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1 The influence of water-saturation on the strength of volcanic rocks
2 and the stability of lava domes

3

4 **Michael J. Heap^{1,2*}, Claire Harnett^{3*}, Jamie Farquharson^{4,5}, Patrick Baud¹, Marina**
5 **Rosas-Carbajal⁶, Jean-Christophe Komorowski⁶, Marie E.S. Violay⁷, H. Albert Gilg⁸,**
6 **and Thierry Reuschlé¹**

7

8 ¹ *Université de Strasbourg, CNRS, Institut Terre et Environnement de Strasbourg, UMR 7063,*
9 *5 rue René Descartes, Strasbourg F-67084, France*

10 ² *Institut Universitaire de France (IUF), Paris, France*

11 ³ *School of Earth Sciences, University College Dublin, Dublin, Ireland*

12 ⁴ *Institute for Research Administration, Niigata University, Ikarashi 2-8050, Nishi-ku,*
13 *Niigata 950-2181, Japan*

14 ⁵ *Research Institute for Natural Hazards and Disaster Recovery, Niigata University, Ikarashi*
15 *2-8050, Nishi-ku, Niigata 950-2181, Japan*

16 ⁶ *Université de Paris Cité, Institut de Physique du Globe de Paris, CNRS, F-75005 Paris,*
17 *France*

18 ⁷ *Laboratory of Experimental Rock Mechanics, Ecole Polytechnique Fédérale de Lausanne,*
19 *Lausanne, Switzerland*

20 ⁸ *TUM School of Engineering and Design, Technical University of Munich, Arcisstrasse 21,*
21 *80333 Munich, Germany*

22

23 *Corresponding authors: Michael Heap (heap@unistra.fr) and Claire Harnett
24 (claire.harnett@ucd.ie)

25

26 **Abstract**

27 The rocks forming a volcanic edifice or dome are typically saturated or partially-
28 saturated with water. However, most experiments aimed at better understanding the
29 mechanical behaviour of volcanic rocks have been performed on dry samples, and therefore
30 most large-scale models designed to explore volcano stability have used parameters
31 representative for dry rock. Here, we present a combined laboratory and modelling study in
32 which we (1) quantified the influence of water-saturation on the mechanical behaviour of
33 variably altered dome rocks from La Soufrière de Guadeloupe (Eastern Caribbean) and (2)
34 used these new data to investigate the influence of water on dome stability. Our laboratory
35 data show that the ratio of wet to dry uniaxial compressive strength (UCS) and Young's
36 modulus are $\sim 0.30\text{--}0.95$ and $\sim 0.10\text{--}1.00$, respectively. In other words, the dome rocks were
37 all mechanically weaker when water-saturated. Further, the ratio of wet to dry UCS decreased
38 with increasing alteration (the wt% of secondary minerals in the rocks). Micromechanical
39 modelling suggests that the observed water-weakening is the result of a decrease in fracture
40 toughness (K_{IC}) in the presence of water. The ratio of wet to dry K_{IC} also decreases with
41 increasing alteration, explaining why water-weakening increased as a function of alteration.
42 To explore the influence of water-saturation on lava dome stability, we numerically generated
43 lava domes in Particle Flow Code using the experimental data corresponding to unaltered and
44 altered rock under dry conditions. The strength of the dome-forming rocks was then reduced
45 to values corresponding to wet conditions. Our modelling shows that, although the stability of
46 the unaltered dome was not influenced by water-saturation, larger displacements were
47 observed for the wet altered dome. Additional simulations in which we modelled a buried
48 alteration zone within an otherwise unaltered dome showed that higher displacements were
49 observed when the dome was water-saturated. We conclude that (1) the water-saturation
50 reduces the UCS and Young's modulus of volcanic rock, (2) larger decreases in UCS in the

51 presence of water are observed for altered rocks, and (3) the stability of a dome can be
52 compromised by the presence of water if the dome is altered, or contains an altered zone.
53 These conclusions highlight that the degree of alteration and water-saturation should be
54 mapped and monitored at active volcanoes worldwide, and that large-scale models should use
55 values for water-saturated rocks when appropriate.

56

57 **Highlights**

- 58 • Water-saturation reduces the strength of volcanic rocks.
- 59 • Strength reduction in the presence of water is higher for altered volcanic rocks.
- 60 • Strength reduction in the presence of water can be explained by a reduction in fracture
61 toughness.
- 62 • The stability of altered flanks and domes, or those containing altered zones, is
63 jeopardised by water-saturation.

64

65 **Keywords:** hydrothermal alteration; uniaxial compressive strength; Young's modulus;
66 fracture toughness; dome collapse

67

68 **1 Introduction**

69 The short- and long-term strength of rock is influenced not only by pore fluid
70 pressures, but also by the presence of water within the void space (e.g., Baud et al., 2000;
71 Brantut et al., 2013). Based on experiments and micromechanical analyses, reductions to the
72 strength of rock associated with the presence of water are thought to be the result of a
73 decrease in specific surface energy, friction coefficient, and fracture toughness (e.g., Baud et
74 al., 2000; Noël et al., 2021), the presence of clay minerals (e.g., Hawkins and McConnell,
75 1992; Heap et al., 2019), adsorption pressures (e.g., Risnes et al., 2005), and/or an increase in
76 the efficiency of sub-critical crack growth (e.g., Atkinson, 1984; Kranz et al., 1982; Masuda,
77 2001; Brantut et al., 2013; Tang et al., 2018).

78 Although water-weakening (the ratio of wet to dry strength) is reasonably well-
79 documented for sedimentary rocks such as sandstone (e.g., Rutter and Mainprice, 1978; Chest
80 and Logan, 1986; Hawkins and McConnell, 1992; Baud et al., 2000; Duda and Renner, 2013;
81 Wasantha and Ranjith, 2014; Baud et al., 2015; Heap et al., 2019; Noël et al., 2021),
82 carbonate rocks (e.g., Risnes et al., 2005; Baud et al., 2016; Nicolas et al., 2016; Castagna et
83 al., 2018), and gypsum (e.g., Caselle et al., 2022), few data exist for volcanic rocks. For
84 example, the onset of inelastic compaction in tuff from Alban Hills (Italy) was reduced by the
85 presence of water (Zhu et al., 2011). The uniaxial compressive strength of tuffs containing
86 alunite (Heap et al., 2015) and zeolites and clay minerals (Heap et al., 2018) was also lower in
87 the presence of water. Zhu et al. (2016) found that the compressive strength of a basalt from
88 Mt Etna (Italy) was reduced in the presence of water. Finally, the uniaxial compressive
89 strength and Young's modulus of lava from Mount Unzen (Japan) (Kendrick et al., 2021) and
90 the uniaxial compressive strength of lavas from Mt. Etna, Volvic (France), Kumamoto
91 (Japan), and Volcán de Colima (Mexico) and a block-and-ash flow from Mt. Meager
92 (Canada) (Heap and Violay, 2021) were reduced when saturated with water. While wet to dry

93 strength ratios in these lavas, which were relatively unaltered, were typically on the order of
94 ~ 0.9 , this ratio in tuffs can reach ~ 0.2 when the tuffs are characterised by high zeolite and clay
95 mineral contents (Heap and Violay, 2021).

96 The importance of understanding the influence of water on the strength and
97 mechanical behaviour of volcanic rocks is threefold. First, volcanoes are often saturated or
98 partially-saturated and, in the case of subaerial, ocean-island, and coastal volcanoes, their
99 saturation state also varies as a function of space and time (Hurwitz et al., 2003; Join et al.,
100 2005; Aizawa et al., 2009; Delcamp et al., 2016). Volcanoes can be saturated, or partially-
101 saturated, with meteoric water (approximately 45% of the world's active volcanoes lie in the
102 Tropics and are therefore prone to intense periods of precipitation; Matthews et al., 2002),
103 magmatic fluids, and, in the case of submarine, ocean-island, and coastal volcanoes, seawater.
104 Second, modelling has shown that the mechanical properties of dome- and edifice-forming
105 rock plays a first-order role in dictating their stability and therefore the risk of collapse and
106 associated hazards (Apuni et al., 2005; Reid et al., 2001; Moon et al., 2009; Borselli et al.,
107 2011; Schaefer et al., 2013; Heap et al., 2021a, b; Harnett and Heap, 2021; Wallace et al.,
108 2022; Mordensky et al., 2022; Harnett et al., 2022; Carr et al., 2022; Heap et al., 2023a) and
109 so, if water-saturation reduces rock strength, it must also decrease dome and edifice stability.
110 As a result, volcano stability models should, where and when appropriate, use the mechanical
111 properties for water-saturated volcanic rocks. Third, the frequency of landslides and the
112 failure and collapse of volcanic slopes and domes increases following heavy rainfall (Kerle
113 and van Wyk de Vries, 2001; Matthews et al., 2002; Matthews and Barclay, 2004; Elsworth et
114 al., 2004; Simmons et al., 2004; Taron et al., 2007; Saucedo et al., 2008; Hicks et al., 2010;
115 Vázquez et al., 2022), which will likely be exacerbated by the increase in heavy rainfall
116 expected given the current trend in global warming (Farquharson and Amelung, 2022).

117 The paucity of laboratory data aimed at understanding water-weakening in volcanic
118 rocks therefore hinders our ability to accurately assess the stability of volcanic domes and
119 flanks. Further, not only does hydrothermal alteration, a very common process at active
120 volcanoes worldwide, influence the physical and mechanical properties of volcanic rocks
121 (e.g., Frolova et al., 2014; Pola et al., 2014; Wyering et al., 2014; Mordensky et al., 2018;
122 Coats et al., 2018; Heap et al., 2021a, 2021b; Darmawan et al., 2022; Kanakiya et al., 2021,
123 2022; Schaefer et al., 2023), but altered rocks can also contain the minerals (e.g., alunite and
124 clay minerals) that are thought to promote water-weakening in volcanic rocks, and so it is of
125 particular importance to assess the influence of hydrothermal alteration on water-weakening
126 in volcanic rocks. Indeed, a deadly debris avalanche at Casita volcano (Nicaragua) in 1998
127 was thought to be the combined result of hydrothermal alteration and heavy rainfall (van Wyk
128 de Vries et al., 2000; Kerle and van Wyk de Vries, 2001; Kerle et al., 2003; Opfergelt et al.,
129 2006), and a landslide on the northeastern side of the dome at Soufrière de Guadeloupe
130 (Eastern Caribbean) that followed extreme rainfall exposed a collapse scar characterised by
131 intensely hydrothermally altered materials (Figure 1b).

132 Here, therefore, we present the results of (1) experiments designed to assess the
133 influence of water-saturation on the mechanical properties of variably-altered volcanic rocks
134 and (2) large-scale numerical modelling in Particle Flow Code (PFC), informed by the
135 experimental data, that investigates the influence of water on the stability of a volcanic dome.
136 For the purpose of this study, we will use La Soufrière de Guadeloupe as a case study.

137

138 **2 Case Study: La Soufrière de Guadeloupe (Eastern Caribbean)**

139 La Soufrière de Guadeloupe is an active andesitic stratovolcano located on the French
140 island of Guadeloupe in the Eastern Caribbean (Komorowski et al., 2005; Moretti et al., 2020;
141 Figure 1a). Volcanic unrest at La Soufrière de Guadeloupe has been steadily increasing in the

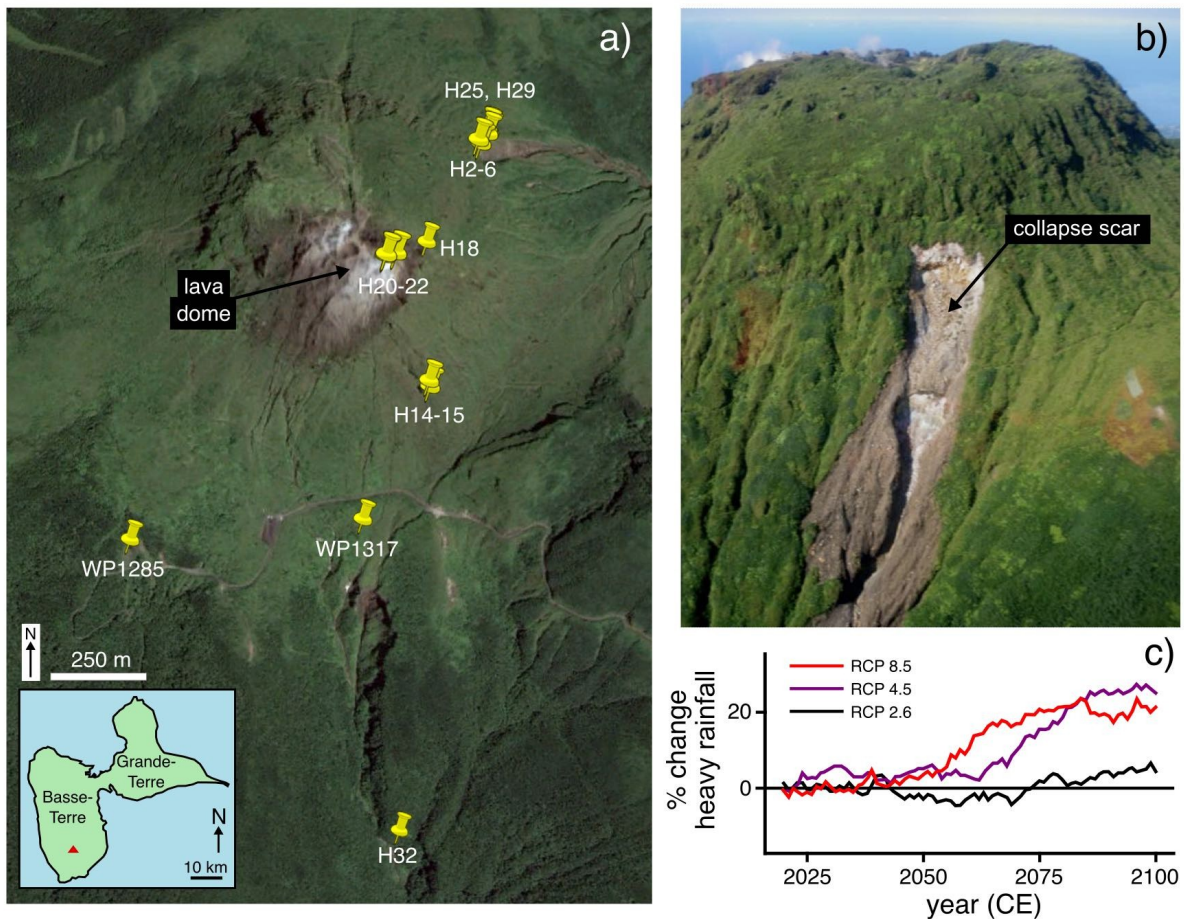
142 last three decades, manifest as an increase in the number of acid chloride-sulfate springs and
143 steam-dominated fumaroles, an increase in the heat output from the dome, an expansion of the
144 outgassing area at the top of the dome, an increase in flank and summit displacement rates,
145 and an abundance of shallow seismicity (including the largest felt tectonic earthquake since
146 the last eruption in 1976–1977, and frequent earthquake swarms that include several hundreds
147 of microearthquakes generated in few days and located within 1 km below the surface)
148 (Brombach et al., 2000; Villemant et al., 2005; Tamburello et al., 2019; Moretti et al., 2020;
149 Heap et al., 2021a; Jessop et al., 2021; Moune et al., 2022). Historically, partial edifice
150 collapse has been common at La Soufrière de Guadeloupe: at least nine flank collapses have
151 occurred in the last 9150 years (Boudon et al., 2008; Komorowski et al., 2005; Legendre,
152 2012; Peruzzetto et al., 2019), the most recent of which occurred in 1530 CE. Extensive
153 hydrothermal alteration has been identified as having a key role in these events, based on the
154 abundance of altered materials within the associated debris avalanche deposits (Komorowski
155 et al., 2005; Le Friant et al., 2006; Salaün et al., 2011; Rosas-Carbajal et al., 2016; Peruzzetto
156 et al., 2019; Heap et al., 2021a). The existence of hydrothermal fluid reservoirs (Brothelande
157 et al., 2014; Rosas-Carbajal et al., 2016, 2017), perched aquifers (Lesparre et al., 2014), and
158 the presence of listric-shaped, low-strength layers formed by the superposition of flank-
159 collapse slip surfaces within and below the La Soufrière de Guadeloupe dome are anticipated
160 to control the dynamics of future flank collapses and other mass-wasting phenomena (Rosas-
161 Carbajal et al., 2016; Peruzzetto et al., 2019).

162 Meteoric fluid input is also significant at La Soufrière de Guadeloupe; for example,
163 fumarole analysis by de Bremond d'Ars and Gibert (2022) highlights that meteoric input
164 exerts an importance control on the otherwise seemingly stochastic dynamics of the shallow
165 hydrothermal system. The volcano consistently receives over 5 m of rainfall annually at its
166 summit (Dessert et al., 2015), with as much as 30% of meteoric input infiltrating into the

167 groundwater (Rad et al., 2007). As a result, frequent smaller mass-wasting events at La
168 Soufrière de Guadeloupe have been directly related to intense rainfall events (Lesparre et al.,
169 2014), including landslides concurrent with the passage of storms and hurricanes (Allemand
170 et al., 2014); one such example is shown in Figure 1b. Very recently, parts of the *Chemin des*
171 *Dames*, a tourist path on the west flank of the dome, required repair following small-volume,
172 rain-induced landslides. This link between geohazards and the high-rainfall tropical climate
173 echoes a broader trend of rainfall-induced hazards observed at volcanoes elsewhere in the
174 Caribbean, including dome collapses (Matthews et al., 2002; Matthews and Barclay, 2004;
175 Carn et al., 2004) and seismicity (Matthews et al., 2009). In a comparative analysis of global
176 climate models, Farquharson and Amelung (2022) showed that models consistently project an
177 increase in heavy rainfall across the volcanic areas of the Caribbean over the next eight
178 decades. In Figure 1c, we plot projected change in heavy rainfall over La Soufrière de
179 Guadeloupe obtained from the MRI-CGCM3 general circulation model of Japan's
180 Meteorological Research Institute: clearly, the propensity for heavy rainfall events is set to
181 increase in the near future (see also Cantet et al., 2014), even under the most ambitious
182 climate change mitigation strategies (i.e. RCP 2.6).

183 Together, these factors highlight that La Soufrière de Guadeloupe represents an ideal
184 natural laboratory to study the influence of water-saturation on the physical and mechanical
185 properties of volcanic rock and volcano stability.

186



187

188 **Figure 1.** (a) Google Earth image (Google Maxar Technologies CNES / Airbus) of La
 189 Soufrière de Guadeloupe showing the sampling locations for the 17 rock blocks collected for
 190 this study. Inset shows a map of Guadeloupe in which the location of La Soufrière de
 191 Guadeloupe is indicated by a red triangle. (b) Photograph of the 2009 landslide scar on the
 192 northeastern flank of the dome (taken on 12 December 2009; photo credit: J.-B. de Chabalier,
 193 IPGP-OVSG). (c) Percentage change in heavy rainfall predicted for La Soufrière de
 194 Guadeloupe. Data are from the MRI-CGCM3 General Circulation Model results, provided by
 195 the Meteorological Research Institute, Japan, via the Earth System Grid Federation servers
 196 (<https://esgf-node.llnl.gov/search/cmip5/>). The change in projected heavy rainfall is resampled
 197 from monthly results corresponding to the 1.121×1.125 degree spatial grid containing La
 198 Soufrière de Guadeloupe, plotted as a function of time. For clarity, data are smoothed (15-
 199 year rolling mean) and normalised to 2023. Results are shown for three separate future

200 climate scenarios: a high emissions scenario (RCP 8.6), an intermediate emissions scenario
201 (RCP 4.5), and a "very stringent" pathway, whereby global carbon dioxide emissions decrease
202 to zero within 80 years (RCP 2.6). Further details are given in Farquharson and Amelung
203 (2022).

204

205 **3 Materials and Methods**

206 3.1 Experimental materials

207 A suite of 17 variably altered andesite blocks from La Soufrière de Guadeloupe were
208 used for this study (sampling locations are shown in Figure 1a). Eight of the 17 blocks were
209 collected from a collapse scar to the northeast of dome summit (blocks H2A, H2B, H3, H4A,
210 H5A, H6, H25, and H29). Four blocks were collected from the dome summit: one block from
211 the wall of the Lacroix Supérieur outgassing fracture on the lava dome (H18), and three
212 blocks from the lava spines that protrude the top of the current dome (one block from Cratère
213 Sud Central, H20, and two blocks from an adjacent site, H21 and H22). A block was collected
214 to the southwest of dome summit from a collapse scar into a highly fractured lava that forms
215 the core of a paleo-collapse mega-block of the former volcanic edifice (WP1285). Blocks
216 were also collected from the West wall of the fault "Faille du 30 août" on the lava dome (H14
217 and H15), and from a thick lava adjacent to the Galion waterfall (H32). The final block is a
218 volcanic non-juvenile bomb from the dome that was ejected during the 1976–1977 explosive
219 eruption (Komorowski et al., 2005) and landed on the roof of a small disused thermal
220 bathhouse to the south of the dome summit (WP1317). We climbed on the roof of the
221 bathhouse to collect the block.

222 These 17 blocks, previously described by Heap et al. (2021a, 2022a, b, c, 2023a, b),
223 are porphyritic andesites characterised by a microcrystalline groundmass containing
224 phenocrysts of dominantly plagioclase and pyroxene (orthopyroxene and clinopyroxene). The

225 mineral assemblage present in each block was identified by a combination of optical
226 microscopy, Raman spectroscopy, and X-ray powder diffraction (XRPD), and quantitative
227 phase analysis was performed using the XRPD data and the Rietveld approach (for more
228 details see Heap et al., 2021a). The XRPD data show that all of the rocks contain variable
229 quantities of secondary (alteration) minerals: kaolinite, alunite or natro-alunite, silica
230 polymorphs (quartz, cristobalite, tridymite, and opal-A), hematite, pyrite, gypsum, and talc
231 (Heap et al. 2021a, 2022a, b, c; Table 1). In this contribution, we quantify alteration as the wt
232 % of secondary minerals in each block. The predominant hydrous alteration phases are
233 kaolinite, natro-alunite, and opal-A (Table 1), suggesting fluid-rock interaction with acidic
234 sulfate-chloride-rich fluids at relatively low temperatures (< 150–200 °C) (Inoue, 1995;
235 Zimbelman et al., 2005; Scher et al., 2013; Fulignati, 2020; Heap et al., 2021a).

236

Mineral	H2A	H2B	H3	H4A	H5A	H6	H14	H15	H18	H20	H21	H22	H25	H29	H32	WP1285	WP1317
Plagioclase	56.7	12.3	46.6	23.3	41.3	30.0	60.7	22.5	61.2	28.7	24.2	59.5	38.7	62.4	64.4	64.7	61.6
Clinopyroxene	8.7	3.4	5.6	4.9	5.2	6.4	6.3	7.3	8.4	8.9	12.4	8.9	5.3	7.8	9.5	5.2	5.9
Orthopyroxene	10.8	9.5	11.8	11.8	11.1	10.8	8.6	9.2	12.2	15.0	19.3	13.6	10.2	11.2	15.1	13.2	15.6
(Ti-) Magnetite	0.7	-	0.8	-	-	-	0.8	-	2.9	2.4	3.1	0.8	-	2.7	4.9	3.5	0.7
Quartz*	1.0	0.5	0.6	0.6	0.5	0.5	1.7	0.7	0.7	0.3	0.2	0.6	0.3	0.4	0.3	0.2	0.7
Cristobalite*	11.3	12.8	10.6	11.8	13.0	11.1	13.5	10.2	11.7	11.4	11.7	10.6	9.8	12.4	5.7	-	-
Tridymite*	-	-	-	-	-	-	-	0.7	-	-	-	-	-	-	-	13.2	13.2
Hematite*	-	-	-	-	-	-	3.4	-	2.8	-	-	-	-	3.1	-	-	-
Pyrite*	3.5	-	3.8	2.3	-	-	-	-	-	-	0.4	3.1	0.6	-	-	-	-
Alunite*	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2.4
Na-Alunite*	1.4	1.6	2.8	1.3	5.4	5.1	5.1	15.0	-	0.5	0.5	-	9.8	-	-	-	-
Gypsum*	-	-	-	0.7	-	-	-	-	-	0.8	1.2	-	-	-	-	-	-
Kaolinite*	6	59.7	17.4	43.3	23.5	36.0	< 1	34.3	-	2.0	2.0	< 1	25.3	-	-	-	-
Talc*	-	-	-	-	-	-	-	-	-	-	-	2.9	-	-	-	-	-
Opal-A*	-	-	-	-	-	-	-	-	-	30	25	-	-	-	-	-	-

237

238 **Table 1.** Mineral contents of the 17 rock blocks from La Soufrière de Guadeloupe measured
239 by X-ray powder diffraction. Values in wt%. Asterisk denotes a secondary mineral (i.e.
240 alteration mineral). Data from Heap et al. (2021a, 2022a, b, c). The relative uncertainties in
241 the quantification are in the order of 5–10%. Sample locations are provided in Figure 1a.

242

243 3.2 Experimental methods

244 Cylindrical samples were prepared from each of the blocks to a diameter of 20 mm
245 and then cut and precision-ground to a nominal length of 40 mm. The cylindrical samples

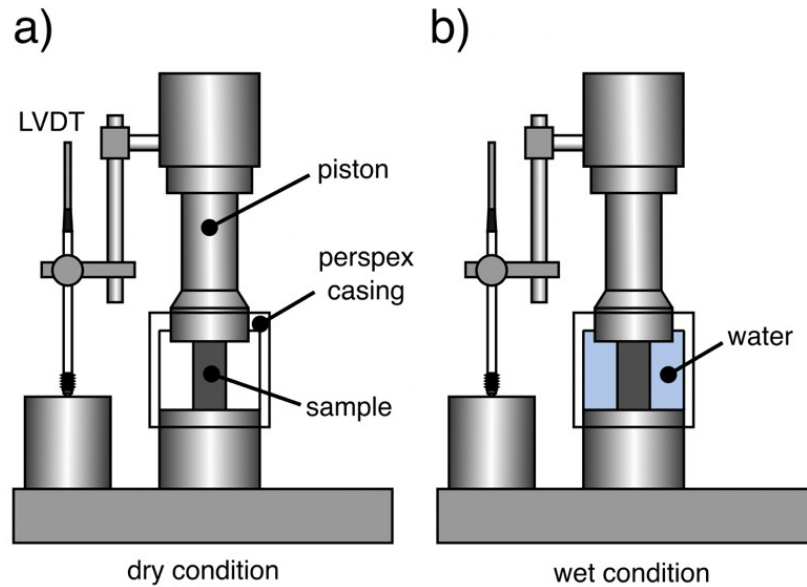
246 cored from a particular block were assumed to have the same mineral contents as the parent
247 block (i.e. those listed in Table 1). The samples were first washed using tapwater and then
248 dried in a vacuum-oven at 40 °C for at least 48 h. The connected porosity of all the samples
249 was calculated using the bulk sample volume and the skeletal (solid) sample volume of the
250 oven-dry samples measured by a helium pycnometer. Relative uncertainties of measurements
251 of connected porosity are < 2%.

252 Dry uniaxial compressive strength was measured on multiple oven-dry samples from
253 all 17 blocks in a uniaxial load frame located in the laboratory at the Strasbourg Institute of
254 Earth & Environment (ITES, France) (Figure 2a). Samples were deformed under ambient
255 laboratory pressure and temperature at a constant axial strain rate of 10^{-5} s^{-1} until macroscopic
256 failure. Axial displacement and axial load were measured using a linear variable differential
257 transducer and a load cell, respectively. Axial displacement (minus the displacement
258 accumulated within the load chain) and axial load were converted to axial strain and axial
259 stress using the sample dimensions. Relative uncertainties of measurements of uniaxial
260 compressive strength are < 1%. The static Young's modulus was determined from the elastic
261 portion of the uniaxial stress-strain curves (Heap et al., 2020a). Relative uncertainties of
262 measurements of Young's modulus are < 2%. The uniaxial compressive strength and Young's
263 modulus of the samples deformed dry were previously published in Heap et al. (2021a) (with
264 the exception of samples from block H32).

265 Water-saturated (wet) uniaxial compressive strength, data unique to this contribution,
266 was measured on multiple samples from all 17 blocks using the same uniaxial load frame,
267 inside a water bath (Figure 2b). The samples to be deformed under saturated conditions were
268 specifically selected so that their appearance and connected porosity was the same, or very
269 similar, to the samples deformed under dry conditions from the same block (so that their dry
270 and wet uniaxial compressive strength and Young's modulus can be compared directly). To do

271 so, multiple samples were prepared from each block and only those with a similar appearance
272 and porosity were selected for experimentation. Using this approach, the porosity difference
273 between each sample pair was in the range 0.00–0.03, which we consider to be sufficient to
274 allow for direct comparisons between the dry and wet uniaxial compressive strength and
275 Young's modulus of each of the blocks. Prior to deformation, the samples were vacuum-
276 saturated in de-aired, deionised water. The water-saturation procedure was as follows. (1) The
277 samples were placed inside a glass container, which was then placed within a bell jar. (2) The
278 samples were vacuumed inside the bell jar for at least 12 h at room temperature. (3)
279 Maintaining the vacuum inside the bell jar, de-aired, deionised water was introduced into the
280 glass container until the samples were completely submerged. (4) The samples were left
281 submerged within the vacuumed bell jar for at least 6 h, after which the vacuum was released
282 and the glass container containing the samples was removed from the bell jar. (5) The samples
283 remained submerged in de-aired, deionised water until deformed. As for the dry experiments,
284 the static Young's modulus was then determined from the elastic portion of the uniaxial
285 stress-strain curves. Samples from block H32, too strong to measure in the uniaxial load
286 frame at the ITES, were measured at the Laboratory of Experimental Rock Mechanics at the
287 Ecole Polytechnique Fédérale de Lausanne (EPFL, Switzerland) using a similar uniaxial
288 setup. We consider the dry samples deformed in Heap et al. (2021a) to have a water saturation
289 of 0%, and the wet samples deformed herein to have a water saturation of 100%.

290 Although rock strength and Young's modulus will increase as a function of pressure
291 (or depth) in the brittle regime, we highlight that lava domes are typically only a few hundred
292 metres high (the lava dome at La Soufrière de Guadeloupe, for example, is about 250–300 m
293 high) and that rock physical and mechanical properties (e.g., uniaxial compressive strength
294 and Young's modulus) will not change significantly under pressures representative of a depth
295 of a few hundred metres.



297

298 **Figure 2.** Schematic diagrams of the uniaxial compression apparatus at the Strasbourg
 299 Institute of Earth & Environment (ITES). (a) Setup for the dry experiments. (b) Setup for the
 300 wet experiments.

301

302 3.3 Numerical modelling

303 To explore the effect of water-saturation on dome stability, we used 2D Discrete
 304 Element Method (DEM) models created in Particle Flow Code (PFC; Itasca Consulting Group
 305 Ltd), following the methods outlined by Harnett et al. (2018) and Harnett and Heap (2021).
 306 PFC has been previously used to successfully model the mechanical behaviour of rock (e.g.,
 307 Potyondy and Cundall, 2004) and, recently, to model dome growth and collapse (Husain et
 308 al., 2014, 2018; Harnett et al., 2018; Husain et al., 2019; Harnett and Heap, 2021; Harnett et
 309 al., 2022; Walter et al., 2022; Heap et al., 2023a). These DEM models consider a particle-
 310 based material in which circular particles interact at interparticle contacts. Contact behaviour
 311 is primarily governed by stiffness and cohesion. At the boundary between the particles and
 312 walls (i.e. the ground surface), the contact behaviour is cohesionless and governed by friction.

313 The particle size in the model is not representative of individual crystals, grains, or rock
314 blocks, but rather represents discretisation of the medium for the purpose of computation.

315 First, we numerically generated two fully solid domes (see Harnett and Heap, 2021)
316 containing 9745 circular particles and 22710 contacts. The contact properties were guided by
317 our experimental data for dry unaltered and altered dome rocks (uniaxial compressive
318 strength, tensile strength, and Young's modulus). An iterative calibration procedure was
319 required to relate contact parameters to bulk rock parameters, whereby uniaxial compression
320 tests are reproduced in PFC (see Harnett and Heap (2021) for details). Using these calibrated
321 parameters, we created initially stable dry unaltered and altered domes. These stable domes
322 will not deform unless perturbed in some way. To explore the effect of water-saturation on the
323 stability of these domes, we modified the mechanical properties of the contacts in the domes
324 to represent wet unaltered and altered dome rocks, also guided by our experimental data. The
325 model then provided the displacement within the dome resulting from water-saturation.
326 Finally, we generated another initially stable fully solid dome using properties representative
327 of dry unaltered rock. In the first scenario, we perturbed the dome by including a dry altered
328 zone within the dome (as in Harnett et al., 2022). In the second scenario, we also included the
329 buried altered zone within the dome, but, instead, we used properties representative of wet
330 unaltered and altered rock. We highlight that, although we use properties representative of
331 uniaxial conditions in our models and, therefore, we do not explicitly consider an increase in
332 strength with depth in the model, the models have an inherent depth-dependence of
333 mechanical properties through the inclusion of gravity: the contact forces at the base of the
334 dome are much higher than those at the surface of the dome. Further, as mentioned above, we
335 also do not expect rock physical and mechanical properties (e.g., uniaxial compressive
336 strength and Young's modulus) to change significantly under pressures representative of a
337 depth of a few hundred metres. For the modelling of much larger, or deeper, features, we

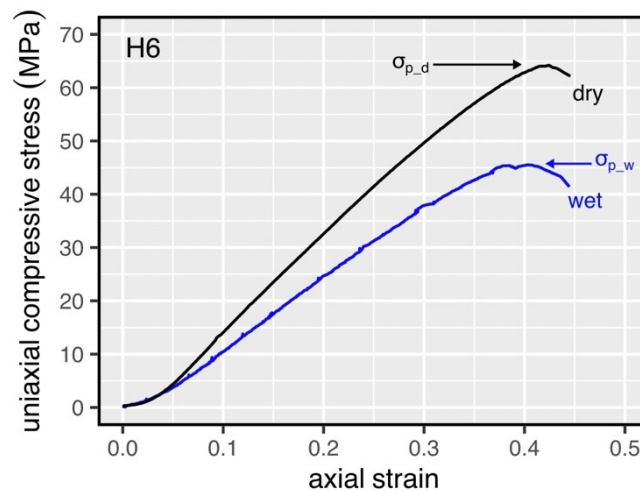
338 recommend that triaxial deformation experiments are performed to understand how rock
339 physical and mechanical properties evolve as a function of pressure (depth).

340

341 4 Results

342 Representative uniaxial stress-strain curves for dry and wet samples are shown in
343 Figure 3 (for block H6). We highlight that the connected porosity of the dry and wet H6
344 samples shown in Figure 3 is 0.18 and 0.16, respectively (Table 2). These curves show that
345 the uniaxial compressive strength and the Young's modulus (the slope of the stress-strain
346 curve in the pseudo-linear elastic regime) are lower when the sample is wet. We also note that
347 the axial strain at the peak stress is slightly lower for the wet sample than for the dry sample
348 (Figure 3).

349



350

351 **Figure 3.** Representative stress-strain curves for dry (black curve) and wet (blue curve)

352 samples from block H6 from La Soufrière de Guadeloupe (Eastern Caribbean). The dry, $\sigma_{p,d}$,

353 and wet, $\sigma_{p,w}$, uniaxial compressive strengths are indicated on the curves.

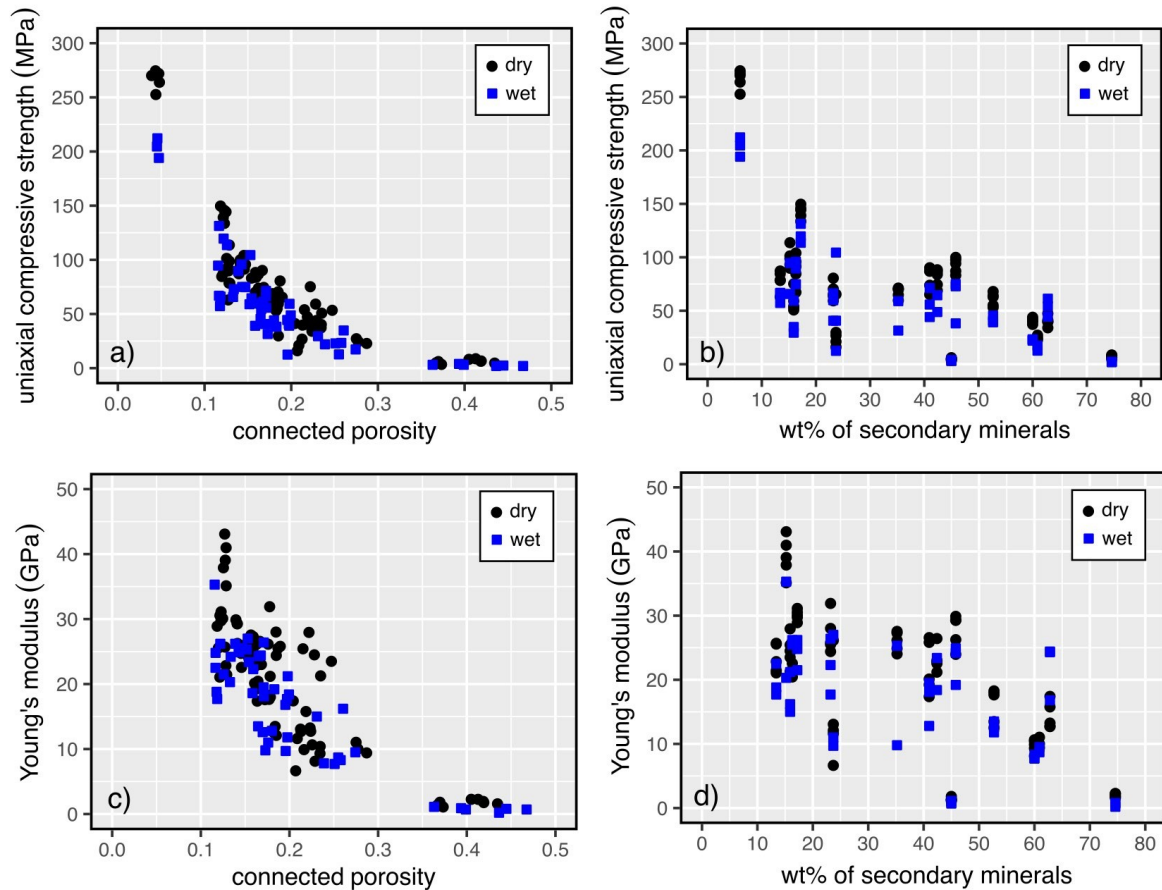
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355 Dry (black circles) and wet (blue squares) uniaxial compressive strength data are
356 plotted as a function of connected porosity and the wt% of secondary minerals in Figures 4a

357 and 4b, respectively (data available in Table 2). These data show that uniaxial compressive
358 strength decreases as a function of connected porosity and the wt% of secondary minerals. For
359 example, strength decreased from ~270 MPa to only a couple of MPa as porosity decreased
360 from ~0.05 to > 0.4 (Figure 4a). These data also show that, for a given porosity, the strength
361 of wet samples is typically lower than the strength of dry samples. Block H2B (porosity
362 ~0.45; alteration 75 wt%; Table 2) and block H2A (porosity ~0.18; alteration 23 wt%) are
363 characterised by the lowest and highest ratio of wet to dry strength, respectively.

364 We also show dry (black circles) and wet (blue squares) Young's modulus as a
365 function of connected porosity and the wt% of secondary minerals in Figures 4c and 4d,
366 respectively (data available in Table 2). Similar to the strength data of Figure 4a and 4b, the
367 data of Figure 4c and 4d show that (1) Young's modulus decreases as a function of connected
368 porosity and the wt% of secondary minerals and (2) for a given porosity, the Young's
369 modulus of wet samples is typically lower than the Young's modulus of dry samples. For
370 example, Young's modulus decreased from ~40 GPa to only a couple of GPa as porosity
371 increases from ~0.05 to > 0.4 (Figure 4c) or as the wt% of secondary minerals increases from
372 ~15 to ~75 wt% (Figure 4d). Block H2B (porosity ~0.45; alteration 75 wt%; Table 2) and
373 block WP1317 (porosity ~0.14; alteration 16 wt%) are characterised by the lowest and highest
374 ratio of wet to dry Young's modulus, respectively.

375



376

377 **Figure 4.** Uniaxial compressive strength of dry (black circles) and wet (blue squares) samples

378 from La Soufrière de Guadeloupe (Eastern Caribbean) as a function of (a) connected porosity

379 and (b) the wt% of secondary minerals. Young's modulus of dry and wet samples from La

380 Soufrière de Guadeloupe as a function of (c) connected porosity and (d) the wt% of secondary

381 minerals. Dry data (apart from data for H32) are from Heap et al. (2021a). The relative

382 uncertainties of the connected porosity, uniaxial compressive strength, and Young's modulus

383 are $< 2\%$, $< 1\%$, and $< 2\%$, respectively; the uncertainties are therefore captured by the

384 symbol size. The relative uncertainties in the quantification of the mineral contents (Table 1)

385 are on the order of 5–10%.

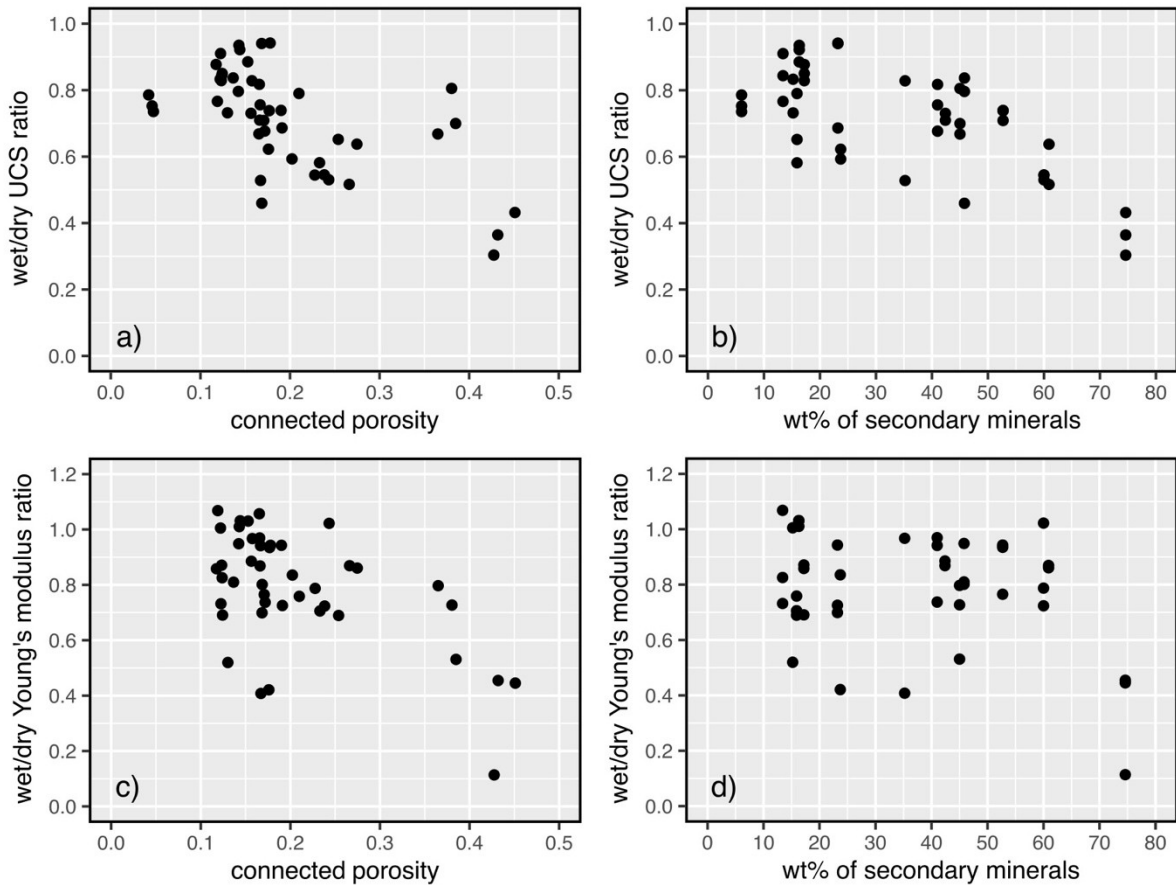
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387 To better highlight the difference between wet and dry strength and Young's modulus,

388 the ratio of wet to dry strength and the ratio of wet to dry Young's modulus are plotted as a

389 function of connected porosity and the wt% of secondary minerals in Figure 5 (data available
390 in Table 3). This ratio is obtained by organising the samples from the same block deformed
391 under dry and wet conditions into pairs, and then dividing the data for the sample deformed
392 under saturated conditions with the data for their corresponding dry counterpart. As
393 mentioned above, the samples deformed under saturated conditions were specifically selected
394 so that their appearance and porosities were the same, or very similar, to those deformed
395 under dry conditions. A ratio of one therefore corresponds to the scenario in which the dry
396 and wet properties are exactly the same, and a ratio below and above one indicates that the
397 sample property is lower or higher when wet, respectively. Figure 5a shows that all of the
398 samples have a wet to dry strength lower than one, indicating that all samples were weaker
399 when saturated with water. These data also show that the wet to dry strength ratio appears to
400 decrease as a function of increasing porosity (Figure 5a) and wt% of secondary minerals
401 (Figure 5b). Figure 5c shows that the majority of the samples have a wet to dry Young's
402 modulus ratio below one. Although the wet to dry Young's modulus ratio appears to decrease
403 as a function of increasing porosity (Figure 5c), there appears to be no discernible trend as a
404 function of increasing wt% of secondary minerals (Figure 5d), although we highlight that the
405 samples with the lowest wet to dry Young's modulus ratio contain the highest proportion of
406 secondary minerals (Figure 5d).

407



408

409

Figure 5. Ratio of wet to dry uniaxial compressive strength (UCS) for samples from La

410

Soufrière de Guadeloupe (Eastern Caribbean) as a function of (a) connected porosity and (b)

411

the wt% of secondary minerals. Ratio of wet to dry Young's modulus for samples from La

412

Soufrière de Guadeloupe as a function of (c) connected porosity and (d) the wt% of secondary

413

minerals. The relative uncertainties of the connected porosity, uniaxial compressive strength,

414

and Young's modulus are < 2%, < 1%, and < 2%, respectively; the uncertainties are therefore

415

captured by the symbol size. The relative uncertainties in the quantification of the mineral

416

contents (Table 1) are on the order of 5–10%.

417

Sample	Weight percentage of secondary minerals	Connected porosity	Condition	Uniaxial compressive strength (MPa)	Young's modulus (GPa)
H2A 2	23	0.18	Dry	63.7	31.9
H2A 5	23	0.19	Dry	80.5	25.5
H2A 6	23	0.19	Dry	65.4	25.8
H2A 11	23	0.19	Dry	59.3	24.4

H2A 12	23	0.18	Dry	70.4	28.0
H2A 7	23	0.17	Wet	66.3	26.4
H2A 10	23	0.16	Wet	59.9	22.3
H2A 14	23	0.20	Wet	40.7	17.7
H2B 3	75	0.42	Dry	6.6	1.8
H2B 10	75	0.42	Dry	6.4	1.9
H2B 11	75	0.41	Dry	8.7	2.3
H2B 12	75	0.41	Dry	8.1	2.3
H2B 15	75	0.43	Dry	4.6	1.6
H2B 7	75	0.44	Wet	2.0	0.2
H2B 9	75	0.47	Wet	2.0	0.7
H2B 13	75	0.44	Wet	2.4	0.8
H3 3	35	0.16	Dry	70.3	27.3
H3 7	35	0.16	Dry	69.9	24.9
H3 8	35	0.16	Dry	64.8	27.5
H3 11	35	0.16	Dry	59.7	24.0
H3 13	35	0.16	Dry	71.2	26.2
H3 14	35	0.17	Wet	31.5	9.8
H3 15	35	0.15	Wet	59.0	25.3
H4A 2	60	0.23	Dry	40.3	10.4
H4A 4	60	0.23	Dry	42.0	10.6
H4A 6	60	0.23	Dry	43.9	8.1
H4A 8	60	0.23	Dry	37.2	9.3
H4A 9	60	0.22	Dry	40.4	9.9
H4A 5	60	0.25	Wet	22.9	.77
H4A 7	60	0.26	Wet	23.3	8.3
H4A 10	60	0.24	Wet	22.0	7.8
H5A 2	42	0.16	Dry	88.3	26.4
H5A 3	42	0.16	Dry	86.6	22.8
H5A 5	42	0.16	Dry	83.9	22.5
H5A 8	42	0.18	Dry	68.8	21.2
H5A 10	42	0.17	Dry	74.5	23.0
H5A 4	42	0.20	Wet	48.8	18.4
H5A 9	42	0.15	Wet	64.5	23.4
H6 6	53	0.18	Dry	55.1	13.5
H6 9	53	0.18	Dry	68.0	18.0
H6 9	53	0.18	Dry	53.0	12.5
H6 12	53	0.18	Dry	64.2	17.6
H6 13	53	0.18	Dry	63.0	18.3
H6 2	53	0.16	Wet	45.5	13.5
H6 3	53	0.20	Wet	39.2	11.8
H6 5	53	0.17	Wet	40.7	12.6
H14 2	24	0.18	Dry	65.4	26.1
H14 3	24	0.21	Dry	21.1	11.6
H14 5	24	0.21	Dry	26.8	13.1
H14 6	24	0.19	Dry	29.8	12.1
H14 10	24	0.21	Dry	16.1	6.6
H14 8	24	0.20	Wet	12.5	9.7
H14 11	24	0.18	Wet	40.7	11.0
H15 3	61	0.28	Dry	24.6	10.0
H15 4	61	0.28	Dry	27.1	11.0
H15 5	61	0.29	Dry	22.8	9.4
H15 6	61	0.27	Wet	17.3	9.5
H15 7	61	0.25	Wet	12.7	8.7
H18 3	15	0.13	Dry	101.3	37.9
H18 4	15	0.13	Dry	89.8	39.1
H18 5	15	0.13	Dry	98.8	41.0
H18 6	15	0.13	Dry	93.2	43.1

H18 7	15	0.13	Dry	113.7	35.1
H18 11	15	0.13	Wet	65.7	20.3
H18 14	15	0.12	Wet	94.7	35.3
H20 2	45	0.37	Dry	4.8	1.2
H20 3	45	0.37	Dry	4.4	1.3
H20 7	45	0.37	Dry	4.6	1.4
H20 8	45	0.37	Dry	6.1	1.8
H20 10	45	0.37	Dry	3.4	1.1
H20 5	45	0.39	Wet	3.9	0.9
H20 6	45	0.40	Wet	3.1	0.7
H20 11	45	0.36	Wet	3.1	1.1
H21 3	41	0.17	Dry	90.2	26.6
H21 8	41	0.16	Dry	65.2	17.4
H21 11	41	0.16	Dry	86.9	25.8
H21 12	41	0.16	Dry	73.8	19.2
H21 13	41	0.16	Dry	87.2	20.1
H21 4	41	0.17	Wet	71.3	19.5
H21 6	41	0.18	Wet	44.1	12.8
H21 15	41	0.17	Wet	55.8	18.1
H22 2	17	0.12	Dry	145.7	29.7
H22 3	17	0.12	Dry	144.3	30.1
H22 4	17	0.12	Dry	139.1	30.5
H22 5	17	0.12	Dry	149.7	28.9
H22 6	17	0.12	Dry	133.6	31.1
H22 7	17	0.12	Wet	119.6	26.2
H22 14	17	0.12	Wet	131.3	24.8
H22 15	17	0.13	Wet	113.6	21.5
H25 2	46	0.14	Dry	99.9	29.3
H25 4	46	0.14	Dry	97.7	29.3
H25 5	46	0.14	Dry	94.2	26.2
H25 9	46	0.14	Dry	87.1	29.9
H25 12	46	0.15	Dry	83.1	24.0
H25 3	46	0.18	Wet	38.2	19.2
H25 10	46	0.13	Wet	72.9	24.2
H25 14	46	0.14	Wet	75.0	24.9
H29 2	16	0.22	Dry	53.9	25.4
H29 8	16	0.23	Dry	59.1	24.5
H29 9	16	0.22	Dry	75.2	27.9
H29 12	16	0.25	Dry	53.4	23.5
H29 16	16	0.24	Dry	50.7	21.3
H29 3	16	0.20	Wet	59.4	21.2
H29 6	16	0.26	Wet	34.8	24.2
H29 13	16	0.23	Wet	29.5	24.9
H32 1	6	0.04	Dry	270.1	-
H32 4	6	0.05	Dry	263.9	-
H32 5	6	0.05	Dry	271.9	-
H32 6	6	0.04	Dry	274.5	-
H32 7	6	0.04	Dry	252.6	-
H32 8	6	0.05	Wet	194.1	-
H32 9	6	0.05	Wet	204.6	-
H32 10	6	0.04	Wet	212.2	-
WP1285 2	13	0.13	Dry	78.5	22.8
WP1285 8	13	0.13	Dry	63.0	25.7
WP1285 10	13	0.13	Dry	78.3	21.4
WP1285 11	13	0.12	Dry	87.2	21.1
WP1285 15	13	0.12	Dry	84.6	25.6
WP1285 4	13	0.12	Wet	66.1	17.7
WP1285 6	13	0.12	Wet	57.3	18.8

WP1285_9	13	0.12	Wet	66.8	22.5
WP1317_2	16	0.16	Dry	67.3	20.4
WP1317_6	16	0.16	Dry	84.5	24.7
WP1317_7	16	0.15	Dry	91.5	22.6
WP1317_8	16	0.15	Dry	95.8	25.9
WP1317_12	16	0.15	Dry	104.0	24.7
WP1317_10	16	0.15	Wet	74.8	25.5
WP1317_13	16	0.14	Wet	89.6	26.2
WP1317_15	16	0.14	Wet	95.9	25.5

418

419 **Table 2.** Experimental condition (dry or wet), connected porosity, uniaxial compressive
420 strength, Young's modulus, and the percentage of secondary minerals (i.e. alteration) for the
421 samples prepared for this study. Sample locations are provided in Figure 1a. Dry data (apart
422 from data for H32) are from Heap et al. (2021a). The relative uncertainties of the connected
423 porosity, uniaxial compressive strength, and Young's modulus are < 2%, < 1%, and < 2%,
424 respectively. The relative uncertainties in the quantification of the mineral contents (Table 1)

425

are on the order of 5–10%.

426

Sample	Weight percentage of secondary minerals	Connected porosity	wet/dry UCS ratio	wet/dry Young's modulus ratio
H2A	23	0.17	0.94	0.70
H2A	23	0.18	0.94	0.94
H2A	23	0.19	0.69	0.73
H2B	75	0.45	0.43	0.45
H2B	75	0.43	0.30	0.11
H2B	75	0.43	0.36	0.45
H3	35	0.17	0.53	0.41
H3	35	0.16	0.83	0.97
H4A	60	0.23	0.54	0.79
H4A	60	0.24	0.55	0.72
H4A	60	0.24	0.53	1.02
H5A	42	0.16	0.73	0.89
H5A	42	0.17	0.71	0.87
H6	53	0.17	0.71	0.77
H6	53	0.19	0.74	0.94
H6	53	0.18	0.74	0.93
H14	24	0.20	0.59	0.84
H14	24	0.18	0.62	0.42
H15	61	0.27	0.64	0.86
H15	61	0.27	0.52	0.87
H18	15	0.12	0.83	1.01
H18	15	0.13	0.73	0.52
H20	45	0.38	0.81	0.73
H20	45	0.38	0.70	0.53

H20	45	0.36	0.67	0.80
H21	41	0.17	0.82	0.97
H21	41	0.17	0.68	0.74
H21	41	0.17	0.76	0.94
H22	17	0.12	0.85	0.69
H22	17	0.12	0.88	0.86
H22	17	0.12	0.83	0.87
H25	46	0.14	0.80	0.95
H25	46	0.14	0.84	0.81
H25	46	0.17	0.46	0.80
H29	16	0.21	0.79	0.76
H29	16	0.25	0.65	0.69
H29	16	0.23	0.58	0.71
H32	6	0.05	0.74	-
H32	6	0.05	0.75	-
H32	6	0.04	0.79	-
WP1285	13	0.12	0.77	1.07
WP1285	13	0.12	0.91	0.73
WP1285	13	0.12	0.84	0.83
WP1317	16	0.14	0.92	1.03
WP1317	16	0.14	0.93	1.01
WP1317	16	0.15	0.89	1.03

427

428 **Table 3.** Percentage of secondary minerals (i.e. alteration), average connected porosity, the
429 ratio of wet to dry uniaxial compressive strength, and the ratio of wet to dry Young's modulus
430 for the samples prepared for this study. Sample locations are provided in Figure 1a. The
431 relative uncertainties of the connected porosity, uniaxial compressive strength, and Young's
432 modulus are < 2%, < 1%, and < 2%, respectively. The relative uncertainties in the
433 quantification of the mineral contents (Table 1) are on the order of 5–10%.

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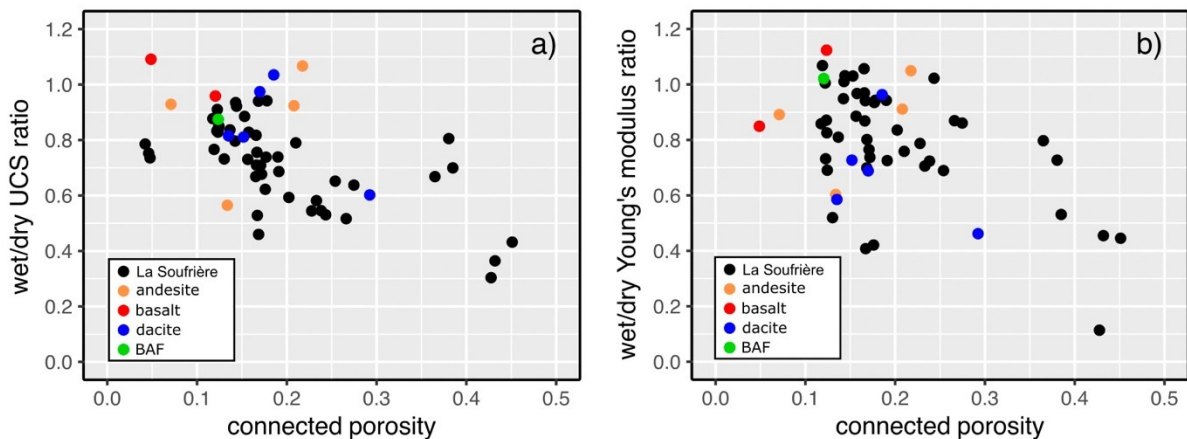
435 5 Discussion

436 5.1 Water-weakening in volcanic rocks: the influence of porosity

437 Our data show that the uniaxial compressive strength and Young's modulus of
438 variably-altered dome lavas from La Soufrière de Guadeloupe are reduced when water-
439 saturated (Figures 4 and 5), in agreement with previous studies on volcanic rocks (Zhu et al.,
440 2011; Heap et al., 2015; Zhu et al., 2016; Heap et al., 2018; Kendrick et al., 2021; Heap and
441 Violay, 2021).

442 We plot the ratio of wet to dry strength and the ratio of wet to dry Young's modulus as
443 a function of connected porosity for our new data alongside published data for volcanic rocks
444 in Figures 6a and 6b, respectively. Figures 6a and 6b show that, based on previously
445 published data for andesite, basalt, dacite, and welded block-and-ash flow, one would
446 conclude that water-weakening in volcanic rocks does not appear to vary systematically with
447 porosity. Previous studies have also shown that porosity does not appear to influence water-
448 weakening in sandstones (Heap et al., 2019), limestones (Baud et al., 2016), and tuffs (Zhu et
449 al., 2011; Heap et al., 2018; Frolova et al., 2021). Our new data for hydrothermally altered
450 andesites from La Soufrière de Guadeloupe, however, suggest that porosity exerts influence
451 on their water-weakening behaviour (Figures 6a and 6b). This may be because the porosity
452 range studied in previous contributions was insufficient to observe a trend (the most porous
453 sample from the previously published dataset is a dacite with a porosity of ~ 0.3 , whereas the
454 maximum porosity in our dataset is ~ 0.45 ; Figures 6 and 6b). Although we conclude that
455 porosity appears to exert influence on water-weaking in our sample suite, we suggest that
456 more data for high-porosity volcanic rocks are now required to make firm conclusions as to
457 the influence of porosity on water-weakening in volcanic rocks (based on reasoning discussed
458 in the next subsection).

459



460

461 **Figure 6.** Ratio of wet to dry (a) uniaxial compressive strength and (b) Young's
462 modulus for samples from La Soufrière de Guadeloupe (Eastern Caribbean) as a function of
463 connected porosity (black circles). Data for andesite (orange circles), basalt (red circles), and
464 block-and-ash flow (BAF; green circles) from Heap and Violay (2021). Data for dacite (blue
465 circles) from Kendrick et al. (2021). For the data for the samples from La Soufrière de
466 Guadeloupe, the relative uncertainties of the connected porosity, uniaxial compressive
467 strength, and Young's modulus are $< 2\%$, $< 1\%$, and $< 2\%$, respectively; the uncertainties are
468 therefore captured by the symbol size.

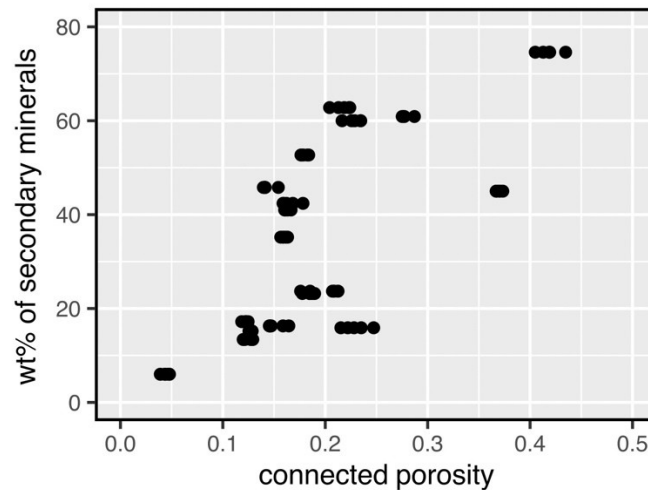
469

470 5.2 Water-weakening in volcanic rocks: the influence of alteration

471 We first note that the uniaxial compressive strength (Figure 4b) and Young's modulus
472 (Figure 4d) of water-saturated andesites from La Soufrière de Guadeloupe are reduced as a
473 function of increasing wt% of secondary minerals. Previously, hydrothermal alteration has
474 been shown to either decrease (e.g., Frolova et al., 2014; Pola et al., 2014; Wyering et al.,
475 2014; Mordensky et al., 2018, 2019; Heap et al., 2021a; Darmawan et al., 2022; Schaefer et
476 al., 2023) or increase the strength of volcanic rocks (e.g., Frolova et al., 2014; Pola et al.,
477 2014; Wyering et al., 2014; Coats et al., 2018; Heap et al., 2020b, 2021b). These studies, and
478 others, have shown that whether or not hydrothermal alteration decreases or increases strength
479 depends on the type of alteration and/or whether the alteration results in increases or
480 decreases to porosity.

481 In the case of hydrothermally altered dome rocks from La Soufrière de Guadeloupe,
482 Heap et al. (2021a, 2022) concluded that alteration had reduced their uniaxial compressive
483 strength and Young's modulus. However, these authors also noted that there is a relationship
484 between porosity and alteration (Figure 7). As discussed by these authors, it is difficult to
485 unravel the influence of porosity and alteration on the strength of these materials because it is

486 unclear whether alteration increased the porosity of the samples or whether the more porous
487 samples are more altered due to their higher fluid-rock ratios. However, based on the presence
488 of abundant clays and dissolution textures in these samples, Heap et al. (2021a, 2022)
489 concluded uniaxial compressive strength and Young's modulus were likely reduced by
490 hydrothermal alteration. We conclude the same here, although we highlight that the main
491 focus of this contribution is to study the influence of water-saturation on the mechanical
492 properties of hydrothermally altered dome rocks, discussed below.
493



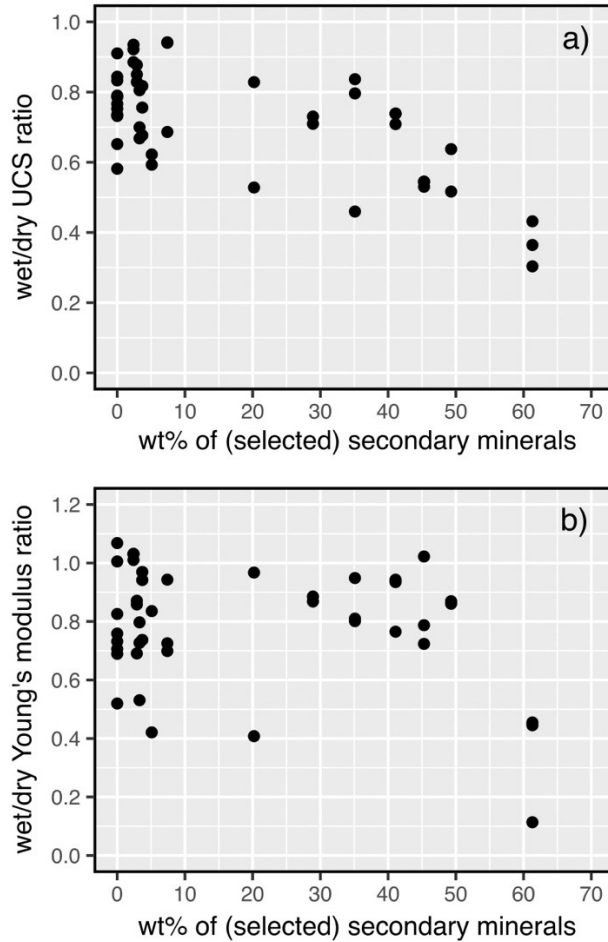
494
495 **Figure 7.** The wt% of secondary minerals as a function of connected porosity for samples
496 from La Soufrière de Guadeloupe (Eastern Caribbean).

497
498 Our data suggest that the wet to dry strength ratio decreases as a function of increasing
499 wt% of secondary minerals (Figure 5b), although there appears to be no discernible trend in
500 the wet to dry Young's modulus ratio as a function of increasing wt% of secondary minerals
501 (Figure 5d). In other words, these data suggest that the reduction in strength due to the
502 presence of water increases as a function of the degree of hydrothermal alteration and,
503 therefore, the abundance of secondary (alteration) minerals. These results are in agreement
504 with the previous laboratory data for tuff, which suggest that the presence of alunite (Heap et

505 al., 2015) and zeolites and clay minerals (Heap et al., 2018) may be responsible for the
506 reduction in uniaxial compressive strength in the presence of water. Expansive clay minerals,
507 for example, can mechanically degrade as a function of increasing water content (Nara et al.,
508 2012). The fact that alteration increases the observed water-weakening (Figure 5b) and that
509 alteration increases as a function of connected porosity for these rocks (Figure 7) also
510 suggests that the observed trends as a function of porosity (Figures 6a and 6b) could be
511 because the high-porosity rocks are more altered, and it is the alteration that is dictating the
512 observed water-weakening. As stated above, more experiments are now required to make firm
513 conclusions as to the influence of porosity on water-weakening in volcanic rocks.

514 To further investigate the influence of alteration on water-weakening, we replot the
515 ratio of wet to dry uniaxial compressive strength and Young's modulus as a function of a
516 subset of the alteration minerals (alunite, Na-alunite, gypsum, kaolinite, and talc only; i.e.
517 excluding cristobalite, tridymite, hematite, pyrite, and opal-A) in Figure 8. We include those
518 minerals that have previously been considered to reduce the strength of rock upon water-
519 saturation. We highlight that the coefficients of determination for simple linear fits to the data
520 shown in Figure 8 are very similar to those for the data shown in Figures 5b and 5d.
521 Therefore, these data suggest that the selected alteration minerals (alunite, Na-alunite,
522 gypsum, kaolinite, and talc) are likely those influencing the uniaxial compressive strength of
523 these rocks in the presence of water (Figure 8a).

524



525

526 **Figure 8.** Ratio of wet to dry (a) uniaxial compressive strength and (b) Young's modulus as a
 527 function of the wt% of a subset of the secondary minerals for samples from La Soufrière de
 528 Guadeloupe (Eastern Caribbean). The alteration minerals included are alunite, Na-alunite,
 529 gypsum, kaolinite, and talc only (i.e. excluding cristobalite, tridymite, hematite, pyrite, and
 530 opal-A). The relative uncertainties of the uniaxial compressive strength and Young's modulus
 531 are $< 1\%$ and $< 2\%$, respectively; the uncertainties are therefore captured by the symbol size.

532 The relative uncertainties in the quantification of the mineral contents (Table 1) are on the
 533 order of 5–10%.

534

535 To investigate the mechanism responsible for the observed weakening in the presence
 536 of water, and the increase in water-weakening as a function of alteration (Figures 4b, 5b, and

537 8a), we use the analytical approximation of Sammis and Ashby's (1986) pore-emanating crack
538 micromechanical model provided by Zhu et al. (2010):

539

$$540 \quad UCS = \frac{1.325}{\phi^{0.414}} \frac{K_{IC}}{\sqrt{\pi r}} (1).$$

541

542 The pore-emanating crack model of Sammis and Ashby (1986) has been widely used to
543 investigate the mechanical behaviour of porous volcanic rocks (see Heap and Violay (2021)
544 for a review). The pore-crack model describes a two-dimensional elastic medium populated
545 with circular pores of a uniform radius. In the model, cracks can propagate in a direction
546 parallel to the maximum principal stress when the stress at the tip of a crack on the curved
547 surface of the pore reaches the fracture toughness of the material, K_{IC} (the resistance of a
548 brittle material to the propagation of cracks under an applied stress). Eventually, the cracks
549 grow long enough to interact and ultimately coalesce, resulting in the macroscopic failure of
550 the medium. Although the andesites studied here contain microcracks, their microstructure
551 can be well approximated by this inclusion model (i.e. pores within a groundmass; Heap et al.,
552 2021a), although we highlight that the model assumes a uniform pore radius. In the analytical
553 solution to the model (Equation (1)), uniaxial compressive strength (UCS) is a function of K_{IC}
554 , porosity, ϕ , and pore radius, r . The two unknowns in Equation (1) are K_{IC} and r . We can
555 estimate r using microstructural analysis. To do so, we calculated the average equivalent
556 macropore diameter (excluding pores with a diameter $< 30 \mu\text{m}$) of each block using scanning
557 electron microscope images and open-source image analysis software ImageJ (data available
558 in Table 4). The average equivalent macropore diameter, $2r$, was calculated using
559 $2r = 3/2(d_F)$, where d_F is the average macropore Feret diameter. An example of how the
560 average equivalent macropore diameter was determined is available as Supplementary

561 Information. Values of dry and wet K_{IC} were then estimated using Equation (1), taking the
562 average values of dry and wet UCS for each block (Table 2). Dry and wet K_{IC} , and the wet to
563 dry K_{IC} ratio, are plotted as a function of alteration in Figures 9a and 9b, respectively (data
564 available in Table 4). The data of Figure 9 highlight that (1) wet K_{IC} is lower than dry K_{IC}
565 and (2) the reduction in K_{IC} in the presence of water is increased as a function of increasing
566 alteration. Using this approach, the wet to dry K_{IC} ratio for the andesites studied herein is
567 estimated to be between 0.32 and 0.93 (Figure 9). For the most part, these values are lower
568 than those typically reported in published studies. For example, ratios for a suite of sandstones
569 were measured to be between 0.65 and 0.95 (Noël et al., 2021), and the ratio for an unaltered
570 andesite (Kumamoto andesite) was measured to be 0.87 (Nara et al., 2012). We emphasise
571 that the values of K_{IC} provided in Figure 9 and Table 4 are estimates made using the pore-
572 emanating crack model (Equation (1)), and not measured values of K_{IC} as in Noël et al.
573 (2021) and Nara et al. (2012).

574 A reduction in the K_{IC} of rocks in the presence of water has been observed by many
575 experimental studies and has been attributed to several, potentially coeval, factors: a reduction
576 in surface energy due to water adsorption (Baud et al., 2000; Noël et al., 2021), crack-tip
577 capillary forces (Nara et al., 2012), grain-contact lubrication (Guha Roy et al., 2017), stress
578 corrosion cracking (Kataoka et al., 2015), mineral dissolution (Maruvanchery and Kim,
579 2019), and the presence of clay minerals (Wang et al., 2007; Nara et al., 2012). The reduction
580 in K_{IC} in the presence of water observed here (Figure 9a), is likely due to a combination of
581 these mechanisms; although, given that we provide here the short-term mechanical strength
582 (UCS experiments performed at a strain rate of 10^{-5} s^{-1}), we anticipate only minor
583 contributions from stress corrosion cracking and mineral dissolution. We consider that the
584 increase in the reduction in K_{IC} in the presence of water as alteration increases (Figure 9b) is
585 most likely explained by the increasing abundance of hydrophilic minerals, such as alunite

586 and clay minerals, as a function of increasing alteration (Table 1). Hydrophilic minerals
 587 expand upon saturation with water, resulting in a reduction in their mechanical resistance
 588 (Nara et al., 2012). We further note that, if hydrophilic minerals expand upon saturation and
 589 reduce ϕ and/or r , then the K_{IC} estimated for the altered rocks using Equation (1) would be
 590 even lower than our estimations presented in Table 4 and Figure 9.

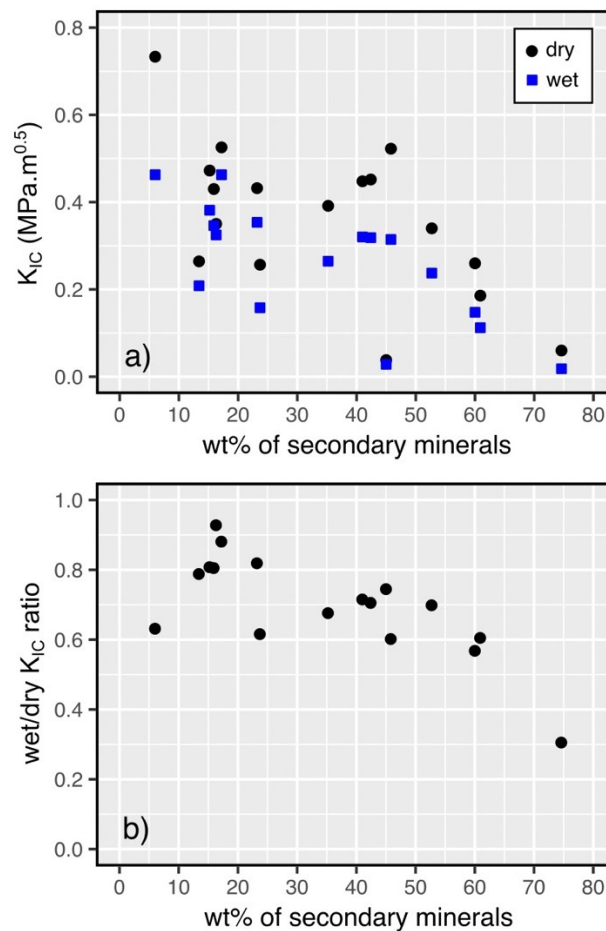
591 We conclude, therefore, that the water-weakening in the dome rocks measured herein
 592 (i.e. the reduction in UCS; Figures 4, 5, 6, and 8) is likely due to a reduction in K_{IC} in the
 593 presence of water (Figure 9a). The increase in water-weakening as a function of alteration
 594 (Figure 5b) can be explained by the decrease in the wet to dry K_{IC} ratio as the degree of
 595 alteration (i.e. the abundance of hydrophilic minerals) increases (Figure 9b). The higher the
 596 wt% of secondary minerals such as alunite and clay minerals, the more K_{IC} is reduced when
 597 the rock is wet, and the greater the macroscopic water-weakening.

598

Sample	Weight percentage of secondary minerals	Average equivalent macropore diameter, r (μm)	Dry K_{IC} ($\text{MPa}\cdot\text{m}^{-1/2}$)	Wet K_{IC} ($\text{MPa}\cdot\text{m}^{-1/2}$)	Wet/dry K_{IC} ratio
H2A	23	179	0.43	0.35	0.82
H2B	75	173	0.06	0.02	0.31
H3	35	174	0.39	0.26	0.68
H4A	60	153	0.26	0.15	0.57
H5A	42	161	0.45	0.32	0.71
H6	53	145	0.34	0.24	0.70
H14	24	156	0.26	0.16	0.62
H15	61	180	0.19	0.11	0.60
H18	15	137	0.47	0.38	0.81
H20	45	167	0.04	0.03	0.74
H21	41	157	0.45	0.32	0.71
H22	17	88	0.53	0.46	0.88
H25	46	182	0.52	0.31	0.60
H29	16	204	0.43	0.35	0.81
H32	6	101	0.73	0.46	0.63
WP1285	13	69	0.26	0.21	0.79
WP1317	16	84	0.35	0.32	0.93

599

600 **Table 4.** Percentage of secondary minerals (i.e. alteration), average equivalent macropore
601 diameter, wet fracture toughness (K_{IC}), dry K_{IC} , and the wet to dry K_{IC} ratio for the samples
602 prepared for this study. Sample locations are provided in Figure 1b. Values of K_{IC} are
603 estimated using the analytical approximation of Sammis and Ashby's (1986) pore-emanating
604 crack micromechanical model provided by Zhu et al. (2010) (Equation (1)). The relative
605 uncertainties in the quantification of the mineral contents (Table 1) are on the order of 5–10%.
606



607
608 **Figure 9.** (a) Fracture toughness (K_{IC}) and (b) the wet to dry K_{IC} ratio as a function of the wt
609 % of secondary minerals (i.e. alteration) for samples from La Soufrière de Guadeloupe
610 (Eastern Caribbean). Values of K_{IC} are estimated using the analytical approximation of
611 Sammis and Ashby's (1986) pore-emanating crack micromechanical model provided by Zhu

612 et al. (2010) (Equation (1)). The relative uncertainties in the quantification of the mineral
613 contents (Table 1) are on the order of 5–10%.

614

615 We highlight that we have performed experiments on completely dry and completely
616 water-saturated samples. However, in nature, the rocks forming a volcanic edifice or dome
617 likely exist between these two end-members (i.e. partially-saturated). Previous experimental
618 work has shown that the water-weakening observed at 100% saturation in sandstones can be
619 observed at saturation levels as low as 10%, whereas the compressive strength of shale
620 decreased almost linearly as a function of increasing saturation level (Schmitt et al., 1994).
621 Systematic uniaxial compressive strength measurements on partially-saturated volcanic rocks,
622 which preserve different porosities and alteration intensities, are now required to understand
623 whether the water-weakening observed here at 100% saturation (Figure 5) is also observed at
624 lower levels of saturation. Finally, we note that we have used deaired, deionised water for our
625 experiments. However, hydrothermal fluids, for example, are often characterised by low
626 values of pH (e.g., Delmelle et al., 2000) which can influence the short-term strength of rock
627 (e.g., Singh et al., 1999). Future experimental studies should also explore the influence of
628 fluid composition and pH on the short-term strength of variably porous and variably altered
629 volcanic rock.

630

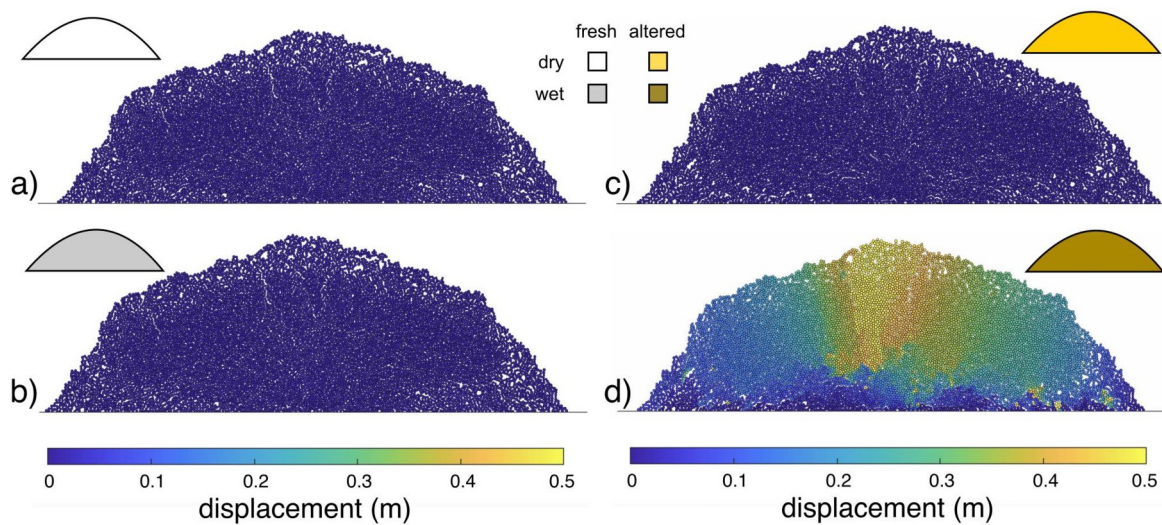
631 5.3 The influence of water-saturation on dome stability

632 We performed numerical simulations in PFC to better understand the influence of
633 water-saturation on dome stability. As explained in our methods section above, we first
634 prepared initially stable dry unaltered and altered domes, guided by our experimental data
635 (Table 2). For the dry unaltered dome, we determined the contact bond parameters required to
636 give uniaxial compressive strength, tensile strength, and Young's modulus values of 100 MPa,

637 10 MPa, and 25 GPa, respectively (Figure 10a). These values were chosen to represent the
638 samples in our dataset that are characterised by low levels of alteration (e.g., samples H18,
639 H22, WP1317). For the dry altered dome, we determined the contact bond parameters
640 required to give uniaxial compressive strength, tensile strength, and Young's modulus values
641 of 10 MPa, 1 MPa, and 2 GPa, respectively (Figure 10c). These values were chosen to
642 represent the samples in our dataset that are characterised by high levels of alteration (e.g.,
643 sample H2B). We used a bulk density of 2400 and 1500 kg/m³ for unaltered and altered dry
644 rocks, respectively, as guided by our experimental data. To investigate the influence of water-
645 saturation at the dome scale, we then changed all of the contact bond parameters in the model
646 to those representative of wet rocks (i.e. a completely water-saturated dome). Guided by our
647 experimental data (Figures 5b and 5d), we assumed wet to dry uniaxial compressive strength
648 and Young's modulus ratios for unaltered and altered dome rocks of 0.9 and 0.4, respectively.
649 Therefore, we determined the contact bond parameters required to give uniaxial compressive
650 strength, tensile strength, and Young's modulus of 90 MPa, 9 MPa, and 20 GPa, respectively,
651 for the unaltered dome (Figure 10b) and, for the altered dome, we used values of 4 MPa, 0.4
652 MPa, and 1 GPa, respectively (Figure 10d). We also note that, similar to the experimental data
653 shown in Figure 3, the strain required for failure in the model is also reduced upon saturation
654 with water. Guided by our experimental data, we used a bulk density of 2550 and 1900 kg/m³
655 for unaltered and altered wet rocks, respectively. Finally, we highlight that, although
656 alteration is thought to have reduced the strength of the dome rocks from La Soufrière de
657 Guadeloupe (Figure 4b; see also discussions in Heap et al., 2021a, 2022a), alteration has also
658 been observed, or inferred, to increase the strength of volcanic rocks (e.g., Frolova et al.,
659 2014; Pola et al., 2014; Wyering et al., 2014; Coats et al., 2018; Heap et al., 2020b, 2021b;
660 Kanakiya et al., 2021, 2022). Although the rocks in these studies contain minerals such as
661 kaolinite and alunite, minerals that may promote water-weakening, it is unclear at present

662 whether volcanic rocks that exhibit a strengthening resulting from alteration would be, as seen
 663 for the rocks from La Soufrière de Guadeloupe (Figures 4, 5, 6, and 8), weaker in the presence
 664 of water. As a result, until new experimental data are available, the numerical modelling
 665 presented herein should be considered applicable for scenarios in which alteration reduces
 666 strength.

667



668

669 **Figure 9.** A stable (i.e. no displacement) dome generated in Particle Flow Code (PFC) for a
 670 (a) dry unaltered dome and (c) dry altered dome. (b) The displacement in the dome shown in
 671 (a) resulting from changing the mechanical properties of the contacts in the stable dome to
 672 represent wet unaltered rock. (d) The displacement in the dome shown in (c) resulting from
 673 changing the mechanical properties of the contacts to represent wet altered rock. Blue - low
 674 displacement; yellow - high displacement.

675

676 The results of the modelling show that displacements across the unaltered dome are
 677 essentially zero when the dome is water-saturated (Figure 10b), suggesting that water-
 678 saturation alone does not significantly influence the stability of an unaltered dome. By
 679 contrast, displacements are high (up to 0.5 m) when the altered dome is water-saturated, with
 680 the highest displacements concentrated at the centre of the dome (Figure 10d). Therefore, if

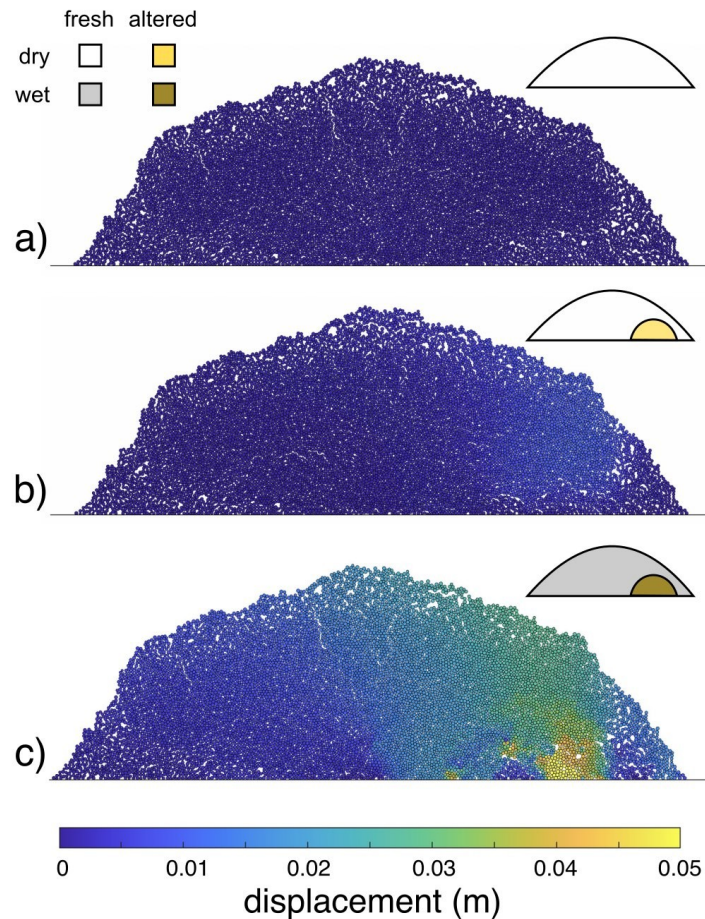
681 an altered dome becomes saturated by, for example, heavy rainfall or rising hydrothermal
682 fluids, the stability of the dome could be compromised, potentially promoting mass-wasting
683 events (e.g., partial dome collapse). We highlight that the propensity for such an altered water
684 saturated dome to collapse will likely be significantly enhanced if the dome is resting on a
685 sloping substratum (see, for example, Harnett and Heap, 2021) or comprises listric low-
686 strength layers from previous partial flank collapses that are opened in the direction of
687 maximum slope.

688 The scenarios shown in Figure 10 represent end-member scenarios in which the dome
689 is either completely unaltered or completely altered. In nature, hydrothermal alteration is
690 likely to be heterogeneously distributed within a dome (e.g., along permeable discontinuities).
691 Therefore, we performed additional modelling in which we included a buried altered zone
692 within an otherwise unaltered dome (Figure 11). In the first scenario, both the unaltered and
693 altered regions were dry (Figure 11b). In this scenario, only minor displacement is observed
694 within and above the buried altered zone (Figure 11b). In the second scenario, both the
695 unaltered and altered regions were water-saturated (Figure 11c). In this scenario, larger
696 displacements are observed within the altered zone, and notable displacements are also
697 observed on the half of the dome containing the buried alteration zone (Figure 11c). In other
698 words, dome instability was exacerbated following saturation in the presence of
699 hydrothermally altered materials. Therefore, the stability of a dome can be compromised by
700 water-saturation (e.g., heavy rainfall, changes to the hydrothermal system, and/or influx of
701 seawater) even when only a small portion of the dome is hydrothermally altered. It is likely,
702 in this scenario, that dome stability would be compromised more by alteration zones at or near
703 the base of the dome, rather than at the top of the dome.

704 The results of our modelling (Figures 10 and 11) may help explain why the frequency
705 of landslides and the failure and collapse of volcanic slopes and domes increases following

706 heavy rainfall (Kerle and van Wyk de Vries, 2001; Matthews et al., 2002; Matthews and
707 Barclay, 2004; Elsworth et al., 2004; Simmons et al., 2004; Taron et al., 2007; Saucedo et al.,
708 2008; Hicks et al., 2010; Vázquez et al., 2022).

709



710

711 **Figure 11.** (a) Stable (i.e. no displacement) dry altered dome generated in Particle Flow Code

712 (PFC). (b) The displacement in the dome shown in (a) resulting from adding a buried dry

713 alteration zone within a dry unaltered dome (as in Harnett et al., 2022). (c) The displacement

714 in the dome shown in (a) resulting from adding a buried wet alteration zone within a wet

715 unaltered dome. Blue - low displacement; yellow - high displacement.

716

717 Extreme or heavy rainfall has been linked to multiple volcanic hazards (McKee et al.,

718 1981; Matthews et al., 2002; Barclay et al., 2006), with theorised mechanisms that range from

719 deep-seated saturation and stress perturbations (McBirney, 1955; Violette et al., 2001;
720 Farquharson and Amelung, 2020; Sahoo et al., 2022) to shallow-seated processes operating in
721 the dome or upper edifice. Of these, the latter is often explained in terms of volumetric
722 expansion of liquids as a function of fuel-coolant interactions (Elsworth et al., 2004; Simmons
723 et al., 2004; Taron et al., 2007), with dome pressurisation and/or weakening driven by thermal
724 stress mechanisms (i.e. contraction of the dome carapace; Mastin, 1994; Yamasato et al.,
725 1998; Elsworth et al., 2004), or the stress on surrounding host-rock caused by the rapid
726 growth of a dome or cryptodome at shallow depth (Voight et al., 1981, 2002; Young et al.,
727 2002). More broadly, the saturation state of the edifice is thought to play an important role in
728 volcano destabilisation (McGuire, 1996; Day, 1996; Delcamp et al., 2016; Ball et al., 2018;
729 Finn et al., 2018; Heap et al., 2021b) and other failure processes, such as the failure of magma
730 reservoirs (Albino et al., 2018). Large-scale sector collapse events are also thought to be
731 facilitated by precipitation-induced retrogressive headward erosion and the weakening of
732 former partial flank collapse slip surfaces following precipitation (Kerle and van Wyk de
733 Vries, 2001; Kerle et al., 2003; Capra, 2006; Tost and Cronin, 2016; Romero et al., 2021).
734 Our new data presented here points to an additional role of dome or edifice rock saturation in
735 promoting volcano instability, in that saturation with water results in a reduction of rock
736 strength of ~10–60% (Figure 5a).

737 The distribution and degree of hydrothermal alteration throughout a volcanic edifice is
738 also thought to be key in determining the potential for collapse-related hazards. Commonly
739 occurring hydrothermal mineral assemblages are thought to have a deleterious effect on
740 volcano stability (López and Williams, 1993; Watters et al., 2000; van Wyk de Vries et al.,
741 2000; Reid et al., 2001; Voight et al., 2002; Reid, 2004; Cecchi et al., 2004; Salaün et al.,
742 2011; Ball et al., 2015; Detienne et al., 2017; Norini et al., 2020; Heap et al., 2021a), as
743 supported by geomechanical studies on the role of hydrothermal alteration in reducing the

744 strength of volcanic materials (in both natural and laboratory settings; Moon and
745 Jayawardane, 2004; de Potro and Hürlimann, 2009; Pola et al., 2014; Wyering et al., 2014;
746 Farquharson et al., 2019; Mordensky et al., 2019; Heap et al., 2021a, 2022a; Darmawan et al.,
747 2022). Indeed, structural failure is often spatially coincident with hydrothermal alteration
748 (Siebert et al., 1987; Delmelle et al., 2015); for example, Zimbelman (1996) notes the
749 association of alteration minerals—including alunite, jarosite, and other sulphates—with
750 exposed collapse scars, and Zimbelman et al. (2005) link collapse events at andesitic arc
751 volcanoes to veins of alteration minerals resulting from the mixing of magmatic sulphate
752 and meteoric water. Indeed, Voight et al. (2002) found varicoloured hydrothermally altered
753 materials within avalanche deposits at Soufrière Hills volcano (Montserrat, Eastern
754 Caribbean). This link is congruent with observations at La Soufrière de Guadeloupe, where
755 avalanche deposits and material ejected during phreatic explosions were found to contain
756 various parts of the active and ancient hydrothermal systems of the volcano (as evidenced by
757 an acid-sulphate mineral assemblage; Salaün et al., 2011; Heap et al., 2021a), and collapse
758 events have revealed visibly altered material beneath (see Figure 1b). La Soufrière de
759 Guadeloupe is characterised by a vigorous hydrothermal system (Komorowski et al., 2005;
760 Villemant et al., 2005; Nicollin et al., 2006; Coutant et al., 2012; Lesparre et al., 2012;
761 Brothelande et al., 2014; Villemant et al., 2014; Rosas-Carbajal et al., 2016, 2017; Moretti et
762 al., 2020; Gibert et al., 2022) and, notably, weathering fluxes on the volcano are among the
763 highest recorded in tropical volcanic settings (Dessert et al., 2015). Our new data underscore
764 the importance of this hydrothermal alteration on the mechanical properties of dome-forming
765 rocks (Figure 5b), with UCS decreasing by as much as ~60% upon water-saturation for highly
766 altered materials (i.e. with a high proportion of secondary minerals). These data further
767 highlight how and why hydrothermal alteration and water (meteoric, hydrothermal, or
768 seawater) conspire to destabilise volcanic structures, as outlined in disastrous effect by the

769 1998 collapse at Casita volcano (van Wyk de Vries et al., 2000; Kerle and van Wyk de Vries,
770 2001; Kerle et al., 2003; Opfergelt et al., 2006) and several large rain-triggered collapses
771 during the period of active dome growth at Soufrière Hills volcano (Elsworth et al., 2004;
772 Simmons et al., 2004; Taron et al., 2007).

773 Based on observations of rainfall-induced dome instability at Soufrière Hills volcano,
774 previous research has focussed on developing mechanistic models of dome collapse as a
775 function of local rainfall rate (Elsworth et al., 2004; Matthews and Barclay, 2004; Taron et al.,
776 2007; see also Harnett et al., 2019) thereby—directly or indirectly—accounting for the
777 saturation state. In contrast, numerous studies that model the influence of alteration on dome
778 or flank stability do not consider the presence or absence of interstitial fluids on the
779 mechanical properties of the dome-forming materials. Critically, our modelling results
780 (Figures 10 and 11) demonstrate that mechanical failure is promoted when both hydrothermal
781 alteration and water-saturation operate in tandem. We emphasise that mechanical water-
782 weakening is an important additional mechanism that has generally not been considered in
783 existing stability modelling of domes, but that can facilitate collapse, particularly in the likely
784 scenario when alteration is also present (Figures 10 and 11). We suggest that, when
785 appropriate and where available, volcano stability models use data relevant for water-
786 saturated volcanic rocks. We consider this especially relevant for submarine, coastal, and
787 ocean-island volcanoes. For the modelling of coastal and ocean-island volcanoes, it is likely
788 important to not only consider the partially saturated subaerial part of the volcano, but also the
789 saturated submarine part (the importance of not neglecting the submarine portion of a coastal
790 volcano is discussed in Urlaub et al., 2018).

791 We also highlight that the link between mechanical water-weakening and
792 hydrothermal alteration may also be further exacerbated when hot gases and rising
793 hydrothermal/magmatic fluids interact with water-saturated altered zones at shallow levels

794 within a dome or volcano, generating large volumes of steam and elevated fluid pore
795 pressures. Indeed, previous modelling has shown that elevated pore pressure negatively
796 impacts volcano stability (Reid, 2004; Heap et al., 2021b). The rise of large volumes of
797 hydrothermal fluids due to major heat pulses was observed during all phreatic unrest and
798 eruptions at La Soufrière of Guadeloupe (Komorowski et al., 2005; Rosas-Carbajal et al.,
799 2016) and Mount Pelée in 1902 (Tanguy, 1994), and is characteristic of the ongoing unrest at
800 La Soufrière of Guadeloupe (Moretti et al., 2020), underscoring the importance of
801 incorporating these phenomena into future, more complex volcano stability models.

802 By combining our experimental results with analytical and numerical mechanical
803 modelling, we link the micro-scale effects of hydrothermal alteration on the fracture
804 mechanics to the scale of the dome. These considerations are particularly important in light of
805 projected climate futures (Aubry et al., 2022). As circulation models results in Figure 1c
806 highlight, La Soufrière de Guadeloupe is projected to receive increasing amounts of heavy
807 rainfall over the next 80 years (see also Cantet et al., 2014), even given widespread adoption
808 of stringent climate change mitigation policies. Accordingly, the propensity for rainfall-
809 induced collapse events will likely increase as well, and detecting where potential collapse
810 events may occur becomes ever more crucial, especially given the absence of detectable
811 geophysical signals preceding such events (Matthews et al., 2002). Our model results
812 highlight that identifying zones of (potentially concealed) hydrothermal alteration may prove
813 to be key in mitigating such hazards (Darmawan et al., 2022; Harnett et al., 2022). Moreover,
814 our results emphasise that close monitoring of domes and slopes during and following heavy
815 rainfall is essential: we echo the call of Barclay et al. (2006) for routine incorporation of
816 meteorological data into monitoring praxis. Finally, direct knowledge of the state of edifice or
817 dome saturation could be invaluable for future stability monitoring. Our new data and models
818 add a new importance to the 3D characterisation of water saturation in volcanic edifices, as

819 well as its evolution over time, using geophysical methods. For example, geophysical
820 methods such as electrical resistivity tomography (e.g. Rosas-Carbajal et al., 2016) and muon
821 tomography (Rosas-Carbajal et al., 2017; Bajou et al., 2023) are sensitive to water content and
822 could be repeatedly employed to assess the distribution of water in the upper parts of the
823 volcanic edifice. Both seismic noise interferometry (Sens-Schönfelder and Wegler, 2006), a
824 method increasingly used to monitor water level variations, and the satellite-based monitoring
825 of groundwater (Rodell et al., 2009) could also be powerful tools to track edifice or dome
826 saturation. Finally, borehole dilatometers and piezometers (Hurwitz and Johnston, 2003)
827 could provide punctual and precise data to calibrate numerical models and interpret
828 geophysical imaging.

829 While La Soufrière de Guadeloupe has been chosen as a case study herein, for the
830 reasons outlined in section two, we emphasise that our conclusions are much more broadly
831 applicable. Volcanoes exhibiting zones of intense alteration and/or vigorous hydrothermal
832 systems (Africano and Bernard, 2000; Zimelman et al., 2005; Rodríguez and van Bergen,
833 2017; Yilmaz et al., 2021), as well as those with persistent acid lakes (van Hinsberg et al.,
834 2010; Rouwet et al., 2014; Delmelle et al., 2015), will likely be particularly prone to extensive
835 water-weakening, and therefore heightened instability, in the event of edifice and/or dome
836 saturation. In tropical environments, increasingly frequent bouts of intense precipitation (not
837 only during the so-called hurricane or rainy season but also at other times of the year in the
838 Tropics) may directly trigger mass-wasting events (as observed elsewhere in the Caribbean:
839 Matthews et al., 2002; Carn et al., 2004; Taron et al., 2007; Matthews et al., 2009 and in other
840 settings: Yamasato et al., 1998; Elsworth et al., 2004), or facilitate other internal or external
841 trigger mechanisms such as magma movement (Reid et al., 2010), pressurisation (Reid, 2004;
842 Heap et al., 2021b), or basal spreading (van Wyk de Vries and Francis, 1997). For the
843 majority of subaerial volcanoes, climate models project an increase in such meteorological

844 events (Farquharson and Amelung, 2022). In the context of water-weakening and related
845 hazards, climate change may also be important in terms of sea level rise. Analysis of drill
846 cores from offshore Montserrat shows that Soufrière Hills volcano underwent periods of
847 heightened instability during periods of rapid sea level rise, with all three large collapse
848 events at the volcano being correlated with sea level changes of > 5 m/ka (Coussens et al.,
849 2016); those authors also highlight that this phenomenon appears to echo a broader trend
850 across other island arc volcanoes. We further posit that variation in edifice saturation due to
851 sea level rise could also promote instability in coastal or island arc volcanoes.

852

853 **6 Conclusions**

854 The goal of this study, using dome stability models informed by experimental data,
855 was to investigate whether water-saturation alone can jeopardise the stability of a lava dome.
856 Our experiments show that water-saturation reduced the strength of variably-altered dome
857 rocks, and that water-weakening increased as the degree of alteration increased.
858 Micromechanical modelling suggested that the observed water-weakening was the result of a
859 decrease in fracture toughness, K_{IC} , in the presence of water. The ratio of wet to dry K_{IC} also
860 decreased with increasing alteration, explaining the trend of increasing water-weakening with
861 increasing alteration. Dome stability modelling, informed by our experimental data, showed
862 that the stability of a lava dome can be compromised by water-saturation if the dome is
863 altered, or contains an altered zone (even without the compounding exacerbating effects of
864 volcanic unrest, such as seismicity, deformation, high heat fluxes, and rising fluids). The
865 models presented herein may therefore help explain why the frequency of landslides and the
866 failure and collapse of altered volcanic slopes and dome increases following heavy rainfall.
867 Our study highlights, and especially in the light of predicted increases in heavy rainfall due to
868 climate change, that the degree of alteration and water-saturation should be monitored at

869 active volcanoes worldwide, volcanic domes and slopes should be closely monitored
870 following heavy rainfall, and that large-scale volcano stability models should use values for
871 water-saturated rocks where and when appropriate.

872

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897

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