements for Computing Equilibria of Thin Elastic Ribbons. ACM Transactions on Graphics (TOG) 43, art. 160. doi:10.1145/3674502.
- Chateau, X., Nguyen, Q.S., 1991. Buckling of elastic structures in unilateral contact with or without friction. European Journal of Mechanics - A/Solids 10, 71-89.
- Coleman, B.D., Swigon, D., 2004. Theory of self-contact in Kirchhoff rods with applications to supercoiling of knotted and unknotted DNA plasmids. Philosophical Transactions of the Royal Society A: Mathematical, Physical and Engineering Sciences 362, 1281-1299. doi:10.1098/rsta.2004.1393.
- Daviet, G., Bertails-Descoubes, F., Boissieux, L., 2011. A hybrid iterative solver for robustly capturing coulomb friction in hair dynamics. ACM Transactions on Graphics (TOG) 30, art. 139. doi:10.1145/2070781.2024173.
- Deboeuf, S., Protière, S., Katzav, E., 2024. Yin-yang spiraling transition of a confined buckled elastic sheet. Phys Rev Res 6, 013100. doi:10.1103/PhysRevResearch.6.013100.
- Dichmann, D., Li, Y., Maddocks, J., 1996. Hamiltonian formulations and symmetries in rod mechanics, in: Mesirov, J., Schulten, K., Sumners, D. (Eds.), Mathematical Approaches to Biomolecular Structure and Dynamics. Springer Verlag. volume 82 of The IMA Volumes in Mathematics and Its Applications, pp. 71-113. doi:10.1007/978-1-4612-4066-2\_6.
- Djondjorov, P., Vassilev, V., Mladenov, I., 2011. Analytic description and explicit parametrisation of the equilibrium shapes of elastic rings and tubes under uniform hydrostatic pressure. International Journal of Mechanical Sciences 53, 355-364. doi:10.1016/j. ijmecsci.2011.02.005.

- Domokos, G., 1994. Global description of elastic bars. Z. Angew. 445 380
- Math. Mech 74, T289-T291. doi:10.1002/zamm.19940740410. 381 446
- Domokos, G., Healey, T., 2005. Multiple helical perversions of finite, 447 382 intrinsically curved rods. International Journal of Bifurcation and 448 383 384
- Chaos 15, 871-890. doi:10.1142/S0218127405012430. Domokos, G., Holmes, P., Royce, B., 1997. Constrained Euler buck-385
- 450 ling. Journal of Nonlinear Science 7, 281-314. doi:10.1007/ 386 451 387 BF02678090. 452
- Eisley, J.G., 1964. Nonlinear vibration of beams and rectangu-388 453 lar plates. Zeitschrift für angewandte Mathematik und Physik 389 454 (ZAMP) 15, 167-175. doi:10.1007/BF01602658. 390
- Elettro, H., Neukirch, S., Vollrath, F., Antkowiak, A., 2016. In-drop 391 456 capillary spooling of spider capture thread inspires hybrid fibers 392 457 with mixed solid-liquid mechanical properties. Proceedings of 458 393 the National Academy of Sciences of the USA 113, 6143-6147. 394 459 doi:10.1073/pnas.1602451113. 395
- Eringen, A.C., 1952. On the non-linear vibration of elastic bars. Quar-396 461 397 terly of Applied Mathematics 9, 361-369. doi:10.1090/qam/ 43694. 398
- 399 Euler, L., 1744. Methodus inveniendi lineas curvas maximi 464 minimivi propreitate gaudentes. Opera Omnia I 24, 231-400 297. URL: https://scholarlycommons.pacific.edu/ 401 466 euler-works/65/. Füssli, Zurich 1960. 402
- Goss, V.G.A., 2009. The History of the Planar Elastica: Insights into 403 Mechanics and Scientific Method. Science & Education 18, 1057-404 469 1082. doi:10.1007/s11191-008-9166-2. 405
- Hearle, J.W.S., 2014. The 20th-Century Revolution In Textile Ma-406 471 407 chines And Processes. Part 2: Textured Yarns And Other Technolo-472 gies. Industrial Archaeology Review 36, 32-47. doi:10.1179/ 408 473 0309072814Z.0000000028. 409
- Henderson, M.E., Neukirch, S., 2004. Classification of the spa-410 475 tial clamped elastica: numerical continuation of the solution set. 476 411 International Journal of Bifurcation and Chaos 14, 1223-1239. 412 477 doi:10.1142/S0218127404009971. 413
- Holmes, P., Domokos, G., Schmitt, J., Szeberényi, I., 1999. Con-414 479 strained euler buckling: an interplay of computation and analysis. 415 480 416 Computer Methods in Applied Mechanics and Engineering 170, 175-207. doi:10.1016/S0045-7825(98)00194-7. 417
- 418 Judah, N., Givli, S., 2024. The Post-Buckling Behavior of a Beam 483 Constrained by Nonlinear Springy Walls. Journal of Applied Me-419 484 chanics 91, 061004. doi:10.1115/1.4064684. 420 485
- von Kármán, T., 1907. Festigkeitsprobleme im Maschinenbau, in: 486 421 Klein, F., Müller, C. (Eds.), Encyklopädie der Mathematischen 422 487 Wissenschaften. Vieweg+Teubner Verlag, Wiesbaden. Mechanik, 423 488 pp. 311-385. doi:10.1007/978-3-663-16028-1\_5. 424
- Katz, S., Givli, S., 2015. The post-buckling behavior of a beam con-425 strained by springy walls. Journal of the Mechanics and Physics of 426 491 Solids 78, 443 - 466. doi:10.1016/j.jmps.2015.02.004. 427
- Kehrbaum, S., Maddocks, J.H., 1997. Elastic rods, rigid bodies, 428 quaternions and the last quadrature. Philosophical Transactions of 429 the Royal Society of London. Series A, Mathematical and Physical 430 Sciences 355, 2117-2136. doi:10.1098/rsta.1997.0113. 431
- Kirchhoff, G., 1876. Vorlesungen über mathematische Physik, 432
- Mechanik. B.G. Teubner, Leipzig. URL: https://archive. 498 433 org/details/vorlesungenberm02kircgoog/. 434 499
- Lacarbonara, W., Yabuno, H., 2006. Refined models of elastic 435 500 beams undergoing large in-plane motions: Theory and experi-436 501 ment. International Journal of Solids and Structures 43, 5066-437 502
- 5084. doi:10.1016/j.ijsolstr.2005.07.018. 438 503 Levien, R.L., 2009. From Spiral to Spline: Optimal Techniques in 504 439
- Interactive Curve Design. Ph.D. thesis. University of California at 505 440 Berkeley. 441
- Love, A.E.H., 1944. A Treatise on the Mathematical Theory of Elas-442
- 443 ticity. 4th ed., Dover Publications, New York.
- Lubinski, A., Althouse, W., 1962. Helical buckling of tubing sealed in 444

packers. Journal of Petroleum Technology 14, 655-670. doi:10. 2118/178-PA

- Manning, R.S., Bulman, G.B., 2005. Stability of an elastic rod buckling into a soft wall. Proceedings of the Royal Society A: Mathematical, Physical and Engineering Science 461, 2423-2450. doi:10.1098/rspa.2005.1458.
- Miller, J., Su, T., Dussan, E., Pabon, J., Wicks, N., Bertoldi, K., Reis, P., 2015. Buckling-induced lock-up of a slender rod injected into a horizontal cylinder. International Journal of Solids and Structures 72, 153 - 164. doi:10.1016/j.ijsolstr.2015.07.025.
- Neukirch, S., Yavari, M., Challamel, N., Thomas, O., 2021. Comparison of the Von Karman and Kirchhoff models for the post-buckling and vibrations of elastic beams. Journal of Theoretical, Computational and Applied Mechanics 6828, 1-18. doi:10.46298/jtcam. 6828
- Pandey, A., Moulton, D.E., Vella, D., Holmes, D.P., 2014. Dynamics of snapping beams and jumping poppers. EPL (Europhysics Letters) 105, 24001. doi:10.1209/0295-5075/105/24001.
- Pocheau, A., Roman, B., 2004. Uniqueness of solutions for constrained elastica. Physica D: Nonlinear Phenomena 192, 161-186. doi:10.1016/j.physd.2003.12.013.
- Roman, B., Pocheau, A., 1999. Buckling cascade of thin plates: Forms, constraints and similarity. Europhysics Letters (EPL) 46, 602-608. doi:10.1209/epl/i1999-00306-3.
- Roman, B., Pocheau, A., 2002. Postbuckling of bilaterally constrained rectangular thin plates. Journal of the Mechanics and Physics of Solids 50, 2379–2401. doi:10.1016/S0022-5096(02)00028-5.
- Schulz, M., Pellegrino, S., 2000. Equilibrium Paths of Mechanical Systems with Unilateral Constraints, I. Theory. Proceedings of the Royal Society A: Mathematical, Physical and Engineering Sciences 456, 2223-2242, doi:10.1098/rspa.2000.0611.
- Thomas, O., 2025. Understanding, computing and identifying the nonlinear dynamics of elastic and piezoelectric structures thanks to nonlinear modes. Springer. volume 614 of Series CISM courses and lectures. chapter 4. pp. 151-236. doi:10.1007/ 978-3-031-67499-0\_4.
- Thomas, O., Sénéchal, A., Deü, J.F., 2016. Hardening/softening behavior and reduced order modeling of nonlinear vibrations of rotating cantilever beams. Nonlinear Dynamics 86, 1293-1318. doi:10.1007/s11071-016-2965-0.
- Thomson, W., Tait, P.G., 1883. Treatise on Natural Philosophy. volume 2. Cambridge University Press.
- Tzokova, P.S., 2020. Confined wrinkling of thin elastic rods, sheets and cones. Ph.D. thesis. University of Cambridge. doi:10.17863/ CAM. 57977.
- Vetter, R., Wittel, F.K., Herrmann, H.J., 2014. Morphogenesis of filaments growing in flexible confinements. Nature Communications 5, 4437. doi:10.1038/ncomms5437.
- Villaggio, P., 1979. Buckling under unilateral constraints. International Journal of Solids and Structures 15, 193-201. doi:10.1016/ 0020-7683(79)90030-1.
- Wachter, A., Biegler, L.T., 2006. On the implementation of an interior-point filter line-search algorithm for large-scale nonlinear programming. Math. Program. Ser. A 106, 25-57. doi:10.1007/ s10107-004-0559-y.
- Wang, J., Deboeuf, S., Antkowiak, A., Neukirch, S., 2025. Constrained Euler buckling: global nonlinear phase diagram. Technical Report. Sorbonne University.
- Woinowsky-Krieger, S., 1950. The effect of an axial force on the vibration of hinged bars. Journal Applied Mechanics 17, 35-36. URL: https://hal.science/hal-03184616.

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