



# Assessment of the impact of climate change and flooding on bridges and surrounding area

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# Assessment of the impact of climate change and flooding on bridges and surrounding area

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Climate change has the potential to significantly impact transportation infrastructure performance. Bridges crossing rivers are designed to withstand a maximum flood level (design flood) considering the expected frequencies and magnitudes of floods in the area. The design flood level ensures the safety of the bridge without being damaged against historical flooding levels. However, flood magnitude and/or frequency are expected to increase in some regions due to climate change, and therefore, bridges may not be able to maintain their serviceability and safety, resulting in significant risk to users and economic losses. This problem is approached in this paper by investigating the effects of flooding and climate change on bridges crossing rivers and surrounding areas. The input of the proposed methodology is the river flow for various climate change scenarios as well as the topography and bridge characteristics. Flood frequency analysis is used to provide information about the magnitude and frequency of annual maximum river discharges under a changing climate. Afterwards, several risk assessment indicators are computed for the bridge and its surrounding area. In addition, stochastic Poisson process is integrated to account for the randomness of floods arrivals and to investigate stochastically the probability of exceeding the design flood level. The proposed methodology is illustrated with a case study in the United Kingdom. The results indicate that the risk of flooding, and associated consequences, would increase for the case study when considering more pessimistic climate change scenarios. Findings from this study can be used to inform decision making for improving bridges' resilience.

## KEYWORDS

climate change, bridges, stochastic Poisson process, risk assessment, flooding, floodplain mapping

## 1 Introduction

Floods are one of the most destructive and frequent natural disasters due to intense rainfall, melting snow, and overflowing rivers and streams. Floods can have a significant impact on the economy and environment, disrupting livelihoods around the world. Floods often inundate floodplains and adjacent lands and might cause river channel shifting, leading to tremendous losses in terms of property, agricultural lands, infrastructure assets, and lives. The force of strong currents can sweep people away or drown them in the rising water level. In a study in the United States for assessing the vulnerability of structures and residential communities to coastal flooding, results indicate that the most damage caused to the communities is due to dune erosion (Hatzikyriakou and Lin, 2018).

The recent technical knowledge associated with bridge engineering is based on the past mechanisms of bridges' failures that have occurred due to various causes, in which each cause of failure has its unique features. The common causes of failures are categorized as natural factors (earthquake, flood, wind, and landslide) and human factors (collision, traffic overloading, fire, lack of inspection and maintenance, improper design) those factors impact the stability of bridges and cause different types of failures (partial failure, total failure, and unserviceability of a structure), in which each type of failure corresponds to the degree of damage on the structure (Zhang et al., 2022). Coastal communities around the world are facing increased coastal flooding and shoreline erosion, this highlights the need for integrated and robust monitoring plans for mitigation plans after implementation (Palinkas et al., 2022). By the end of the 21st century, the sea level rise alone can claim more than 50% of Tarawa, the capital of the Republic of Kiribati, and pose risk to over 60% of the population under the most pessimistic climate change scenario (Sabūnas et al., 2021). Flooding represents the cause of almost of bridge failures over rivers due to a range of factors, scouring, hydrodynamic loads and pressures on the deck and piers, overtopping, and debris accumulation (Davidson et al., 2013; Shen et al., 2017; Mondoro and Frangopol, 2018). In the 20th century, floods were responsible for 6.8 million deaths from natural disasters across the world (Doocy et al., 2013). Globally, 2.82 trillion people were affected by floods between 1980 and 2009. In Europe, the number of people affected by floods was 1.41 million in 2013. In the United Kingdom, nearly 350,000 people were affected by floods in 2008 (Hannah et al., 2022).

The lifespan of infrastructure assets significantly varies depending on the type of infrastructure, environment, materials of construction, degree of maintenance, degradation factors, and extreme events, e.g., temperature, humidity, corrosion, floods, etc. For example, bridges can last up to 100 years or more, such as the Abraham Lincoln Bridge in the United States (Brad et al., 2019). However, bridges located in areas with extreme weather conditions may have a shorter lifespan (TRID, 2017). For example, the average life expectancy of bridges in the United States has been drastically cut to around 50 years as a result of floods in the 1990s (Crawford, 2023). Bridges could suffer a loss in their reliability over time due to progressive deterioration (corrosion, fatigue, creep, etc.) (Bastidas-Arteaga et al., 2020; Truong et al., 2022). When bridges are crossing rivers, they also might suffer a sudden loss in their reliability due to shock-based deterioration (floods, collisions, earthquakes, etc.) and their reliability depends on the severity of the shock size (Sánchez-Silva and Klutke, 2016). Therefore, inspections and maintenance are essential after the occurrence of any shock-based deterioration to ensure bridge structural safety (Collins, 1989).

In the case of flooding, the impact of unpredictable intense flows on the reliability of bridges and surrounding areas might be catastrophic. In addition, the effect of compound flooding is more severe on bridges due to the short time period between two or more events. Compound flooding is more frequent during winter/autumn than summer/spring (Robins et al., 2021). Therefore, engineers must be able to design the bridges to withstand the forces of pre-evaluated shock sizes and consider urban planning by identifying areas that are at risk of flooding, and then developing strategies to reduce the risk of flooding in those areas. In this context, natural features have been used as protection for infrastructures to reduce the risk of flooding, their application has grown steadily in

recent years to adapt to the changing climate (Bridges et al., 2022). In addition, railway engineers should adopt different mitigation methods for different failure scenarios under a changing climate (Setsobhonkul et al., 2017). As a result, recovery planning strategies should be implemented to conduct an assessment of the flooded road network performance throughout the recovery stage to provide the policymakers with several scenarios, this aims to improve the recovery process (Serdar and Al-Ghamdi, 2023).

Bridges over rivers are designed to sustain a design flood level determined by the hydrological flood frequency analysis (Bailey, 1989; Alexander, 1995; Stedinger and Griffis, 2008; Black and Fadipe, 2009; Watson and Biedenbarn, 2011; Bridge Scour Manual, 2019; England et al., 2019). This analysis interprets the historical flood events to evaluate the likelihood of future occurrences, estimate the frequencies of floods (recurrence intervals), and determine the magnitudes of these floods for bridge design purposes. This analysis can be used to assess the risk of flooding, identify areas that are more prone to flooding, inform decisions about flood management, and develop strategies for flood risk mitigation. Hydrologic flood frequency analysis is commonly used to determine the magnitude of the design flood level of a 200-year return period which represents the probability of 0.5% of a flood occurring in a given area based on the historical annual maximum river flow discharge values (Benn, 2013).

Over time, the climate has changed over the earth due to natural processes (Plate tectonics, volcanic activity, variations in the Earth's orbit, and solar variability). In the last century, the increase in the emissions of greenhouse gases have caused global warming (IPCC, 2014). Uncertainties of climate change are due to incomplete understanding of the interaction between the Earth's climate systems, natural variability of the Earth's climate system, and variability of the climate models (Reilly et al., 2001; Shepherd, 2014; Kundzewicz et al., 2018). Climate models are used to simulate the complex interactions between the Earth's climate system and provide predictions of climate indicators based on the degree of changing planetary processes, e.g., land use and emissions. However, their predictions are uncertain due to climate models' variability, forcing factors, aerosols, jet stream impact, resolution, etc. (van Vuuren et al., 2011).

Under a changing climate, the river discharge values in some areas are expected to increase in intensity and frequency due to a wide range of natural processes causing long-term shifts in temperature and weather patterns leading to more volatile precipitation patterns (Habeeb and Bastidas-Arteaga, 2022). Developing an assessment for flood frequency analysis methods under a changing climate is essential for effectively implementing flood risk assessment and management directives. For example, the European Parliament and the Council of the European Union recommended a comprehensive European assessment of different flood frequency analysis methods under varying climatologic and geographic conditions to ensure robust flood risk management strategies (THE EUROPEAN PARLIAMENT AND THE COUNCIL OF THE EUROPEAN UNION, 2007). This assessment should consider the potential of adaptation and mitigation strategies to reduce the impacts of climate change on the reliability of bridges across rivers. Subsequently, the design flood level of a bridge is expected to be affected in terms of the flood magnitude and probability of occurrence. In this context, the Intergovernmental Panel on Climate Change (IPCC) mentioned that

currently the intensity and frequency of floods in some regions caused the 100-year design flood level to occur every 2–5 years (Parry, 2007). Therefore, the design flood level of a bridge may need to be adjusted to account for higher values and more frequent floods to inform future decisions about flood management and future strategies for flood risk mitigations.

Economic losses in Europe under a changing climate increased by almost 2% per year between 2010 and 2020, Europe has lost around €145 billion in the last decade due to climate change-related events according to Eurostat (Eurostat, 2022). The devastating impacts of climate change on infrastructure resilience due to flooding in Europe are expected to rise between €14 Billion to €21.5 Billion by the end of this century (Ciscar, 2012). In the United Kingdom, the impact of climate change has already cost an average of over £50 million each year and this value is expected to increase due to the increased risk of failure of bridges (Dikanski et al., 2017).

The nature of most hydrological events, such as floods is random and varies with time and space, it is possible to estimate return periods by utilizing different probability distributions (Singh and Yadava, 2003). Flood frequency analysis (Todd, 1957) estimates the frequency of occurrence of a hydrological event, such as floods, and relates the magnitude of extreme events to their frequency of occurrence through the use of probability distributions (Stedinger and Cohn, 1986; Te Chow et al., 1988). This analysis can be approached using theoretical techniques based on probability distributions and applied research techniques, such as regionalization and floodplains-simulating software (Hamed and Rao, 2019). Floodplains should be generated using models that consider fluid dynamics to account for the impact of flood velocity on the extents (Marsooli and Wang, 2020). Theoretical techniques analyze the past annual peak flow discharge data in a certain location to calculate statistical information, such as mean values, standard deviations, and skewness. The analysis fits statistical distributions to the data, such as Log-Pearson Type III, Gumbel, Log Normal, Weibull, and Generalized Extreme Value (Cunnane and Singh, 1987) by using methods to estimate the parameters; then a flood with a specified probability of exceedance can be inferred from the distributions to provide frequency distributions, which are graphical representations that indicate the likelihood of different discharge levels occurring and relating the peak discharge values to their recurrence interval or exceedance probability. Parameters of the distributions can be estimated from data using estimation methods, such as the method of moments, maximum likelihood, and L-moments (Arnell, 2002). In particular, the method of moments is recommended by Bulletin 17B to fit the log-Pearson Type III (Tim, 1982). To ensure the performance of the methodology presented by Bulletin 17B, an investigation using Monte Carlo simulations indicated the consistency of the method (Griffis and Stedinger, 2007).

The objective of this paper is to propose a framework to assess the serviceability and safety of a bridge and its surrounding area prone to flooding under a changing climate. Several distributions of flood frequency analysis are investigated and the selection of the best fit involves goodness of fit tests and accuracy indicators to determine the flood levels for specific return periods, annual exceedance probability, and return period under a changing climate. Then risk assessment indicators assess the bridge performance and its surrounding areas under a changing climate. In addition, the serviceability of the bridge

being out of service due to flooding events under a changing climate is investigated stochastically using a Poisson process in which the rate of the bridge being out of service is determined based on the number of exceedances of the design flood level.

This paper is organized as follows. Section 2.1 presents a general description of the methodology proposed to analyze the effects of flooding events and climate change for bridges performance. Section 2.2 presents the flood frequency analysis distribution used in this paper. Section 2.3 provides the applied risk assessment approaches. Section 3 presents the case study and the database used in the analysis. Section 4 presents a comprehensive analysis of the potential impacts of climate change on the serviceability of a bridge in UK when subjected to flooding events. In addition, this section presents the impact of climate change on floodplain mapping.

## 2 Methodology

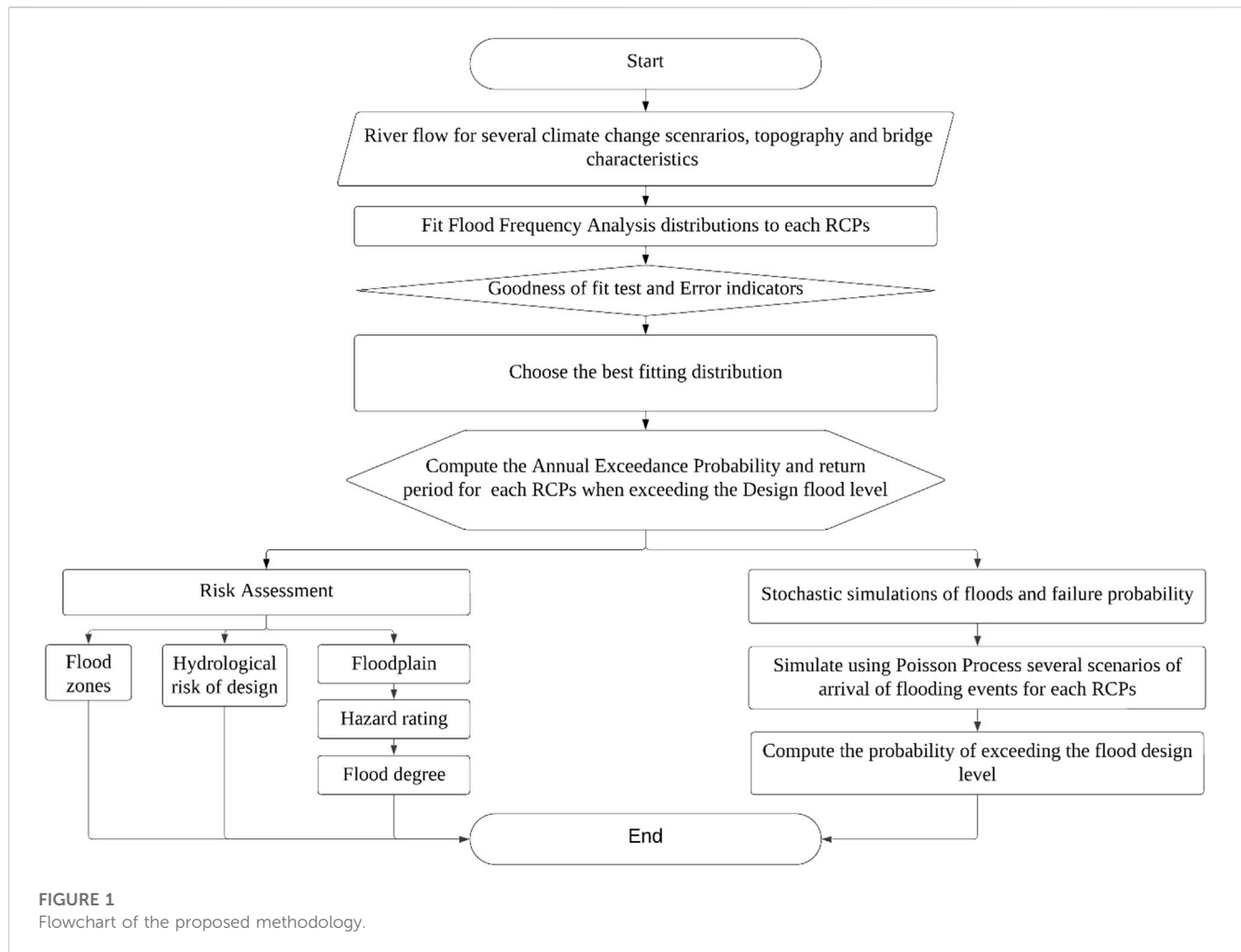
### 2.1 General description

This paper investigates the evolution of the annual probability of exceedance and peak discharge values under a changing climate, assesses accordingly the risk on the surrounding area of the bridge (road/inhabitant areas), and investigates stochastically the probability of exceeding the design flood level. The structural failure is beyond the scope of this paper.

Figure 1 presents the procedure used in this study. The main input is a database containing river flow predictions for several climate change scenarios. This database serves to fit the best flood frequency distribution that is then used to determine the flood levels for specific return periods and their annual probability of exceedance under a changing climate. Afterwards, risk assessment indicators are computed to assess the impact of exceeding the design flood level of the bridge on its surrounding area under a changing climate as follows. The bridge and its surrounding area are categorized into flood zones (Section 2.3.1) by considering the annual probability of exceedance. Hydrological risk of design (Section 2.3.2) presents the probability of a flood level that a structure can withstand without suffering damage during its design life. Flood hazard rating (Section 2.3.3) is based on the evolution of the floodplain and flood velocity; this indicator is used to indicate the flood degree which presents the degree of danger that floods can pose to people. In addition, the probability of the bridge being out of service (Section 2.4) is approached by a stochastic Poisson process with the bridge's rate being out of service determined by the peak annual discharge values from climate change scenarios exceeding the design flood level of the bridge since the bridge was designed to maintain its performance in the face of scour with a 200-year return period for a design flood level value of 114.19 m<sup>3</sup>/sec.

### 2.2 Flood frequency analysis

Gumbel distribution which is also known as the Extreme-Value Type I distribution is based on a double exponential distribution (Gumbel, 1941). This distribution is generally used to estimate the peak discharge values and probability of exceedance for return periods



of extreme hydrological events, such as floods. The probability of flood event exceedance  $P_T$  for a return period  $T$ , writes:

$$P_T = 1 - \left[ e^{-e^{-Y_T}} \right] = \frac{1}{T} \quad (1)$$

$$Y_T = -\ln \left[ \ln \left[ \frac{T}{T-1} \right] \right] \quad (2)$$

where  $Y_T$  is the reduced variate.

The annual peak discharge of a flood  $X_T$  for a return period  $T$ , writes:

$$X_T = \mu + \left[ \sigma \times \left[ \frac{Y_T - \mu_R}{\sigma_R} \right] \right] \quad (3)$$

where  $\mu$  is the mean of the sample,  $\sigma$  is the standard deviation of the sample,  $\mu_R$  is the reduced mean, and  $\sigma_R$  is the reduced standard deviation. Detailed illustrations of the Gumbel Extreme Value distribution, in addition to the values of the reduced mean and reduced standard deviation are discussed in (Phien, 1987; Ponce, 1989; Onni et al., 2007) and obtained from the Gumbel's probability table (Gumbel, 1958).

The annual peak discharge value in (Eq. 3) is based on the mean value and standard deviation of several projection scenarios of climate models, in which the projections are uncertain due to

climate models' variability, forcing factors, aerosols, jet stream impact, resolution, etc. Therefore, confidence intervals (CI) including Upper Limit (UL) and Lower Limit (LL) values are applied to the annual peak discharge to consider the uncertainties within the distribution, as follows:

$$X_{T(UL,LL)} = X_T \pm [CI \times S_e] \quad (4)$$

$$S_e = b \left[ \frac{\sigma}{\sqrt{N}} \right] \quad (5)$$

$$b = \sqrt{1 + 1.3K_T + 1.1K_T^2} \quad (6)$$

$$K_T = \frac{Y_T - \mu_R}{\sigma_R} \quad (7)$$

where  $X_{T(UL,LL)}$  in (Eq. 4) is the general equation of hydrologic frequency analysis which indicates the limits of the annual peak discharge values of a flood  $X_T$ , within these limits, the values are presented with a specific probability based on sampling errors,  $S_e$  is the probable error of the distribution which depends on the dispersion characteristic (standard deviation  $\sigma$ ) of the sample, the sample size  $N$ , and the frequency factor  $K_T$  which is based on the reduced variate in (Eq. 2) and the characteristics of the Gumbel Extreme Value distribution (the reduced mean and reduced standard deviation) as mentioned and discussed in (Eq. 3).

**TABLE 1** Flood zones [Adapted from Table D.1 in (Development and flood risk, 2006)].

Category	Description	Annual probability of flooding
1	Low probability	Less than 0.1%
2	Medium probability	Between 0.1% and 1%
3a	High probability	Between 1% and 3.3%
3b	Functional floodplain	More than 3.3%

**TABLE 2** Debris factor [Source Table 3.1 in (Wallingford, 2006)].

Flood depth $H$	Land use		
	Pasture/Arable	Woodland	Urban
0–0.25 m	0	0	0
0.25–0.75 m	0	0.5	1
> 0.75 m and/or $V > 2$ m/sec	0.5	1	1

## 2.3 Risk assessment

This section presents the impact of climate change on the serviceability of the bridge and its surrounding areas by determining the following indicators under a changing climate, i.e., flood zone categories, hydrologic risk of design, flood hazard rating, and degree of flood.

### 2.3.1 Flood zones categories

Flood zones are geographical areas that are designed according to the risk of flooding. This indicator can be used to determine the flood insurance rates and regulations for buildings in flood-prone areas. Table 1 describes and categorizes the flood zones based on the annual probability of flooding in (Eq. 1) (Flood Risk Assessment Guidance, 2017).

### 2.3.2 Hydrological risk of design

When hydrologists perform a design based on the  $T$ -year return period flood, they consider investigating the probability of a flood level that a structure can withstand without suffering damage during a design life  $D_L$ . The risk of hydrological design  $r$  (Vogel and Castellarin, 2017), is:

$$r = 1 - \left[ 1 - \frac{1}{T} \right]^{D_L} \quad (8)$$

### 2.3.3 Flood hazard rating

There is a general agreement on the degree of danger that floods can pose to people. In this context, flood hazard rating is affected by the severity of floods and is used to measure the risk of flooding in flood-prone areas to people. The flood hazard rating  $H_R$  (HR Wallingford, Flood Risks to People, 2006; Helen et al., 2005), writes:

$$H_R = H \times [V + 0.5] + D_F \quad (9)$$

where  $H$  is the flood depth,  $V$  is the velocity, and  $D_F$  is the debris factor which depends on the flood depth, velocity, and land use. The flood

depth is estimated based on the evolution of the floodplain that considers the topography and the flood characteristics. The debris factor value is determined from Table 2 as a function of the land use and flood depth to present a probability that a debris will lead to a hazard.

Table 3 presents the degree of the flood hazard and its impact on people based on the hazard rating value  $H_R$  (Eq. 9).

## 2.4 Stochastic simulations of floods and probability of exceeding the design flood level

A stochastic Poisson process is used to simulate various scenarios of flood occurrences exceeding the design flood level. The number of floods  $N = \{N_t: t \in [0, \infty)\}$ , where  $N_t$  is the number of arrivals in  $[0, t]$  is defined as a Poisson process by supposing that floods are shocks of a sequence of independent, identically distributed random variables with a rate  $\lambda \in [0, \infty]$ . This approach is used to stochastically investigate the impact of climate change on the serviceability of the bridge, in which the bridge is supposed to be out of service when an annual maximum river discharge value exceeds the magnitude of the design flood level. Subsequently, the probability of the bridge being out of service  $P_f(t)$  at time  $t$  involves the concept of the bridge's rate being out of service, which assumes that the lifetime of the bridge follows an exponential distribution. In this study, the probability of the railway bridge being out of service is related to maximum annual river discharge values exceeding the magnitude of the design flood level of the bridge under a changing climate and does not present the structural failure probability. The probability of the bridge being out of service  $P_f(t)$ , writes:

$$P_f(t) = 1 - \sum_{i=0}^n \frac{(\lambda t)^i}{i!} e^{-\lambda t}, \quad n \in [0, t] \quad (10)$$

where  $n$  presents the number of shocks (floods) to be out of service which in this case is  $n = 1$  to represent the state of exceeding the design flood level.

## 3 Application

### 3.1 Case study description

The case study bridge used in this study is located over the Cherwell River in Oxfordshire, United Kingdom. The railway bridge is a dual-span bridge with a steel deck, one pier of 2 m width, and a bed-to-soffit height of around 4.5 m. The bridge is designed to maintain its performance due to the impact of scour with a 200-year return period for a design flood level value of 114.19 m<sup>3</sup>/sec (Matthew, 2013). Figure 2 presents the cross-sectional view of the case study bridge, including materials used in construction, and ground elevations.

### 3.2 River flow projections database

The impact of climate change is considered in this study to assess the risk of the bridge subjected to flooding events caused by various annual maximum river discharge values from three

TABLE 3 Degree of a flood [Source Table 3.2 in (Wallingford, 2006)].

Hazard rating	Degree of flood	Description
< 0.75	Low	Caution: Flood zone with shallow flowing water or deep standing water.
0.75 to 1.25	Moderate	Danger: Flood zone with deep or fast-flowing water.
1.25 to 2.5	Significant	Danger: Flood zone with deep, fast-flowing water. Dangerous for most people.
> 2.5	Extreme	Extreme danger: Flood zone with deep fast flowing water. Dangerous for all people.

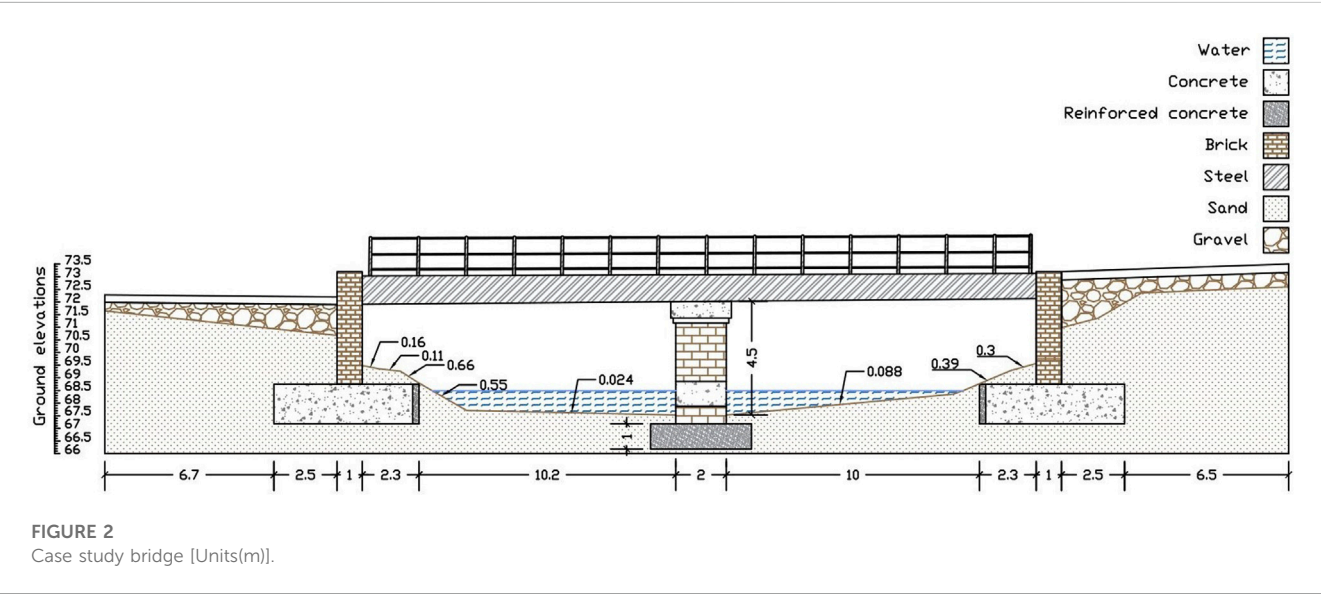


FIGURE 2 Case study bridge [Units(m)].

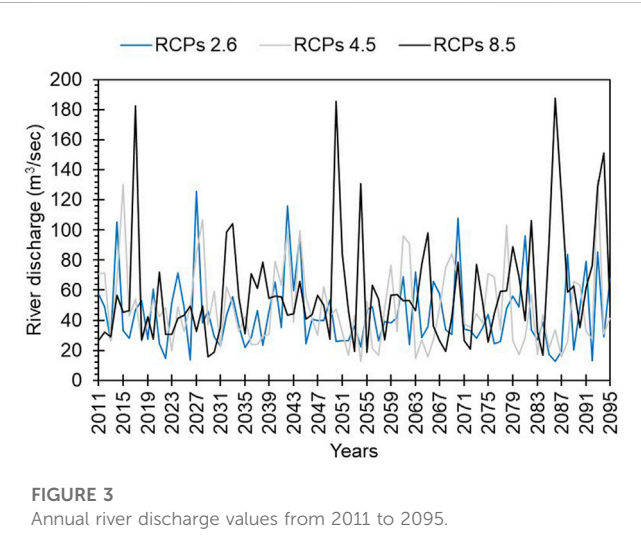


FIGURE 3 Annual river discharge values from 2011 to 2095.

projections scenarios (RCPs 2.6, RCPs 4.5, and RCPs 8.5) from 2011 to 2095, which are representative concentration pathways scenarios of greenhouse gases concentration that are labelled after a possible range of radiative forcing values (2.6, 4.5, and 8.5 Watts/m<sup>2</sup>). In this study, the KNMI-RACMO22E regional climate model which is derived from the KNMI-EC-EARTH global climate model is used to assess the risk caused by flooding events on the serviceability of the bridge under a

changing climate. The database used was produced through the IMPACT-2C research project funded by the European Union Seventh Framework Programme EU-FP7. The database is based on hydrological models which were driven by regional climate model data. The output data has been spatially interpolated, adjusted to the standard Gregorian calendar, and evaluated by the scientific partners involved in the IMPACT2C project (Greuell et al., 2015). The database is presented and discussed in Figure 3 (Section 4.1).

## 4 Results and discussions

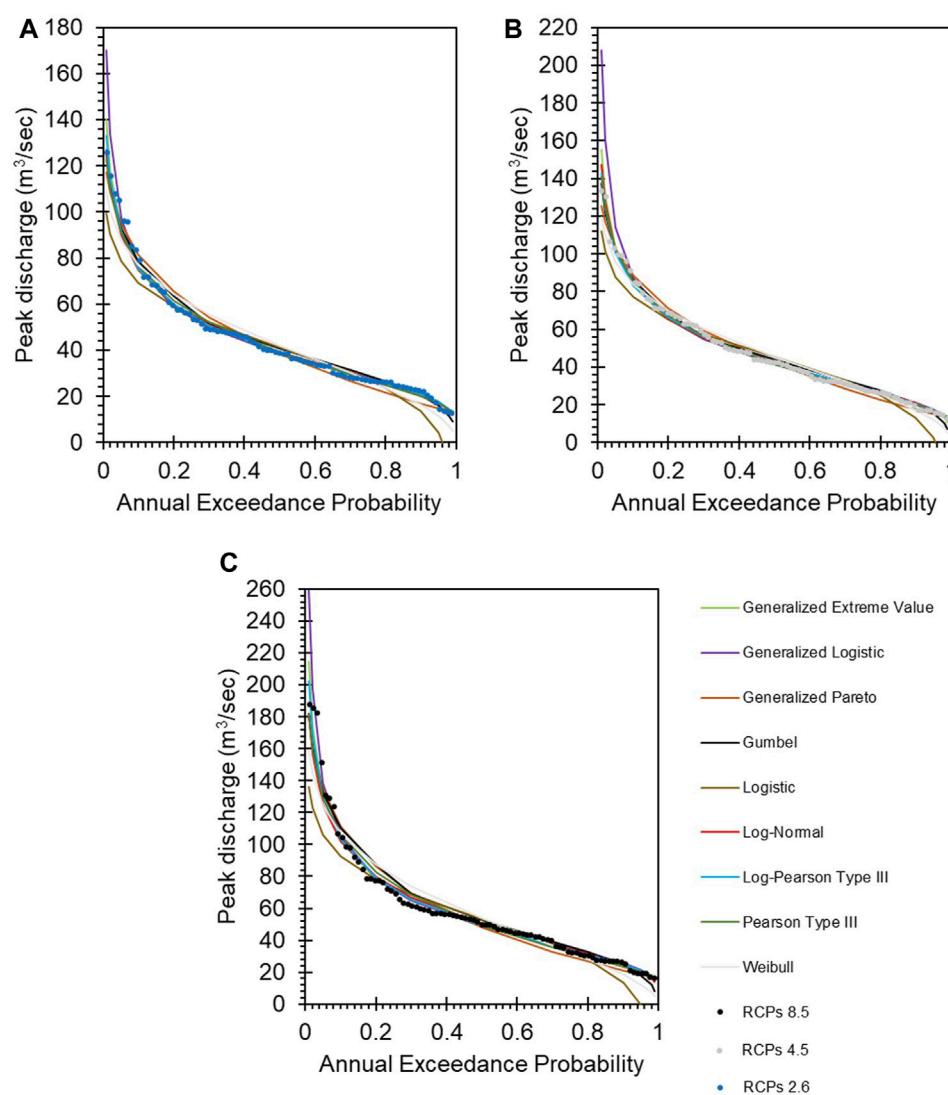
The main objective of this section is to investigate the evolution of the annual exceedance probability and peak discharge values under a changing climate. This analysis is useful to evaluate the degree to which such events are expected to affect people in the future. In addition, this section presents the floodplain mapping scenarios under a changing climate and investigates stochastically the probability of exceeding the design flood level under a changing climate.

### 4.1 River flow projections under a changing climate

The river discharge values are expected to increase for some regions in mean and variance under a changing climate. Figure 3

**TABLE 4** Statistical information of the river discharge values.

Climate model scenario	Mean value ( $m^3/sec$ )	Standard deviation ( $m^3/sec$ )
RCPs 2.6	44.7	24.0
RCPs 4.5	48.3	26.4
RCPs 8.5	58.7	36.5

**FIGURE 4**

Distributions fitting under a changing climate. (A) RCPs 2.6, (B) RCPs 4.5, and (C) RCPs 8.5.

presents the river discharge values for RCPs 2.6, RCPs 4.5, and RCPs 8.5 from 2011 to 2095 for the considered river. The observations indicate a higher river discharge values for RCPs 8.5, while the differences between RCPs 2.6 and RCPs 4.5 are not significant.

Table 4 presents the mean value and standard deviation of the river discharge for each RCPs. Higher mean and standard deviation values are observed for more pessimistic scenarios, while the differences between RCPs 2.6 and RCPs 4.5 are not statistically significant. These values are used in the flood frequency analysis to

fit the distributions which will control the shift (location) and the spread (skewness) of the distributions, thereby affecting its ability to present extreme river discharge values.

## 4.2 Flood frequency analysis

This section justifies first the choice of the best-fit distribution for several distributions including the commonly used distributions,

TABLE 5 Error indicators of the flood frequency analysis distributions.

	Gen. Extreme Value	Gen. Logistic	Gen. Pareto	Gumbel	Logistic	Log-Normal	Log-Pearson Type III	Pearson Type III	Weibull
RMSE (RCP 2.6)	6.03	18.06	5.65	3.23	15.80	3.61	3.71	3.93	10.02
ME (RCP 2.6)	1.65	8.90	-0.22	0.34	-10.28	-1.31	0.32	-1.43	-3.77
RMSE (RCP 4.5)	8.47	30.99	6.39	3.74	14.70	5.68	4.45	3.79	9.71
ME (RCP 4.5)	2.45	15.59	-1.39	-0.10	-9.96	1.09	-0.12	0.25	-4.57
RMSE (RCP 8.5)	11.21	27.63	10.67	7.67	32.46	11.74	8.66	10.76	19.91
ME (RCP 8.5)	1.52	12.64	-1.45	0.30	-20.79	-5.91	-0.65	-3.86	-7.69

RMSE, Root mean square error. ( $\text{m}^3/\text{sec}$ ).

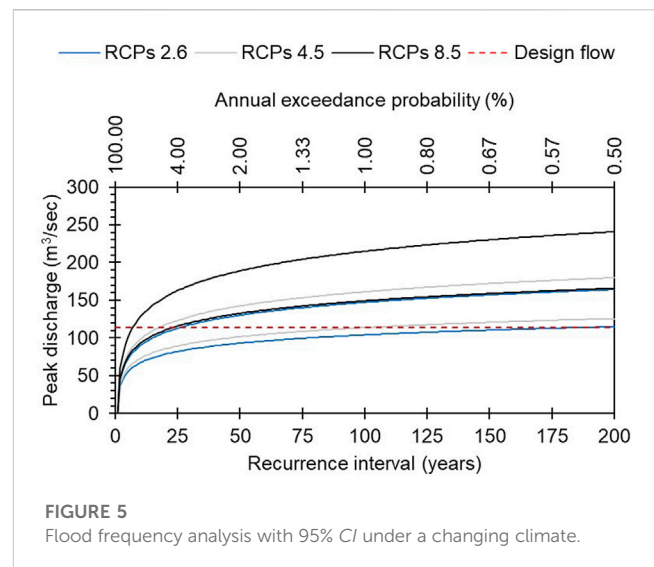
ME, Mean error. ( $\text{m}^3/\text{sec}$ ).

i.e., Generalized Extreme Value, Extreme Value Type I, Extreme Value Type III, Pearson Type III, Log-Pearson Type III, and Log-Normal distributions to determine which distribution fits better. The selection of the best fit involves goodness of fit tests and error indicators to determine the flood levels for specific return periods and their annual exceedance probability under a changing climate. As in (Hodson, 2022), the root mean square error (RMSE) and the mean error (ME) are used in this study. Furthermore, this section presents the relation between the peak discharge of flooding events and their frequency of occurrence.

Figure 4 presents the fit of the distributions under a changing climate. The observations indicate that Generalized Pareto, Generalized Logistic, Logistic, and Weibull distributions show the highest variations for each RCPs when compared to the other distributions.

The Kolmogorov-Smirnov goodness of fit test is first used in this study to investigate whether there is sufficient evidence that the distributions fit each RCPs. The results for all the distributions present a  $p$ -value  $> 0.05$ , and therefore, no rejection of any of the distributions was found for this goodness of fit test. Further investigation should be conducted using the error indicators to choose the best-fitting distribution. Consequently, the error indicators of the fitted distributions for RCPs 2.6, 4.5 and 8.5 are presented in Table 5 to determine the best-fitting distribution. The results indicate that the best-fitting distribution for all RCPs is the Gumbel distribution confirming the suitability of this kind of distribution for flood frequency analysis in this specific study case when considering climate change. Gumbel distribution is then used in this study to determine the flood levels for specific return periods and their annual exceedance probability under a changing climate. This is illustrated in Figure 5 by presenting the relation between the peak discharge values with their return period and annual exceedance probability for the Upper Limit  $UL$  and Lower Limit  $LL$  values of 95%  $CI$  for RCPs 2.6, RCPs 4.5, and RCPs 8.5. The results indicate that the flood design level of a 200-year return period is exceeded for more pessimistic scenarios, i.e., RCPs 8.5, RCPs 4.5, and RCPs 2.6, respectively.

Table 6 presents the change in the flood design level of a 200-year return period under a changing climate and also provides the change in the annual exceedance probability due to the decrease in the recurrence intervals as a result of the increase in peak discharge



values. The results indicate that higher peak discharge values are found for a higher probability of exceedance for RCPs 8.5, RCPs 4.5, and RCPs 2.6, respectively, in which the peak discharge values and the annual exceedance probability for the 200-year return period are exceeded under a changing climate which may impact the serviceability of the bridge.

## 4.3 Risk assessment

This section studies the impact of climate change on the risk of the bridge and surrounding areas by analyzing the flood zone categories, hydrological risks, flood hazard rating, and degree of flood.

### 4.3.1 Flood zones categories

Table 7 presents the expected flood zone categorization as defined according to the classification given in Table 1 under a changing climate. The results indicate the design flood level is

TABLE 6 Flood frequency analysis with a 95% CI.

	Design flood	Climate scenarios					
		RCPs 2.6		RCPs 4.5		RCPs 8.5	
		LL	UL	LL	UL	LL	UL
Peak discharge ( $m^3/sec$ )	114.19	114.89	164.15	125.62	179.89	165.78	240.08
Annual exceedance probability (%)	0.5	0.52	3.70	0.96	5.55	4.16	12.5

TABLE 7 Flood zone category under a changing climate.

	Design flood	Climate scenarios					
		RCPs 2.6		RCPs 4.5		RCPs 8.5	
		LL	UL	LL	UL	LL	UL
Category	2	2	3b	2	3b	3b	3b
Description	Medium	Medium	Functional floodplain	Medium	Functional floodplain	Functional floodplain	Functional floodplain

TABLE 8 Hydrological risk.

	Design flood	Climate scenarios					
		RCPs 2.6		RCPs 4.5		RCPs 8.5	
		LL	UL	LL	UL	LL	UL
Hydrological risk of design (%)	34.70	35.84	95.95	56.01	99.22	97.31	99.99

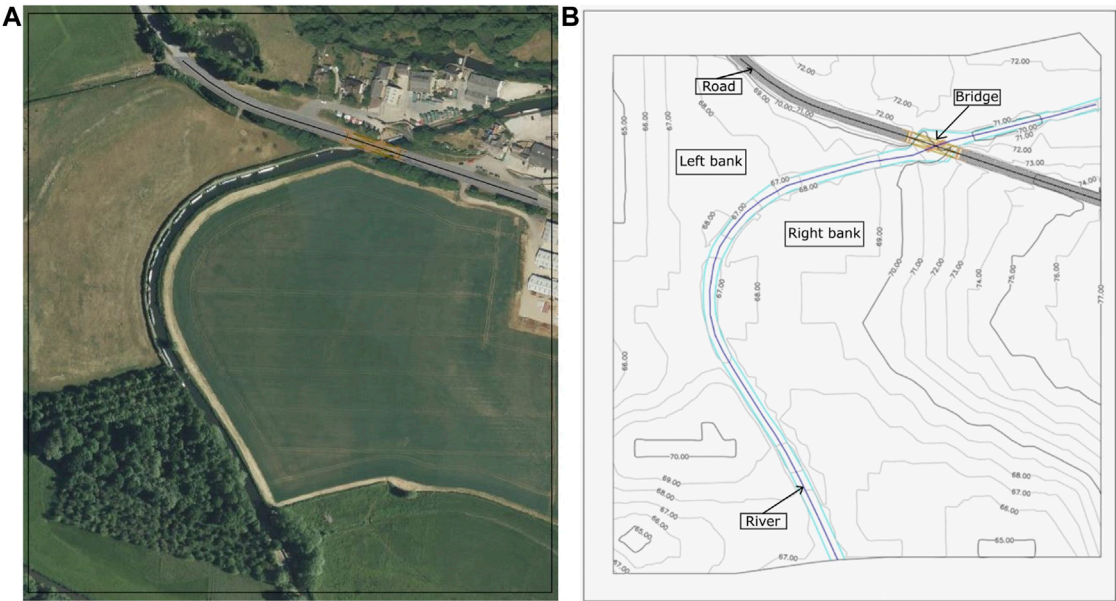
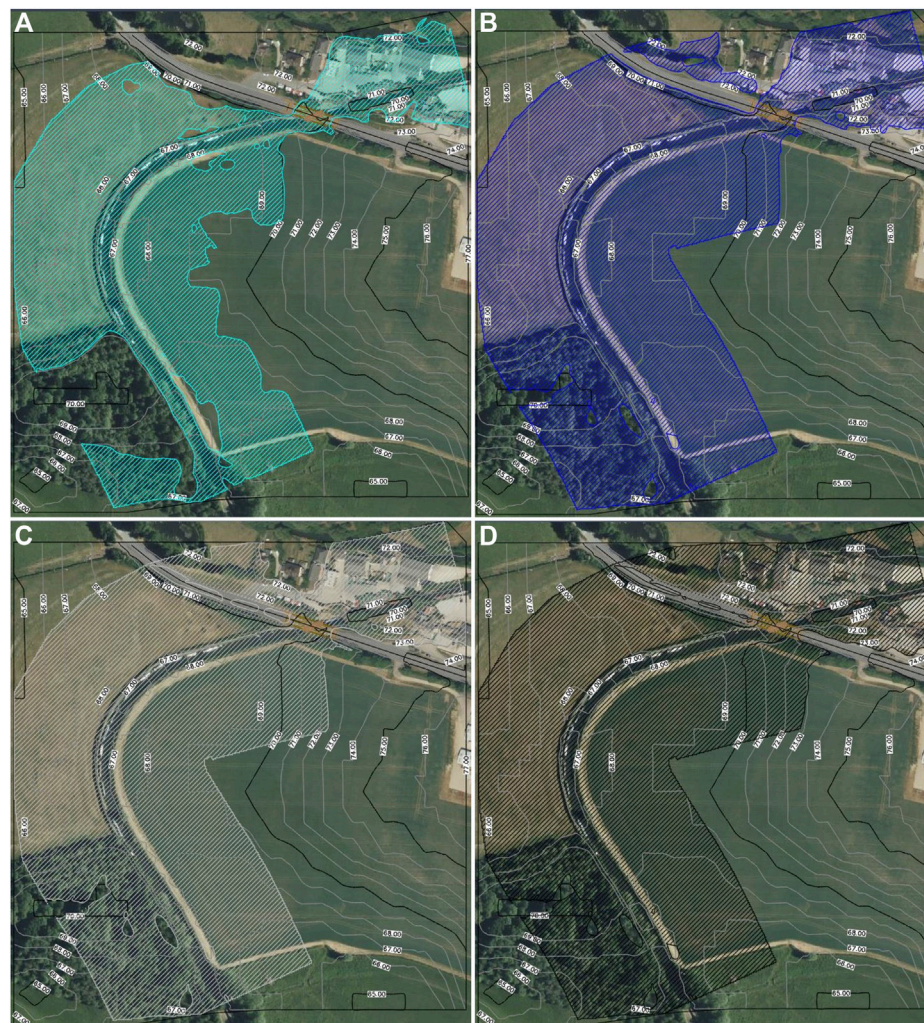


FIGURE 6  
Area of study. (A) Satellite image, (B) Topographical view.



**FIGURE 7**  
Floodplain using satellite imagery for the design flood level and several climate change scenarios. (A) Design flood, (B) RCPs 2.6, (C) RCPs 4.5, and (D) RCPs 8.5.

categorized as a medium floodplain. When investigating the potential impacts of flood risk under a changing climate, it is evident that RCPs 2.6 and RCPs 4.5 exhibit similar flood zone categories, ranging from medium to functional floodplain. On the other hand, the flood zone category for both *UL* and *LL* RCPs 8.5 is a functional floodplain. This suggests that for more pessimistic climate scenarios, there is a higher likelihood of experiencing more severe and frequent flooding events.

### 4.3.2 Hydrological risk of design

Table 8 presents the impact of climate change on the hydrological risk of a 200-year return period design flood level according to (Eq. 8). The results indicate that the hydrological risk associated with a changing climate increases for more pessimistic climate scenarios. The hydrological risk values for the *UL* and *LL* RCPs 8.5 indicate a higher likelihood of associated risks due to the increase of flooding events, while the hydrological risk values are not significantly affected for the *LL* of both RCPs 2.6 and RCPs 4.5 when compared to the design flood. This suggests that the hydrological

risk remains relatively stable for the *LL* of these scenarios, implying a lower likelihood of associated risks. However, there is a significant influence for the *UL* of both RCPs 2.6 and RCPs 4.5, and thereby there is high variability for RCPs 2.6 and RCPs 4.5.

### 4.3.3 Flood hazard rating

Flood hazard rating requires an assessment of the flow depths for the different climate change scenarios (Eq. 9). Therefore, it is necessary to assess the impact of climate change on the floodplain. This analysis uses the InfraWorks 3D design engineering software to generate the topographical mapping of the surrounding area of the bridge. The River and Flood Analysis Module for Civil 3D is used to simulate flooding scenarios by considering the peak discharge values of the design flood, RCPs 2.6, RCPs 4.5, and RCPs 8.5. This analysis assesses the impact of climate change on the floodplain and hazard rating on the two banks of the river, as well as determining which scenarios can affect the road crossing the river.

Figure 6 presents the topographical mapping and satellite image of the surrounding area of the bridge. The observations of the

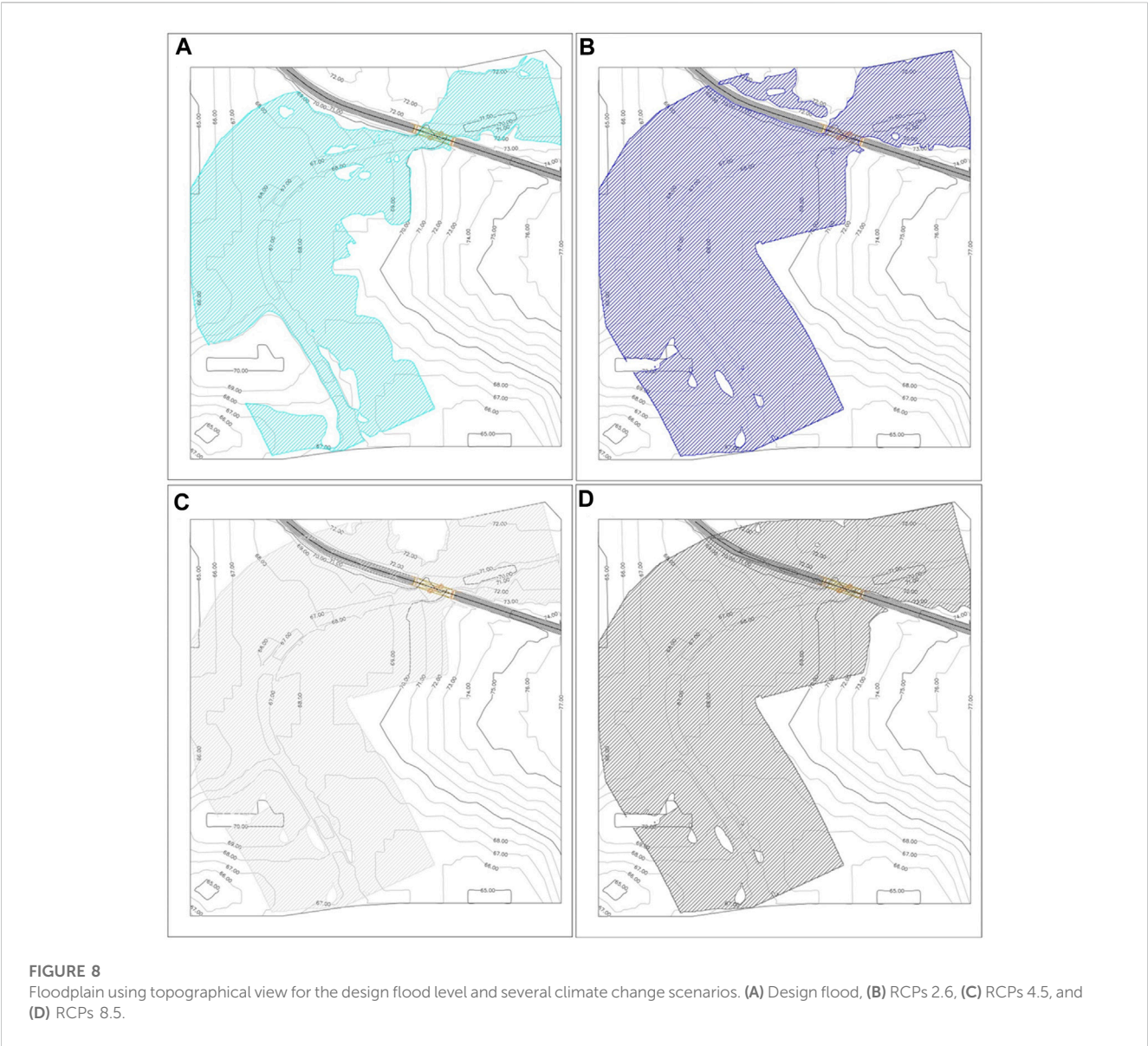


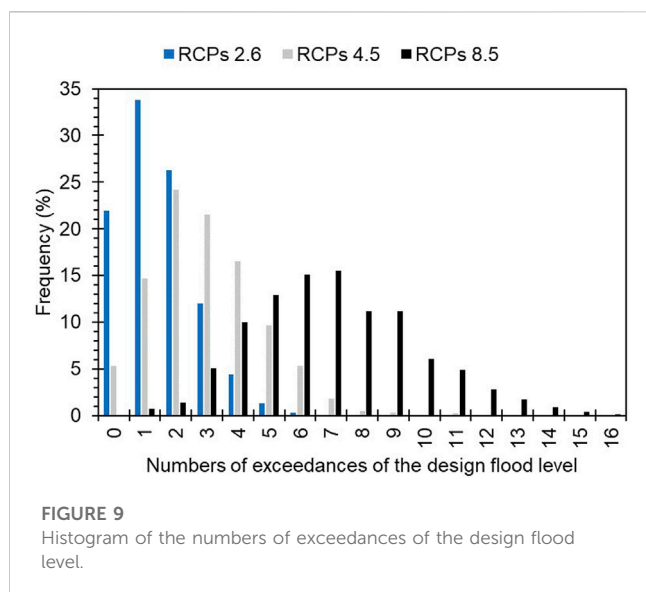
TABLE 9 Degree of floods under a changing climate.

	Design flood	Climate scenarios					
		RCPs 2.6		RCPs 4.5		RCPs 8.5	
		LL	UL	LL	UL	LL	UL
Left bank	Moderate	Moderate	Extreme	Significant	Extreme	Extreme	Extreme
Right bank	Low	Low	Moderate	Low	Significant	Moderate	Extreme

topographical view indicate that the right bank of the river is higher in elevation compared to the left bank of the river. In addition, the upper part of the satellite image shows that there are residential areas on the two banks of the river close to the bridge.

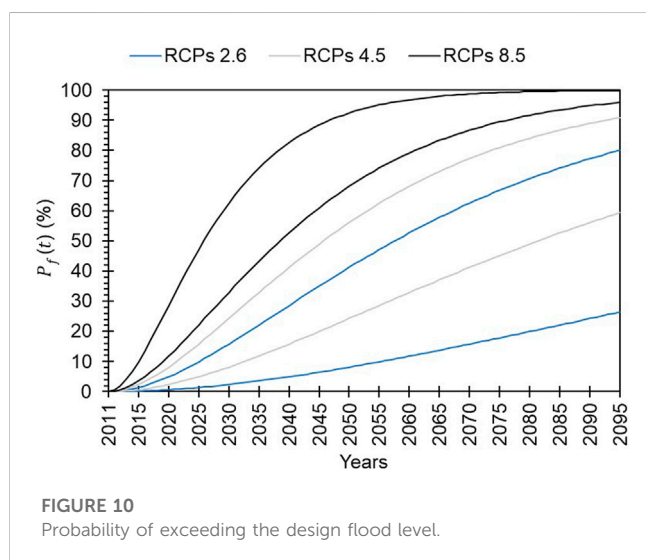
It is interesting to investigate the evolution of the floodplains for different flooding events and their impact on the case study bridge and its surrounding area. The floodplain is presented using the peak discharge

value related to a 200-year return period. The peak discharge value of a 200-year return period for the design flood is 114.19 m<sup>3</sup>/sec, while for RCPs 2.6, RCPs 4.5, and RCPs 8.5, the peak discharge value for each is computed using (Eq. 3). Figure 7 presents the satellite imagery of the flooded areas and Figure 8 presents the topographical view of the flooded areas. It is observed that the floodplain is expected to increase when considering RCPs 2.6, RCPs 4.5, and RCPs 8.5, respectively. The



**TABLE 10** Expected number of exceeding the design flood level under a changing climate.

	Climate scenarios					
	RCPs 2.6		RCPs 4.5		RCPs 8.5	
	LL	UL	LL	UL	LL	UL
Number of exceedances	1	3	2	4	5	9



serviceability of the road is not expected to be affected by the design flood and RCPs 2.6 flood scenarios. However, the road on the left bank of the river is likely to be out of service when considering RCPs 4.5 and RCPs 8.5 floods. On the other hand, the road on the right bank of the river is not affected by any of the flooding scenarios.

The hazard rating using (Eq. 9) assesses the degree of danger posed by floods to people and is controlled by the flood level, flow velocity, and topographic terrain according to Table 3. As the topographic terrain on both banks of the river varies, the assessment of the degree of danger posed by floods on both banks is expected to be more severe on the left bank of the river due to its lower land elevation, which puts buildings inhabited by people at greater risk on the left bank of the river. Table 9, presents the impact of climate change on the degree of floods. The results indicate that for each scenario, the left bank of the river is expected to face a higher impact on people in comparison to the right bank of the river due to the topographic terrain of the area. Furthermore, a higher degree of floods is expected for more pessimistic climate scenarios. For the UL of RCPs 8.5 on both banks of the river, the degree of the flood poses an extreme danger to people, indicating a flood zone with deep, fast-flowing water.

#### 4.4 Stochastic simulations of floods and probability of exceeding the design flood level

Stochastic assessment of the bridge and its surrounding area involves the concept of the rate of floods under a changing climate exceeding the value of the design flood level ( $114.19 \text{ m}^3/\text{sec}$ ). The quartile method (Hyndman and Fan, 1996) presents the Upper Limit UL and Lower Limit LL of the simulations resulting from Poisson Process. An increase in the impacts of climate change is predicted by all the climate models for RCPs 8.5 for this location (Pörtner et al., 2022). Figure 9 presents the frequency of the number of exceedances of the design flood level under a changing climate from 2011 to 2095. The results indicate an increase in the frequency of exceedances of the design flood level when considering more pessimistic climate scenarios.

Table 10 presents the UL and LL of the number of exceedances of the design flood level. The results indicate that a higher number of exceedances of the design flood level is found when considering more pessimistic scenarios. The results are used to compute the rate of exceeding the design flood level, which is the number of exceedances of the design flood level over the period from 2011 to 2095 to be used in computing the probability of the bridge being out of service in (Eq. 10).

Figure 10 investigates the probability of the bridge being out of service (Eq. 10) under a changing climate from 2011 to 2095 using a stochastic Poisson process. In accordance with previous results, Figure 10 indicates that there is a higher probability of the bridge being out of service under a changing climate when considering more pessimistic climate scenarios. A more refined analysis is required to estimate the probability of structural failure of the bridge at network and asset levels (Argyroudis and Mitoulis, 2021; Loli et al., 2022) but this point is beyond the scope of this paper.

## 5 Conclusion

This paper proposes a framework to assess the impact of climate change on the bridge's serviceability in the face of flooding events that combines river flow predictions for climate change scenarios,

flood frequency analysis, risk assessment indicators, and a stochastic process. The approach is applied to the analysis of the case study bridge. The main conclusions are summarized as follows:

- The proposed framework presented an interesting set of methods for investigating the impact of flooding events on the probability of bridges being out of service and their impact on the surrounding areas of bridges, in addition to evaluating the degree to which such events are expected to affect people in the future under a changing climate. This framework requires the knowledge of the design flood level of the bridge and does not consider structural failure.
- An investigation of the distributions of flood frequency analysis is mandatory to determine the best-fitting distribution under a changing climate based on a regional basis. The results of the investigation concluded that the Generalized logistic is no longer suitable to describe the flood growth in all regions in the United Kingdom under a changing climate as recommended in the (Flood Estimation Handbook) (Robson and Reed, 2008). In this study, the Gumbel distribution provided the best fit to the annual maximum river discharge values of the Cherwell River in the United Kingdom for all climate change scenarios when compared with other flood frequency analysis distributions.
- The stochastic Poisson process is suitable for assessing the serviceability of bridges subjected to floods to account for the randomness of floods' arrivals. In this study, a higher likelihood of the case study bridge being out of service (exceeding the design flood level) is expected when considering more pessimistic climate change scenarios.
- The flooded areas are expected to increase in regions where river discharge values are expected to increase when considering more pessimistic climate change scenarios. The following conclusions are based on the effects of the flooded areas under a changing climate in this study.
  - o The serviceability of the road is not expected to be affected when considering the design level and RCPs 2.6 flood scenarios. However, the road on the left bank of the river is expected to be out of service when considering flooding scenarios of RCPs 4.5 and RCPs 8.5.
  - o The left bank of the river is expected to face a higher impact on people under a changing climate in comparison to the right bank of the river due to the topographic terrain of the area.
- The risk assessment of bridges and their surrounding areas under a changing climate concluded the following:
  - o The annual exceedance probability of the 200-year return period flood is expected to increase in regions where river discharge values are expected to increase under a changing climate. In this study, the annual exceedance probability of the 200-year return period flood for the Cherwell River in the United Kingdom has increased when considering more pessimistic climate change scenarios. This indicates that bridges crossing the Cherwell River are expected to face more severe flooding events in the future.
  - o The risk of flooding indicated by flood zones becomes more severe in some regions under a changing climate. The case

study was categorized as a medium probability zone by considering the design flood level. The flood annual exceedance probability becomes more severe under a changing climate, and therefore, the risk of flooding indicated by flood zones becomes more severe under a changing climate, in which a functional floodplain zone is found for each RCPs Upper Limit *UL*.

- o A higher degree of floods is expected in regions where river discharge values are expected to increase when considering more pessimistic climate change scenarios. In this study, the bridge and its surrounding area are categorized extremely dangerous flood zones with deep fast-flowing water which is dangerous for all people when considering RCPs 8.5.
- o The impact of floods on road accessibility is expected to increase in some regions under a changing climate. Facilities and services such as, hospitals and working facilities are indirectly affected by floods due to inaccessibility of roads. Detecting the affected roads under a changing climate can help transportation administrators to prioritize their urban planning development in the future.
- Future research should consider the proposed framework to assess the risks posed by floods regionally, provide flood risk maps to inform decisions about flood management and develop strategies for flood risk mitigation. The author suggests presenting a complete framework to assess the impact of flooding events by also investigating the structural failure of bridges due to several damage states, such as the impact of local scour on the stability of bridges and the resulting impacts of hydrodynamic loads and pressures on the deck and piers.

## Data availability statement

The raw data supporting the conclusion of this article will be made available by the authors, without undue reservation.

## Author contributions

BH: Visualization, Writing—original draft, Writing—review and editing, Conceptualization, Data curation, Formal Analysis, Investigation, Methodology, Software, Validation. EB-A: Conceptualization, Formal Analysis, Funding acquisition, Investigation, Resources, Supervision, Validation, Writing—review and editing.

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## Conflict of interest

The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

The authors declared that they were an editorial board member of Frontiers, at the time of submission. This had no impact on the peer review process and the final decision.

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## Nomenclature

$CI$	Confidence intervals
$D_F$	Debris factor
$D_L$	Design life
$H$	Flood depth
$H_R$	Hazard rating
$K_T$	Frequency factor
$LL$	Lower Limit
$L_t$	Total length of years
$ME$	Mean error
$N$	Sample size
$N_t$	Number of arrivals
$n$	Number of shocks
$P_T$	Probability of exceedance
$P_f(t)$	Probability of the bridge being out of service
$r$	Risk of hydrological design
$RMSE$	Root mean square error
$S_e$	Probable error
$T$	Return period
$UL$	Upper Limit
$V$	Velocity of the flow
$X_T$	Annual peak discharge value
$Y_T$	Reduced variate
$\mu$	Mean of the sample
$\lambda$	Rate of shocks
$\mu_R$	Reduced mean
$\sigma_R$	Reduced standard deviation
$\sigma$	Standard deviation of the sample