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# Cellular concrete waste: an efficient new way for H<sub>2</sub>S removal

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## Abstract

The ability of cellular concrete waste (CC) to remove hydrogen sulfide (H<sub>2</sub>S) from a mimic biogas composed of nitrogen, oxygen and H<sub>2</sub>S was investigated using abiotic filtration systems for H<sub>2</sub>S concentration up to 1500 mg m<sup>-3</sup> and Empty Bed Residence Time (EBRT) ranging from 30 to 180 s. The influence of the humidity of the material in H<sub>2</sub>S removal was studied. In wetted conditions (relative humidity > 97%), 169 g of H<sub>2</sub>S were removed per kg of material. This value is probably lower than the maximum capacity of the cellular concrete since the experiment was ended before the end-of-life of the material. The H<sub>2</sub>S elimination capacity of cellular concrete waste could reach up to 32 g m<sup>-3</sup> h<sup>-1</sup>. These performances are due to complex physico-chemical mechanisms operating simultaneously between H<sub>2</sub>S and the different components of cellular concrete (mainly calcium oxide CaO from the calcium silicate hydrate CaO·SiO<sub>2</sub>·nH<sub>2</sub>O, and ferric oxide Fe<sub>2</sub>O<sub>3</sub>) leading to a modification of the mechanical structure of the material to calcium sulfate (gypsum CaSO<sub>4</sub>·2H<sub>2</sub>O) and the production of elemental sulfur (octasulfur S<sub>8</sub>). Empirical relationships based on H<sub>2</sub>S concentration and EBRT were established (i) to predict the Removal Efficiency (RE) of the filtration system (logistic equation); (ii) to assess the operating time of the material. Results obtained suggest that this simple, robust and cheap H<sub>2</sub>S removal technology would be a suitable and valuable technique to treat gas flowrates generated by decentralized domestic biogas digesters.

33

34 **Keywords:** Hydrogen sulfide; Cellular Concrete Waste; Desulfurization; Separation; Calcium  
35 oxide

36

### 37 **Highlights**

38 - H<sub>2</sub>S removal from a mimic biogas (N<sub>2</sub>-O<sub>2</sub>-H<sub>2</sub>S) using cellular concrete was investigated

39 - Cellular concrete is efficient to remove H<sub>2</sub>S in abiotic and wetted conditions

40 - The elimination capacity of the cellular concrete is at least 169 g<sub>H<sub>2</sub>S</sub> per kg<sub>material</sub>

41 - The elimination capacity of the filtration system reached up to 32 g m<sup>-3</sup> h<sup>-1</sup>

42

### 43 **1. Introduction**

44 Biogas, a renewable energy source produced by anaerobic digestion of raw materials such as  
45 agricultural and municipal waste, manure, green waste or food waste..., is an alternative to  
46 fossil fuels to produce energy. It is a mixture of gaseous compounds, mainly methane (CH<sub>4</sub>)  
47 and carbon dioxide (CO<sub>2</sub>) but contains also trace amounts of other components such as nitrogen  
48 (N<sub>2</sub>), water vapor (H<sub>2</sub>O), ammonia (NH<sub>3</sub>), oxygen (O<sub>2</sub>) and hydrogen sulfide H<sub>2</sub>S [1].  
49 According to the biogas plants and the feedstocks considered, the H<sub>2</sub>S gaseous concentrations  
50 can reach several thousands of ppm [2]. Hydrogen sulfide is known as a major contaminant in  
51 gas since it is both hazardous and corrosive. Its burning leads to the formation of sulfur oxides  
52 (SO<sub>x</sub>) which can affect both the environment and animal health. A H<sub>2</sub>S level limited to the range  
53 200-500 ppm is recommended even if a H<sub>2</sub>S-free biogas is advised [3]. Different technologies  
54 exist for H<sub>2</sub>S removal, such as biological treatments using microalgae or sulfide oxidizing  
55 bacteria and physicochemical treatments (membrane separation, chemical absorption, and  
56 physical adsorption techniques including adsorption on activated carbon or iron oxides, and  
57 pressure swing adsorption) [1,4–9]. Each of these techniques have their pros and cons in terms  
58 of complexity of operation, performance and cost effectiveness. They are suitable for large  
59 industrial gas treatment systems, however, they are not well adapted to treat small gas flowrates  
60 generated by decentralized domestic biogas digesters such as encountered in India and China  
61 for instance [10]. As unpurified biogas burning from small domestic installations used for  
62 cooking and lighting leads to significant SO<sub>x</sub> emissions, the development of cheap techniques  
63 adapted to these decentralized raw gas production systems must be encouraged. A simple

64 filtration using a bed filled with cellular concrete waste (CC) was recently highlighted to  
65 eliminate H<sub>2</sub>S from diluted in air under abiotic conditions, i.e. without any microbial population  
66 [11,12]. It was suggested that the ability of CC to remove H<sub>2</sub>S was due to reactions between  
67 H<sub>2</sub>S and some components of the material, mainly with calcium carbonate leading to potential  
68 gypsum formation [12]. Literature data highlight that H<sub>2</sub>S filtration using CC had never been  
69 tested to clean raw gas with high H<sub>2</sub>S content. The objective of this study was therefore to  
70 investigate the ability of a filter bed filled with cellular concrete waste to remove H<sub>2</sub>S in a mimic  
71 biogas to reach the recommended levels. The influence of the material moisture was  
72 investigated. Different operating conditions were applied to determine the filtration  
73 performance, in terms of elimination capacity (EC) and removal efficiency (RE). Interactions  
74 between H<sub>2</sub>S and cellular concrete components, were also studied.

75

## 76 2. Materials and methods

77 The properties and composition of the cellular concrete waste used (Figure 1) were detailed in  
78 previous studies dedicated to air cleaning [11,12]. This material, called “copolight”, is a mineral  
79 material provided by the Florentaise company (France). It is a light (density  $547 \pm 5 \text{ kg m}^{-3}$ )  
80 and porous (porosity 64%) material, slightly alkaline (pH of surface  $9.0 \pm 0.1$ ). Its composition  
81 in weight is: 50.5% SiO<sub>2</sub>, 24.6% CaO, 19.7% SO<sub>3</sub>, 2.2% Al<sub>2</sub>O<sub>3</sub>, 1.4% P<sub>2</sub>O<sub>5</sub>, 1.3% Fe<sub>2</sub>O<sub>3</sub>, 0.2%  
82 K<sub>2</sub>O [11].

83



84

85 *Figure 1 Cellular concrete waste; scale in centimeters. Left: raw material; Middle: extracted*  
86 *from the top of filter (a) at the end of experiment; Right: extracted from the bottom of filter (a)*  
87 *at the end of experiment*

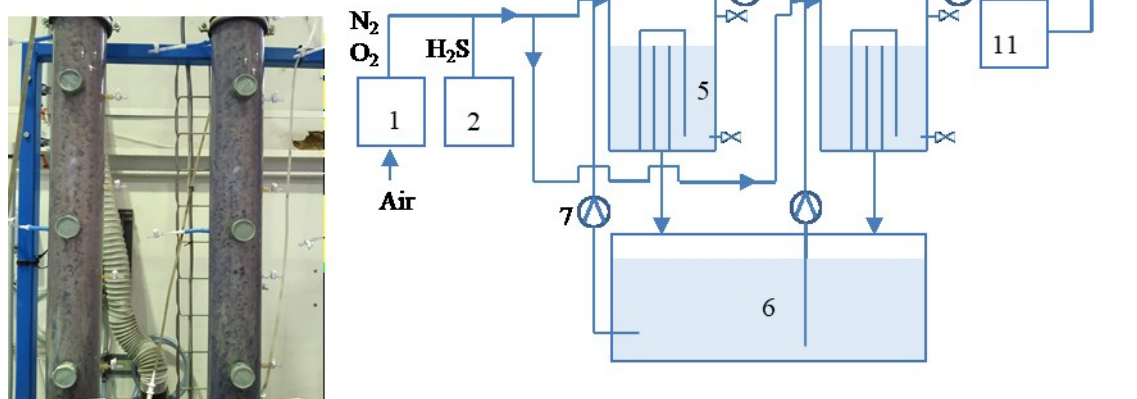
88

89 The experimental set up, depicted in Figure 2, consisted of two identical PVC cylindrical  
90 columns (internal diameter 100 mm) filled with the same volume of material (1 m height,  $V =$   
91 7.8 L). For safety reasons, a mimic biogas was used,  $\text{CH}_4$  being replaced by  $\text{N}_2$ . A nitrogen  
92 generator BrezzaNiGen LC-MS (40-1) (purity up to 99.9%) from Gengaz Company  
93 (Wasquehal, France) was used to supply  $\text{N}_2$  continuously into filters (called “Filter (a)” and  
94 “Filter (b)”). In spite of the ability of the generator to efficiently purify  $\text{N}_2$  from air, a weak  
95 fraction of oxygen, ranged from 0.5 to 0.8%, was always measured in the nitrogen gas. The  $\text{N}_2$   
96 flow rate entering a filter was controlled by a mass flowmeter (Model 5850S, Brooks  
97 Instruments, Hatfield, USA) and a stream of  $\text{H}_2\text{S}$  (99.7% purity), controlled by another mass  
98 flow meter (Model 5850S, Brooks Instruments, Hatfield, USA), was mixed with the  $\text{N}_2$  flow  
99 rate at the bottom of the filter. As a result, the gas flowing through the bed of CC was a mixture  
100 of  $\text{N}_2\text{-H}_2\text{S-O}_2$  mimicking an actual raw gas. Thermocouples (K type) were installed on each  
101 filter to measure temperatures. To maintain an optimal bed humidity, CC was humidified by a  
102 water drop-by-drop system. To respect the operating configuration of an industrial process, the  
103 water used was continuously recirculated in each filter. Wet conditions were controlled by  
104 humidity sensors (Model EE08, E+E Elektronik, Sevres, France) located at the top of filters.  
105  $\text{H}_2\text{S}$  concentrations were measured all along filters by an electrochemical analyzer (Biogas  
106 5000, Geotechnical Instruments Ltd, United Kingdom). Six sampling ports located at 0, 20, 40,  
107 60, 80 and 100 cm from the bottom were used for this purpose. The sampling ports were also  
108 used for measuring the pressure drops (pressure sensor Setra, Setra Systems, Inc, Boxborough,  
109 USA; 0–700 Pa).

110

**Legend:**

1. Nitrogen generator
2. Pure H<sub>2</sub>S bottle
3. Abiotic filters filled with cellular concrete
4. Watering distribution system
5. Water guard (3 L)
6. Water tank (20 L)
7. Volumetric pump
8. Valve
9. Temperature sensor
10. Relative humidity and temperature sensor
11. NaOH trap for H<sub>2</sub>S neutralisation



111

112 *Figure 2 Experimental setup (Filter (a): left; Filter (b): right)*

113

114 The parameters used to determine filters performances are reported in Table 1 and the operating  
115 conditions are summarized in Table 2. Experiments were performed for two distinct phases.  
116 During Phase I (from 97th to 124th day), the influence of the relative humidity on RE was  
117 evaluated using Filter (a) at a constant EBRT and H<sub>2</sub>S concentration. During Phase II (from  
118 146th to 215th day), the influence of EBRT and inlet H<sub>2</sub>S concentration on RE were studied for  
119 a relative humidity of the gas higher than 93% using the two identical Filters (a) and (b). Before  
120 Phase I, to fix the different operation problems encountered, both filters were operated  
121 intermittently during a preliminary stage for 96 days. Filters were fed with mimic biogas at day  
122 1 and all experimental parameters were continuously recorded from day 43. The H<sub>2</sub>S  
123 concentrations selected (up to 1500 mg m<sup>-3</sup>) were chosen according to the concentrations  
124 encountered in a raw biogas. Gas and material temperatures were measured, not controlled. For  
125 the running period, temperatures ranged from 17 to 27 °C according to climatic conditions (June  
126 to August 2021).

127 The composition of CC samples during H<sub>2</sub>S filtration was determined using an energy  
128 dispersive X-ray fluorescence (XRF) spectrometer (EDX-800HS, Shimadzu Company) at

129 different operating times, i.e. at day 1, day 43, day 138 and day 215. X-ray diffraction (XRD)  
 130 analysis was done to assess the modification in crystalline structure taking place within H<sub>2</sub>S  
 131 filtration (Bruker AXS - D8 advance diffractometer with Cu K $\alpha$  radiation,  $\lambda = 1.5406 \text{ \AA}$ , step  
 132 size of  $0.02^\circ$  in  $2\theta$  ranged from  $5$  to  $75^\circ$ , step time of  $576 \text{ s}$ ). Samples of the raw material at the  
 133 beginning of experiment (day 1) and on the used material at the end of experiment (day 215)  
 134 were analyzed.

135

136 *Table 1 Parameters used*

Parameter	Definition	Nomenclature
Loading Rate LR ( $\text{g m}^{-3} \text{ h}^{-1}$ )	$\frac{Q}{V} C_{\text{in}}$	$C_{\text{in}}$ : Inlet gas concentration ( $\text{g m}^{-3}$ )
Elimination Capacity EC ( $\text{g m}^{-3} \text{ h}^{-1}$ )	$\frac{Q}{V} (C_{\text{in}} - C_{\text{out}})$	$C_{\text{out}}$ : Outlet gas concentration ( $\text{g m}^{-3}$ )
Empty Bed Residence Time EBRT (s)	$\frac{V}{Q}$	Q: Gas flow rate ( $\text{m}^3 \text{ s}^{-1}$ )
Removal Efficiency RE (%)	$100 \frac{C_{\text{in}} - C_{\text{out}}}{C_{\text{in}}}$	V: Bed volume of the filter ( $\text{m}^3$ )

137

138 *Table 2 Operating conditions*

Phase	Inlet H <sub>2</sub> S concentration $C_{\text{in}}$ ( $\text{mg m}^{-3}$ )	EBRT (s)	Loading rate LR ( $\text{g m}^{-3} \text{ h}^{-1}$ )	Relative humidity RH (%)	
Filter (a)	I	320	90	12.8	15-98
	II(a)	350		14.0	
		500		20.1	
		630	90	25.2	>93
		770		30.3	
		910		36.4	
		500	60	30.2	
		500	45	40.2	>93
			30	60.3	
		Filter (b)	350		7.0
500				10.0	
900	180		18.0	>93	
1200			24.0		
1500			29.0		
1500	150		36.0		
1500	120		45.0	>93	
	100		54.0		

139

### 140 3 Results and discussion

#### 141 3.1 Influence of the relative humidity (RH)

142 The influence of the gas relative humidity is displayed at Figure 3(A) (phase I). It can be seen  
143 that the higher the relative humidity, the higher the removal efficiency. This finding clearly  
144 confirms that wet conditions are required to obtain H<sub>2</sub>S removal as evidenced by Lebrun et  
145 al. [12] for air treatment. Results from Figure 3(A) demonstrate that relative humidity close to  
146 100% is needed to significantly increase RE. To guarantee the removal efficiency, the humidity  
147 of the gas to be treated must be controlled. To some extent, this finding can be compared to  
148 results reported in the literature dedicated to the degradation of concrete sewer pipelines in  
149 wastewater infrastructure systems [13,14]. From controlled chamber experiments, Joseph et al.  
150 [13] investigated the role of relative humidity, temperature and H<sub>2</sub>S concentration in air on the  
151 concrete degradation. They showed that the influence of the relative humidity was not obvious  
152 at low temperature (16-18°C) whereas at higher temperature (25 and 30 °C), its influence was  
153 quite clear. This finding was confirmed by Wells and Melchers [14] who studied the corrosion  
154 of concrete in 6 different sewers throughout Australia during 48 months. These authors  
155 developed a model to predict the corrosion rate. According to this model, the corrosion rate is  
156 proportional to (i) the square root of the H<sub>2</sub>S concentration in the gas phase, (ii) the temperature  
157 via an Arrhenius equation, and (iii) the moisture content of the concrete expressed as the  
158 empirical functional relationship depending on the relative humidity (RH) for RH ranged from  
159 85 to 100%:

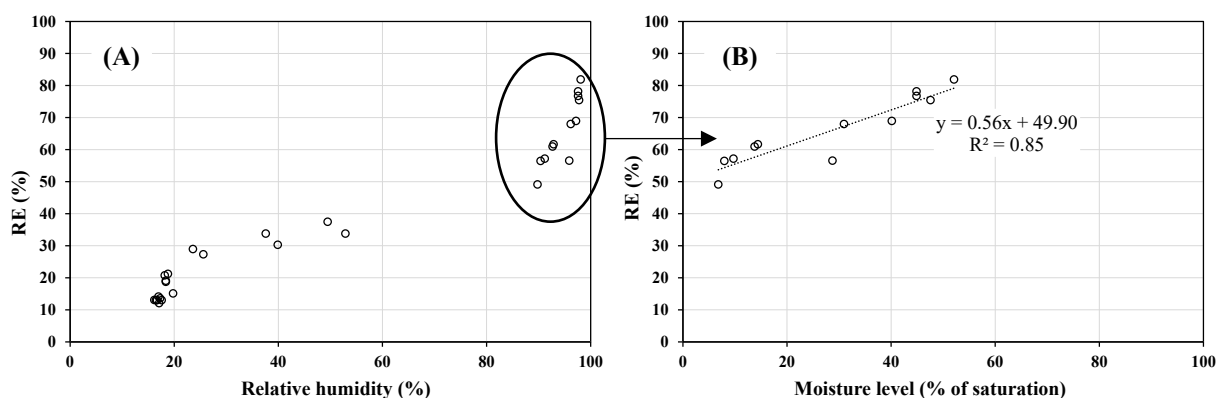
$$\text{Moisture level} = \frac{0.1602 \text{ RH} - 0.1355}{1 - 0.9770 \text{ RH}} \quad \text{Equation 1}$$

160

161 Applying this empirical relationship to the data reported in Figure 3(A) for RH>85%, the RE  
162 vs moisture level is displayed at Figure 3(B). As observed, a clear linear trend between RE and  
163 the moisture level exists, even if the regression coefficient is only 0.85. This figure confirms  
164 that cellular concrete must be largely wet to achieve a significant removal efficiency. H<sub>2</sub>S is  
165 then probably first transferred from the gas phase to the aqueous phase to react with CC  
166 components.

167





168

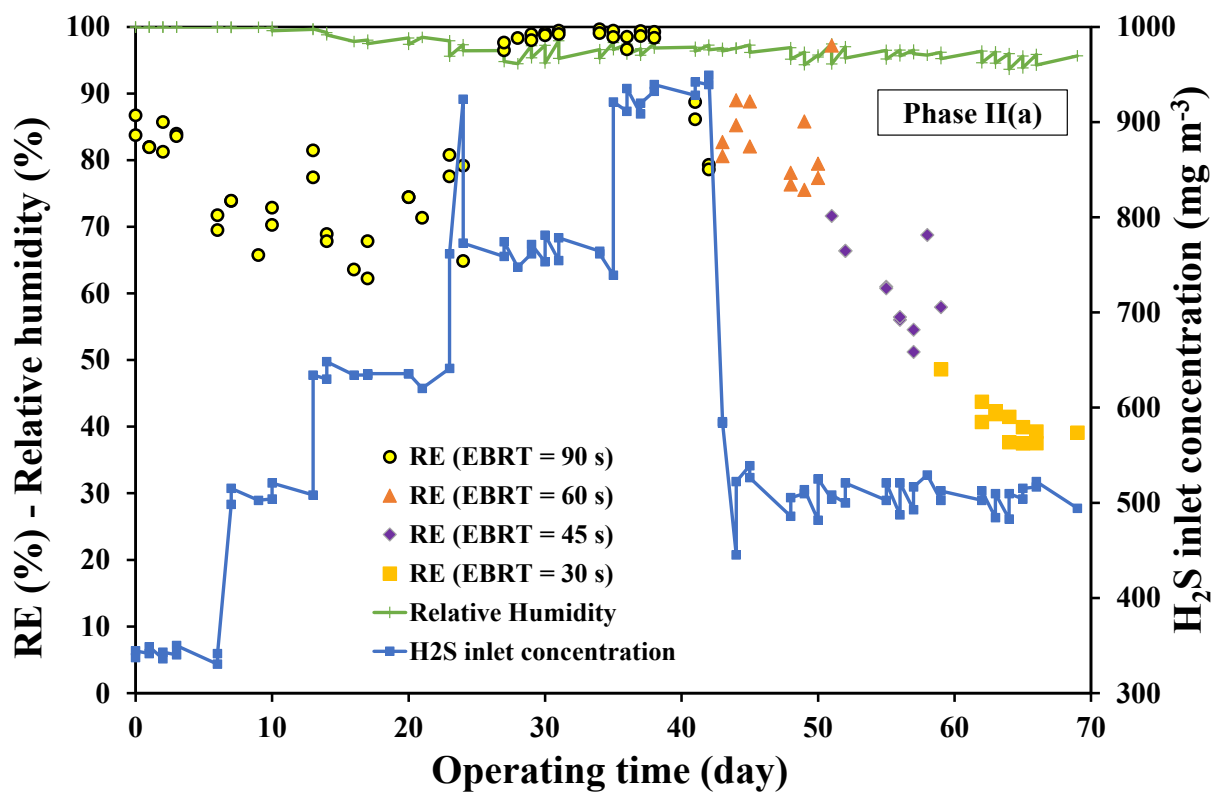
169 *Figure 3 Influence of the relative humidity of the gas on H<sub>2</sub>S removal efficiency. EBRT = 90 s.*  
 170 *H<sub>2</sub>S inlet concentration = 320 mg m<sup>-3</sup>. (A): RE vs relative humidity of the gas (RH). (B): RE*  
 171 *vs moisture level of the material calculated from the empirical relationship proposed by [14]:*  
 172 *moisture level = (0.1602 RH - 0.1355)/(1 - 0.9770 RH) for RH ranged between 85 and 100%*

173

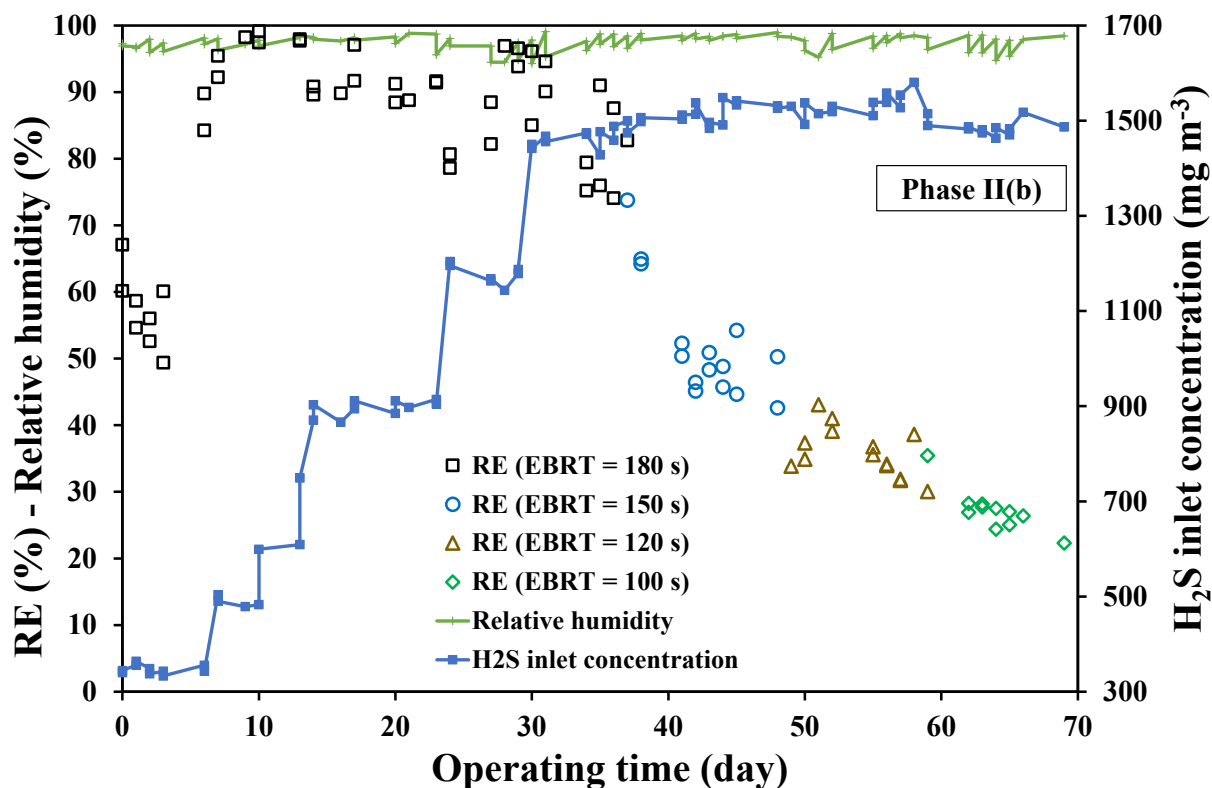
### 174 **3.2 Influence of EBRT and H<sub>2</sub>S inlet concentration**

175 The influence of both EBRT and H<sub>2</sub>S concentration on RE, recorded for the two identical filters,  
 176 is given in Figure 4. For a high EBRT (90 s and 180 s for phases II(a) and II(b), respectively),  
 177 high RE values can be achieved for the overall range of H<sub>2</sub>S concentrations applied. RE close  
 178 to 100% were even recorded for H<sub>2</sub>S concentration of 910 mg m<sup>-3</sup> at EBRT 90 s (phase II(a)).  
 179 These very high results were obtained during a period of high outdoor temperatures (near to 27  
 180 °C), well above the values measured in the experiments (from 17 to 20 °C). The temperature of  
 181 the filtering material must have a large influence on RE. This finding is supported by Wells and  
 182 Melchers [14] who studied the influence of the temperature of the sewers atmosphere on the  
 183 corrosion of concrete pipes due to H<sub>2</sub>S presence, as indicated above. The contact time between  
 184 the gas and the material has to be sufficiently long to allow the reaction to take place. In fact,  
 185 for a given H<sub>2</sub>S concentration to be treated (500 and 1500 mg m<sup>-3</sup> for phases II(a) and II(b),  
 186 respectively), the decrease of EBRT led to a decrease of performances. Since wet conditions  
 187 are required for high efficiency, the mass transfer of H<sub>2</sub>S between the gas phase and the liquid  
 188 phase surrounding the material is likely to be limited, especially for a short EBRT. This issue  
 189 is similar to the one reported in the current literature dedicated to H<sub>2</sub>S removal by biofiltration  
 190 [15], since H<sub>2</sub>S has to be dissolved in the aqueous phase to be degraded by microorganisms  
 191 suspended in the liquid or attached at the surface of the material. To consider the influence of  
 192 both EBRT and H<sub>2</sub>S concentration, all results are gathered in Figure 5 introducing the  
 193 elimination capacity of filters according to the loading rate to be treated (Table 1). As observed,  
 194 RE ranged from 70 to 100% for LR (loading rate) up to 30 g m<sup>-3</sup> h<sup>-1</sup>. This finding is in the same

195 order of magnitude that those reported in the recent literature dedicated to the anoxic removal  
196 of H<sub>2</sub>S using conventional biotrickling filters, as reviewed by Valdebenito-Rolak et al. [16].  
197 Compared to anoxic biotrickling filtration, the main advantage to proceed a simple H<sub>2</sub>S  
198 filtration using CC lies in the fact that micro-organisms and chemical addition of nitrate as  
199 electron acceptor are not needed. This technology should be surely more robust, economical  
200 and therefore more reliable over the long term than conventional biofiltration at equal  
201 performance. In the future, H<sub>2</sub>S removal using CC could probably be an interesting alternative  
202 to biofiltration depending on the industrial application. Note that the present results obtained  
203 for a mimic biogas are better than results found for air treatment since a maximum elimination  
204 capacity of 7.8 g m<sup>-3</sup> h<sup>-1</sup> was calculated by Lebrun et al. [12], without controlling the humidity  
205 of the material. For LR higher than 30 g m<sup>-3</sup> h<sup>-1</sup>, a decay of performance is observed (Figure 5).  
206 Experimental results are significantly scattered, probably partly due to temperature variations  
207 mentioned above. As a first approach, the decrease in EC (elimination capacity) for high LR  
208 (Figure 5) seems similar to an inhibition curve sometimes observed in H<sub>2</sub>S biofiltration  
209 although the filter bed was not inoculated. Filters can be naturally colonized by  
210 microorganisms, but the role of this potential biomass on H<sub>2</sub>S removal should be limited.  
211 Moreover, all attempts to model the experimental results from procedures developed for  
212 biofilter analysis failed [17,18]. Another modelling approach mainly based on both parameters  
213 H<sub>2</sub>S concentration and EBRT must be favored.



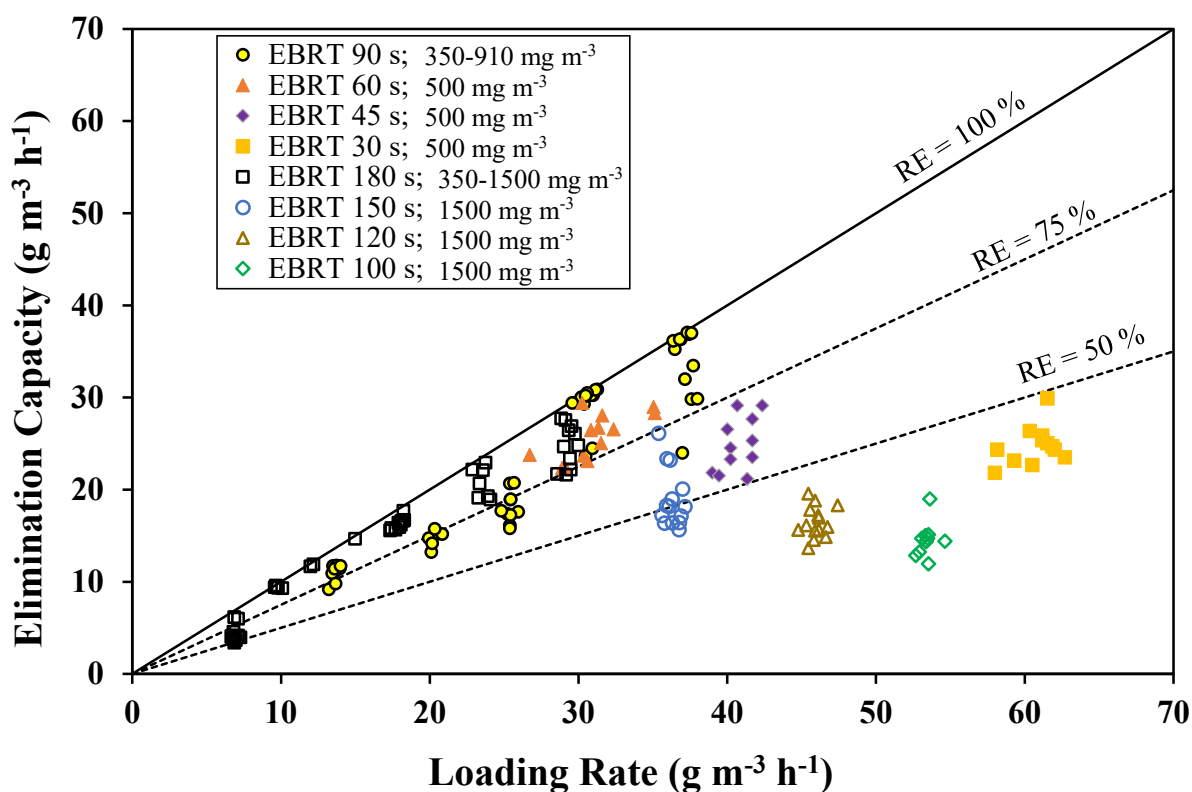
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215

216 *Figure 4 Influence of EBRT and H<sub>2</sub>S concentration on RE (Phase II(a) and II(b) correspond to*  
 217 *results recorded using Filter (a) and Filter (b), respectively)*

218



219

220 *Figure 5 EC vs LR for Phases II(a) and II(b) (II(a) closed symbols; II(b) open symbols)*

221

### 222 3.3 Modeling removal efficiency

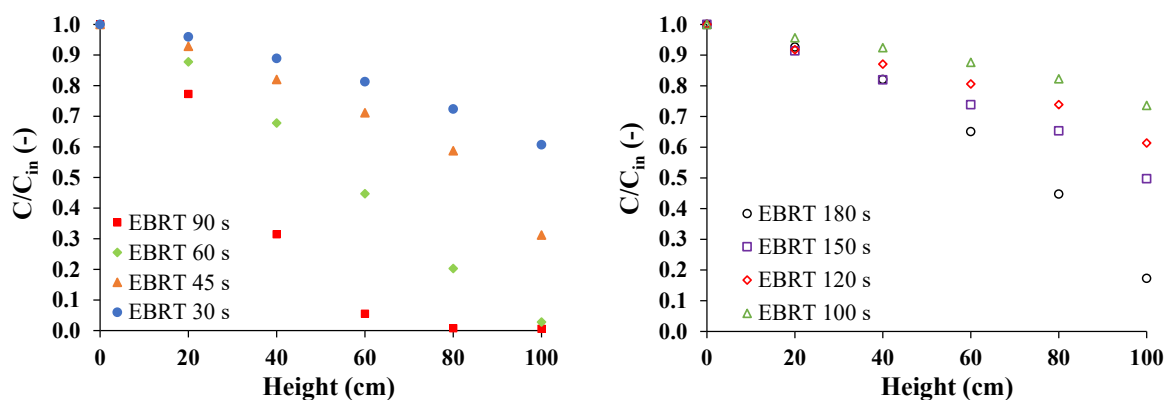
223 H<sub>2</sub>S concentration profiles along the filters length (H) are given in Figure 6 for two inlet  
 224 concentrations (500 and 1500 mg m<sup>-3</sup>, respectively) and for different EBRT. Analysis of these  
 225 profiles can give information about the kinetic regime operating in filter. The decrease in the  
 226 H<sub>2</sub>S concentration along the height of the filter can be well fitted using quadratic forms, but  
 227 functions are concave down (the second derivatives of these functions being negative) whereas  
 228 the contrary is usually observed (concave up). All results are gathered in Figure 7 as RE = (1-  
 229 C/C<sub>in</sub>) versus (H/U). H/U is the time taken for the gas to reach the height H in the filter. For  
 230 each experiment, H/U is then ranged between 0 at the bottom of the filter (H = 0 m) and EBRT  
 231 at the top (H = 1 m). For both ranges of H<sub>2</sub>S concentration applied, RE clearly depends on the  
 232 time spent by the gas in the filter. For instance, for a H<sub>2</sub>S concentration of 500 mg m<sup>-3</sup>, RE is  
 233 around 40% when the time spent is 30 s, 70% when the time spent is 45 s, and close to 100%  
 234 when the time spent is 60 s. A similar finding is achieved for a H<sub>2</sub>S concentration of 1500 mg  
 235 m<sup>-3</sup>. Whatever EBRT is applied to a given inlet concentration, all experiment results are well  
 236 fitted using a logistic function (RE=1/[1 + α exp(-β (H/U) C<sub>in</sub><sup>δ</sup>)])) involving three constants α,

237  $\beta, \delta$  determined using Microsoft Excel<sup>®</sup> solver (the equation of the parity plot between modeled  
 238 RE and experimental RE values is  $y = 0.98 x$ ;  $R^2 = 0.98$ ; not shown):

$$\text{RE} = \frac{1}{1 + 26.94 \exp\left(-321.60 \left(\frac{H}{U}\right) C_{\text{in}}^{-1.3}\right)} \quad \text{Equation 2}$$

239 From a physical point of view, the logistic function (sigmoid curve) is a suitable equation for  
 240 the modelling of experimental data. Indeed, for a residence time of the biogas in the filter ( $H/U$ )  
 241  $\rightarrow \infty$ , RE logically tends to the asymptote 100% (Figure 7). Moreover, the parameter  $\beta C_{\text{in}}^\delta$ ,  
 242 which affects the steepness of the curve, shows that the curve approaches the asymptote  $\text{RE} =$   
 243 1 more rapidly while the inlet  $\text{H}_2\text{S}$  concentration is low. The parameter  $\alpha$  corresponds to the  
 244 number of times the value  $\text{RE}_{(t=0)}$  must be multiplied to reach  $\text{RE} = 1$ . As observed in Figure 7,  
 245 modeled logistic curves begin at  $\text{RE}_{(t=0)} = 0.036$  (3.6%) whereas RE should be zero. More  
 246 sophisticated logistic equations can be found to begin at  $\text{RE}_{(t=0)}$  closed to 0, however, the curves  
 247 are not affected for  $\text{RE} > 0.036$  and the simple form reported in Figure 7 is preferable. This  
 248 equation will be useful to design industrial and applied filters.

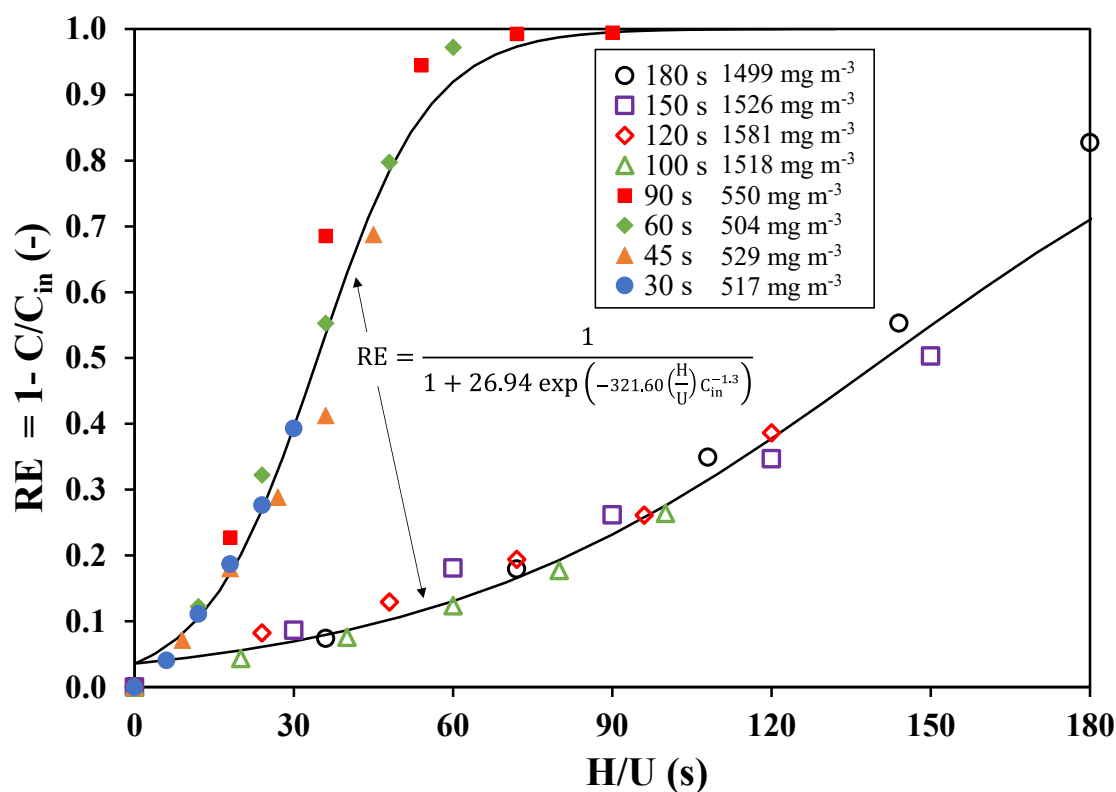
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251 *Figure 6  $\text{H}_2\text{S}$  concentration profiles according to the length of filters (Left:  $C_{\text{in}} \approx 500 \text{ mg m}^{-3}$ :  
 252 phase II(a); Right:  $C_{\text{in}} \approx 1500 \text{ mg m}^{-3}$ : phase II(b))*

253



254

255 *Figure 7 RE profiles according to the gas residence time in filters (calculated from data*  
 256 *reported in Figure 6). Symbols: experimental points; solid curves: logistic equation.*

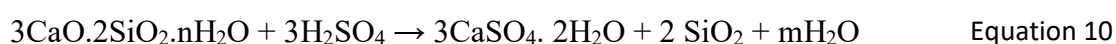
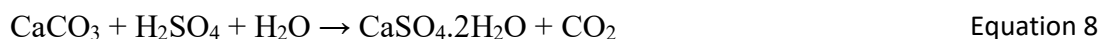
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### 258 **3.4 Possible reactions between H<sub>2</sub>S and CC**

259 The X-ray diffraction (XRD) analysis performed on samples of the raw material at the  
 260 beginning of experiment and on the used material at the end of experiment is shown in Figure  
 261 8. Comparison between spectrograms shows the loss and formation of various components. The  
 262 raw material was mainly characterized by the presence of tobermorite, chemically represented  
 263 by CaO.SiO<sub>2</sub>.nH<sub>2</sub>O, and calcite (i.e. calcium carbonate CaCO<sub>3</sub>). At the end of experiment, both  
 264 mineral components disappeared while new components appeared, mainly elemental sulfur S<sup>0</sup>  
 265 (under the octasulfur form S<sub>8</sub>), bassanite (CaSO<sub>4</sub>.0.5H<sub>2</sub>O) and anhydrite (CaSO<sub>4</sub>), which  
 266 certainly come from gypsum dehydration (CaSO<sub>4</sub>.2H<sub>2</sub>O) during the drying of samples for  
 267 analysis. The disappearance of calcium oxides (CaO.SiO<sub>2</sub>.nH<sub>2</sub>O and CaCO<sub>3</sub>) and the  
 268 appearance of calcium sulfate (CaSO<sub>4</sub>.nH<sub>2</sub>O) are consistent with the degradation mechanisms  
 269 of concrete sewer pipelines due to H<sub>2</sub>S formation in wastewater infrastructure systems [19].  
 270 The formation of calcium sulfate (gypsum) is due to chemical reactions between the main  
 271 calcium components of CC with H<sub>2</sub>S and sulfuric acid H<sub>2</sub>SO<sub>4</sub> (Figure 9). Possible reactions with  
 272 H<sub>2</sub>S are:



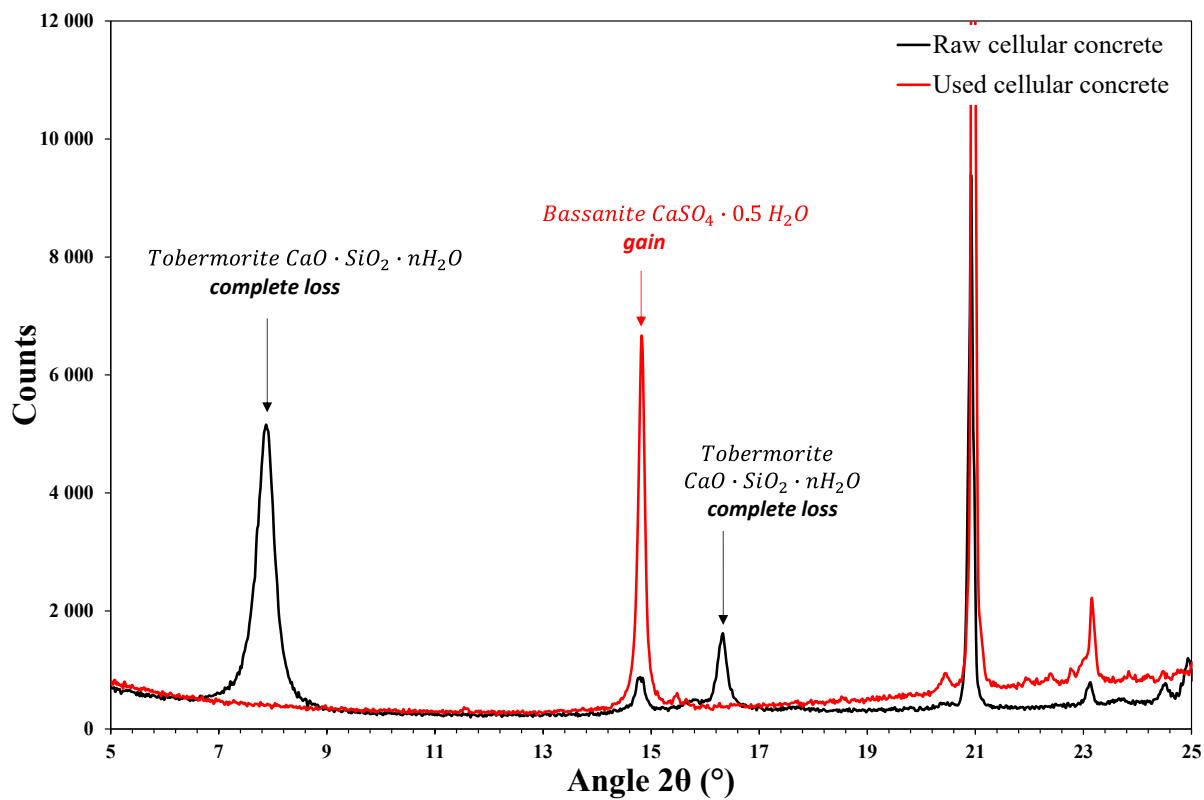
273 Possible reactions with  $\text{H}_2\text{SO}_4$  are:



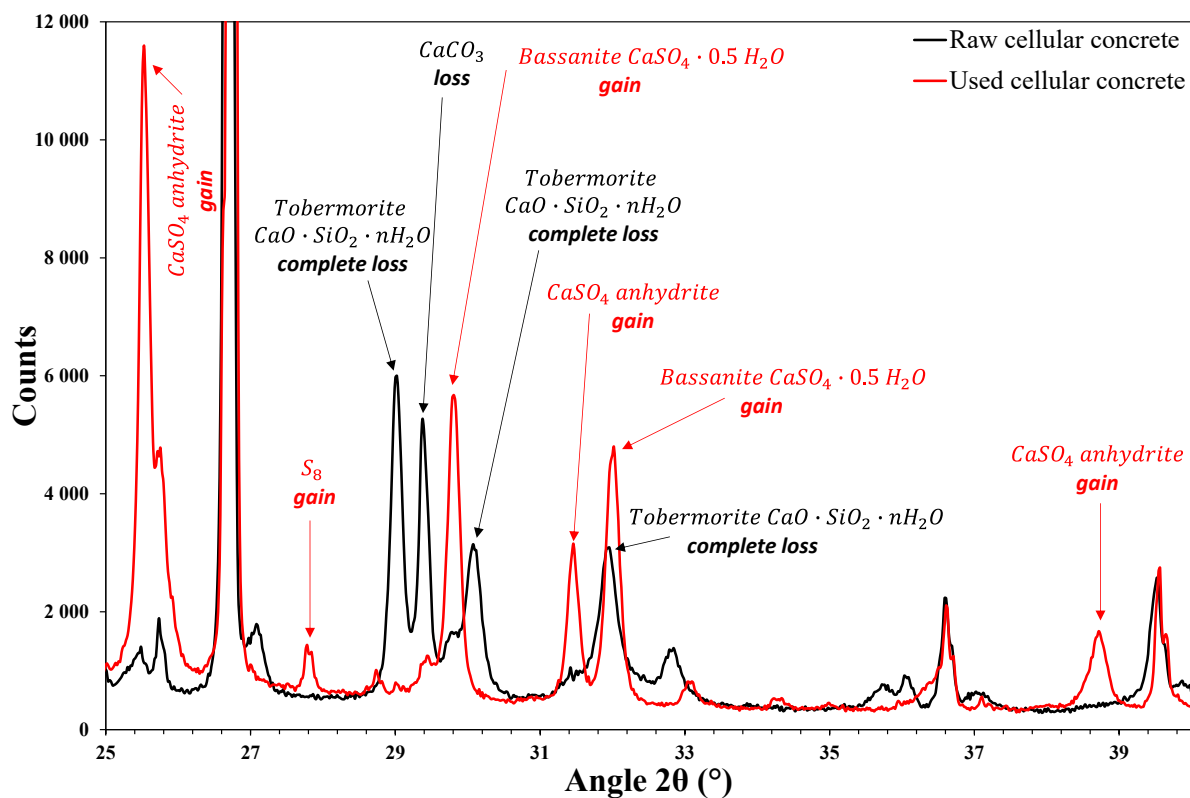
274 The presence of sulfuric acid was confirmed by the decrease in pH (from 8.0 to 1.7) of the  
275 liquid contained in the water tank and used to humidify the filters. The formation of  $\text{H}_2\text{SO}_4$  can  
276 be due to reaction between the sulfur trioxide ( $\text{SO}_3$ ) contained in the CC and water, and to the  
277 direct oxidation of  $\text{H}_2\text{S}$  with oxygen contained in the mimic biogas (up to 0.8%). According to  
278 the following stoichiometric equations, the first product of the  $\text{H}_2\text{S}$  oxidation is  $\text{S}^0$ , which can  
279 be converted into  $\text{SO}_4^{2-}$  in the case of oxygen excess amounts:



280 Sulfuric acid can also be formed biologically by sulfur oxidizing bacteria (SOB), but an attempt  
281 to extract DNA from the surface of a few pieces of CC taken from the filter at the end of the  
282 experiment did not reveal the presence of bacteria.



283

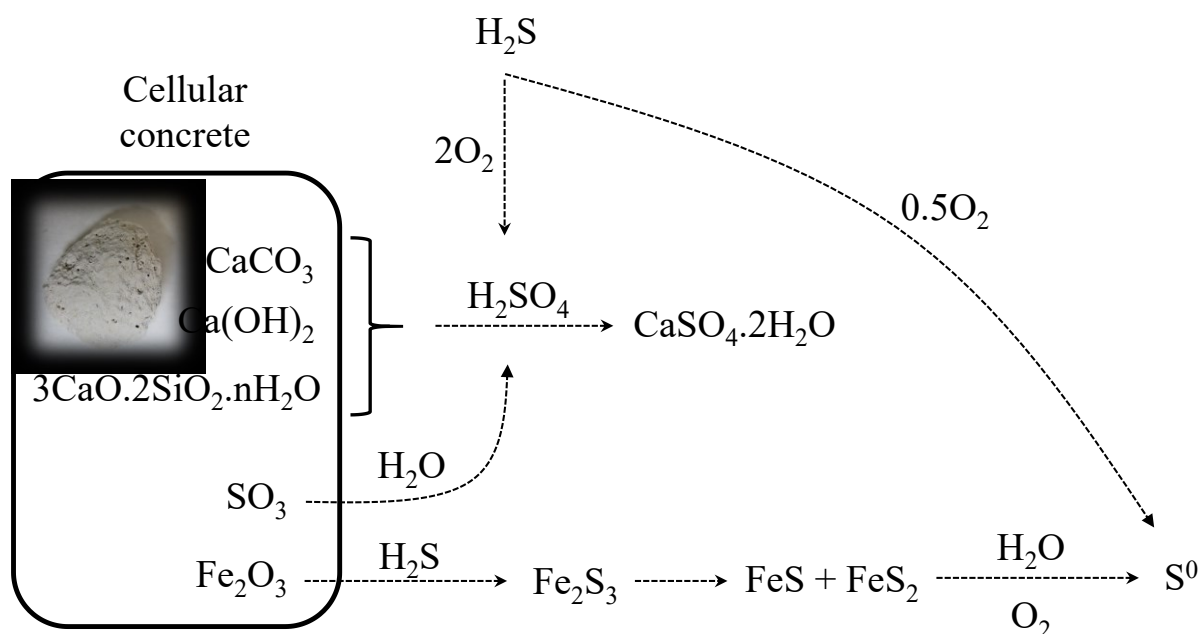


284

285 Figure 8 X-ray diffractograms of samples of cellular concrete. Black line: raw material  
 286 (beginning of the experiment); red line: used material (end of the experiment)

287





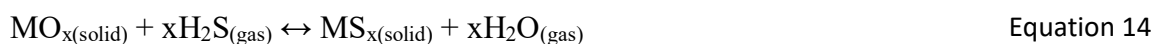
288

289 *Figure 9 Main reactions between the cellular concrete components and H<sub>2</sub>S in wet conditions*

290

291 The accumulation of sulfur on CC is clearly evidenced by the increase of the sulfur proportion  
 292 in the material measured using the energy dispersive X-ray Fluorescence spectrometer (XRF).  
 293 As Figure 10 shows, the sulfur percentage in the material extracted from Filter (a) increased  
 294 from 15.4% at day 43 (beginning of continuous measurement records) to 36.2% at day 215,  
 295 corresponding to an increase of 20.8% in weight. The transformation of the material can also  
 296 be seen in Figure 1 and Figure 11. The yellow deposit on the surface of the material (Figure 1)  
 297 clearly reveals the presence of elemental sulfur S<sup>0</sup> whereas the origin of the black color at the  
 298 core has to be identified. This black color is maybe due to the reaction between H<sub>2</sub>S and Fe<sub>2</sub>O<sub>3</sub>  
 299 leading to the formation of iron sulfide (FeS) as will be discussed later. Note that the use of  
 300 “iron sponges” based on chemical reactions occurring between Fe<sub>2</sub>O<sub>3</sub> and H<sub>2</sub>S is an old  
 301 technology to remove this component from natural and coal gases. Nonetheless, as observed in  
 302 Figure 1 and Figure 11, the black color develops from the core to the surface which is  
 303 counterintuitive since a diffusion-reaction front progresses from the surface to the core. This  
 304 preliminary observation should be investigated in the future. The reactions between the CC  
 305 components and the gas also modified the mechanical structure of the material, which became  
 306 easily friable. As the pieces of material extracted from the filter were highly moist, a simple  
 307 finger pressure was sufficient to crush them and highlight the dark coloration inside, as  
 308 observed in Figure 11. Such an operation is not possible using raw cellular concrete waste.

309 In addition to the reaction of H<sub>2</sub>S with O<sub>2</sub>, the production of elemental sulfur can also be due to  
310 the presence of oxides into the material itself (Figure 9). The chemistry of H<sub>2</sub>S removal at  
311 ambient temperature by oxides such as ZnO, CuO, and Fe<sub>2</sub>O<sub>3</sub> was recently reviewed by  
312 Watanabe [9]. According to the composition of cellular concrete waste, Al<sub>2</sub>O<sub>3</sub> and Fe<sub>2</sub>O<sub>3</sub> are  
313 the main oxides of metals. Copper, chromium, strontium and zirconium were also detected  
314 (Figure 10). Watanabe [9] reported that H<sub>2</sub>S reacts with metal oxides to form metal sulfides as:



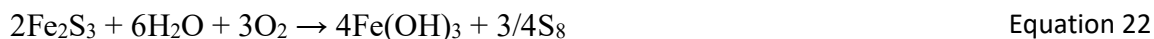
315 This reaction consists of the mechanism described by the following reactions:



316 Where MO<sub>x</sub> is the form of the initial metal oxide and MS<sub>x</sub> is the produced metal sulfide after  
317 the reaction. Reactivity potential theoretically follows the order Fe<sub>2</sub>O<sub>3</sub> > CuO > ZnO [9]. Since  
318 the proportion of ferric oxide Fe<sub>2</sub>O<sub>3</sub> in CC is 1.3% in weight, its presence probably led to the  
319 following reactions with H<sub>2</sub>S, as reported by [20].



320 Subsequently, FeS and Fe<sub>2</sub>S<sub>3</sub> can react with oxygen leading to the regeneration of ferric oxide:

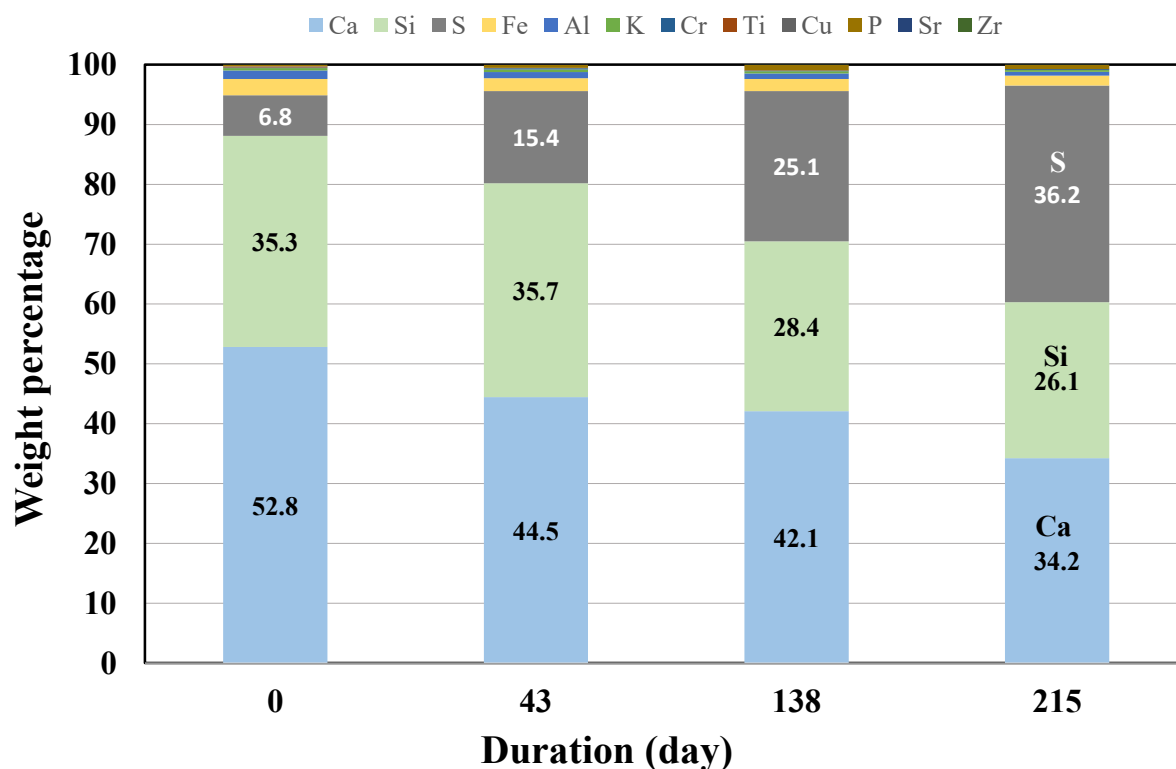


321 The overall stoichiometry is:



322 Because disulfur S<sub>2</sub> and octasulfur S<sub>8</sub> are allotrope forms of elemental sulfur (octasulfur being  
323 its main component) the continuous regeneration of Fe<sub>2</sub>O<sub>3</sub> is probably one of the mechanisms

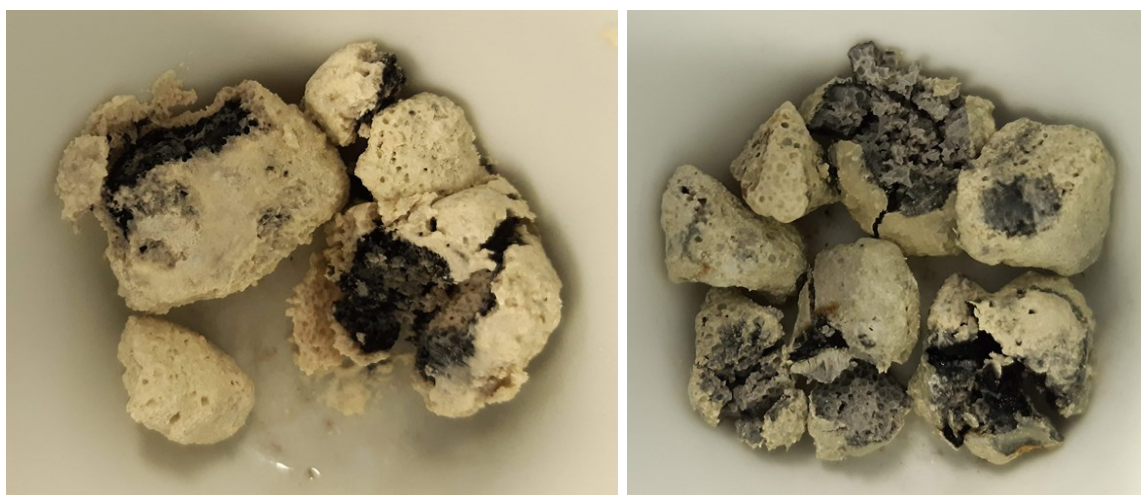
324 involved in H<sub>2</sub>S removal by cellular concrete waste (Figure 9). Although the mechanisms of the  
 325 reactions occurring between H<sub>2</sub>S and the CC components are partly revealed, the macroscopic  
 326 analysis from XRD and XRF confirmed the ability of the cellular concrete to react with H<sub>2</sub>S  
 327 and clean the gas.



328

329 *Figure 10 Modification in composition of the cellular concrete over time extracted from Filter*  
 330 *(a)*

331



332

333 *Figure 11 Cellular concrete waste extracted from Filter (a) at the end of experiment. Left: top of*  
 334 *filter. Right: bottom of filter*

335

### 336 *3.5 H<sub>2</sub>S elimination capacity*

337 The cumulative amounts of H<sub>2</sub>S removed by both filters are displayed in Figure 12 for the  
338 different experimental phases including the preliminary stage. Between day 43 and day 215  
339 (end of experiment), Filter (a) removed 422 g<sub>H<sub>2</sub>S</sub> meaning that over the period, 60% of the H<sub>2</sub>S  
340 entering the filter were removed from biogas. For Filter (b), results are lower, 262 g<sub>H<sub>2</sub>S</sub> and 45%  
341 of the H<sub>2</sub>S entering the filter. This result is due to the fact that the EBRT applied during Phase  
342 II (Table 2) were shorter than the calculated residence time needed for high RE in relation with  
343 the high H<sub>2</sub>S concentration (1500 mg m<sup>-3</sup>; Figure 7). The ability of the CC to react with H<sub>2</sub>S is  
344 by and large remarkable since 169 g of H<sub>2</sub>S were removed per kg of material for Filter (a) at  
345 the end of the experiment (105 g per kg for Filter (b); Figure 12). This result is equivalent or  
346 higher than data reported in the literature [1,9,21–24] even compared with activated carbons  
347 considered as references for H<sub>2</sub>S removal (Table 3). Galera Martínez et al. [22] reported, for  
348 example, that the capacity of fixed-bed or fluidized-bed reactors using carbon-based materials  
349 for H<sub>2</sub>S abatement is below 70 g of H<sub>2</sub>S per kg of material. These authors studied the ability of  
350 two solid wastes containing mainly calcium carbonate to remove H<sub>2</sub>S from air at low  
351 concentration (280 mg m<sup>-3</sup>) in a triphasic gas-solid-liquid reactor. They reported a removal of  
352 75 g of H<sub>2</sub>S per kg of material. The ability of CC to remove H<sub>2</sub>S in this study is more than twice  
353 as high as those reported in the literature dealing with the reuse of solid waste.

354 As a first approach, assuming that all H<sub>2</sub>S removed has reacted with the CC components to  
355 form mainly gypsum and elemental sulfur, the increase of the proportion of sulfur (%S) in the  
356 CC over time can be assessed by Equation 25 (Appendix A):

$$\% S_{(t)} = \% S_{(t=0)} + 100 \frac{EC}{\rho_{\text{filter}}} \frac{M_S}{M_{\text{H}_2\text{S}}} t \quad \text{Equation 25}$$

357 With EC the elimination capacity (expressed in kg m<sup>-3</sup> h<sup>-1</sup>),  $\rho_{\text{filter}}$  the CC bed density (kg m<sup>-3</sup>),  
358  $M_S$  and  $M_{\text{H}_2\text{S}}$ , the sulfur and hydrogen sulfide molar mass (kg mol<sup>-1</sup>), and  $t$  the operating time  
359 (h). From data reported in Figure 12, because the average EC value calculated for Filter (a) over  
360 the period day 43 to day 215 was 10.4 g m<sup>-3</sup> h<sup>-1</sup>, the increase in sulfur percentage calculated  
361 from Equation 25 should be around 15.9% (with  $\rho_{\text{filter}}$  calculated from the mass of cellular  
362 concrete introduced in the filter bed at the beginning of the experiment). The increase in sulfur  
363 proportion determined from XRF measurement (Figure 10) is 20.8% during the same period,

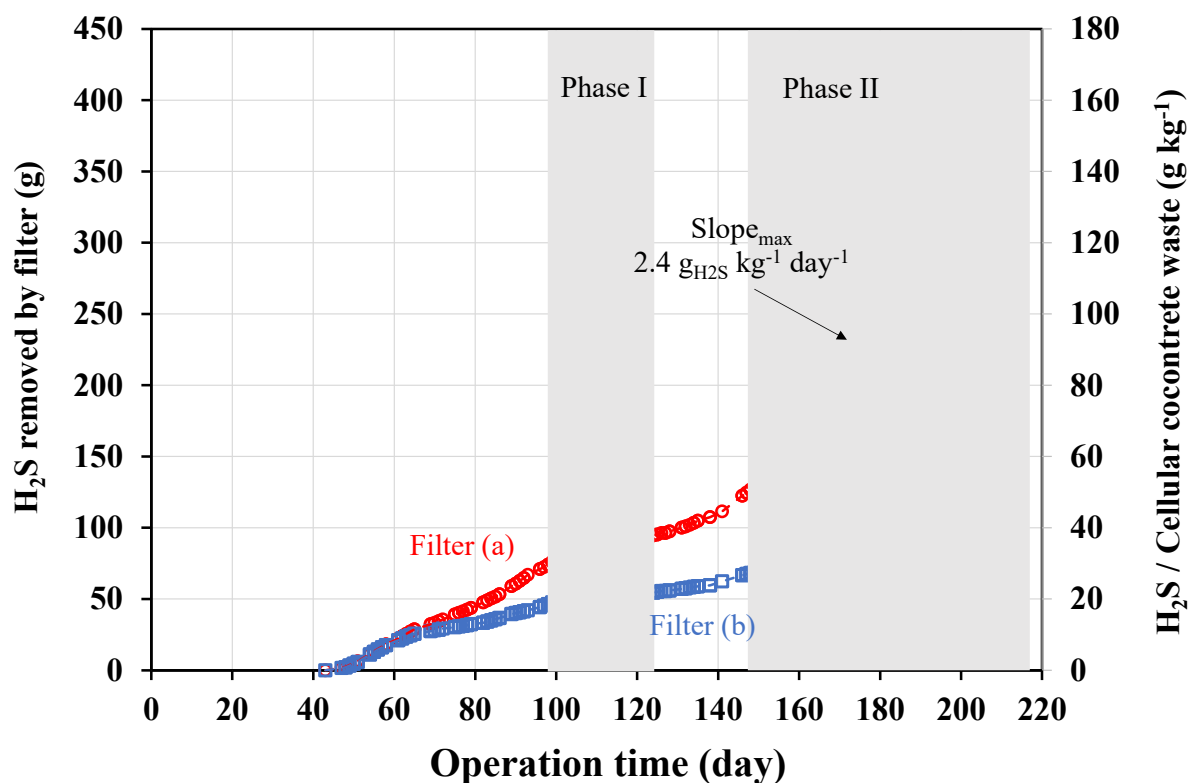
364 i.e. slightly higher than value determined from Equation 25. Assuming that the pieces of material  
365 extracted from the filter (top and bottom) for subsequent analysis are representative of all pieces  
366 of material filling the filter, the difference between the two results is probably due to the  
367 variation of the  $\rho_{\text{filter}}$  over time. Because the progressive accumulation of sulfur in the material  
368 led indubitably to a modification in its mass and its structure, the density of the filter bed varied  
369 over time. Although Equation 25 gives only an order of magnitude of the increase in the sulfur  
370 content in the material over time, the formula does give a valuable indication of the ability of  
371 CC to remove H<sub>2</sub>S. At day 215, the sulfur and the calcium contents in Filter (a) were in the  
372 same proportion (Figure 10), the cellular concrete was possibly close to its end-of-life.

373

374 *Table 3 Summary of main adsorbents used for H<sub>2</sub>S removal (25 °C and atmospheric pressure;*  
375 *adapted from [1])*

<b>Adsorbents for H<sub>2</sub>S removal</b>	<b>H<sub>2</sub>S elimination capacity (g kg<sup>-1</sup>)</b>
Iron oxide-based (Regenerative, commercial)	12.7 to 200
Activated carbons (non-Regenerative, commercial)	1.17 to 211.3
Functionalized adsorbents (non-Regenerative, lab-based)	1.7 to 496.6
Zeolites (non-Regenerative, commercial)	0.5 to 53
Metal organic frameworks (non-Regenerative, lab-based)	13.6 to 618

376



377

378 *Figure 12 Amounts of H<sub>2</sub>S removed by both filters over time (the operating conditions are*  
 379 *summarized in Table 2)*

380

### 381 **3.6 Lifetime assessment of cellular concrete**

382 On the basis that reactions between H<sub>2</sub>S and calcium oxides lead to gypsum formation  
 383 (Equation 3 to Equation 10), that fundamentally modifies the structure of the material, the  
 384 lifetime of the cellular concrete can be assessed by considering the percentage of CaO in the  
 385 raw material. This lifetime (t in h) can then be determined as a first approach using the Equation  
 386 26 (Appendix B):

$$t = \rho_{\text{filter}} \frac{1}{\text{EC}} \% \text{CaO} \frac{M_{\text{H}_2\text{S}}}{M_{\text{CaO}}} \quad \text{Equation 26}$$

387 As the average EC value calculated over the period (day 43 to day 215) is 10.4 g m<sup>-3</sup> h<sup>-1</sup>, the  
 388 operating time of a cellular concrete having 24.9% of CaO, is around 4650 h, i.e. 194 days. This  
 389 time is by and large consistent with the time life of the material observed during the experiment  
 390 since the measured performance declined progressively after the end of experiment. Given that  
 391 t is inversely proportional to EC, the operating time will be shorter as EC increases. According  
 392 to Figure 12, the maximum amount of H<sub>2</sub>S removed was 2.4 g<sub>H<sub>2</sub>S</sub> kg<sub>CC</sub><sup>-1</sup> day<sup>-1</sup> corresponding to

393  $EC = 32 \text{ g m}^{-3} \text{ h}^{-1}$ . If these conditions could be continuously maintained, the operating time will  
394 be 3 times shorter. Once transformed into gypsum CC waste becomes a new waste, its future  
395 must be considered. This new waste could eventually be used as a soil amendment, but it will  
396 have to be characterized in terms of toxicity before use. To conclude, as the lifetime of the  
397 cellular concrete was calculated for 24.9% of CaO, the result could be improved for higher  
398 contents of calcium oxide. Recent work highlighted that synthetic tobermorite ( $\text{CaO Al}_2\text{O}_3 \text{ SiO}_2$   
399  $\text{H}_2\text{O}$ ) can be prepared using bio-waste (snail shell) and domestic waste such as soda-lime-silica  
400 glass [25]. The synthesis of a functional material especially dedicated to  $\text{H}_2\text{S}$  removal, and made  
401 from various waste, would be attractive to imagine. However, the results of the present study  
402 have shown that various components of the cellular concrete react with  $\text{H}_2\text{S}$ , and synergetic  
403 effects probably occurred. The porosity of the material is also a significant parameter that have  
404 to be considered [12].

405 The simple  $\text{H}_2\text{S}$  removal technology based on filtration using cellular concrete waste could be  
406 applied at different scales. For large industrial applications, long horizontal filters filled with  
407 different crates of cellular concrete would be more appropriate than a very high column. For  
408 small decentralized domestic biogas digesters, the size of filters would be small (some liters)  
409 and consequently a simple system could be use. Whatever the scale of the application,  
410 maintaining the moisture content of the material and changing the material at the end of its life  
411 are the two parameters that will dictate the technology and the design of the filters. Concerning  
412 the future of the cellular concrete waste having reacted with  $\text{H}_2\text{S}$ , the new waste material  
413 obtained could be possibly recyclable as a soil amendment as it is mainly composed of gypsum.  
414 Nonetheless, it will have to be characterized in terms of toxicity before use. As the findings  
415 reported in this study were based on a mimic biogas, future experiments should be performed  
416 in real conditions by using a real biogas. The presence of others gases in the biogas, mainly  
417 carbon dioxide but also oxygen, could have a significant influence on the empirical relationship  
418 predicting the removal efficiency (Equation 2).

419

#### 420 **4 Conclusions**

421 The ability of cellular concrete waste to satisfactorily remove  $\text{H}_2\text{S}$  from a biogas mimic  
422 composed of  $\text{N}_2\text{-O}_2\text{-H}_2\text{S}$  was evidenced. Many complex physico-chemical mechanisms must  
423 occur simultaneously between  $\text{H}_2\text{S}$  and the components of cellular concrete waste (mainly  
424 calcium oxides and ferric oxide), leading to a modification of the composition of the material.

425 During the study, 169 g of H<sub>2</sub>S were removed per kg of material at the end of the experiment,  
 426 it is significantly higher than data reported in the literature. This result is probably lower than  
 427 the maximum capacity of the CC since the experiment was ended before the end-of-life of the  
 428 material. The H<sub>2</sub>S elimination capacity of the CC was also measured to be up to 32 g m<sup>-3</sup> h<sup>-1</sup>,  
 429 which is equal to or better than the performance of biofiltration systems. This simple H<sub>2</sub>S  
 430 removal technology, certainly more robust and economical than conventional biofiltration with  
 431 the same performance, could take its place in numerous industrial cases. A first empirical  
 432 relationship, depending of both the H<sub>2</sub>S concentration of the biogas and the EBRT applied, was  
 433 proposed to predict the removal efficiency of a filtration system (logistic equation, Equation 2).  
 434 An equation was also proposed to predict the lifetime of the material. These findings suggest  
 435 that H<sub>2</sub>S removal using cellular concrete waste could be suitable to treat small gas flowrates  
 436 generated by decentralized domestic biogas digesters. This pre-experiment based on a mimic  
 437 biogas must be, in the future, validated in real conditions by using a real biogas.

438

## 439 **Appendix A**

440 Assuming that H<sub>2</sub>S reacts with components of CC to mainly form gypsum and elemental sulfur,  
 441 the increase in sulfur proportion of CC over time can be assessed according to the following  
 442 way (S = sulfur):

$$\text{Mass } S_{(t)} = \text{Mass } S_{(t=0)} + \text{Mass } S_{(\text{transferred from gas to cellular concrete})} \quad \text{Eq. (A.1)}$$

$$\text{Mass } S_{(t)} = \text{Mass } S_{(t=0)} + Q (C_{\text{in}} - C_{\text{out}}) \frac{M_S}{M_{\text{H}_2\text{S}}} t \quad \text{Eq. (A.2)}$$

443 With Q the gas flowrate (m<sup>3</sup> h<sup>-1</sup>), C<sub>in</sub> and C<sub>out</sub> (kg m<sup>-3</sup>) the gas H<sub>2</sub>S concentrations at the inlet  
 444 and outlet of the filter, respectively, t (h) the operating time, M<sub>S</sub> and M<sub>H<sub>2</sub>S</sub>, the of sulfur and  
 445 hydrogen sulfide molar mass (kg mol<sup>-1</sup>). Dividing by the mass of CC used (Mass<sub>CC</sub> in kg), the  
 446 following equation is obtained:

$$\frac{\text{Mass } S_{(t)}}{\text{Mass}_{\text{CC}}} = \frac{\text{Mass } S_{(t=0)}}{\text{Mass}_{\text{CC}}} + \frac{Q (C_{\text{in}} - C_{\text{out}})}{\text{Mass}_{\text{CC}}} \frac{M_S}{M_{\text{H}_2\text{S}}} t \quad \text{Eq. (A.3)}$$

447 As the mass of cellular concrete in the filter is (Mass<sub>CC</sub> = V ρ<sub>filter</sub>) with V (m<sup>3</sup>) the volume of  
 448 the filter bed and ρ<sub>filter</sub> (kg m<sup>-3</sup>) the CC bed density, Eq. (A.3) gives:



$$\text{Fraction } S_{(t)} = \text{Fraction } S_{(t=0)} + \frac{Q (C_{in} - C_{out})}{V \rho_{\text{filter}}} \frac{M_S}{M_{H_2S}} t \quad \text{Eq. (A.4)}$$

449 According to the definition of the elimination capacity (EC) given in Table 1, and multiplying  
 450 each term by 100, the increase in sulfur proportion over time is given according to the following  
 451 relation:

$$\% S_{(t)} = \% S_{(t=0)} + 100 \frac{EC}{\rho_{\text{filter}}} \frac{M_S}{M_{H_2S}} t \quad \text{Eq. (A.5)}$$

452

## 453 **Appendix B**

454 The lifetime (t in h) of the cellular concrete can be assessed by considering the percentage of  
 455 CaO in the raw material:

$$t = \frac{\text{Mass of } H_2S \text{ reacting with CaO}}{\text{Mass flow of } H_2S \text{ removed from the gas}} \quad \text{Eq. (B.1)}$$

$$t = \frac{\%CaO \text{ Mass}_{CC} \frac{M_{H_2S}}{M_{CaO}}}{Q (C_{in} - C_{out})} \quad \text{Eq. (B.2)}$$

456 With parameters given in Appendix A and  $M_{CaO}$ , the molar mass ( $\text{kg mol}^{-1}$ ) of calcium oxide.  
 457 As previously detailed, the mass of cellular concrete  $\text{Mass}_{CC} = (V \rho_{\text{filter}})$  and:

$$t = \rho_{\text{filter}} \frac{V}{Q (C_{in} - C_{out})} \%CaO \frac{M_{H_2S}}{M_{CaO}} \quad \text{Eq. (B.3)}$$

458 This equation can be rewritten under the following forms, either by introducing the Elimination  
 459 Capacity or the Removal Efficiency and the Empty Bed Residence Time given in Table 1 using  
 460 the appropriate units (EC in  $\text{kg m}^{-3} \text{ h}^{-1}$  and EBRT in h):

$$t = \rho_{\text{filter}} \frac{1}{EC} \%CaO \frac{M_{H_2S}}{M_{CaO}} \quad \text{Eq. (B.4)}$$

$$t = \rho_{\text{filter}} \frac{V}{Q C_{in} RE} \%CaO \frac{M_{H_2S}}{M_{CaO}} \quad \text{Eq. (B.5)}$$

$$t = \rho_{\text{filter}} \frac{EBRT}{C_{in} RE} \%CaO \frac{M_{H_2S}}{M_{CaO}} \quad \text{Eq. (B.6)}$$

461

462

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### 467 **Conflict of interest**

468 The authors declare no conflict of interest.

469

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