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Electrochemical analysis of carbon steel embedded in mortars with pretreated copper tailings as supplementary cementitious material

Carlos Sepúlveda-Vásquez^a, Nicolás Carrasco-Astudillo^b, Lisa Muñoz^c, Paulo Molina^d, Armelle Ringuedé^e, Carolina Guerra^f, Mamié Sancy^{b,d,g,*}

^a Departamento de Ingeniería Mecánica y Metalúrgica, Facultad de Ingeniería, Pontificia Universidad Católica de Chile, Santiago 7820436, Chile

^b Escuela de Construcción Civil, Facultad de Ingeniería, Pontificia Universidad Católica de Chile, Santiago 7820436, Chile

^c Instituto de Química, Facultad de Ciencias, Pontificia Universidad Católica de Valparaíso, Valparaíso, Chile

^d Millennium Institute in Green Ammonia, Pontificia, Universidad Católica de Chile, Santiago 7820436, Chile

^e Chimie ParisTech, PSL Research University, CNRS, Institut de Recherche de Chimie Paris, France

^f Optics and Photonics Group, Faculty of Engineering, University of Nottingham, University Park, Nottingham NG7 2RD, UK

^g Center for Research in Nanotechnology and Advanced Materials, Pontificia Universidad Católica de Chile, Pontificia Universidad Católica de Chile, Santiago 7820436, Chile

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ABSTRACT

The cement industry, responsible for 8% of global greenhouse gas emissions, necessitates developing sustainable materials to replace cement partially. This investigation examined the feasibility of using copper tailings, a byproduct of mining, as alternative materials for cement within mortars and reinforced mortars (0–15 wt%). The microstructural composition of the tailings was analyzed using scanning electron microscopy and X-ray diffraction. The corrosion resistance of mortars reinforced with copper tailings was elucidated through open-circuit potential measurements and electrochemical impedance spectroscopy. The results showed that incorporating 5 and 10 wt% of sieved copper tailings improved the mechanical strength and significantly enhanced the electrochemical stability, as indicated by more noble open-circuit potential values. Specifically, the sieved tailings played a crucial role in forming a more stable oxide film, which was confirmed by higher impedance values, suggesting a reduced corrosion rate. In contrast, mortars with 5 wt% of milled tailings exhibited properties like those of the control group. This electrochemical understanding highlights the potential of processed copper tailings in mitigating the environmental impact of cement production and enhancing the durability of cementitious composites.

1. Introduction

The cement industry produces nearly 4 billion tons annually of cement [1,2], which is equivalent to 8% of global carbon dioxide (CO₂) emissions [3], producing 1 ton of CO₂ for every 1 ton of cement [4,5]. It has been proposed that substituting cement with supplementary cementitious materials (SCMs) reduces cement production and associated emissions [6,7]. SCM consists of soluble powders mainly composed of silica, alumina, and calcium oxide [8,9]. The use of SCM contributes to the cement matrix through the effects of (1) pozzolanic and (2) filler [10], improving the mechanical behavior and durability.

The pozzolanic reactions have been related to forming portlandite $(Ca(OH)_2)$ and silicates. In this context, it was reported that the

replacement of cement by silica fume and metakaolin at 15 wt% reduced by half the water absorption when compared to concrete without replacement at 28 days, also increasing the surface electrical resistance of the concrete from 2.7 to 2.0 times compared to the concrete without replacement. Furthermore, it was reported that adding silica fume and metakaolin improved chloride ion penetration by more than 50% after 28 days, especially when using silica fume [11]. Also, Güneyisi et al. [12] reported that adding metakaolin between 5 and 15 wt% to the mixture increased its compressive strength by approximately 40% after 28 days, attributable to the improved cement paste and aggregate quality. The authors reported that the concrete durability improved due to decreased porosity generated by dense calcium silicate hydrate (C-S-H) production [9].

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^{*} Corresponding author at: Escuela de Construcción Civil, Facultad de Ingeniería, Pontificia Universidad Católica de Chile, Santiago 7820436, Chile. *E-mail addresses:* cdsepulveda@uc.cl (C. Sepúlveda-Vásquez), mamiesancy@uc.cl, mamiesancy@gmail.com (M. Sancy).

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Mining waste, such as copper tailings, has been proposed as an SCM. Copper production is nearly 20.000 tons per year, generating 200 tons of copper tailings per 1 to of refined copper [13]. Copper tailing is composed mainly of Albite (Na,Ca)Al(Si, Al)₃O₈), α-quartz (α-SiO₂), Sanidine (KAlSi₃O₈), and calcium silicates [14]. Several pre-treatments have been studied for copper tailing as SCM, such as calcination, which has shown better response between 700 and 850 °C. However, this process also generates CO₂ emissions and high energy costs. It is worth mentioning that cement replacement by copper tailing has focused between 0 and 20 wt% due to more significant replacements, which have reduced the mechanical properties and required further pre-treatment, increasing the energy costs. In this context, Onuaguluchi and Eren [15] reported replacing 10 wt% with copper tailings increased concrete compressive strength by 16.2% at 28 days. Dandautiya and Singh [16] studied the combined use of fly ash and copper tailings, finding that the most favorable mixture was using 20 wt% of cement replacement by fly ash and 5 wt% by copper tailings, increasing compressive strength by 8.27% and reducing CO₂ equivalent emissions by 14%. Esmaeili et al. [17,18] studied the replacement of copper tailings from mines in Iran, finding that compressive strength improved with cement replacement of up to 30% at 90 days, associated with the delayed response of the pozzolanic and filler effects, determining the optimal cement replacement to vary between 15 and 20 wt%. However, the SCM must often be pre-treated to optimize its performance through industrial processes involving significant energy costs. For instance, using kaolinite clays requires 10% of the thermal energy to be added as cement replacement [19]. Table 1 shows a resume of the mechanical behavior of different types of SCMs, revealing that industrial wastes, such as fly ash and stainless-steel slag, have better compressive responses than copper tailings, which was related primarily to the water-binder ratios with a similar percentage of cement replacement.

The effect of Cu tailing on corrosion over metallic bars embedded in mortar is currently unknown. Several studies have been conducted on another kind of SCM. For instance, Stefanoni et al. [23] and Hemkemeier et al. [24] have reported a reduction in the corrosion current density (I_{corr}) using fly ash and slag as SCM in reinforced structures. Koga et al. [25] studied the effect of fly ash addition through open-circuit potential (E_{OC}) measurements, reporting a shift of potential towards more positive values in samples with added fly ash, also reaching similar I_{corr} values after 365 days of exposure [26]. Montemor et al. [27] observed an increase in load transfer resistance with 30 wt% fly ash as cement replacement, increasing by a factor of 4 compared to samples without substitution. Zhang et al. [26] investigated copper tailings, proposing that the mortar's quality is the main factor determining the corrosion behavior, reporting matrix failure with replacements above 30 wt%.

This work studied the copper tailing as SCM after a milling treatment as cement replacement between 0% and 15%. Morphological, microstructural, and mechanical behavior of mortars were investigated using scanning electron microscopy, X-ray diffraction, compressive and flexural tests. The corrosion of steel carbon embedded in mortars was also analyzed using electrochemical techniques to understand the effect of the hydration and hardness processes over time.

2. Methodology

2.1. Conditions of copper tailing

The copper tailings were characterized upon receipt and then used as received with particles under the size of 75 μ m to replicate the condition of the commercial cement, which has particle size lower than this value [10,28,29]. Vargas et al. [10] characterized the particle size distribution for copper tailings as SCM, obtaining a size lower than 100 μ m. Cu tailing was also used after ball milling for 6 h in a ratio of copper tailings/balls of 1:2 with 600 rpm.

2.2. Copper tailing characterization

The chemical composition of the tailings was determined using Energy-Dispersive X-ray Spectroscopy (EDS) on an FE-SEM Model QUANTA 250 FEG. The crystallographic phase composition of the tailings was analyzed qualitatively using X-ray diffraction (XRD) on a Bruker D8 Advance diffractometer operating at 40 kV. Rigaku with a Cu K_{\alpha1,2}; K_{\beta} source. The diffractograms were recorded between angles $2\theta = 10^\circ - 80^\circ$, using a scan rate of 0.02 °•s⁻¹ at room temperature.

2.3. Sample fabrication

Mortar mixtures were manufactured with Portland cement replacement according to the ASTM C305- 20, using copper tailings as partial cement replacement, between 5 and 15 wt%, as received with particles under the size of 75 μ m and milled, as mentioned in Section 2.1. The water/cement (w/c) ratio was 0.5. Portland cement was also used according to ASTM C 595: Type P (Portland-Puzolanic Cement, Melon, Chile) and a commercial fine river sand (Maipo, Chile). Prismatic mortar samples were manufactured to be mechanically tested according to ASTM C109/C109M - 21, with dimensions of 40 mm ×40 mm x 160 mm, as shown in Fig. 1(a).

For corrosion analysis, carbon steel bars A630–420 H were used (CAP Acero, Chile) with a nominal composition of (wt%) 0.18–0.44 C, 0.6–1.1 Mn, \leq 0.012 P, \leq 0.04 S, \leq 0.15 Si, and Fe as the balance [30]. The bars were 150 mm long and 15 mm in diameter, which were cleaned through chemical pickling (3 times for 10 min.), drizzled with an anhydrous ethanol solution, and dried with warm air in agreement with ASTM G1. The carbon steel bars without striated were then painted with anticorrosive paint to isolate a specific geometrical area, as shown in Fig. 1(b). After 24 h, the metal samples were embedded in a mortar matrix. Fig. 2 shows an optical microscope image of interface carbon steel - mortar used for electrochemical measurements before exposure.

2.4. Mechanical evaluation

Compressive strength and flexural tests were performed on the prismatic samples after 7, 28, and 90 days of exposure to a saturated solution $Ca(OH)_2$ [31]. A constant velocity of 4 mm•min⁻¹ in compressive strength and 1 mm•min⁻¹ in flexural tests was applied

Table 1

Mechanical behavior of replacing different SCMs compared to a mix without replacement at 28 days.

Water/Binder Ratio	Cement replacement (wt%)	Compressive Strength at 28 days (f _{c- 28 days}) (MPa)	Flexural Strength at 28 days (f _{c- 28} _{days}) (MPa)	References				
0.485	25	9		[20]				
0.485	32	8	-					
0.485	10	26	7.5					
0.485	15	24	7					
Varied	0–10	63–40	Varied	[20]				
Varied	10-30	57.37-33.55	Varied	[20]				
0.43	25–40	52.2-41.6	Varied	[21,22]				
0.44-0.45	5–10	52.6–57.6	Varied	[21]				
	Water/Binder Ratio 0.485 0.44-0.45	Water/Binder Ratio Cement replacement (wt%) 0.485 25 0.485 32 0.485 10 0.485 15 Varied 0-10 Varied 10-30 0.43 25-40 0.44-0.45 5-10	Water/Binder Ratio Cement replacement (wt%) Compressive Strength at 28 days (f_{c-28} days) (MPa) 0.485 25 9 0.485 32 8 0.485 10 26 0.485 15 24 Varied 0–10 63–40 Varied 10–30 57.37–33.55 0.43 25–40 52.2–41.6 0.44–0.45 5–10 52.6–57.6	Water/Binder Ratio Cement replacement (wt%) Compressive Strength at 28 days (f_{c-28} $days$) (MPa) Flexural Strength at 28 days (f_{c-28} days) (MPa) 0.485 25 9 - 0.485 32 8 - 0.485 10 26 7.5 0.485 15 24 7 Varied 0-10 63-40 Varied Varied 10-30 57.37-33.55 Varied 0.43 25-40 52.2-41.6 Varied 0.44-0.45 5-10 52.6-57.6 Varied				



Fig. 1. Scheme representation of samples for (a) mechanical and (b) electrochemical analysis.



Fig. 2. Optical microscope image of interface carbon steel-mortar before exposure.

using a TC 10001 (homemade), which was done according to standards ASTM C109 – 21 and ASTM C348. All tests were measured in triplicate.

2.5. Thermogravimetry

The mortar samples were analyzed using thermogravimetry measurements after 28 days of hydration, which was halted by immersing in isopropanol using a Setsys Evolution ATG 16/18 easy fit by SETARAM equipment, which was set to stabilize at 40 °C for 20 min. The analysis was performed between 60 °C and 1200 °C using air as the sweep gas, with a heating rate of 10 °C min⁻¹.

2.6. Electrochemical evaluation

The electrochemical tests were measured during 56 days of exposure to a saturated Ca(OH)₂ solution at 23 °C [31]. Open circuit potential (E_{OC}), electrochemical impedance spectroscopy (EIS), and linear sweep voltammetry (LSV) measurements were carried out using a potentios-tat/galvanostat (Bio-Logic, VSP), a three-electrode electrochemical cell, with graphite and a saturated calomel electrode (SCE) employed as the counter and reference electrodes, respectively. The carbon steel embedded in mortar, described in Section 2.3, was used as the working electrode, with a geometrical exposure area of 45.23 cm². EIS data were collected between 65 kHz and 3 mHz, with seven points per decade, using a ± 10 mV peak-to-peak sinusoidal voltage at E = E_{OC} and E = $E_{OC} \pm 200$ mV. The EIS data were fitted using SIMAD (LISE UPR 15 CNRS, France) between 1 MHz and 1 μ Hz.

3. Results and discussion

3.1. Copper tailing characterization

Fig. 3 shows the morphology of Cu tailings at different conditions, which reveals a broader particle size distribution for the samples as received and milled (see Fig. 3a,c) than sieved Cu tailings, which have particle size under the size of 75 μ m (see Fig. 3b), which was determined using image analysis. The Cu tailing's particle size was slightly more significant than the cement particle size, as reported for SCM particle size by Serai et al. [29] and Vargas and Lopez [10]. It has been reported that using small particle sizes reduces the permeability, segregation, and attack of concrete sulfates, improving mechanical resistance and durability. The latter was related to the pozzolanic activity in ordinary Portland cement, forming calcium silicate hydrate (C-S-H) of second order [32] due to the presence of Portlandite (Ca(OH)₂) in the cement paste. In addition, it has been proposed that smaller particle sizes can benefit the cement hydration kinetic for SCM, improving the mechanical strength. However, the viscosity can also increase, reducing the workability [33,34].

Table 2 shows the chemical composition of Cu tailing obtained by XRD analyses, revealing that Si, Ca, and Al are the main chemical elements, forming different compounds, such as Albinite ((Na, Ca)Al(Si, Al)₃O₈) and a-quartz low (α -SiO₂), as main compounds in sieved and milled Cu tailing. For sieved Cu tailings, XRD analysis revealed the presence of Sanidine (KAlSi₃O₈) and other calcium aluminum silicates, such as 1.7 wt% Gehlenite (Ca₂Al(SiAl)O₇) 1.2 wt% Gmelinite (Ca₂K₄Na₄)(Si₈Al₄)O₂₄x11H₂O. For milled Cu tailings, XRD analysis also revealed the formation of quartz (SiO₂).

Fig. 4 shows the XRD patterns of copper tailing as received and milled, revealing the presence of Albite and α -quartz low for both samples, as reported by Qing et al. [35,36]. It is well known that the presence of α -quartz low in OPC influences the cement hydration and pozzolanic activity [37]. A reduction in pozzolanic activity was reported due to the presence of Albite and Sanidine [38]. In this case, the presence of Gehlenite and Gmelinite in the sieved Cu tailings and the trigonal structure of quartz in the milled Cu tailings could improve the pozzolanic activity [39], which could also play an essential role in the hydration of the mortar, consequently, in the mechanical properties.

The Cu tailing was also analyzed using X-ray fluorescence, determining that Fe, S, K, Ca, Cu, Pb, and Cd are permissible values according to Standard 503–40 CFR/1993 [40] and Australian Standard AS 4454–1999 [41], as previously reported by our group. These concentration values are similar to those found by other research groups [14, 18,42]. Therefore, copper tailing does not present harmful amounts for use as SCM.



Fig. 3. FE-SEM images of Cu tailing. (a) As received, (b) sieved under 75 µm, and (c) milled as pre-treatment.

Table 2 Chemical composition (in wt%) analysis of copper tailing obtained by XRD analyses.

Sample	Albite	α -quartz low	Sanidine	quartz	Others
Sieved	57.3	30.7	9.1		2.9
Milled	50.5	25.2	-	24.3	-



Fig. 4. X-ray pattern of Cu tailing as received (low) and milled (up).

3.2. Mechanical mortars analysis

Fig. 5 shows the samples' compression strength (f_c) as a function of exposure time and cement replacement of Cu tailing sieved and milled. Both graphs reveal an enhancement by adding 5 and 10 wt% sieved Cu tailings over time, which is even more significant than control mortar without replacement. However, incorporating milled Cu tailings decreased the compression strength, except using 5 wt% of milled Cu tailing after 90 days of exposure, suggesting that the milled Cu tailing

can delay the hydration reactions in mortars. In all cases, it is observed that the mortar strengthening is adversely affected as the replacement percentage increases. Fig. 5(a) shows that after 90 days of maturation, the mortar with 5 wt% Cu tailing can reach 38 MPa while the control comes to 31 MPa, demonstrating that Cu tailing with small particles is suitable from a mechanical point of view. Onuaguluchi and Eren [15] reported increased compressive strength, adding 10 wt% of Cu tailings to concrete after 28 days, using 50% of particle size was lower than 0.1 mm. Dandautiya and Singh [16] determined that the compressive strength improved by incorporating 20 wt% of fly ash and 5 wt% of Cu tailings without any pre-treatment by the SCM. Esmaeili et al. [17,18] also reported better compressive strength using 15–20 wt% of Cu tailings after 90 days, which was milled for 3 h at 300 rpm to reach particle size finer than 45 μ m for the 98%.

Fig. 6 shows samples' flexion strength (f_m) as a function of exposure time and cement replacement, revealing an improvement using sieved Cu tailing compared with the milled treated, which is more drastic for more prolonged exposure. However, no clear trend was observed for milled samples, and better results were found for longer exposure time. Vargas et al. [43] analyzed the pre-treatment of Cu tailing as SCM by a modeling approach, reporting an enhancement of the mechanical behavior on the concrete mixture by adding 20 wt% of Cu tailings thermally treated [44].

3.3. Thermogravimetry

Fig. 7 shows the thermogravimetric analysis of the mortars with different replacements after 28 days, revealing thermal zones related to (I) gypsum dehydrated at below 150 °C, (II) Portlandite (Ca(OH)₂) dehydroxylates (Ca(OH)₂ \rightarrow CaO + H₂O) between 400 and 600 °C, and (III) Calcium carbonate (CaCO₃) decomposes above 600°C to CaO and CO₂, as previously reported by Lothenbach et al. [45]. Also, Fig. 7 shows that the first thermal step between room temperature and 150 °C was reduced by adding 5 wt% of sieved Cu tailing. A similar peak is observed for samples with 5 and 15 wt% milled Cu tailings. Madali and Wei [46]



Fig. 5. Maximum compressive strength of mortars with different cement replacements after exposure to saturated Ca(OH)₂ solution at room temperature. (■) 0 wt%, (●) 5 wt%, (▲) 10 wt%, (★) 15 wt% of cement replacement.



Fig. 6. Maximum flexion strength of mortars with different cement replacements after exposure to saturated Ca(OH)₂ solution at room temperature. (a) Sieved to particle size $< 75 \ \mu\text{m}$ and (b) milled Cu tailings. (**b**) 0 wt%, (**b**) 5 wt%, (**b**) 10 wt%, (**c**) 15 wt% of cement replacement.



Fig. 7. The effect of Cu tailings replacement on the thermogravimetric analysis of the mortars after 28 days. (a) Sieved to particle size <75 µm and (b) milled Cu tailings. (-) 0 wt%, (-, -) 5 wt.%, and (-, -) 15 wt% of cement replacement.

related this step thermal to the evaporation of the adsorbed and interlayer water (30–109 °C) and dehydration of C-S-H [45]. However, a step thermal at 130 °C was previously attributed to the dehydration of Ettringite and gypsum [47], as described by Katsioti et al. [48]. Additionally, the incorporation of 5 wt% of sieved Cu tailings and 15 wt% of milled Cu tailings increased the peak at 450 °C, which was associated with the Portlandite dehydroxylates (Ca(OH)₂) [45,48]. A few dehydration products were formed by adding 15 wt% of Cu tailing, as sieved and milled.

3.4. Electrochemical behavior of carbon steel embedded in mortars

Fig. 8 shows the evolution of the carbon steel bar's open circuit potential (E_{oc}) embedded in the mortar through exposure, with different cement replacement percentages. As can be seen, the E_{oc} was shifted to more positive values at earlier stages, which was more drastic for sieved samples than milled samples, suggesting a slight improvement in the protection of the steel bar, according to the ASTM C876 standard [49]. All samples reached a more stable E_{oc} value between -200 mV and



Fig. 8. Open circuit potential of carbon steel embedded in mortars after 56 days of exposure to $Ca(OH)_{2,sat}$ at room temperature with different cement replacements by Cu tailing over time. (a) Sieved to particle size <75 μ m and (b) milled Cu tailings. (\blacksquare) 0 wt%, (\blacklozenge) 5 wt%, (\bigstar) 10 wt%, (\bigstar) 15 wt% of cement replacement.

-300 mV for longer exposure time. Fig. 7 shows the E_{oc} values with their standard deviation, which had excellent reproducibility. Stefanoni et al. [23] proposed that the more negative potentials of the metallic bar embedded in carbonated concrete were associated with a high corrosion rate. Hemkemeier et al. [24] reported a shift in the E_{oc} values using industrial wastes in cementitious composites, which was attributed to the wetting/drying cycles, increasing the porosity.

Fig. 9 shows the Nyquist diagrams, at different frequency ranges, of reinforced mortar samples after exposure at $E=E_{oc}$ as a representative example of studied systems, revealing two capacitive loops superimposed, one of them at very high-frequency ranges (HF), between 65 kHz and 100 Hz, and another between medium and low-frequency ranges (MF-LF), between 100 Hz and3 mHz. Fig. 9(a) shows the impedance response at the whole frequency range with slight differences between samples between HF and LF ranges. Fig. 9(b) shows a zoom of the VHF, revealing a shift of the real part of the impedance with 10 wt% and 15 wt% of cement replacement concerning the control specimens. Fig. 9(c) shows the experimental and simulated impedance data.

Novoa et al. [50,51] proposed that the impedance response at VHF can be related to the porosity, pore size distribution, and dielectric properties of cementitious cover. Therefore, the changes produced at VHF could be provoked by the cement replacement, which modified the mortar bulk morphology, as was demonstrated by the compressive strength. Fig. 9(c) shows the Nyquist diagrams between MF-LF, which is associated with the response of the carbon steel bars embedded in the cementitious matrix, clearly revealing the changes between samples. Meyer et al. [52] related the impedance response of reinforced concretes to the porous electrode behavior, reporting two-time constants.

Fig. 10 shows the Nyquist diagrams of reinforced mortar samples without partial cement replacement after 28 days of exposure, comparing the impedance response at open circuit potential ($E=E_{oc}$) with cathodic and anodic overpotentials ($E=E_{OC}\pm 200$ mV). No significant differences are observed, suggesting that the impedance response at $E=E_{oc}$ is under mixed control between the anodic and cathodic reactions.

Fig. 11 shows the equivalent circuits proposed for reinforced mortar samples after exposure, based on that proposed by Novoa et al. [50] for the range VHF to MF and for the range MF to LF based on that proposed by Shi et. al, [53], Shi et al. [54] and Feng et al. [55], where R_{PP} represents the percolating pore resistance, which is interconnected pores, and in parallel is R_{OP} corresponds to the occluded pore resistance. In this case, non-ideal capacitances were included for the bulk part and occluded in mortar parts, which were expressed by a constant phase element (CPE) [56,57] by Q coefficient and α . Therefore, Q_{bulk}, α_{bulk} , and Q_{OP}, α_{OP} , respectively. The experimental impedance data at VHF,



Fig. 10. Nyquist diagrams of carbon steel embedded in mortar samples without Cu tailing addition after 28 days of exposure to Ca(OH)_{2,sat} solution after 28 days at (\Box) E = E_{OC}, (x) E = E_{OC} +200 mV, and (+) E = E_{OC}-200 mV.

collected between 65 kHz and 100 Hz, were adjusted and simulated between 40 MHz and 100 Hz, as shown in Fig. 8(b). In addition, R_{oxide} represents the oxide resistance, $R_{ct,a}$ is the anodic charge transfer resistance, CPE_{oxide} corresponds to the oxide capacitance, and CPE_{dl} is the double-layer pseudo-capacitance.

Fig. 12 shows the evolution of the CPE parameters of the bulk cementitious part as a function of exposure time, revealing that the Q_{bulk} values decreased drastically up to 24 h, from 10^{-6} to 10^{-10} F•s^(α -1)•cm⁻², with higher variation for sieved than milled samples, possibly due to the hydration reactions in mortars. Later, the Q_{bulk} coefficient reached more stable values, around 10^{-10} to 10^{-9} F•s^(α -1)•cm⁻² for both samples. It should be noted that using 15 wt% of sieved Cu tailing, the Q_{bulk} coefficient did not vary dramatically during the exposure.

Liu et al. [58] reported Q values slightly higher using basalt fiber as a new anti-corrosion for steel bars in reinforced concrete, which is environmentally friendly and improves the durability of samples. Meyer et al. [52] studied the corrosion behavior of embedded steel bars into concretes, estimating Q values for the concrete parts that varied between 10^{-12} to 10^{-9} F•cm⁻² between 7 and 28 days of exposure. Fig. 12 also shows the variation of α related to the bulk cementitious part as a



Fig. 9. Nyquist diagrams of carbon steel embedded in mortar samples after 28 of exposure to Ca(OH)_{2, sat} at room temperature, and $E = E_{OC}$. (a) all frequency range, (b) VHF, and (c) HF to LF range. (**b**) 0 wt%, (**a**) 10 wt%, (**b**) 15 wt% of Cu tailings. (-) Simulated impedance data.



Fig. 11. A proposed equivalent circuit for carbon steel embedded in mortars after $Ca(OH)_2$ exposure at $E=E_{OC}$.

function of exposure time, revealing a drastic increase up to 28 days, from 0.5 to 0.9 approximately, keeping constant close to 0.9, which can be related to an evolution of the hydration process more homogenous. Andrade et al. [59] proposed that the morphological changes in mortars and concretes can be attributed to the structure's durability related to changes in porosity and conductivity.

Fig. 13 shows the evolution of the percolating and occluded pores

resistances (R_{PP} , R_{OP}) as a function of exposure time, revealing that the R_{PP} increased up to 28 days, from 10^3 to $10^5 \Omega \bullet cm^2$, which was more significant for sieved samples than milled samples. It should be noticed that for sieved samples, the R_{PP} increased more dramatically as the cement replacement increased. However, R_{PP} was not influenced significantly by adding Cu tailing for milled samples, suggesting that the filler effect was determined by the particle size of the SCM and its distribution. Fig. 13(c,d) shows that R_{OP} increased during exposure, from 10^3 to $10^5 \Omega \circ cm^2$, revealing again a more significant effect applying sieved than milled as pre-treatment for Cu tailings, suggesting an influence of the surface area on the pozzolanic activity of Cu tailings as SCM. R_{OP} of sieved Cu tailing is increasing in value compared to the control sample as the replacement rises, demonstrating a decrease in the quantity of occluded porous due to the filled effect of sieved Cu tailing. Instead, the ROP of Cu milled tailing showed similar values to the control sample, which reveals that the pretreatment provokes a change in the crystallinity of some species, observed in the increase of peaks in the Xray diffraction (XRD), as shown in Fig. 4, which loses its pozzolanic effect compared to sieved tailing.

As mentioned above, the impedance data of carbon steel bars embedded in mortars in MF and LF ranges were measured between 100 Hz to 3 mHz, which were adjusted using the equivalent circuit shown in Fig. 11 [51,59,60]. CPE parameters were used to replace the pure capacitance, as proposed by Gonzalez et al. [61] and Liu et al. [62]. Impedance data were also simulated, reaching frequency values close to 10^{-6} Hz, as shown in Figs. 9(c) and 14, showing the experimental impedance data as symbols and simulated data as lines. Fig. 14(a) reveals that the impedance modulus was influenced by incorporating Cu tailings at the MF to LF range, showing lower values using sieved samples with 15 wt% of cement replacement. Moreover, Fig. 14(b) shows that the corrected phase was close to 75° between 10^{-2} and 10^{1} Hz for all samples, which was attributed to a CPE behavior possibly due to the formation of an oxide layer on the steel bars, which is n. The significant effect of the electrolyte correction on Bode plots can be associated with



Fig. 12. Constant phase element parameters related to carbon steel's bulk cementitious part mortar embedded in mortars after Ca(OH)_{2,sat} exposure at T_{amb} and $E=E_{oc}$. (a,c) Sieved to particle size < 75 µm and (b,d) milled Cu tailings. (\blacksquare) 0 wt%, (\bullet) 10 wt%, (\star) 15 wt% of cement replacement.



Fig. 13. Effect of copper tailings replacement on the (a,b) percolating pore and (c,d) occluded pore resistance of carbon steel embedded in mortars after Ca(OH)_{2, sat} exposure at room temperature. (a,c) Sieved to particle size $< 75 \,\mu\text{m}$ and (b,d) milled Cu tailings. (\blacksquare) 0 wt%, (\blacklozenge) 5 wt%, (\bigstar) 10 wt%, (\bigstar) 15 wt% of cement replacement.



Fig. 14. Bode plots corrected by the electrolyte resistance of carbon steel embedded in mortar after 28 days of exposure to saturated Ca(OH)₂ solution at $E = E_{OC}$. (a) Modulus and (b) phase as a function of frequency range. (-) Simulated data at the LF range. (\blacksquare) 0 wt%, (\blacktriangle) sieved 10 wt%, and (\bigstar) milled 15 wt% of cement replacement.

the geometrical effect of the electrochemical cell used for this kind of system.

Fig. 15 shows the variation of the impedance modulus obtained from the simulated impedance data between medium and low frequency, 10^{-4} and 10^{-6} Hz ($|Z|_{MF}$ to LF), revealing an increase up 28 days of exposure in the control sample but also with sample with its thickness and passive properties up to 28 days, but decreased after 28 days. Therefore, a continuous oxide film formation/dissolution mechanism can be developed in this system. It is worth mentioning that the sample with sieved Cu tailings reached similar $|Z|_{LF}$ values for more prolonged exposure, possibly related to the reduction of the hydration reaction rate. In addition, *a* much better reproducibility is observed using sieved Cu tailing than milled, suggesting *a* more stable oxide film formed in samples using sieved Cu tailing than milled. Fig. 16 shows the evolution of the R_{oxide} determined from the impedance fit parameters. Adding milled Cu tailing increased R_{oxide} more significantly than the control sample and sieved Cu tailings. However, a reduction of R_{oxide} was determined using cement replacement after 56 days, suggesting that the Cu tailings can temporarily increase the oxide conductivity. Shi et al. [54] and Feng et al. [55] reported R_{oxide} values slightly lower values for this kind of system in marine media, close to $10^5 \ \Omega \ \text{cm}^2$, which also increased over time.

4. Conclusion

Mortar and reinforced mortar specimens were prepared using copper tailings contents to evaluate their suitability as supplementary cementitious materials, analyzing the mechanical and electrochemical effects.



Fig. 15. The effect of cement replacement on the impedance fit module between medium and low frequency of carbon steel embedded mortars in the MF to LF range. (a) Sieved to particle size $< 75 \mu m$ and (b) milled Cu tailings. (**b**) 0 wt%, (**b**) 5 wt%, (**b**) 10 wt%, (**c**) 15 wt% of cement replacement.



Fig. 16. The effect of Cu tailings as cement replacement on oxide resistance of carbon steel embedded mortars after Ca(OH)_{2, sat} exposure at T_{room} . (a) Sieved to particle size < 75 µm and (b) milled. \blacksquare) 0 wt%, (\blacklozenge) 10 wt%, (\bigstar) 15 wt% of cement replacement.

The results allowed us to conclude that Cu tailing in as received condition but with small particles has an adequate pozzolanic capacity. However, a critical cement replacement would be close to 10 wt%. This conclusion is based on:

- (1) The mechanical strengthening response was enhanced when Cu tailing was sieved in contrast with a control mortar. However, when Cu tailing is milled, it demonstrates a loosening of its strengthening properties.
- (2) A pretreatment such as milling for Cu tailing reduced its pozzolanic properties due to an increase in the crystallinity of the SiO₂, which resulted in a poor reaction during the mortar's maturity.
- (3) The electrochemical response of carbon steel embedded in mortars revealed that samples with Cu tailing as cement replacement can reach similar corrosion properties after more prolonged exposure.

Declaration of Competing Interest

The authors declare the following financial interests/personal relationships which may be considered as potential competing interests. Carlos Sepulveda-Vasquez reports financial support was provided by National Agency for Research and Development. If there are other authors, they declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

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