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Influence of laboratory aging and asphaltene content of asphalt on frictional coefficient by tribological analysis - a case study

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Abstract:

An increase in use of reclaimed asphalt pavement (RAP) materials was observed for the last few years. These materials are composed of aged asphalt which can cause implementation issues due to its high viscosity. This work suggests a new way to explain a part of this problematic by tribological analysis and to assess the lubricating ability of aged asphalt. The influence of asphalt aging is investigated on the frictional properties using two 35/50 asphalt penetration graded specimens and a third one extracted from RAP. Short and long-term as well as natural effects of aging are examined on penetration grade, ring and ball temperature, infrared indices, and asphaltene content to demonstrate a physical hardening of the studied asphalts. The influence of natural or thermal aging are also studied by tribological measurements on lubricating properties. A ball on a three-plate equipment are used to evaluate the coefficient of friction (CoF) as a function of sliding velocity (Stribeck Curve) from 120°C to 80°C. The increase in asphaltene content after oxidation increases the frictional coefficient in the hydrodynamic regime. These observations were confirmed by tribological analysis performed on

asphalts with controlled asphaltene contents (0, 15, 20wt%). In addition of the physical hardening, asphalt oxidation causes a decrease of the lubricant properties. The evaluation of frictional coefficient can be added to the other well-known parameters to explain workability issues in presence of aged materials.

Keywords:

Asphalt, Aging, Tribology, Asphaltene, Coefficient of Friction

1 Introduction:

Nowadays, with environmental issues, reclaimed asphalt pavement (RAP) is increasingly used as raw materials for new asphalt concrete formulations¹. Workability issues are now of strong interest and a better understanding of the asphalt present in the RAP is required.

Asphalt materials are generally described as complex mixtures of different hydrocarbons with a polarity gradient in Saturate, Aromatic, Resin and Asphaltene (SARA) fractions^{2,3}, which can be viewed as a colloidal suspension^{4,5}. Understanding the aging of these materials is a keypoint for future developments concerning the addition of reclaimed asphalt pavement to asphalt concrete. Laboratory asphalt aging was extensively described in literature⁶⁻¹⁰. The natural aging of the asphalt binders is simulated using two thermal tests. The first one is the rolling thin-film oven test (RTFOT) for short-term aging¹¹. This experiment attempts to generate the same hardening and physico-chemical behavior modifications as after the mixing process for the hot-mix asphalt. The aim of the second experiment, referred to as the Pressure-Aging Vessel (PAV)¹², is designed to represent the long-term aging of binders after long-term in-service roads. After these treatments, the asphalt binders are also analyzed using infrared spectroscopy to evaluate the degree of oxidation using carbonyl and sulfoxide indices^{6,9}. These parameters are generally used to quantify asphalt aging and correlate with physico-chemical properties such as viscosity and viscoelastic properties^{7,8}.

The oxidation of asphalt is generally described in the literature by different physical and chemical modifications. Regarding the chemical aspect, oxidation can be followed by studying S=O and C=O bonds by FTIR measurements^{7,10}, and information can be gained also measuring SARA fraction which varying with oxidation degree¹³. This variation of SARA fraction was mainly due to the oxidation of sulfur and carbon to sulfoxide and carbonyl¹⁴. These heteroatoms were in principle in aromatic and resin fractions. This oxidation generates new kind of polar molecules in asphalt which are called oxidized asphaltenes. These chemical changes are generally induced by thermal oxidation. More complex reactions occur with ultraviolet radiation, as it was listed by N. C. L. Madeira and col. ⁶.

26 These chemical modifications have an influence on mechanical behavior of asphalt resulting in an
27 increase of the storage modulus (G') and viscosity parameters. Moreover, it increases the ring and ball
28 temperature (RBT) and decreases penetration grade. The loss of workability and mechanical properties
29 when reclaimed asphalt materials are used in asphalt concrete formulations is generally induced by an
30 asphalt evolution after oxidation. In some cases, these parameters were not sufficient to explain the
31 workability and compaction properties. Tribological analysis are expected to explain some aspects of
32 these physical behaviors.

33 Tribology science is used to observe the lubricant power of oil for engineering pieces¹⁵. Stribeck worked
34 principally on friction phenomena present in steel balls bearing and established the so-used Stribeck
35 curve. M. Woydt described historical events related to discovery dealing with tribological science and
36 tell that Thuston was the first to develop an accurate system to measure the Coefficient of Friction
37 (CoF) in 1870s¹⁵. Tribological experiments are used to compare different compounds based on their
38 ability to decrease the CoF between two contact layers. In many cases, lubricants exhibit different
39 behaviors, as identified by Stribeck. Many different tests exist to evaluate frictional parameters under
40 different movements and conditions¹⁶. The first thin film analysis of asphalt were conducted by G.L.
41 Baumgardner and col.^{17,18} using a classic rheometer. Subsequently, the ball on a three-plate system is
42 most commonly used¹⁹⁻²⁹ and the use of four-ball apparatus analysis is occasionally mentioned^{23,29};
43 however, these experiments require more material and temperature homogenization requires more
44 time. It is interesting to note that the use of tribology to study asphalt binders increased in the last
45 decade^{16,22,29,30}.

46 With respect to asphalt applications, recent studies have shown that viscosity, and gyratory shear press
47 workability on asphalt concrete^{31,32} could be correlated to the minimal CoF, measured by tribology,
48 and that the influence of chemical additives could be evaluated.

49 This study proposes the first evaluation of the influence of asphalt aging on the frictional properties.

50 2 Materials and methods:

51 General asphalt characterization

52 Characterizations were performed on different asphalt samples before and after aging based on two
53 35/50 1/10mm penetration graded specimens arbitrarily called Asphalt A and B with relatively similar
54 physical-chemical properties. Those characterizations include the asphaltene content with an internal
55 method describe afterwards in “Asphaltene extraction” part, ring and ball temperature [NF EN 1427]³³,
56 needle penetration [NF EN 1426]³⁴, aging index by Infra-Red (IR) spectroscopy, and rheological tests.
57 The results are summarized in Table 1. The ring and ball temperature measurement follows the
58 standard procedure with calibrating rings and balls. A bath of water was used to vary the temperature
59 (in °C) and a laser was used to precisely determine the softening point of the asphalt sample. Needle
60 penetration was measured using a calibrated needle with a weight of 100 g for 5 s at 25°C.

61 Infrared Data treatment

62 Aging indices were calculated by the integration of the signals at different wavenumbers and estimated
63 using the C-H strain peak as a reference peak (IR spectra are shown in Figures S1 and S2). The oxidation
64 indices were calculated from the area under the peak, using the limits described in Table S1. The
65 carbonyl index (I_{CO}) and sulfoxide index (I_{SO}) were calculated using the equations S1 and S2.

66 Rheological Analysis

67 To respect the “natural cooling” and erase the potential thermal history of asphalt, the two following
68 rheological protocols began from higher to lower temperatures. All experiments were performed
69 under inert atmosphere (N_2) except for Brookfield rotational viscosimeter experiments.

70 Rheological analysis was performed using an Anton Paar rotational controlled stress rheometer (MCR
71 302) to evaluate the viscosity of each asphalt specimen. Isothermal analyses, from 120°C to 80°C
72 (increments of 10°C), were performed at a constant shear rate ($=1s^{-1}$) with a gap equal to 1 mm and a
73 25 mm plate-plate geometry. The sample was maintained at the analysis temperature for 10 min to

74 obtain a homogeneous sample temperature, and acquisition was performed for 10 min to ensure that
75 the temperature did not evolve during the test. The average viscosity was then calculated and plotted
76 as a function of the temperature (Figure S3).

77 Viscosity of asphalt A and B were also evaluated using a Brookfield viscometer with a coaxial cylinder's
78 geometry in a temperature range from 180 to 60°C and following the standard NF EN 13302³⁵. Two
79 different cylinder diameters (SC4-21 and SC4-27) were used to adapt the measurement to viscosity
80 evolution with decreasing temperature. For each measurement, the sample was maintained at the
81 analysis temperature for 10 min and the viscosity value was collected with a value of torque at 90% of
82 the maximum value that can be detected by the viscometer. The viscosity is plotted as a function of
83 temperature (Figure S4).

84 [Asphaltene / maltene extraction protocol](#)

85 The asphaltenes were extracted from the asphalt binder *via* filtration. First, 1 g of asphalt binder was
86 dissolved in 30 mL of N-heptane during 24 h. Then a filtration was carried out using a Büchner system
87 with a PTFE filter with a porosity of 45µm to isolate the asphaltenes aggregates/particles from the
88 filtrate solution constituted of a mixture of maltene and N-heptane. This solution was placed under
89 vacuum to remove the N-heptane (Figure S5).

90 The first step was repeated with the asphaltene extract at 70°C for 2 h to wash the high-melting
91 crystallizable fraction. Retentate asphaltene particles were collected and the filtrate was eliminated.
92 The collected asphaltene was weighted to determine the asphaltene content in the asphalt using
93 equation [S3].

94 This extraction protocol was performed on all samples and was used to control the quantity of
95 asphaltene in the reference and aged specimens.

96 Laboratory aging of asphalt sample

97 Asphalts A and B from different origins were used and subjected to thermal aging treatment. The first
98 concerns short-term aging using the Rolling Thin-Film Oven Test (RTFOT) process following the EN
99 12607-1 standard¹¹. To proceed, 35 g of asphalt was added to each vial and placed in an RTFOT machine
100 for 1h15 min at 163°C under oxidative flux at atmospheric pressure. This treatment simulates the aging
101 process that occurs during the mixing process. The second method involved a Pressure Aging Vessel
102 (PAV, long-term aging) following the EN 14769 standard¹². This process was realized with 50 g of
103 asphalt in each plate with an oxidation time of 40h at 100°C after one RTFOT and under 21 bars of
104 oxidative flux.

105 Asphaltene control process for model asphalt specimen

106 Asphaltenes and maltenes were added to the asphalt mixture to obtain modified asphalt with
107 controlled asphaltene content. Asphalt A has a natural content of 20.3 wt%, and asphalt B has 11.4
108 wt% asphaltenes. The maltene extract from asphalt A was reused to decrease the asphaltene content
109 by dilution and obtain 15.0 wt% asphaltenes. Similarly, the asphaltene extract from asphalt B was
110 added to increase the asphaltene content to obtain asphalt B with 15.0 wt% asphaltene. All the
111 compounds (maltene and asphaltenes) were mixed by mechanical stirring for 5 min after heating the
112 asphalt to 150°C for 30 min under Nitrogen atmosphere to avoid oxidation.

113 Asphalt extraction from Reclaimed Asphalt Pavement (RAP)

114 This process consists of solubilizing asphalt from RAP with a perchloroethylene solvent and following
115 the NF EN 12697-1 standard³⁶. RAP (1 kg) was added to the engine, mixed with the solvent, and
116 centrifuged to extract the filler particles. A mixture of asphalt solubilized in a solvent was collected and
117 placed in a rotavapor to extract the solvent. The amount of residual solvent determined by Infrared
118 spectroscopy was negligible (<0,4%).

119 Finally, ten different asphalts were obtained and evaluated through tribological analysis (Table 1).

120 *Table 1: Asphaltene content, Ring and ball temperature, needle penetration, oxidative index and viscosity at 135°C of*
 121 *asphalts A and B before and after RTFOT, after RTFOT + PAV 40h, at 15wt% of asphaltene, maltene A and asphalt from RAP*

Asphalt origin	Name	Asphaltene content (%) [+/- 1%]	Ring and Ball Temperature (°C) [+/- 1°C]	Needle Penetration (1/10 mm) [+/- 2 dmm]	Index I _{CO} (a.u.)	Index I _{SO} (a.u.)	Viscosity at 135°C (Pa.s)
Asphalt A	¹ AA _{REF}	20.3	52.6	50	1.1	10.8	0,842 +/- 0,039
	² AA _{RTFOT}	22.4	58.0	28	0.6	13.0	1,107 +/- 0,042
	³ AA _{RTFOTPAV}	29.8	73.8	15	6.7	19.2	3,043 +/- 0,045
	⁴ AA ₁₅	15	-----	-----	-----	-----	-----
	⁵ AA ₀	0	-----	-----	-----	-----	-----
Asphalt B	¹ AB _{REF}	11.4	52.4	38	1.2	11.1	0,727 +/- 0,045
	² AB _{RTFOT}	12.9	59.0	29	1.7	11.8	1,032 +/- 0,045
	³ AB _{RTFOTPAV}	25.6	70.6	16	7.5	18.5	2,283 +/- 0,043
	⁴ AB ₁₅	15.0	-----	-----	-----	-----	-----
Naturally aged asphalt extract from RAP	⁶ A _{RAP}	29.0	72.2	12	8.0	22.6	4,375 +/- 0,374
Standard	-----	Internal Method	NF EN 1427	NF EN 1426	Internal Method	Internal Method	
Measurement Sample number	-----	1 2	1 2	3 1	1 1	1 1	

122 ¹Reference asphalt without modification,

123 ²Asphalt obtained after Rolling Thin Film Oven Test oxidation time 1 hours 15 minutes at 163°C,

124 ³Asphalt obtained after RTFOT and Pressure Aging Vessel during 40 hours at 100°C,

125 ⁴Asphalt obtained after modification by addition of maltene or asphaltene to get a modified asphalt at 15 wt% of asphaltene,

126 ⁵Maltene obtained after the internal method for asphaltene extraction by N-heptane at room temperature (first extraction step),

127 ⁶Asphalt extract by Infratest protocol from reclaimed asphalt pavement.

130

131 Tribological analysis

132 Tribological analysis was performed with an Anton Paar rotational controlled stress Rheometer MCR

133 302 with three-plates ball system as shown in Figure S6.

134 Stainless steel plates were used and handled with nitril gloves for all the following experiments and

135 washed with technical ethanol and acetone to remove every trace of grease which can influence the

136 experimental values. The experimental conditions were the same as in the work of Ingrassia L. *et.al.*²⁷

137 (except for the sliding velocity ramp) and are described in details in the supporting information (Figures

138 S6 and S7). This work also demonstrates the accuracy of measurements. For this reason, 5 runs were
 139 performed at each temperature for one sample to obtain average Stribeck curves. Balls and plates
 140 were changed between every samples to limit measurement error. The evolution of the CoF versus
 141 sliding velocity (or RPM) is divided into four domains, as shown in Figure 1. The first domain (Figure 1-
 142 1) at a low sliding velocity is called the boundary regime and shows that the CoF is influenced by the
 143 large amount of frictional force provided by the substrate. Then at higher sliding velocity, a second
 144 domain appears (Figure 1-2) in which the normal force from the specimen began to spread the plates
 145 and ball leading to a pronounced decrease of the CoF; however, some frictional forces from the
 146 substrate still remain. This second domain is called a mixed regime. The third domain (Figure 1-3) is
 147 called the elasto-hydrodynamic domain and presents a minimum of CoF. In the last domain (Figure 1-
 148 4) an increase in the CoF, caused by an increase in the film thickness of the sample, is observed. This is
 149 the hydrodynamic behavior of the specimen.

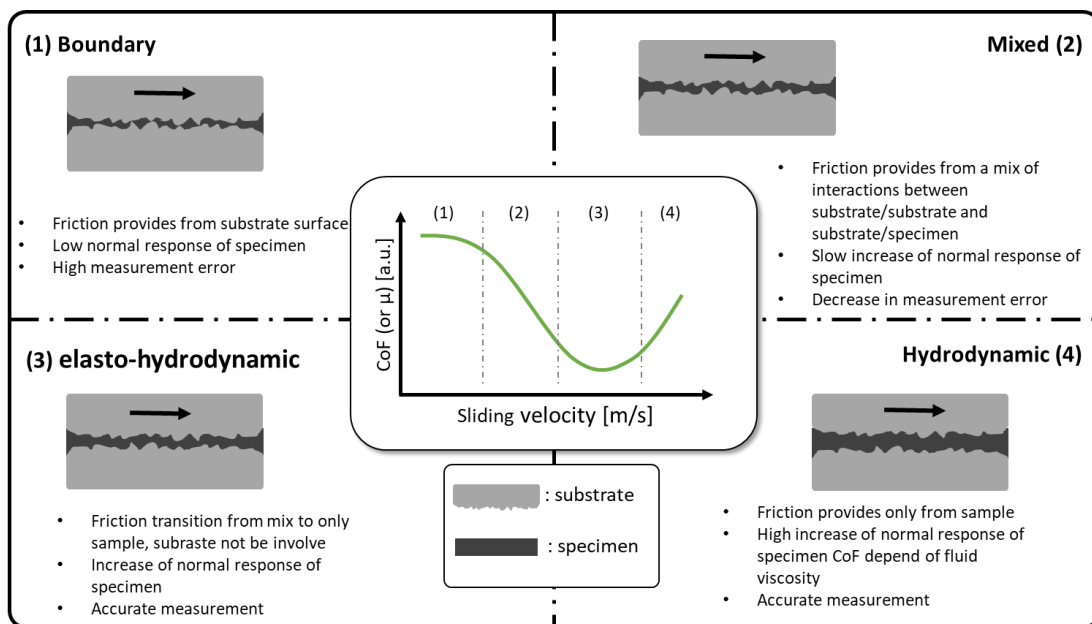


Figure 1: Schematic overview of the Stribeck curve and all different domains with (1) boundary (2) mixed (3) elasto-hydrodynamic and (4) hydrodynamic ones

150

151 3 Results and discussion:

152 This work is based on two reference asphalt samples with nearly the same high-temperature viscosity,
153 penetration grade, and crystallizable fraction content. The crystallizable fractions were determined by
154 differential scanning calorimetry (Figures S8 and S9) and differ by only 1% in weight, and therefore is
155 considered as non-influencing. The major differences between asphalts A and B were the sources and
156 asphaltene content (gap of $\approx 10\text{wt}\%$). These two asphalt samples allow a comparison of the influence
157 of asphaltene content before and after aging and its influence on the coefficient of friction for the
158 same asphalt penetration grade (35/50) (Table S2).

159 The first set of analyses investigated the influence of short- and long-term laboratory aging on the
160 penetration grade, ring and ball temperature, asphaltene content, and aging index using FTIR. Table
161 shows all the results that characterize asphalt aging and its physical-chemical evolution. These tests
162 revealed that there was a gradual increase in the asphaltene content (+47% for asphalt A and +125%
163 for asphalt B), in ring and ball temperature (RBT) (+40% for asphalt A and +35% for asphalt B), and a
164 decrease in the penetration grade (-70% for asphalt A and -58% for asphalt B) after RTFOT + PAV for
165 40h. There were similarities between specimens A and B in terms of the evolution of the penetration
166 grade (Figure S10). However, differences were observed in the RBT and asphaltene content. The
167 asphaltene generated by the thermal oxidative process causes the global hardening of the asphalt
168 binder and generally results to the creation of new asphaltene molecules known as oxidized
169 asphaltenes.

170 The next part investigates the influence of these types of thermal modifications on the tribological
171 behavior of asphalts. Tribological analysis of aged samples were performed at different temperatures
172 to investigate the influence of these treatments on the evolution of the Coefficient of Friction (CoF)
173 with sliding velocity.

174 Figure 2 shows the results of the tribological analysis performed on asphalt A before and after short-
 175 and long-term aging. Four domains of the Stribeck curve were observed for all temperatures, except
 176 at 120°C where the hydrodynamic domain is not yet reached in the sliding velocity range investigated
 177 and where all samples show similar behavior. The absence of this domain is related to the low viscosity
 178 and the Newtonian behavior in a wide range of shear rate of the asphalt at this temperature. The
 179 friction between two plates still exist at high sliding velocity which can explain high measurement
 180 error. When the temperature decreases, all domains shift to smallest sliding velocity, and the
 181 hydrodynamic behavior of the specimen becomes more and more pronounced. An increase of the CoF
 182 in this domain is related to an increase of the film thickness, indirectly observed by the gap evolution
 183 (Figures S11-14). As the film thickness increases for a given sliding velocity, the shear rate decreases
 184 ($\dot{\gamma} \propto \frac{1}{h}$).

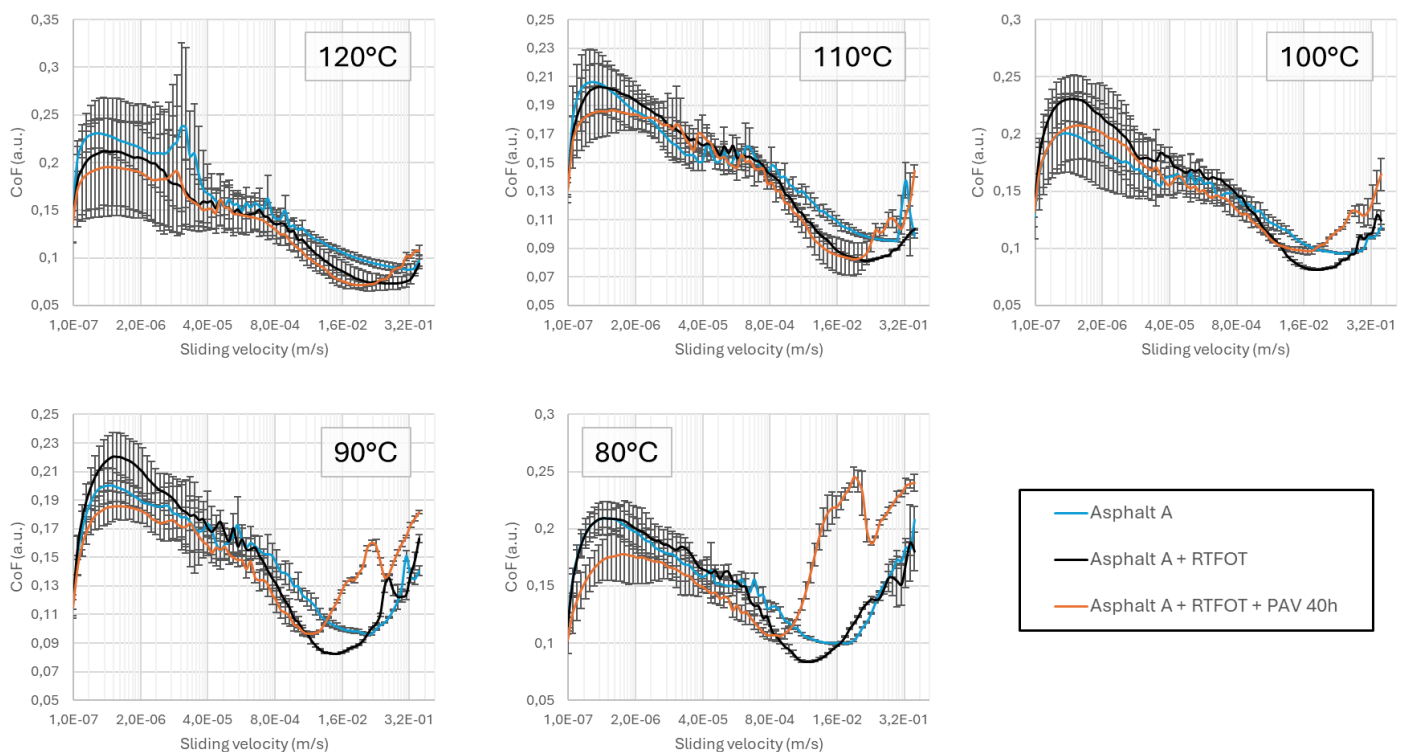


Figure 2: Stribeck curves of reference asphalt A in blue, after RTFOT in black and after RTFOT + PAV 40h in orange at 120, 110, 100, 90 and 80°C

185

186 An increase in asphalt viscosity may cause the increase of the coefficient of friction in hydrodynamic
 187 domain. The shear rate in tribological analysis can be estimated over 1000 s^{-1} , and the probability of

188 observing a nonlinear behavior is high, as shown in Eyssautier J. al. works³⁷, where the shear thinning
 189 behavior of crude oil appears at 80°C with a shear rate of 100 s⁻¹.

190 The minimal friction coefficient obtained on the Stribeck curves at the different temperatures studied
 191 are presented in Figure 3 for all asphalt A type samples. These results reveal that there was a gradual
 192 increase in the minimal coefficient of friction when the temperature decreased after aging. Reference
 193 asphalt shows a low influence of temperature on minimal coefficient of friction as compared to other
 194 works^{26,27}. But frictional coefficient cannot be resume to minimal coefficient of friction. The coefficient
 195 of friction of this sample in the hydrodynamic domain increases for all samples when the temperature
 196 decreases. As previously discussed, the asphalt material can show a non-Newtonian behavior and this
 197 phenomenon, related to the normal response of the sample, induces the appearance of the
 198 hydrodynamic behavior. The increase in the minimal CoF was caused by a reduction in the mixed
 199 domain, which was governed by the coexistence of friction from substrate / substrate and substrate /
 200 asphalt. The influence of the asphalt specimen appeared at lower sliding velocity. This was observed
 201 for all the aged and non-aged asphalts. It is interesting to note that higher minimal friction coefficient
 202 is obtained for asphalt A aged with RTFOT + PAV process at 80°C.

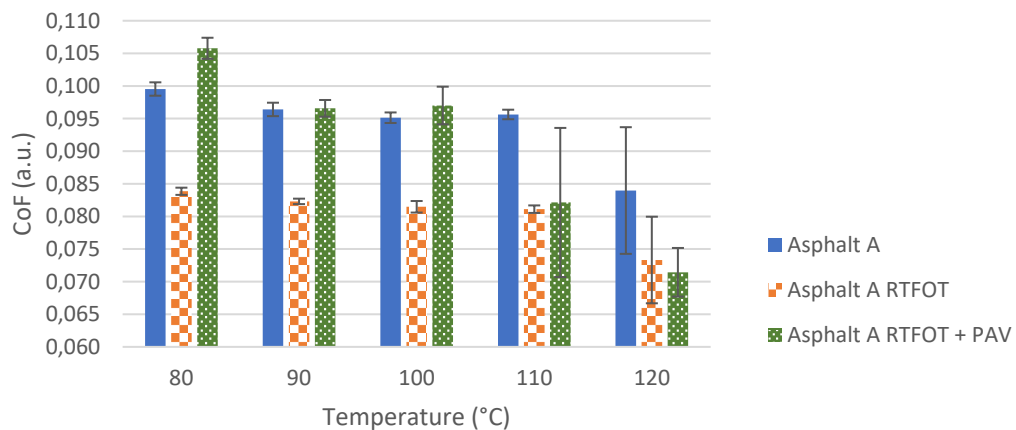


Figure 3: Comparison between minimal CoFs obtained with asphalt A in blue, after RTFOT in orange and after RTFOT + PAV 40h in green at 120, 110, 100, 90 and 80°C

203 Short- and long-term aging exhibit similar behavior with respect to a shift of all Stribeck domains at
 204 lower sliding velocities (Figure 2). Mixed and boundary regimes were usually less used because of the

205 high measurement error. However, these treatments have different effects on elasto-hydrodynamic
206 and hydrodynamic domains.

207 For Asphalt A, Rolling Thin Film Oven Test aging induces a decrease in the coefficient of friction in the
208 elasto-hydrodynamic domain and maintains a similar behavior in the hydrodynamic regime compared
209 to the reference asphalt. The first observation can be explained by the oxidation of asphalt, which
210 leads to an increase in the sulfoxide index, which differs from natural asphaltene. Lee et al.³⁸ shows
211 four different effects of nanoparticles on lubricating properties with rolling, mending and polishing
212 effects and generation of protective film. Another work²⁵ deals with the incorporation of graphene
213 nanoparticles in asphalt and explained the decrease in the coefficient of friction by the mending effect.
214 For asphalt A after RTFOT, a possible hypothesis consists of the generation of a new nanoaggregate of
215 oxidized asphaltene, which can help lubrication by one of these different effects.

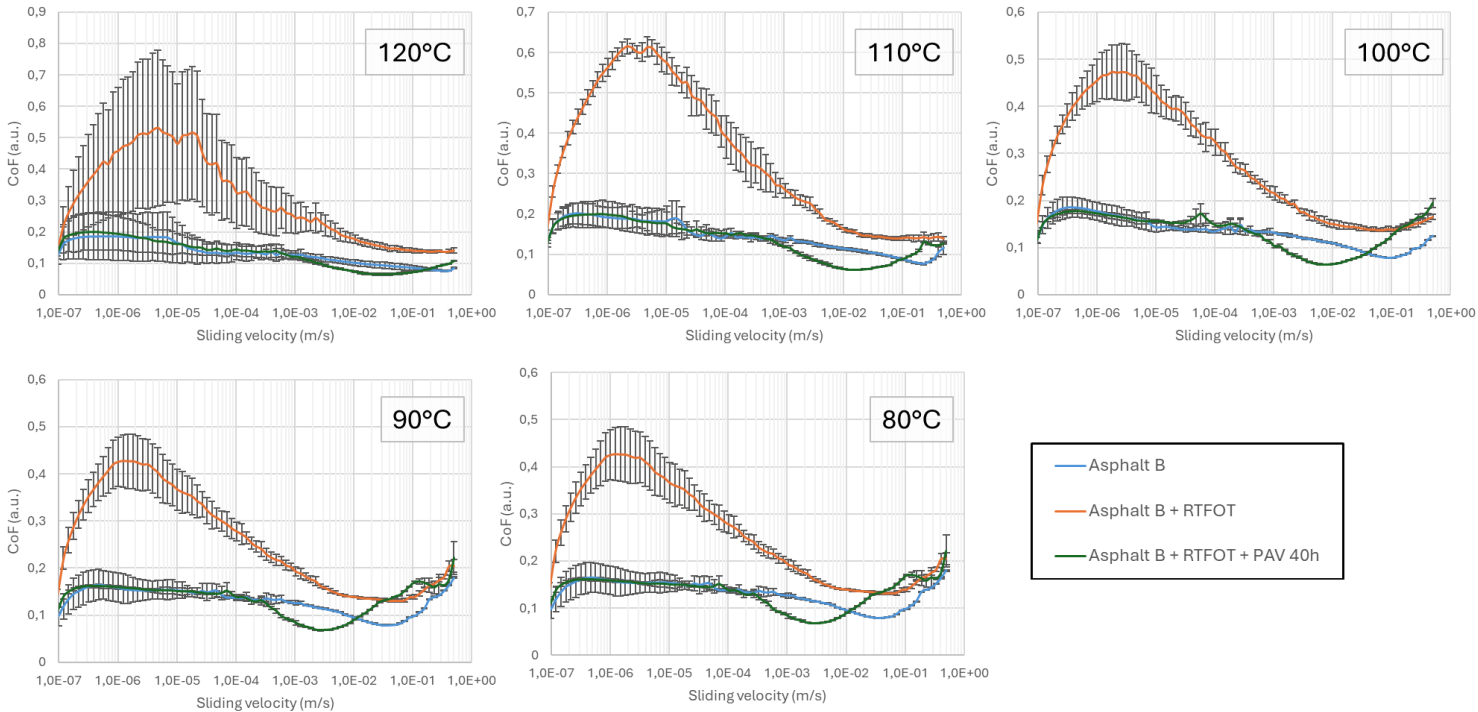
216 RTFOT + Pressure Aging Vessel (40h) induces an increase of the minimum coefficient of friction in the
217 elasto-hydrodynamic and hydrodynamic Stribeck domains due to the increase in asphaltene content.
218 In relation to the previous hypothesis dealing with mending effect, in this case, it was possible to
219 identify a limit concerning nanoparticles content to improve the lubrication properties. These limits
220 were reached with this laboratory aging and asphalt.

221 The same general trend is obtained with asphalt B for long-term aging samples Figure 4. All Stribeck
222 domains were identified at temperatures ranging from 110 °C to 80°C. A shift in all Stribeck domains
223 at lower sliding velocities and an increase in the coefficient of friction in the hydrodynamic domain is
224 observed as for Asphalt A. Regarding the elasto-hydrodynamic and mixed domains of long-term aging
225 asphalt B, the CoF was lower than that obtained for reference asphalt B at each temperature tested.

226 Short-term aging sample B presents a more complex modification of the coefficient of friction. It was
227 always higher in all the domains and the results were reproducible. In this case, short-term aging
228 appears to have an important influence on the lubricity ability of the binders, which is difficult to

229 explain. However, the needle penetration results are similar to those of asphalt A and the behavior of
 230 this asphalt after long term aging was more coherent as compared to asphalt A.

231



232

Figure 4: Stribeck curves of reference asphalt B in blue, after RTFOT in orange and after RTFOT + PAV40h in green at 120, 110, 100, 90 and 80°C

233

234 The minimal coefficients of friction for all the asphalt B samples are compared in Figure 5. The same
 235 behavior was observed as a function of temperature for asphalt B reference (i.e. unaged) and the long-
 236 term aging. The minimal CoF slightly increased with the decrease of temperature. In the case of Rolling
 237 Thin Film Oven Test treatment (short-term aging) a different behavior from that of asphalt A with the
 238 same aging state was observed. The CoF decreases slightly with the decrease in temperature only for
 239 this sample and the influence of temperature is difficult to explain again.

240 These aging processes show that it is difficult to compare different samples using only the minimal
241 coefficient of friction obtained from the elasto-hydrodynamic domain.

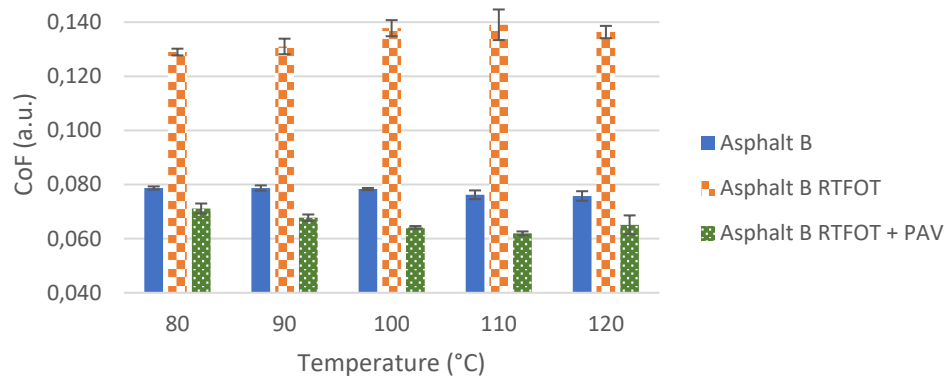


Figure 5: Comparison between minimal CoFs obtained with asphalt B in blue, after RTFOT in orange and after RTFOT + PAV 40h in green at 120, 110, 100, 90 and 80°C

242

243 To summarize, some similarities were observed after thermal oxidative aging. The hydrodynamic
244 domain occurred at the smallest sliding velocity and was more pronounced for the two asphalts tested
245 after RTFOT + Pressure Aging Vessel for 40h. This behavior may be related to the increase in viscosity
246 and the elastic behavior of asphalt in hydrodynamic domain which induces an increase of the normal
247 response (i.e. increase the film thickness) of the asphalt at each temperature investigated. A final
248 comment concerns the influence of asphaltene on the coefficient of friction in elasto-hydrodynamic
249 and hydrodynamic domains; asphalt A has twice as much asphaltene as asphalt B and presents a higher
250 coefficient of friction.

251 To confront laboratory aging and natural aging processes, the results obtained with the Rolling Thin
252 Film Oven Test + Pressure Aging Vessel treatment on asphalts A and B were compared to the results
253 obtained for asphalt extracted from the Reclaimed Asphalt Pavement (different from asphalt A and B),
254 which was naturally aged. This comparison was realized for information purposes only because no link
255 exists between asphalt A, B, and asphalt from RAP, and the information cannot be generalized to every
256 asphalt. The complete Stribeck curves are shown in Figure 6. It is important to keep in mind that the
257 origin and aging history of this RAP extract are unknown; however, the behavior of this asphalt seems

258 to be similar to that of asphalt A and less lubricating than asphalt B. In terms of asphaltene content,
 259 penetration grade, Ring and Ball Temperature, and Infra-Red oxidative index, asphalt A and RAP extract
 260 have some similarities (Table 1). This may explain the close behavior in the elasto-hydrodynamic and
 261 hydrodynamic domains. However, boundary and mixed domains presented higher coefficients of
 262 friction for the extracted asphalt, particularly at 110 °C and 100°C. However, the oxidation indices
 263 presented in Table 1 are slightly higher for A_{RAP} than for AA_{RTFOTPAV} [$I_{CO} (A_{RAP}) = 8,0$ $I_{CO} (AA_{RTFOTPAV}) = 6,7$
 264 $I_{SO} (A_{RAP}) = 22,6$ $I_{SO} (AA_{RTFOTPAV}) = 19,2$]. This difference in the oxidation state of asphalt from RAP
 265 originates from ultraviolet radiation, and water exposure which were not simulated in laboratory
 266 treatments. This radiation oxidation provides asphalt with higher viscosity (Table 1 and Figure S3) and
 267 may explain higher coefficient of friction in almost all Stribeck domains.

268 It was observed that aging induces an increase in asphaltene content which seems to influence
 269 lubricant behavior of asphalt. In this part, the effect of asphaltene content on the coefficient of friction

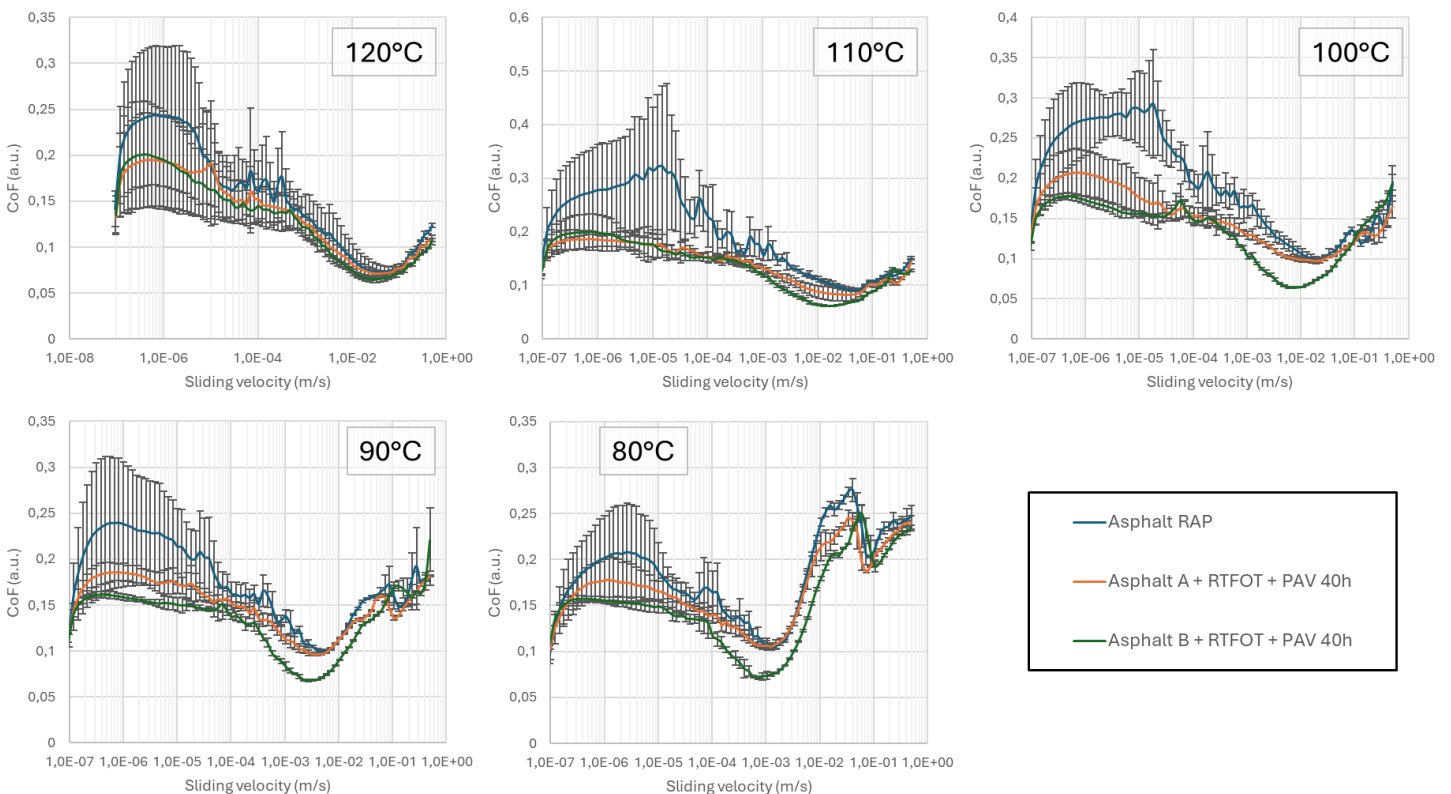


Figure 6: Comparison between asphalt sample A and B after RTFOT + PAV40h respectively in orange and green and asphalt extract from RAP in blue at 120, 110, 100, 90 and 80°C

270 is investigated based on a model system with controlled asphaltene content. Oxidized asphaltenes are
271 not considered to simplify the comprehension of these complex materials.

272 Systems with controlled asphaltene contents were first prepared. Samples with 0 (i.e., 100% maltene),
273 15 and 20.3 wt% of asphaltene were prepared for asphalt A. These specimens were analyzed using the
274 same experimental protocol, and the results are shown in Figure 7. Maltene, without N-heptane
275 extracted asphaltene, shows only the three first domain of the Stribeck curve expected at 80°C where
276 hydrodynamic regime appears. The other specimens presented all Stribeck domains, except at 120°C,
277 where the sample viscosity induced by the lower asphaltene content was probably too weak to
278 generate a hydrodynamic pressure in the velocity range investigated. These results indicate that an
279 increase in asphaltene content modifies the elasto-hydrodynamic, hydrodynamic regimes³⁹ and
280 viscosity^{37,40}. In these two domains, an increase of the asphaltene content generates an increase in the
281 coefficient of friction due to increased interaction between the particles. This probably generates
282 higher values of the first normal stress difference, implying an increase of the normal force generated
283 from the asphalt sample on the rheometer axis. In comparison, the boundary and mixed regimes are
284 not significantly affected.

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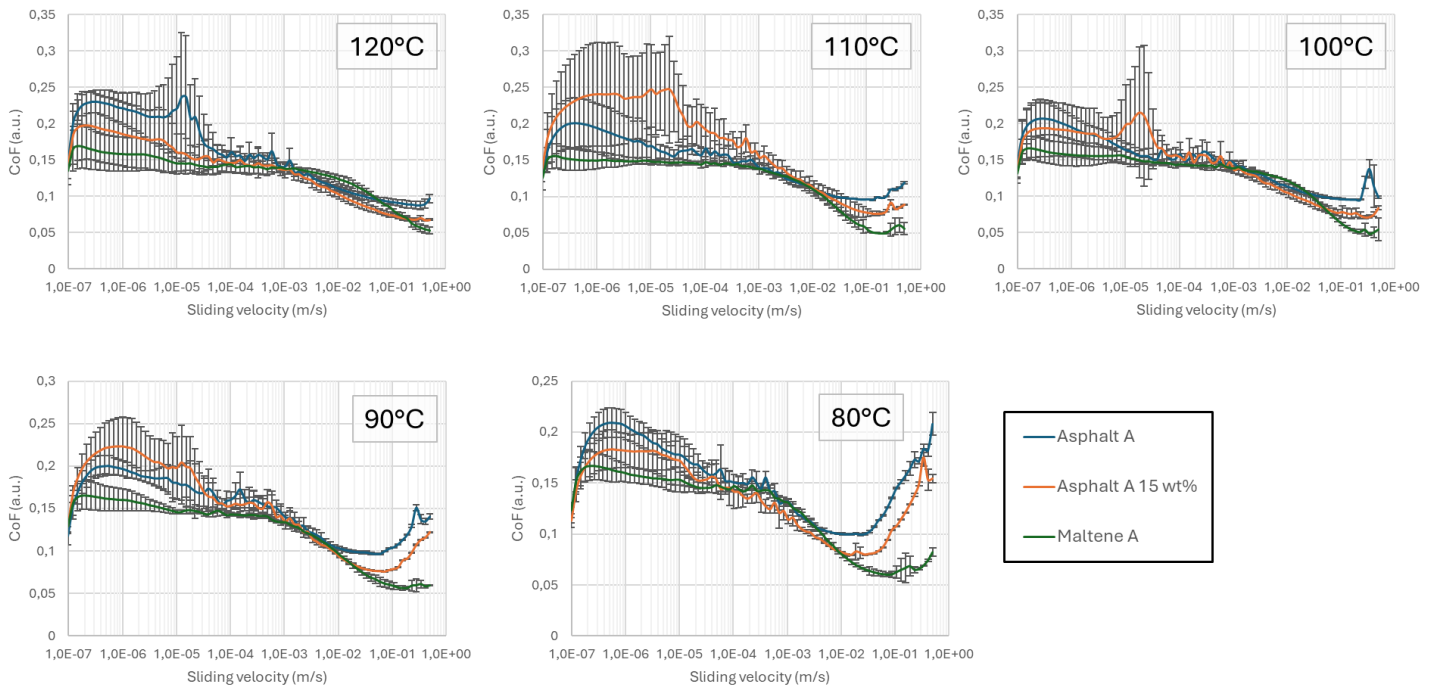


Figure 7: Stribeck curves of modified asphalt A reference asphalt at 20wt% in blue, asphalt A orange with 15 wt% of asphaltene in orange and maltene A in green at 120, 110, 100, 90 and 80°C

287

288 Similar experiments were performed on asphalt B to obtain two different asphaltene contents (11.4
 289 and 15.0 wt%) and results are shown Figure 8. All Stribeck domains appeared at all investigated
 290 temperatures. As a result, the influence of asphaltene content was highlighted. For asphalt B, the
 291 mixed, elasto-hydrodynamic, and hydrodynamic regimes were modified. An increase of the asphaltene
 292 content induces a higher CoF in all domains and a shift of these regimes at lower sliding velocities. As
 293 seen in the previous part of this study, asphalt B appears to be more sensitive to asphaltene variation
 294 as the oxidation state varies.

295 These results show that samples with different origins and similar viscosities, RBT, and penetration
 296 grades (Table 1) can evolve differently.

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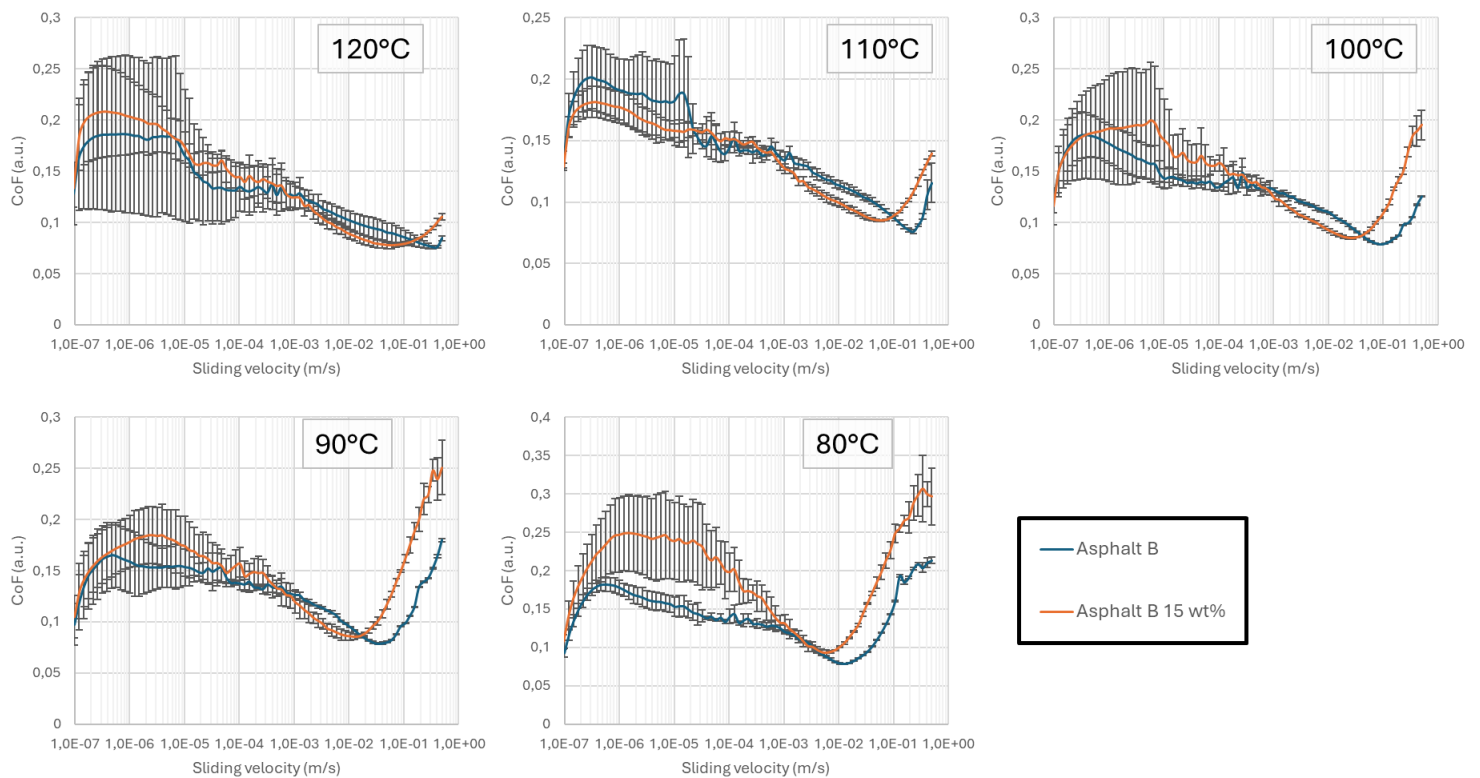


Figure 8: Stribeck curves of modified asphalt B reference asphalt at 10wt% in blue, asphalt A orange with 15 wt% of asphaltene in orange at 120, 110, 100, 90 and 80°C

300 The asphaltene content parameter can explain the shift of the four Stribeck domains at lower sliding
 301 velocities when it increases. For the modified asphalts presented in this section, the results show a
 302 trend when the asphaltene content increases, the minimal coefficient of friction also increases.
 303 However, for aged samples, this hypothesis cannot be applied. Nevertheless, the influence of the
 304 asphaltene content on the CoF in the hydrodynamic regime was similar for model and aged asphalts.
 305 The peptization of asphaltene (i.e. the amount of resin fraction), polarity of asphaltenes, and the initial
 306 aggregation state of asphaltenes may explain the differences between the model and aging methods.

307

308 4 Conclusion:

309 The aim of the present study was to examine the influence of natural aging of asphalt coming from
 310 reclaimed asphalt pavement and the effect of laboratory thermal aging of asphalts (RTFOT and PAV)
 311 on lubrication properties. The variation of the asphaltene content that occurs during the aging process

312 is the key parameter that governs tribological properties and were specifically studied using model
313 asphalts with controlled asphaltene content.

314 After short- and long-term laboratory aging, asphalts showed physical hardening induced by oxidative
315 reaction which generated an increase in ring and ball temperature, oxidative indices and asphaltene
316 content and a decrease in the needle penetrability. An increase of the asphaltene content increased
317 the viscosity at high temperature. Those observations agree with literature data.

318 The Rolling Thin Film Oven test of asphalts with a penetration grade of 35/50 showed different
319 behaviors depending on the origin of the asphalt. Generation of low quantity of oxidized asphaltene
320 could modify the lubricity ability of the asphalts both negatively and positively. In general, short-term
321 aging slightly modifies the coefficient of friction in elasto-hydrodynamic and hydrodynamic domains.
322 After long-term aging (RTFOT + Pressure Aging Vessel for 40h), the asphalt binders behaviors were
323 modified by a shift of all Stribeck domains at the smallest sliding velocities and a slight variation of the
324 minimal coefficient of friction (CoF). The CoF in hydrodynamic domains sharply increased, due to the
325 increase in asphaltene content, which induced a higher hydrodynamic pressure.

326 In general, asphalt aging induced an increase in viscosity and a similar effect can be observed in the
327 lubricity ability of asphalts in a hydrodynamic domain which comes from an increase of the coefficient
328 of friction. The model system analysis confirms the influence of asphaltene in the hydrodynamic
329 domain. Higher asphaltene content degrades the lubricity ability induced by an increase of particles
330 interactions. Asphalt lubricity, in addition to the viscosity parameter, can explain a part of asphalt
331 concrete workability issues when reclaimed asphalt pavement is used in the composition of a road.

332 This work extends collected data on asphalt samples from different sources with aging treatment. It
333 also demonstrates the efficiency of tribological analysis as a complementary tool to viscosity
334 measurement in order to investigate asphalt formulations composed of reclaimed asphalt pavement.
335 More experiments are still necessary to confirm and expand these general hypotheses to other
336 asphalts from different origins.

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