

Wave Velocities and Poisson Ratio in a Loose Sandy Martian Regolith Simulant Under Low Stresses: 1. Laboratory Investigation

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1	Wave Velocities And Poisson Ratio In Loose Sandy Martian Regolith Simulant
2	Under Low Stresses. Part 1: Laboratory Investigation
3	J. P. Castillo Betancourt ^{1,2} , P. Delage ¹ , B. Caicedo ² , Ph. Lognonné ³ , B. Banerdt ⁴
4	¹ Ecole des Ponts ParisTech, lab. Navier-CERMES, CNRS, UGE, Marne la Vallée, France.
5	² Universidad de los Andes, Bogota, Colombia.
6	³ Université Paris-Cité, Institut de Physique du Globe, CNRS, Paris, France.
7	⁴ Jet Propulsion Laboratory, California Institute of Technology, Pasadena, California, USA.
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10	
11	Key Points:
12	• Bender Element measurements of V_p and V_s carried out on a novel device on loose
13	Fontainebleau sand samples used as a Mars regolith simulant.
14	• Investigation carried out within a so far unexplored low stress range, below 10 kPa and as
15	low as 1.75 kPa.
16	• Poisson ratio observed to remain mostly constant at low stress, with a possible decrease
17	in the zone below 5 kPa.

18 Abstract

Wave velocity measurements were performed on Fontainebleau sand samples used as Martian 19 regolith simulant to investigate the elastic properties of the surface material at the InSight landing 20 site on Mars (Elysium Planitia). Loose samples (density 1.4 Mg/m³, density index 6%) were 21 prepared by using the pluviation method, to mimic the low regolith density at the InSight landing 22 site. A novel device derived from triaxial testing was designed to measure wave velocities at low 23 stresses along a horizontal cylindrical specimen. Four tests were made, in which the confining 24 stress was applied by applying vacuum between 1 and 80 kPa. Wave velocities were measured by 25 using bender elements under stresses as low as 1.75 kPa, a very low value compared to the standard 26 stress ranges generally considered in terrestrial geotechnics (> 10 kPa). The changes in 27 compression and shear wave velocities obey a standard power law, with however two slightly 28 different exponents for V_p and V_s , indicating a not perfectly elastic behavior. Data showed greater 29 variability below 5 kPa, indicating some limitations of the bender elements technique in this range. 30 A slight decrease in Poisson ratio was detected below 5 kPa, which certainly deserves more 31 investigation. This investigation is useful to better analyze the data of the InSight mission, both in 32 terms of wave propagation at surface and to interpret some in-situ elastic tests carried out with the 33 scoop. These data are interpreted in the light of a granular contact mechanics theory in a companion 34 35 paper.

36 Plain Language Summary

The lander of the InSight mission placed on Mars a highly sensitive seismometer to detect seismic 37 38 waves from Marsquakes and meteorite impacts. To help analysing the seismic waves, a special device was developed to measure wave velocities on a loose sandy simulant mimicking the surface 39 regolith, with wave emitting and receiving transducers placed on both sides of a cylindrical sand 40 sample submitted to an external stress. Due to reduced gravity, stresses, that control the wave 41 velocity (the larger the stress, the faster the waves) are very low at the surface of Mars. To impose 42 low stresses (down to 1.75 kPa), we used a horizontal sample with a careful stress calculation 43 along the sample axis, along which waves propagate. We determined the changes in wave velocity 44 with stress (governed by a power law) and observed greater variability under 5 kPa, showing the 45 limit of our transducers at low stresses. Based on the elasticity theory, we derived from waves 46 velocities some parameter governing the sample deformations under changes of strain. We suspect 47

a slight decrease of the Poisson ratio (that characterises the radial elastic strain of a cylindrical
sample submitted to an axial load) below 5 kPa. These data are deemed useful to better constrain
the response of Mars sandy surface materials, at the InSight site and also in other areas for future
missions on Mars.

52 **1 Introduction**

Recent interest into the elastic behavior at small strain of sands under low stresses has been 53 gained through the investigation of the mechanical properties of the surface layer at the InSight 54 landing site on Mars, submitted to reduced gravity ($g = 3.721 \text{ m/s}^2$). Briefly, the NASA InSight 55 mission is a geophysical mission landed on Mars (Elysium Planitia) in November 2018. The 56 InSight lander installed on Mars' surface a highly sensitive seismometer, funded by CNES (Centre 57 National d'Etudes Spatiales, the French Space Agency) aimed at investigating both the seismic 58 activity and the meteorite impact activity on Mars. The lander also installed the so-called HP³ 59 thermal device funded by DLR (Deutsches Zentrum für Luft- und Raumfahrt, the German 60 Aerospace Center) that comprised a self-driving penetrometer aimed at measuring the thermal flow 61 emitted by Mars and at characterising the surface thermal properties (Spohn et al., 2018). The 62 seismometer (Lognonné et al., 2019, 2020) provided valuable information about the seismic 63 activity on Mars (e.g. Banerdt et al., 2018, Giardini et al. 2020, Hobiger et al., 2021) and about 64 65 meteorite impacts (e.g. Garcia et al., 2022; Posiolova et al., 2022). Unfortunately, the self-driving penetrometer could not penetrate more than 40 cm deep, due to a lack of friction along its shaft 66 (Spohn et al., 2022a and b). It provided however the surface thermal conductivity (0.039 ± 0.002) 67 W m⁻¹ K⁻¹), in agreement with orbital and in-situ thermal inertia estimates. Thermal data also 68 69 allowed to derive a quite low surface density of 1.2 Mg/m³ (Grott et al., 2021).



Figure 1. Image taken on Sol 1770 (i.e., the 1770th martian day of the mission) by the Instrument 71 72 Context Camera of the Lander showing the hemispherical Wind and Thermal Shield (WTS, diameter 720 73 mm) covering the SEIS instrument, with the Instrument Deployment Arm vertically pushing on the soil at 74 the right of the SEIS. A flat footprint made by a former push test by the IDA can also be observed. These push tests were carried out to estimate the surface elastic properties based on the sstrain derived from the 75 SEIS response. The (black) HP³ support system is observed on the left side, close to the place where the 76 77 self-driving penetrometer has been buried. Some scraps made by the IDA scoop to cover the regolith tether 78 for better thermal insulation can also be seen at the left of the WTS. All instruments are covered by a dust 79 layer progressively brought by the Martian winds (Image Credit: NASA-JPL).

80 The InSight mission provided much information on the mechanical properties of the surface (Morgan et al., 2018, Spohn et al., 2021, Delage et al. 2022a, 2022b, 2023, Marteau et al. 81 82 2021, 2022, 2023, Verdier et al. 2023). Significant input in this regard was gained by using the Instrument Deployment Arm (IDA). The IDA, designed to place the instruments on the surface of 83 84 the planet, was also used in various scrapping and piling activities carried out with the scoop at its edge (Golombek et al., 2023). Figure 1 shows a photo taken by the Instrument Context Camera of 85 86 the lander on sol 1771, close to the end of the mission, illustrating these activities. The photo shows that the landing site is flat and characterised by a sandy deposit with little rock abundance, as 87 88 planned from orbiter data during the landing site selection (Golombek et al., 2017). The photo also shows the hemispherical Wind and Thermal Shield (WTS) placed above the SEIS seismometer 89 and the scraps made by the Instrument Deployment Arm (IDA) placed with its scoop in contact 90 with the soil on the right hand side of SEIS). One also observes, left of SEIS, the scraps made by 91 92 the IDA scoop during the operation of covering the SEIS tether to improve thermal insulation and

93 reduce glitches in the seismic signals (Golombek et al. 2023). The support system of the self 94 penetrating thermal probe is observed on the left hand side, close to the place where the probe has 95 been penetrated. One can observe that all devices are covered by a dust layer that finally hampered 96 the functioning of the solar arrays, resulting in a too weak power supply to keep the instruments 97 working. This lead to the end of the mission on 18 December 2022.

The surface layer properties are not fully understood. Due to long term wind saltation, Mars 98 regolith particles at surface are sub-rounded to rounded (Goetz et al. 2010). Direct visual 99 observations of the pits excavated by the rocket exhausts during landing evidenced a layer of 100 101 cohesive sandy matrix containing some pebbles, called duricrust (see Warner et al. 2022, Delage et al. 2022a). It is suspected that the surface low density is made possible by inter-grains cemented 102 103 bonds within a loose arrangement of regolith particles. The inter-grains cement is suspected to be 104 due to salts deposited by thin films of water via interactions of atmospheric water vapor and soils, as suggested by chemical measurements by Viking and Mars Exploration Rover spacecraft (Banin 105 et al. 1992, Haskin et al. 2005, Hurowitz et al. 2005). 106

The in-situ determination of the surface regolith elastic properties could be made by recording with the seismometer the waves emitted during the hammering sessions of the penetrometer (Brinkman et al., 2022). The mean values of compression and shear wave velocities were $V_p = 119$ m/s and $V_s = 63$ m/s, respectively, in reasonable agreement with previous lab estimations on loose sands used as regolith simulants (Delage et al., 2017).

112 In spite of the difference between the sandy simulants used in terrestrial lab testing (Seiferlin et al. 2008, Delage et al. 2017, Delage et al. 2022b) and the regolith at the InSight landing 113 114 site, it was found interesting to further investigate the elastic properties of sandy simulants under stresses lower than those currently considered in terrestrial Soil mechanics to better analyse the 115 116 properties of surface sandy regolith on Mars, that are submitted to very low stresses. To do so, a specific device was developed in the laboratory to measure wave velocities in loose sand with 117 118 bender elements under low stresses. This paper presents the device and the results obtained in terms of wave velocities and Poisson ratio. In a companion paper (Caicedo et al. 2023), the data 119 are analysed through a theory of contact mechanics accounting for the roughness of particles 120 (Bachrach et al. 2000, Bahrami et al. 2005, Bachrach and Avseth 2008, Butt et al. 2015). 121

122 **2** Wave velocity and Poisson ratio in sands

The Poisson ratio v of sands has been determined through wave velocity measurements by various authors, including Bates (1989), Nakagawa et al. (1996), Prasad (2002), Kumar & Madhusudhan (2010) and Suwal & Kuwano (2013). The expression of v with respect to V_p and V_s is as follows, assuming an isotropic elastic medium:

127
$$\nu = 0.5 \frac{\left(\frac{V_p}{V_s}\right)^2 - 2}{\left(\frac{V_p}{V_s}\right)^2 - 1}$$
(1)

where V_p and V_s are the compression and shear wave velocities, respectively, given by the following expressions:

130
$$V_P = \sqrt{\left(K + \frac{4}{3}G\right)/\rho_b} \text{ and } V_S = \sqrt{\frac{G}{\rho_b}}$$
(2)

where *K* and *G* are the bulk and shear elastic moduli, respectively, and $\rho_{\rm b}$ the bulk unit mass of the sample.

In the stress ranges considered by these authors, the Poisson ratio decreased under 133 increased stress. Under high stresses, Prasad (2002) observed a decrease from 0.38 to 0.30 between 134 1 and 10.2 MPa ,with v = 0.30 up to 20 MPa. For Toyoura sand ($D_{50} = 0.19$ mm, $I_D = 0.68$), Suwal 135 and Kuwano (2013) showed that v decreased from 0.175 down to 0.15 between 50 and 400 kPa. 136 Kumar and Madhusudhan (2010) investigated the effects of stress (between 100 and 500 kPa), 137 density index I_D between 0.4 and 0.8 $(I_D = \frac{e - e_{min}}{e_{max} - e_{min}}$, where e_{max} and e_{min} are the sand 138 maximum and minimum void ratios, respectively) and grain size of well-sorted sands. All the 139 140 Poisson ratios measured were below 0.26 and decreased with increased density index. Some data under lower stresses in a dense random pack of identical glass beads were presented by Bachrach 141 142 et al. (2000), with ν ranging between 0.130 and 0.151 for stresses between 5 and 40 kPa. They 143 also conducted in-situ wave measurements in Moss Landing beach (California, dry angular sand) using a seismic line parallel to the shoreline of 20 geophones distant of 30 cm, and a hammer 144 (about 50 kg) applied on a metal block as source. The Poisson ratio derived from the velocity 145 profile between 0 and 5 m (vertical stress between 0 and 78.5 kPa with a dry unit mass of 1.7 146 Mg/m^3) was equal to 0.15, with no increase with depth. The Poisson ratios that they determined in 147 the lab were between 0.13 and 0.26, with a tendency to decrease i) under increased stress - more 148

significantly for fine-grained sand - and ii) under increased density index, whereas the in-situ determination by Bachrach et al. (2000) evidenced a constant v with respect to stress.

151

152 **3 Materials and Methods**

153 3.1 Fontainebleau sand as a Martian Regolith simulant

To account for the rounded shape of regolith grains on Mars (Goetz et al. 2010), we adopted 154 the NE34 Fontainebleau sand (see Delage et al. 2022b). It is a well-sorted silica sand (grain density 155 $\rho_{\rm s} = 2.651 \text{ Mg/m}^3$) from the Paris Basin with a median diameter D_{50} of 220 μ m, (see Figure 2, 156 Benahmed 2005), to compare with the 175 μ m diameter derived from thermal inertia 157 measurements in the InSight landing site (Golombek et al. 2017). The uniformity coefficient C_u is 158 equal to 1.65. The grain size distribution is presented in Figure 2a, together with a scanning 159 electron microscope photo (b) that shows that grains are sub-rounded to rounded. The minimum 160 and maximum void ratios are $e_{\min} = 0.54$ and $e_{\max} = 0.86$, respectively (Andria-Ntoanina 2011). 161 162



163 164

165 Figure 2. NE34 Fontainebleau Sand: (a) grain size distribution curve, and (b) SEM image.

166 3.2 Sample preparation

167 The sample was prepared, as for standard triaxial testing, by using a 170 mm high mould 168 of internal diameter 100 mm into which a neoprene membrane, fixed to the base of the triaxial cell 169 and the external face of the mould at the top by using two O-rings, was maintained by applying vacuum (80 kPa). The pluviation technique (Kolbuszewski, 1948; Cresswell et al., 1999,
Benahmed, 2001, Hariprasad et al. 2016) was used to prepare a low density sample, adopting an
almost null fall height (resulting in an upward speed of the sieve of 3 cm/s) and a small sand flow.

The final dimensions of the sample are 100 mm in diameter and 150 mm in height, with a density around 1.4 Mg/m³ (void ratio e = 0.84 - 0.82, density index $I_d = 6 - 10\%$). After pluviation, the sand in excess was carefully removed, the top cap was carefully placed on the flattened sand surface and the membrane was wrapped around it and fixed by an O-ring. Before dismounting the mould, the vacuum was decreased to 10 kPa, a pressure that appeared to be high enough to maintain the cylindrical shape of the sample.

179 3.3 Experimental setup at low stress

The wave velocity measurements were carried out by using 3 mm height bender elements (GDS brand, able to send and receive both compression P and shear S waves) inserted into the top and bottom ends of the sample. They were controlled via a signal generator producing sinusoidal waves at a fixed frequency of 10 kHz and 14 V amplitude. An amplifier was used to increase the magnitude of the received signal. Data acquisition was made through an oscilloscope, using two channels spatially located the furthest from each other, thus reducing as much as possible the crosstalk effect between bender elements (Lee & Santamarina, 2005).

In a vertical position, with a density of 1.4 Mg/m³, the vertical stress σ_v at the base is around 2.21 kPa (the weight of the soil column). Given the low-stress range considered (below 10 kPa), the stress difference between the top and bottom of the sample was not acceptable. It was then decided to place the sample in a horizontal position (Figure 3).

191



193	Figure 3. a) Sample preparation (vertical); b) Sample tested (horizontal).
194	The wave travel path along the axis of the horizontal cylindrical sample is submitted a total
195	stress σ equal to the sum of three components:
196	$\sigma = \sigma_{vac} + \sigma_m + \sigma_{gh} \tag{3}$
197	where σ_{gh} is the gravity-induced stress within the horizontal sample, σ_m the membrane isotropic
198	confining stress and $\sigma_{\rm vac}$ the vacuum-induced isotropic stress.

199 The stress produced by the membrane was evaluated using the solution for a small wall 200 cylinder under pressure as follows:

- The stiffness of the membrane was measured by imposing different forces on a membrane, as shown in Figure 4a. From the linear response obtained (Figure 4b), one can derive the spring coefficient *k* of the membrane as the ratio between the half of the applied unit load and the axial strain of the membrane. The data of the Figure provide k = 1613.85 N/m.
- The force F_m applied by the membrane around the sample (Figure 4c) is given by $F_m = k$ $\Delta r/r$, where k is the membrane spring coefficient, Δr the increase in the radius of the membrane between the free state and the stressed state around the sample and r the sample radius. Δr was obtained by subtracting the sample perimeter from the length of the extended membrane, L_m , divided by 2π ($\Delta r = r - L_m/2\pi$).
- Using the solution for a small wall cylinder under pressure, as shown in Figure 4c, the radial stress σ_m applied by the membrane per unit length on the sample is $\sigma_m = F/r$.
- With k = 1613.85 N/m, $\Delta r = 2.5$ mm and r = 50 mm, the confining stress due to the membrane is $\sigma_m = 1.6$ kPa, a result similar to that obtained by Henkel & Gilbert (1952) and Newland & Allely (1959).
- 215





Figure 4. (a) Setup used for measuring the membrane stiffness; (b) Force/vertical strain elongation curve of the membrane; (c) Schematic drawing for computing the stress on a cylinder.

The gravity-induced stress was computed in elastic conditions by using the Plaxis 3D finite element code (see Figure 5), applying the membrane stress as a boundary condition, providing σ_{gh} = 0.75 kPa at a density of 1.4 Mg/m³.





Figure 5. FEM (Plaxis 3D) elastic analyses of the sample self-weight to estimate the horizontal stress σ_{gh} on the sample axis.

The constant stress due to both gravity and the membrane is $\sigma_{\rm m} + \sigma_{\rm gh} = 0.75$ kPa. The total stress applied to the sample was changed by changing the vacuum applied within the sample. Once the (vertical) sample pluviated and the membrane fixed around it, a 10 kPa vacuum was applied before removing the mould and rotating the sample in the horizontal position, with no change observed in its size. The total horizontal stress applied on the sample axis was obtained by adding 0.75 kPa to the applied vacuum. Then, the loading path was applied by changing vacuum as follows:

233

234

• A step unloading path from 10 kPa to 1 kPa, (10 - 9 - 8 - 7 - 6 - 5 - 4 - 3 - 2 - 1 kPa) followed by reloading back to 10 kPa.

A step loading path from 10 kPa up to 80 kPa, (10 - 20 - 30 - 40 - 50 - 60 - 70 - 80 kPa).
 Moreover, some tests were performed at 5 and 15 kPa to investigate eventual hysteresis effects.

Various wave measurements were carried out at each stress to assess variability and repeatability. Five waves were recorded for stresses below 10 kPa, as a higher variability was observed in this stress range. In contrast, three measurements were deemed sufficient at 15 kPa and above (the number of repetitions was chosen after interpreting the results of the initial tests).

242 **3 Experimental results**

The tests were carried out on 4 pluviated samples (EP 1 to 4), with the following measured densities: EP1=1.443 Mg/m³, EP2=1.443 Mg/m³, EP3=1.447 Mg/m³ and EP4=1.452 Mg/m³.

Determining the accurate travel time of the waves is crucial in bender element tests, and 245 various methods have been used in the literature. They include the first inversion of the output 246 247 signal and the first zero value immediately after the signal inversion (Shirley & Hampton, 1978; Dyvik & Madshus, 1985; Jamiolkowski et al., 1995; Jovičić et al., 1996; Jovičić & Vilhar, 2009), 248 the peak-to-peak-method (Viggiani & Atkinson, 1995a), the cross-correlation frequency-domain 249 method (Viggiani & Atkinson, 1995b; Arulnathan et al., 1998; Boulanger et al., 1998; Mohsin & 250 251 Airey, 2003; Murillo et al., 2009; Murillo et al., 2011) and some methods simultaneously analysing the travel time of both P and S waves (Lee & Santamarina, 2005; Wang et al., 2017). 252

As seen in Figure 6, which presents the data for 70.75 kPa, an accurate determination of the travel time could be made by considering, for both P and S waves, the starting point of the sinusoidal emitted signal and that of the received P and S signals. Interestingly, there is a clear coupling between the reception of both waves. The S wave arrival could be detected in the received

257 P wave signal (at 0.57 ms), as commented by Wang et al. (2017). Note also that some slight

oscillations due to the P wave arrival (after 0.4 ms) are detected in the S wave signal, whereas a

clear increase in amplitude in the P wave signal is observed when the S wave arrives after 0.57

260 ms.



261

Figure 6. P and S waves under 70.75 kPa, with good correspondence between both: the S wave arrival can also be detected in the P wave signal; the arrival of the P wave creates some slight perturbations in the S wave signal (time is in milliseconds).

265

As seen in Figure 7, that presents both P (pink) and S (grey) signals under lower stresses (< 10 kPa), the signals become less clear at small stresses, particularly below 2.75 kPa. Besides carefully examining both signals and their coupling, the arrival time was also determined by extrapolating the trend resulting from larger stresses. The data show that bender elements can be used in (loose) sands, even under low stresses, smaller than those currently investigated in terrestrial geotechnical engineering.



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Figure 7. P (pink) and S (grey) waves detected under smaller stresses below 10 kPa.

Good agreement was also found with peak-to-peak data (calculating velocity with the tiptip distance between bender elements, Lee & Santamarina 2005). A software (Appendix A) was implemented to interpret arrival times, visualise all P/S waves of each experimental program in a group and suggest the departing and arriving points.

Figure 8 shows the change in wave velocities with stress in the tests on samples EP1 to EP4. The shape of the curves is comparable to other data on sands (e.g. Bachrach et al. 2000, Zimmer et al. 2002, Delage et al. 2017). As indicated in the Figure, the stress path followed first decreases from 10.75 kPa to 0.75 kPa, before increasing to 10.75 kPa up to 80.75 kPa.



Figure 8. Measured wave velocities obtained using bender elements compared with those measured by Brinkman et al. (2022) on Mars (red and blue points).

Inspection of data along the unloading-reloading path indicates no significant hysteresis effect in both V_p and V_s curves, showing satisfactory reversibility in the response, in conjunction with slight changes in the grain assembly. Also, the variability of the measured wave velocities between the different samples is low, demonstrating good repeatability and providing confidence in the results.

As expected, the velocities decrease with decreased applied stress, according to the following equations:

$$V_{\rm p} = 109.47 \left(\frac{\sigma}{\sigma_1}\right)^{0.314}$$
 $R^2 = 0.98$ (4)

(5)

295
$$V_s = 72.73 \left(\frac{\sigma}{\sigma_1}\right)^{0.306}$$
 $R^2 = 0.98$

296 where $\sigma_1 = 1$ kPa is a reference stress.

297 **4 Discussion**

The exponents derived from Figure 8 (Eqs 4 and 5) are slightly different for V_p and V_s , showing that our experimental data are not fully in agreement with the elasticity theory. When trying to fit V_p and V_s with the same exponent, one gets a value of 0.31, with no dependency of the Poisson ratio with respect to the confining stress. This leads however to a Poisson ratio of 0.105, in disagreement with our experimental results. The exponents values derived from Figure 13 are higher than those measured (0.22) at higher stresses between 25 and 450 kPa by Delage et al. (2017) on three other Martian simulants (Mojave Mars simulant, MSS-D and Eifel sand).

Interestingly, these data agree reasonably well with the in-situ measurements performed on Mars during the hammering campaign of the HP³ self-penetrating dynamic probe (see Figure 1), during which emitted waves were detected by SEIS. The V_p/V_s values determined by Brinkman et al. (2022) are reported in the figure, considering that they correspond to a vertical stress of 0.9 kPa, resulting from an average depth of 0.2 m at a density of 1.2 Mg/m³ (Grott et al. 2022) under a gravity of 3.721 m/s².

Figure 9, that presents the Poisson ratios computed using Eq. 1 from V_p - V_s measurements at each point, exhibits more variability below 5 kPa. This is probably related to the difficulty of

- 313 properly measuring travel times at low stresses (see Figure 12), in a stress range for which the use
- of bender elements becomes more tricky.



Figure 9. Changes in Poisson ratio with respect to confining stress determined from bender element measurements (points) and from the fitted curves of Figure 8 (curve).

319

Since the values of the exponents of Eqs 4 and 5 are close, the relationship V_p/V_s is slightly dependent on stress, with $V_p/V_s = (\sigma/\sigma_1)^{0.008}$. Therefore, from Eq. 1, the fitting equation for the Poisson ratio becomes:

323
$$\nu = 0.5 \frac{2.265 \left(\frac{\sigma}{\sigma_1}\right)^{0.016} - 2}{2.265 \left(\frac{\sigma}{\sigma_1}\right)^{0.016} - 1}$$
(6)

324

This low dependency of the Poisson ratio with respect to stress is illustrated in the curve of Figure 9 that indicates a slight decreasing trend below 10kPa, derived from both individual points and fitted curves.

The larger dispersion of our experimental data below 5 kPa indicates that bender elements are less efficient in this area. Inspection of the data of Figure 9 also shows that the changes in V_p/V_s with stress seems to somewhat differ below 10 kPa, with a less regular curvature of the data. The changes in Poisson ratio below 5 kPa would certainly deserve more investigation, in an area where the efficiency of bender elements is perhaps limited.

The velocities may be used to calculate the elastic shear modulus G (from Eq. 2) and the Young modulus $E = 2G(1 + \nu)$. The results, presented in Figure 10, show how the moduli decrease with decreased stress. Interestingly, the data at low stress are not far from the measured

value of E from interaction tests carried out with the SEIS foot on Fontainebleau sand (20 MPa at

- 337 3.5 kPa, Delage et. al 2022b).
- 338



339

Figure 10. Changes in shear and Young modulus with respect to confining stress determined from model predictions. The results from an interaction with the SEIS foot (red point) is in good agreement (Delage et al. 2022b).

343 **5** Conclusions

344 Wave measurements were performed under low stresses by using bender elements on loose samples of Fontainebleau sand used as a Martian regolith simulant. This was made to better 345 constrain the elastic properties of the regolith at the surface of the InSight landing site and to help 346 347 interpreting in-situ elastic data obtained at surface through wave measurements (Brinkman et al. 2022) of indentation experiment carried out with the IDA (Delage et al. 2023). Current knowledge 348 on wave transfer in sands was extended to low stresses (< 10 kPa) by using a horizontal cylindrical 349 sample submitted to changes in vacuum (between 2 and 80 kPa). Additional stresses due to gravity 350 351 and to the confining membrane were carefully calibrated and accounted for. Uncertainty in wave velocity determination was reduced by performing 3 to 5 measurements at each stress, and tests 352 were carried out on 4 different samples. Bender elements were able to provide wave velocity 353 measurements at stresses as low as 1.75 kPa, with however greater variability below 5 kPa. The 354 change in V_p and V_s with respect to stress followed a power law, with however two slightly 355 different exponent values, indicating that elastic hypotheses were not fully respected within the 356

sand sample. The calculation of the Poisson ratio ν from wave velocities measurements confirmed that ν was reasonably constant above 10 kPa. Conclusions are less clear below 5 kPa, where the bender elements measurements are less accurate. The possibility of having a decreasing Poisson ratio in this area seems still worth of investigation, to be conducted through other experimental techniques.

This study enlarged the knowledge of the elastic behaviour of sands under low-stress and low strain, a novel topic in wave velocities investigations, that was useful to interpret local wave velocity measurements carried out at the InSight landing site (Brinkman et al. 2022). Further information was also gained about the Poisson ratio under low stress, another important parameter to estimate the surface elastic response of the surface.

However, the comparison of our laboratory data on a Martian simulant with what could be 367 the response of a true Martian regolith is not straightforward. First, one have to consider possible 368 differences in the grain package, that is known to have significant effects on seismic wave travel. 369 370 It is very probable that the packing resulting from our pluviation method on a vertical sample, that has been put horizontally afterwards, differs from the packing of surface regolith grains on Mars, 371 that results from very long term wind saltation (through many repetitive small lateral jumps of 372 sand grains along the wind direction). It could be that the post convenient way to mimic the 373 resulting packing would be by trying to reproduce the saltation process on a (subrounded) sand 374 field submitted to constant wind. Then, the best would probably to run in situ wave determination 375 by using a line of geophones aimed at detecting an impact made on the ground, as done by 376 Bachrach et al. 2000 on a beach in California. 377

Note also that direct observations and measurements made on the InSight landing site 378 379 showed that the soil profile was not only made up of a sandy material. The true profile is composed 380 of a surface 1 cm thick sand/dust layer, overlaying an around 20 cm thick loose duricrust made up of a cohesive matrix containing some pebbles, located above a 12 cm layer of sand overlaying a 381 382 gravel/sand deposit (Golombek et a. 2020, Warner et al. 2022, Delage et al. 2022, Spohn et al. 383 2022). However, in-situ wave velocity measurements carried out during the hammering session of the HP³ self-penetrating gauge (Brinkman et al. 2022) provided values of velocity and Poisson 384 ratio close to those found in this work. Also, our investigation may be useful, as a first step, for 385 386 other areas of Mars, where thicker deposits of sand, with dunes and ripples, have been observed (e.g., Ehlmann et al. 2017). This can also be the case for future Mars missions. 387

Some of the questions raised in this experimental investigation are further considered in a companion paper (Caicedo et al. 2023) in which data at small stress are analysed through a contact mechanics theory derived from Bachrach et al. (2000).

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- 566

567 **Open Research**

- All the Waveform data csv files from the Bender Element measurements performed on the four
- samples, used for wave velocity calculations in the study are available at the public MRA-
- 570 Bender-element-repository repository (Castillo-Betancourt Juan Pablo. (2023)). Instructions may
- 571 be found on the "README" file of the repository.

572

Appendix A: Methodology for travel time determination

A Matlab script was developed to enable accurate and quick identification of the departing and arrival times on a considerable amount of data within a reasonable timeframe (about 150 waves were acquired in each of the four experimental programs, with a total of 1200 wave measures to analyse).

The MATLAB script allows to simultaneously consider up to three methodologies for identifying the departing and arrival times. The procedure identifies local maximum, minimum and zero values, that make up the potential points of interest of the signal (shown in Figure A1). This identification is an automatic initialisation method of suggested arrival and departure points. To achieve this, the following procedure was implemented.

1. Initially, a set of candidates for local maximums and minimums is established after evaluating the sign function of numerically calculated derivatives between every point in the signal and its surrounding neighbours. This procedure is a recursive method based on a parameter selected to determine an adequate number of points (seeking to not saturate the visual interface with too many choices of potential candidate points).

After that, the search is widened from each candidate point by following the curve up to
the previous existing inflection point (or the beginning of the signal, should it be the case). A new
candidate point is then proposed.

592



594 Figure A1. The interface of the Matlab script for single wave data analysis.

595

- 596 The script also allows simultaneous visualisation of the waves under various confining stresses, as
- seen in Figure A2. All the signals acquired at the same stress value are averaged to account for
- variability and repeatability. By observing all the measures, the user can choose the time arrival
- 599 points by clicking on a visual interface.



601 Figure A2. Interpretation script showing all waves measured at different stresses.

Proper signal visualisation is critical when using Wang's arrival time approach, because it requires 602 viewing P and S waves simultaneously on the same timescale, as seen in Figure A2. In contrast to 603 usual methods like peak-to-peak or arrival-to-arrival, a matching of P and S wave signals are 604 compared. While determining the P wave arrival time is relatively simple (the choice is given by 605 the first departure of the signal from the zero axis in the P signal), Wang's criterion indicates that 606 the location of the S wave arrival time correspond to the peak of the "initial main excursion in the 607 opposite direction of movement compared to that of the S-wave received signal". This means that 608 the first sizeable negative peak in S wave will characterise the S wave arrival time. Choosing this 609 610 last peak can often be a challenge and the associated P wave hence allows an objective and repeatable measurement. The chosen point for the S wave arrival time is to be found at the exact 611 location when the first negative peak with considerable amplitude appears in the P wave signal, 612 indicating the arrival of the S components produced by this P wave signal. This also implies that 613 614 the motion registered in the S wave signal before the chosen time of arrival corresponds to the

- 615 faster P wave produced by the action of the bender that emitted the S wave and its rebounds on the
- 616 sample walls.
- 617 Figure A3 shows an example of the waveforms acquired during one of the four experimental
- programs, along with lines marking the identified locations of the departure and arrival of the
- 619 waves for the three utilised criteria.



- Figure A3. Simultaneous visualisation of the P and S waves following Wang's method.
- 623

The following figures present the waves acquired during the other three experimental programs, along with the lines identifying the departure and arrival times for both P and S waves. The figures are shown as generated in the data processing tool built for data interpretation.



627 628

Figure A4. Waves acquired on sample EP1. Depart and arrival times are indicated both for P and S waves.









634 Figure A6. Waves acquired on sample EP3. Depart and arrival times are indicated both for P and S waves.



Figure A7. Waves acquired during on sample EP4. Depart and arrival times are indicated both for P and Swaves.