

1DV bottom boundary layer modeling under combined wave and current: Turbulent separation and phase lag effects

Katell Guizien, Marjolein Dohmen-Janssen, Giovanna Vittori

▶ To cite this version:

Katell Guizien, Marjolein Dohmen-Janssen, Giovanna Vittori. 1DV bottom boundary layer modeling under combined wave and current: Turbulent separation and phase lag effects. Journal of Geophysical Research, 2003, 108 (C1), pp.3016. 10.1029/2001JC001292. hal-03883194

HAL Id: hal-03883194 https://hal.science/hal-03883194v1

Submitted on 8 Dec 2022

HAL is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers. L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.

1DV bottom boundary layer modeling under combined wave and current: turbulent separation and phase lag effects

Katell Guizien ^{1*}, Marjolein Dohmen-Janssen ² and Giovanna Vittori ³

¹ Laboratoire des Ecoulements Géophysiques et Industriels,

BP 53, 38041 Grenoble Cedex 9, France.

* now at Laboratoire d'Océanographie Biologique de Banyuls,

BP 44, 66651 Banyuls sur Mer Cedex, France.

Tél: +33 4 68 88 73 19 / Fax: +33 4 68 88 73 95 / E-mail: guizien@obs-banyuls.fr

² University of Twente, Department of Civil Engineering,

PO BOX 217, 7500 AE Enschede, The Netherlands.

³ Dipartimento di Ingegneria Ambientale Università di Genova,

Via Montallegro 1, 16145 Genova, Italy.

Abstract

On the basis of Wilcox transitional $k-\omega$ turbulence model (1992, [37]), we propose a new $k-\omega$ turbulence model for 1DV oscillating bottom boundary layer in which a separation condition under a strong adverse pressure gradient has been introduced and the diffusion and transition constants have been modified. This new turbulence model agrees better than Wilcox original one with both a direct numerical simulation (DNS) of a pure oscillatory flow over a smooth bottom in the intermittently turbulent regime and with experimental data from Jensen et al. (1989, [14]) who attained the fully turbulent regime for pure oscillatory flows. The new turbulence model is also found to agree better than the original one with experimental data of an oscillatory flow with current over a rough bottom by Dohmen-Janssen (1999, [5]). In particular, the proposed model reproduces the secondary humps in the Reynolds stresses during the decelerating part of the wave cycle and the vertical phase lagging of the Reynolds stresses, two shortcomings of all previous modeling attempts. In addition, the model

predict suspension ejection events in the decelerating part of the wave cycle when it is coupled with a sediment concentration equation. Concentration measurements in the sheet flow layer give indication that these suspension ejection events do occur in practice.

1 Introduction

In coastal zones, the suspension associated to waves and currents in the bottom boundary layer can have an impact on both human activities and ecological equilibrium. Indeed, it is well known that suspension plays a major role in sediment transport and affects human works and biological species through morphodynamical changes which may affect the stability of the former and destroy the habitats of the latter. Moreover suspension can also affect directly the life cycle of some species and hence play a role in their population dynamics. This is the case for instance for benthic invertebrates with planktonic larvae. Indeed, the larvae settlement on the bed may be limited by strong suspension events and lead to dramatic cut in the population. Studying the suspension dynamics under waves and currents is hence of great interest, especially over flat bed since this corresponds to the more severe hydrodynamical conditions.

As a conclusion of the MAST II G8-M Coastal Morphodynamics European project, some shortcomings in modeling sand transport by combined waves and currents have been identified which are reported in Davies et al. (1997, [4]). In their paper, an inter-comparison of experimental data with four research sediment transport models under combined waves and currents was presented. The four models mainly differed in the complexity of the turbulence closure schemes (from zero to two-equations) used to compute the eddy-viscosity in the bottom turbulent boundary layer. In Fredsøe's model (1984, [8]), a time-dependent eddy viscosity is derived from integration of the momentum equation over the wave boundary layer, assuming a logarithmic velocity profile (zero-equation model). Ribberink and Al Salem (1995, [22]) used a time- and height-dependent eddy viscosity by extending Prandtl's mixing length theory to an unsteady flow (zero-equation model). Li and Davies (1996, [19]) used a k-equation model with a similarity l-scaling (one-equation model) and Huynh Than et al. (1994, [12]) used a k-L model (two-equations model)

to compute a time-varying eddy viscosity. The concentration is computed from a convection-diffusion equation in which vertical sediment diffusivity is assumed to be equal to the time-dependent eddy viscosity, except in Huynh Than et al. model where turbulence damping is taken into account so that eddy viscosity and sediment diffusivity are related through some coupling terms. Despite the difference in the complexity of the turbulence closure, all the models show similar shortcomings when predictions are compared to flat bed experiments which correspond to strong wave plus current conditions ("sheet flow" regime).

All the models lead to underestimation of the phase lag between concentration and velocity in the upper part of an oscillatory boundary layer and to unreliable estimates of sediment load predictions. Recent experiments in clear water (without sediment) by Dohmen-Janssen (1999, [5]) show a relevant phase lag over depth in Reynolds stress time series thus showing that the phase lag between concentration and velocity is partially inherent to the oscillatory boundary layer dynamics and not totally due to the sediment feedback on the turbulence structure. Therefore efforts should be done to improve turbulence modeling for oscillatory boundary layers before working on flow and sediment coupling.

In particular none of the aforementioned models reproduce correctly the phase lag between Reynolds stress and velocity. This phase lag is related to the Reynolds stress vertical decay in the region far from the wall: the quicker it decays, the larger the phase lag is. In a recent paper, Sana and Tanaka (2000, [26]) present a comparison between five low-Reynolds-number $k - \epsilon$ models and the Direct Numerical Simulation (DNS) by Spalart and Baldwin (1989, [30]) for sinusoidal oscillatory flows over smooth beds. They show that the Jones and Launder's (1972, [15]) model provides better predictions of transition initiation and of the Reynolds stress vertical decay in the region far from the wall. These results suggest that the introduction of low-Reynolds-number modifications could improve the modeling of phase lag between Reynolds stress and velocity. However it should also be pointed out that Jones and Launder's model underestimates the peak value of the turbulent kinetic energy and overestimates the bottom shear stress enhancement after transition. It can be concluded that none of the low-Reynolds-number modifications proposed in these five $k - \epsilon$ models enable to predict correctly the whole dynamics of the oscillating boundary layer.

A second shortcoming of the models considered by Davies et al. (1997, [4]), concerns concentration secondary peaks which are sometimes observed near flow reversal in experimental measurements close to the bottom (Katapodi et al., 1994, [17] and Dohmen-Janssen, 1999, [5]) and are not reproduced by models. Although the very sharp concentration peaks that show in the measurements close to the bottom may be partly caused by the measuring technique, there are indications that suspension ejection events really occur before flow reversal (see section 4). These may be attributed to shear instabilities in the wave boundary layer (Foster et al., 1994, [7]).

The contribution of these secondary concentration peaks to the near-bed sediment load is limited, since they occur at time when the velocity is nearly zero. However, the huge amount of sediment picked up from the bed around flow reversal, especially for fine sand, may affect turbulence and at the same time may be redistributed along time in the upper suspension layer. Hence, these concentration secondary peaks can be of great importance with respect to total sediment load predictions. Besides, such suspension ejection events can play a crucial role in benthic life.

In 1996, Savioli and Justesen (1996, [27]) proposed a modified condition for the reference concentration that enables to reproduce secondary peaks on the concentration time series with a standard (without low-Reynolds-number effects) $k-\epsilon$ model (Rodi, 1980, [24]) in a 1DV fully rough turbulent oscillating boundary layer model, taking advantage of a narrow diffusivity peak just before flow reversal. A much smaller narrow peak, is also present near flow reversal in the eddy viscosity time series computed using a standard $k-\omega$ turbulence model (Wilcox, 1988, [36]), whereas a k-L turbulence model (Huynh Than et al., 1994, [12]) does not produce such peaks. However, although showing discrepancies on the eddy viscosity time series, the three turbulence models produce similar time series of the bottom shear stress, without any significant increase near flow reversal (Guizien et al., 2001 [9]). In fact, differences in the eddy viscosity time series are due to the closures of the models, namely to the singularity in the behavior of the eddy viscosity, that reads $\nu_T = k/\omega$ in the $k-\omega$ model and $\nu_T = 0.09k^2/\epsilon$ in the $k-\epsilon$ model. The singularity arises when k and the other value ω or ϵ approach zero, for instance when the outer flow velocity decreases to zero during a wave cycle. At that phase, the instantaneous local Reynolds number decreases rapidly and the eddy viscosity strongly increases if the fully turbulent value for the model constants is applied. In steady boundary layers, it is well known that the constants used in $k-\epsilon$ standard models should be modified using low-Reynolds-number damping function to avoid the singular behavior of the eddy viscosity near the wall when computing the viscous sub-layer. It is worth noticing that, in standard $k-\omega$ models, the viscous sub-layer can be easily included for both smooth and rough bottom (Saffman, 1970, [25]), avoiding this latter near-wall singularity. In addition, under stationary conditions with an adverse pressure gradient and for low-Reynolds-numbers, standard $k-\omega$ models perform better than standard $k - \epsilon$ models (Wilcox, 1998 [38]). This is consistent with the fact that the near-reversal eddy viscosity peak is smaller in the standard $k-\omega$ computations than in the standard $k - \epsilon$ computations and that a much smaller time step (50 times smaller, strongly depending on the velocity amplitude) is required to deal with the singularity in computations with a standard $k-\epsilon$ model compared to computations performed with a standard $k - \omega$ model. However, introducing low-Reynolds-number effect in a $k-\epsilon$ turbulence model (e.g. Chien model, 1982, [1] used by Thais et al., 1999, [31]), the peak in the eddy viscosity time series for an oscillating boundary layer vanishes (Thais, Pers. Comm.). Similarly, when using Wilcox (1992, [37]) transitional $k-\omega$ turbulence model, that includes low-Reynoldsnumber effect, the eddy viscosity time series for oscillating boundary layers do not present any peak.

Recently, clear water experiments by Dohmen-Janssen (1999, [5]) shed a new light on this question. During these experiments, stronger turbulent activity was detected in the Reynolds stress time series close to the wall in the decelerating part of the wave cycle. This turbulence enhancement occurs at phases when the concentration secondary peaks are observed for the same hydrodynamical conditions. It should be outlined that fluctuations similar to the ones measured by Dohmen-Janssen, were observed by Sleath (1987, [29]). He also measured a 180 degrees phase shift of the phase of the Reynolds stress maximum at a certain height from the bed and explained it by assuming the existence of jets of fluids associated with vortex formation over the bottom roughness. He suggested that these jets of fluid would dominate the flow close to the wall whereas turbulence would dominate far from it. This explanation clearly implies that a detailed modeling of rough oscillating

boundary layers should be three-dimensional and include a mechanism for vortex generation by bottom roughness. Even though such a sophisticated model is beyond the scope of this paper, it is clear that the strong turbulence activity which takes place during the decelerating phases of the cycle should be taken into account since it contributes to put more sediment in suspension. In this paper, starting from the Wilcox transitional $k-\omega$ model (1992, [37]), a new transitional $k-\omega$ turbulence model is proposed in order to improve the 1DV modeling of oscillating bottom boundary layers. A $k-\omega$ turbulence model is preferred to a $k-\epsilon$ one for its simplicity, its ability to include the viscous sub-layer and for its good predictions under adverse pressure gradients which occur during the decelerating phases of the wave cycle. The improvement brought to the Wilcox transitional $k-\omega$ model concerns vertical phase lagging and suspension ejection events. The damping of turbulence by the stratification is also introduced. The model is presented in section 2. The ability of the new model to predict laminar-turbulent transition is tested for a pure oscillatory flow over a smooth bottom by comparison with Direct Numerical Simulations in section 3.1 and with the experimental data from Jensen et al. (1989, [14]) in section 3.2. The model is then compared with experimental data in the rough turbulent regime for an oscillatory flow plus current in section 4.1 (Dohmen-Janssen, 1999, [5]). Finally concentration predictions corresponding to these latter hydrodynamical conditions are described in section 4.2.

2 The new $k-\omega$ model

2.1 Basic formulation

The basis of the Reynolds Averaged Navier-Stokes (R.A.N.S.) model we use to compute the turbulent bottom boundary layer under an oscillatory flow (with or without current) is the transitional $k - \omega$ model devised by Wilcox (1992, [37]) in its 1DV formulation. In addition, turbulence damping by stratification is introduced into the original Wilcox formulation through coupling terms between turbulence and the density field $\rho(z,t) = \rho_0 + C(z,t)(\rho_s - \rho_0)$ resulting from the sediment suspension (ρ_0 is the fluid density, ρ_s is the sediment density and C(z,t) is the sediment concentration per volume). The coupling terms are similar to those introduced by Lewellen

(1977, [18]) in a k-L model. The hydrodynamical model (i.e. without sediment) is easily retrieved taking $\frac{\partial \rho}{\partial z} = 0$.

The horizontal velocity u inside the boundary layer, the turbulent kinetic energy k and the energy dissipation rate ω fulfill the following set of equations (1)-(6), where U is the horizontal velocity outside the boundary layer (outer flow) and $\frac{\partial \bar{P}}{\partial x}$ is the mean pressure gradient generating the current (note that for pure oscillatory flow, $\frac{\partial \bar{P}}{\partial x} = 0$). In this 1DV formulation, we assume no x-dependence for all averaged quantities (no horizontal convective or diffusive transport) and no vertical velocity at the top of the boundary layer. These assumptions correspond strictly to the tunnel experiment conditions we will compare with in the next sections.

$$\frac{\partial u}{\partial t} = -\frac{1}{\rho_0} \frac{\partial \bar{P}}{\partial x} + \frac{\partial U}{\partial t} + \frac{\partial}{\partial z} \left((\nu + \nu_t) \frac{\partial u}{\partial z} \right) \tag{1}$$

$$\frac{\partial k}{\partial t} = \nu_t \left(\frac{\partial u}{\partial z}\right)^2 - \beta^* k \omega + \frac{\partial}{\partial z} \left[(\nu + \sigma^* \nu_t) \frac{\partial k}{\partial z} \right] + \frac{g}{\rho_0} \frac{\partial \rho}{\partial z}$$
 (2)

$$\frac{\partial \omega}{\partial t} = \underbrace{\alpha \nu_t \frac{\omega}{k} \left(\frac{\partial u}{\partial z}\right)^2}_{nreduction} - \underbrace{\frac{\partial \omega^2}{\partial z}}_{dissipation} + \underbrace{\frac{\partial}{\partial z} \left((\nu + \sigma \nu_t) \frac{\partial \omega}{\partial z}\right)}_{diffusion} + \underbrace{\frac{\omega}{2k} \frac{g}{\rho_0} \frac{\partial \rho}{\partial z}}_{nreduction}$$
(3)

$$\nu_t = \alpha^* \frac{k}{\omega} \frac{1 - C_3 \Omega}{(1 - C_1 \Omega)(1 - C_2 \Omega)} \tag{4}$$

$$\gamma_t = \nu_t \frac{1 - C_2 \Omega}{(1 - C_3 \Omega)} \tag{5}$$

$$\Omega = 2 \frac{g}{\rho_0} \frac{d\rho}{dz} \frac{4}{\omega^2} \tag{6}$$

with

$$\alpha^* = \frac{\alpha_0^* + Re_T/R_K}{1 + Re_T/R_K}; \quad \alpha = \frac{13}{25} \frac{\alpha_0 + Re_T/R_\omega}{1 + Re_T/R_\omega} (\alpha^*)^{-1}; \quad \beta^* = \frac{9}{100} \frac{4/15 + (Re_T/R_\beta)^4}{1 + (Re_T/R_\beta)^4}$$

where
$$Re_T = \frac{k}{\nu\omega}$$
, $\beta = \beta_0 = \frac{9}{125}$, $\alpha_0^* = \frac{\beta_0}{3}$, $\alpha_0 = \frac{1}{9}$, $R_\omega = 2.95$, and $c_0 = 0.8$. It

should be recalled here that, unlike most of the above coefficients, no simple argument can be found to estimate the values for σ , σ^* , R_K and R_{β} . For given values for R_K and R_{β} , there is a unique value of R_{ω} that yields the value measured by Nikuradse of the constant appearing in the law of the wall for smooth wall $C_w = 5.0$. Hence, Wilcox (1992, [37]) proposed values for $R_K=6,\,R_\beta=8$ and $\sigma=\sigma^*=0.5$ that give the best agreement both with experiments and Direct Numerical Simulations of steady boundary layers with and without adverse or favorable pressure gradient. However, he already outlined that taking a smaller value for σ^* should improve the model's prediction for boundary layers with variable pressure gradient. Hence, on the basis of a preliminary analysis of the performances of the model we suggest to use the following values for oscillatory boundary layers (oscillatory pressure gradient): $\sigma = 0.8$, $\sigma^* = 0.375$, $R_K = 20$ and $R_{\beta} = 27$. The original value for $R_{\omega}=2.95$ is kept and gives a constant for the law of the wall $C_{w}=7.6$ for a steady boundary layer in the smooth regime. These values provide better predictions than the values suggested by Wilcox when the results of the model are compared with a DNS computation of a pure oscillatory boundary layer in the smooth regime (see section 3.1). The equations (1)-(3) for u, k and ω are solved using the implicit finite control volume method of Patankar (1980, [20]) on an exponential mesh with the following boundary conditions. At the bottom, we prescribe the true value of k and u (Saffman, 1970 [25]), meanwhile the value of ω is fixed depending on whether a smooth or rough wall should be modeled (Wilcox, 1998 [38], p 177). At the top of the boundary layer we force the outer flow and the vanishing of turbulence. These conditions write:

- at z=0: u=0, k=0 and $\omega_{wall}=\frac{u_f^2}{\nu}S_R$ with $S_R=(50/k_N^+)^2$ if $k_N^+<25$ and $S_R=100/k_N^+$ if $k_N^+\geq 25$. The quantity k_N^+ is equal to k_Nu_f/ν where k_N is the Nikuradse roughness, $u_f=\sqrt{\tau/\rho_0}$ is the friction velocity and $\tau=\rho_0(\nu+\nu_T)\frac{\partial u}{\partial z}$ is the total bottom shear stress. Note that this boundary condition for a rough surface describes also a smooth surface provided k_N^+ is small enough (in practice, $k_N^+<5$).
- at $z = z_h$: $u = U = U_0 \sin(2\pi t/T) + U_c$ (where U_0 , T are the amplitude and period of the velocity oscillation and U_c the current velocity) and $\partial k/\partial z =$

 $\partial \omega/\partial z = 0$. For pure oscillatory flows $z = z_h$ is just outside the bottom boundary layer. When a steady current in a free surface flow is present, $z = z_h$ is the location of the free surface. Due to symmetries in tunnel experiments, $z = z_h$ is the location of the tunnel axis.

When sediment suspension is considered, the following sediment concentration balance is solved together with the set of equations (1)-(6):

$$\frac{\partial C}{\partial t} = \frac{\partial (w_s C)}{\partial z} + \frac{\partial}{\partial z} \left(\gamma_t \frac{\partial C}{\partial z} \right) \tag{7}$$

where w_s is the sediment settling velocity and the following boundary conditions are applied:

- at $z = 2d_{50}$ with d_{50} the median grain diameter: $C = \max(C_a, C_b)$ where C_a is a reference concentration obtained by applying instantaneously Engelund and Fredsøe's (1976, [6]) formula (quasi-steady approximation) and C_b results from particles settling from the upper layers.
- at $z = z_h$, zero flux of sediment reads $\partial C/\partial z = 0$.

In this paper, we don't discuss the value of the coupling constants C_1 , C_2 and C_3 . For the sediment suspension application at the end of the paper, we consider a simplified version of the coupled model after linearization, taking $\Omega = 0$. Explanations on the derivation of the coupling constants can be found in Lewellen (1977, [18]).

2.2 Modeling of turbulent separation under the effect of an adverse pressure gradient

We now discuss the modeling of turbulence separation near flow reversal. Starting from experimental observations in an oscillating tunnel, a stronger turbulent activity shows in Reynolds stress time series (secondary humps) when the pressure gradient is adverse and nearly maximum, i.e. at the end of the decelerating parts of the cycle in the rough fully turbulent regime. Indeed large values of Reynolds stresses are observed for positive velocity under strong positive pressure gradient and for negative velocity under strong negative pressure gradient. In tunnel experiments, the horizontal pressure gradient is equal to the opposite of the outer velocity time

derivative and it is then 90 degrees out of phase with the velocity. The phase for both the oscillatory part of the outer velocity and the corresponding oscillatory pressure gradient are defined on figure 1. Under the effect of the adverse pressure gradient at the end of the decelerating phases of the cycle, the velocity profiles become more and more S-shaped with a large inflection point and finally a back-flow hereafter called flow separation, is present close to the wall. In the laminar regime, the presence of an inflection point is associated to instability and to transition to turbulence (Foster et al., 1994, [7] and Wu, 1992, [39]). In the turbulent regime, separation under the effects of an adverse pressure gradient occurs as well, even though the turbulent boundary layer can resist to separation longer than the laminar boundary layer, at the expenses of an increased wall friction. On the basis of the experimental results, we think that this resistance to separation also depends on the wall roughness, namely a rough wall is more resistant than a smooth wall. Hence, we suggest to model this wall friction enhancement before flow separation under the effect of the adverse pressure gradient for fully developed turbulence and rough walls only, as follows.

Firstly, it is necessary to establish when the enhancement in the wall friction begins. A criterion for the wall shear stress enhancement before turbulent separation under adverse pressure gradient can be found by extending to oscillatory flows the definition of the equilibrium parameter (8) first defined by Clauser for steady flows (1954, [2]):

$$\beta_T(t) = \frac{\delta^*(t)}{\tau(t)} \frac{dP}{dx}(t) \tag{8}$$

where
$$\delta^*(t) = \int_0^\delta \left(1 - \frac{u}{U}\right) dz$$
 is the instantaneous displacement thickness, $\tau(t)$

is the instantaneous bottom shear stress and $\frac{dP}{dx}(t)$ is the instantaneous pressure gradient. This suggestion, like the use of a Reynolds averaged model, relies on a quasi-steady assumption. Indeed, it is assumed that in such oscillatory flows (with oscillating frequency smaller than turbulence frequencies) it is possible to find a time step for which the outer flow can be considered steady for applying Reynolds averaging and hence define Clauser parameter.

Turbulent separation occurs when β_T tends to infinity (in practice when β_T is larger than 20), that is when $\tau = 0$. For steady flows, β_T is positive for adverse pressure gradient and negative for favorable pressure gradient. For oscillatory flows, this classification does no longer hold. Indeed, β_T is positive under a favorable pressure gradient when the near-wall velocity is negative. Hence, to define the adverse pressure gradient in oscillatory flow, we should compare the pressure gradient action to the near-wall velocity. However, Clauser parameter still tells the relative strength of the pressure gradient compared to the flow. For steady flows, Wilcox ranges the pressure gradient strength from low ($\beta_T < 2$) to moderate ($2 < \beta_T < 4$) and strong ($\beta_T > 4$). For oscillating flows, we suggest to initiate the wall shear stress enhancement resisting and preceding turbulent separation when the pressure gradient is adverse and $\|\beta_T(t)\|$ is larger than a threshold value β_{sep} (strong adverse pressure gradient).

Secondly, we model the wall shear stress enhancement, when these conditions are fulfilled, prescribing a much lower value for the energy dissipation rate at the wall, ω_{wall} , than the one given by Wilcox condition cited above (1998 [38], p 177). Indeed, a decrease of ω_{wall} leads to an increase of the eddy viscosity and hence to an increase of the Reynolds stress, which is consistent with what is observed in the experiments. In the light of Wilcox' rough wall condition i.e. $\omega_{wall} = 100 u_f/k_N$, it can be understood that decreasing ω_{wall} is a way to take into account, in a 1DV model, the 3D vortex shedding mentioned by Sleath (1987, [29]). Indeed, we suggest that vortex shedding increases the apparent bottom roughness in relation with the vortex size, and consequently decreases ω_{wall} . However and as long as it is not possible to prescribe the dynamics of such an apparent bottom roughness according to the vortex shedding, we suggest a shortcut by imposing a fixed constant low value ω_{vortex} for ω_{wall} . It is worth noticing that the resulting wall shear stress enhancement tends to decrease the value of $\|\beta_T(t)\|$ which would cause a feedback on this condition. This feedback can lead to numerical instabilities in Reynolds stress time series close to the wall if ω_{vortex} is much smaller than the usual value for ω_{wall} . We think it is not relevant to allow for such a feedback as long as the apparent bottom roughness dynamics in relation with the vortex shedding is not included. That is to say that after $\|\beta_T(t)\|$ reaches for the first time β_{sep} during the decelerating parts of the wave cycle, we apply $\omega_{wall} = \omega_{vortex}$ until the pressure gradient becomes favorable again. However, a higher time resolution is required in computations to deal with this separation condition under the effect of a strong adverse pressure gradient. Indeed, without the separation condition, reliable computations are obtained with a 1 degree phase resolution. In contrast, a 0.1 degree phase resolution is required to obtain converged computations with the separation condition.

Concerning spatial resolution, computations are done on a vertical grid whose step size increases exponentially from bottom to top, thus giving a higher resolution near the bed where velocity gradients are important. Computations with near bed step size of 10^{-6} m and 10^{-7} m have been compared and give the same results. Thus, for all the computations presented in this paper, a near bed step size of 10^{-6} m was used, with 173 grid points exponentially distributed over the boundary layer thickness.

We discuss now the model sensitivity to the values chosen for β_{sep} and ω_{vortex} . On figures 2.a and b, we plot computed Reynolds stress time series close to the wall corresponding to the flow condition of Dohmen-Janssen experiment G4 (see section 4.1). For β_{sep} ranging from 4 to 40, it is clear that the lower β_{sep} is, the earlier shear stress enhancement begins (as can be expected) and the larger shear stress enhancement is. We will see in section 4.1 that a good agreement with tunnel experiments is found for $\beta_{sep} = 20$ which is in line with the usual practical value for turbulent separation. On figures 2.b, we show the computations with $\beta_{sep} = 20$ for ω_{vortex} ranging from 30 to 3000 (usual values for ω_{wall} for this flow condition is 10^4). As expected, the larger ω_{vortex} is, the smaller the shear stress enhancement is. We will see in section 4.1 that a good agreement with tunnel experiments is found for $\omega_{vortex} = 300$.

3 Pure oscillatory flow over a smooth bottom

3.1 The $k - \omega$ models versus DNS

Vittori and Verzicco (1998, [34]), by integrating numerically the Navier Stokes equations (DNS), observed wall shear stress enhancements during the decelerating phases of the oscillatory flow over a smooth bottom for moderate values of the Reynolds number $R_{\delta} = U_0 \delta / \nu = \sqrt{2R_e}$ (U_0 , T are the amplitude and period of the velocity oscillations outside the boundary layer, $\delta = \sqrt{\nu T/\pi}$ is the viscous boundary layer thickness, ν is the kinematic viscosity). Vittori and Verzicco (1998, [34]) also observed the "disturbed-laminar" and "intermittently turbulent" regimes (Hino et al., 1976, [10]) and investigated the transition from these two regimes. The "intermittently turbulent" regime, observed for values of R_{δ} larger than a critical value which depends on the intensity of the external disturbances but ranges around 600, shows turbulence activity during the decelerating phases of the cycle, while in the rest of the cycle the flow tends to recover a laminar-like behavior. When turbulent fluctuations start, at the beginning of the decelerating part of the cycle, a sudden strong increase in the wall shear stress is observed which might cause an increase of the sediment pick-up from the bed and an enhancement of the concentration.

Hence, in this subsection, we discuss the ability of the original Wilcox transitional model to predict flow field transition from the laminar to the turbulent regime under oscillatory pressure gradient for smooth wall and moderate Reynolds number conditions. In order to test its performance, a DNS has been performed for the following hydrodynamical conditions: $U_0 = 1.1 \text{ m/s}$, T = 4 s, $R_{\delta} = 1241 \text{ and}$ $\delta = 1.128 \text{ } 10^{-3} \text{ m}$. For details on the DNS, the reader should refer to Vittori and Verzicco (1998). For these conditions, we suggest that flow separation induces the laminar-turbulent transition, the modeling of which is already included in the transitional model.

In the following the DNS results presented have been phase-averaged in order to remove the stochastic behavior characteristic of a single realization of the process. For details on the averaging procedure the reader is referred to Vittori and Verzicco (1998) and Costamagna et al. (2002, [3]). In figure 3.b, the wall shear stress computed with the DNS and the Wilcox original transitional model are plotted. Wilcox model is indeed able to predict the wall shear stress enhancement at the laminar-turbulent transition as shown in the DNS computations. In particular, the maximum wall shear stress predicted by the Wilcox original model is close to DNS predictions. However, the initiation of the transition occurs earlier in Wilcox model than in the DNS In the transitional $k-\omega$ model, the initiation of this transition is controlled by the two parameters R_K and R_ω , whereas the amplitude of the bottom shear stress enhancement is linked to the R_K/R_β ratio. The chosen values of $R_K = 20$, $R_\beta = 27$ and $R_\omega = 2.95$ lead to much better results (figure 3.b).

Velocity (figure 3.a), Reynolds stress (figure 4) and turbulent kinetic energy (figure 5) vertical profiles through the boundary layer at different phases during half oscillation are also plotted. It is noticeable that the original Wilcox model underestimate the vertical decay of all these variables. By changing the value of the diffusion constants in the k and ω -equations (taking $\sigma^* = 0.8$ and $\sigma = 0.375$), we can improve the $k - \omega$ model results for the vertical decay of the velocity, the Reynolds stress and the turbulent kinetic energy compared to DNS results. The agreement between the so-called new $k-\omega$ model and the DNS is indeed remarkable except close to the wall ($z \leq 5\delta$) for the turbulent kinetic energy. Indeed, the original Wilcox model give better estimation of the near-wall turbulent kinetic energy predictions, have reduced effects on the wall shear stress prediction (figure 3.b).

3.2 The $k-\omega$ models versus Jensen et al. experiments (1989, [14])

In this section, we compare predictions of the two transitional models with the experimental measurements of Jensen et al (1989) which were made within the boundary layer of a pure oscillating flow over a smooth bottom. Details concerning these experiments can be found in Jensen et al. (1989, [14]).

Jensen et al. carried out experiments using a smooth wall for various Reynolds number ranging from the laminar regime ($R_e = 3.3 \times 10^4$) up to the fully turbulent regime ($R_e = 6.0 \times 10^6$) and in particular, they measured bottom shear stress. On figure 6, the non-dimensional bottom shear stress time series (bottom shear stress time series divided by the maximum bottom shear stress) computed using the original Wilcox model and the new one are plotted. For the sake of clearness, we do not show Jensen et al's measurements on this graph. To compare the theoretical predictions with the experimental data, this figure should be compared to figure 9 of Jensen et al. It is then clear that in the original Wilcox model, the laminar-turbulent transition develops much quicker for R_e larger than 3.3×10^4 , whereas the new model with modified value for R_K and R_β gives results closer to the measurements. This is confirmed by figure 7 where the computed dimensional bottom shear stresses are plotted along with the measured values. However, it can be noticed that in the

measurements, for R_e larger than 6.0×10^6 , turbulence seems to have pervaded the whole cycle whereas in the new model results the boundary layer still recovers its laminar behavior at the very beginning of the accelerating phases of the cycle, even though for these low value of the outer flow velocity, the difference between fully turbulent and laminar value of the bottom shear stress are small.

Then we compare measured and modeled characteristics of the boundary layer for a specific experiment, in particular test 8 from Jensen et al. ($R_e = 1.6 \times 10^6$, $R_{\delta} = 1789$) is considered. This test was also chosen by Thais et al. (1999, [31]) to test the Chien low-Reynolds-number $k - \epsilon$ model. Hence this comparison enables a cross discussion of the results of all the models. The comparison plots are presented with the outer flow scaling. Vertical profiles through the boundary layer at different phases during half cycle are plotted for the velocity (figure 8), turbulent kinetic energy (figure 9) and Reynolds stress (figure 10). On these plots, we clearly see that the original Wilcox model does not correctly reproduce the vertical dependency on these three quantities. The better achievements of the new transitional $k - \omega$ model and in particular the improved vertical decay description of the velocity, turbulent kinetic energy and Reynolds stress are obtained by changing the diffusion constants.

However some discrepancy is present when a detailed comparison of the model results is made with the experimental data. Part of this discrepancy might be due to experimental errors. In particular, turbulent quantities measured using the Laser Doppler Anemometer (LDA) may be underestimated near the wall. Indeed, according to Prandtl mixing length theory the turbulent length scale is equal to κz , with κ the Von Kármán constant ($\kappa = 0.4$). Hence, part of the turbulence may be averaged out over the sampling volume of the LDA when turbulent length scale is smaller than the size of the sampling volume. In Jensen et al. experiments, the sampling volume sizes are 0.15 mm x 0.15 mm x 2.5 mm. This means that up to $z = 3.5\delta = 6.25$ mm, turbulent quantities might be underestimated. This might explain why the measured turbulent kinetic energy is smaller than even the Wilcox model predictions close to the wall at phases around flow reversal ($\varphi = 2\pi t/T =$ 0, 15, 30, 150 and 165°). Another error source might be due to the prescription in the models of an exact sinusoidal outer flow whereas in the measurements, the outer flow is not perfectly sinusoidal. Finally, it should be recognized that the Chien low-Reynolds-number model used by Thais et al. (1999, [31]) performs also very well in

4 Oscillatory flow plus current over a rough bottom: the $k-\omega$ model versus tunnel experiments

4.1 Dohmen-Janssen clear water experiments (1999, [5])

The experiments were carried out in the Large Oscillating Water Tunnel (LOWT) of WL||Delft Hydraulics. For a detailed description of the LOWT, the reader can refer to Ribberink and Al Salem (1994, [21]). This facility enables a full-scale simulation of the horizontal orbital motion near the bed associated to waves. The LOWT is a U-shaped tube, with a piston in one of the cylindrical risers to generate a horizontal oscillatory flow in the test section. Due to this configuration, vertical orbital velocities are not generated in the tunnel. A re-circulation system allows the generation of a net current in addition to the oscillatory flow. The test section is 0.3 m x 0.8 m by 12 m long. For clear water experiments, a fixed rough bottom was used, made of glued sand of diameter $d_{50} = 0.21$ mm on a wooden bottom. Timedependent flow velocities, including turbulent components, were measured with a two-components LDA in forward-scatter mode. The LDA has a relatively small sampling volume, with a height and width (in flow direction) of only 0.215 mm. The width in cross direction is larger (6.47 mm). The experimental conditions of the clear water experiments we first consider in this section are named G4 and G5 and given in table 1.

Figures 11 and 12 show the Reynolds stress time series measured by Dohmen-Janssen (1999, [5]) at various levels above the rough bed (experiments G4 and G5), along with the computations obtained by Wilcox transitional $k - \omega$ model and our new $k - \omega$ model taking $\beta_{sep} = 20$ and $\omega_{wall} = 300$. We clearly observe on figures 11a, b and c, secondary humps at the end of the decelerating phase in the measured Reynolds stress time series. These humps are also present in our new model computations. Yet, the computed values of Reynolds stress are larger than those measured up to z = 10 mm. In fact, the experimental values might be underestimated due to the size of the sampling volume of the LDA as explained in section 3.2. This means that up to z = 16 mm, part of the turbulence may be

averaged out, resulting in smaller value of $\overline{u'w'}$. Above z=10 mm, the agreement between the measured and the new model's results is noticeable on both the amplitude and the phase predictions. In contrast, Wilcox model mainly fails on the phase prediction. The better performance of the new model in the phase prediction is related to the better description of the vertical decay of Reynolds stress, velocity and turbulent kinetic energy as already noticed when the model predictions were compared with the DNS This finding is a further confirmation of the better choice for the diffusion constants σ and σ^* in our new model.

4.2 Janssen et al. experiments with sediments (1997, [13])

Hereafter, we present an application of the new $k-\omega$ model described in section 2 to predict sediment suspension. For this purpose, we compare the results obtained using the original Wilcox transitional model and the new $k-\omega$ model. In both models, turbulence damping by the stratification induced by suspended sediment is included as described in section 2. The concentration equation is thus solved simultaneously with the hydrodynamics using Engelund and Fredsøe's (1976, [6]) reference concentration condition at the bottom. We apply the two models for the test conditions H6 and E2 of Janssen et al. (1997, [13], see table 1). In figure 13 the measured and modeled concentration time series taken at different levels above the bed are shown. Concentration measurements shows sharp increases before flow reversal near the bottom (figures 13.b, c, e,f). Doubts can be cast on these sudden concentration peaks that could be linked to the measuring technique. Indeed, a sediment deposition between the electrodes of the conductivity probe (CCM) at low velocity could cause such a signal. Nevertheless, concentration peaks are also observed in time series measured using optical conductivity probes (figures 13.a and d) further from the bottom. Hence, we feel that the peaks in the measured concentration time series are the effective signature of suspension ejection events, although a possible bias in CCM probes measurements should be investigated. The separation condition introduced in the new transitional $k-\omega$ model enables to reproduce the secondary peaks observed in measured concentration time series before flow reversal. Moreover, by changing the diffusion constants, we have improved the description of the phase concentration in the upper part of the boundary layer. However a significant discrepancy still remains between the predicted and the measured values. Indeed, the concentration is underestimated throughout the entire boundary layer. It should be stressed that the overall concentration underestimation in the new model contrast with the traditional model predictions. Indeed, the traditional model, despite its underestimation of the concentration in the lower levels, agrees better with the data at z = 20 mm. In our opinion, this better agreement of the traditional model is coincidental. Indeed, discrepancies between the lower and upper levels is in line with the fact that the Reynolds stress vertical decay is not correctly predicted by the traditional model (as shown in comparison with the DNS on figure 4). In contrast, in the new model, the concentration is underestimated at all levels (see figures 13). The model predictions can be improved at all levels by taking into account the inter-granular forces in the "sheet flow" layer (highly concentrated bottom layer). Indeed, following Guizien et al. (2001, [9]) suggestion of enhancing the hindering in the sheet flow layer i.e. taking a lower settling velocity for volume concentration larger than 0.01, the agreement of the new model with the data is improved at all levels as shown on figures 14. In contrast, in the traditional model, although the agreement is improved at the lower levels, it turns worst in the upper levels (see figures 14.a and b). This is consistent with the improvement in the new model predictions of the turbulence vertical decay as already mentioned in section 3.1. However, discrepancies still remains between predicted and measured values. We suggest this could be improved by looking into more details in the modeling of the inter-granular forces in the "sheet flow" layer and also in the sediment feedback on turbulence, but this is beyond the scope of this paper.

5 Conclusions

A new transitional $k-\omega$ model has been devised introducing a turbulent separation condition under adverse pressure gradient and modifying the diffusion and transition constants of the Wilcox original $k-\omega$ transitional model (1992, [37]). The constants modification is suggested by a comparison with a D.N.S computation of a pure oscillatory flow over a smooth bottom in the laminar-turbulent transitional regime ($R_e = 7.7 \times 10^5$). The new model is then validated by comparing its predictions with experimental data by Jensen et al. (1989, [14]) for Reynolds number

 R_e ranging from 3.3×10^4 to 6×10^6 . In this regime, flow separation induces the transition from the disturbed-laminar to the intermittently-turbulent regimes which is modeled by the dependence of the constants of the high-Reynolds Wilcox model (1988, [36]) upon a local Reynolds number. We are thus able to reproduce the wall shear stress sharp increase which takes place at transition in good agreement with Jensen et al. data. The change of the diffusion constants improves also the description of the vertical distribution of both velocity and Reynolds stress compared to the original transitional Wilcox model (1992, [37]). The new model gives fairly good results, except concerning the value of the turbulent kinetic energy close to the wall around $\varphi = 90^\circ$, and is low-time consuming.

The change of the diffusion and transition constants proposed in this new model also enables to improve the description of Reynolds stress vertical distribution in the fully rough turbulent regime. Indeed, as shown by a comparison with experimental data in clear water, the model reproduces accurately the vertical phase lagging of Reynolds stress. In addition, for these experimental conditions, the turbulent separation condition acts and enables to reproduce the secondary humps in the measured Reynolds stress time series. This feature has never been reproduced in standard R.A.N.S. models. Finally, we use the validated new $k-\omega$ model to predict sediment transport and compare the results with experimental data. By solving the concentration convective-diffusive equation with turbulence damping, this new model reproduces secondary peaks in concentration time series before flow reversal in accordance with the measurements of Janssen et al. (1997). However, it should be outlined that the concentration is still underestimated in the sheet flow layer. This finding stimulate us to go on working on this important issue for morphodynamics predictions.

6 Acknowledgments

This work was funded by the EC through a MAST-III project SEDMOC (contract MAS3-CT97-0115). First author would like to thank the Instituto do Mar (Universidade de Coimbra - Portugal) for awarding a postdoctoral grant.

Bibliography

- [1] Chien K.Y., 1982. Predictions of channel and boundary layer flows with a low-Reynolds-number turbulence model. AIAA J. 20, 33-38.
- [2] Clauser F.H., 1954. Turbulent boundary layers under adverse pressure gradients.
 J. Aeronautics Sci. 21, 91-108.
- [3] Costamagna P., Vittori G. and Blondeaux P., 2002. Coherent structures in oscillatory boundary layers. To appear in J. Fluid Mech.
- [4] Davies A.G., Ribberink J.S., Temperville A. and Zyserman J.A., 1997. Comparison between sediment transport models and observations made in wave and current flows above plane beds. *Coastal Eng.* 31, 163-198.
- [5] Dohmen-Janssen M., 1999. Grain size influence on sediment transport in oscillatory sheet flow. *PhD Thesis*, Delft Technical University.
- [6] Engelund F. and Fredsøe J., 1976. A sediment transport model for straight alluvial channel. *Nordic Hydrology* 7, 293-306.
- [7] Foster D.L., Holman R.A. and Beach R.A., 1994. Sediment suspension events and shear instabilities in the bottom boundary layer. *Proc. Coastal Dyn.* '94, ASCE, Barcelona, 712-726.
- [8] Fredsøe J., 1984. Turbulent boundary layer in wave-current interaction. J. Hydr. Eng. ASCE, 110, HY8, 1103-1120.
- [9] Guizien K., Silva P. and Seabra-Santos F.J., 2001. Mathematical modeling of sand transport by combined waves and current in the sheet flow regime using R.A.N.S. turbulence models: hindering and flow reversal ejections. SEDMOC book, Aqua Publications, BD, 1-8.
- [10] Hino M., Sawamoto M. and Takasu S., 1976. Experiments on transition to turbulence in an oscillatory pipe flow. *J. Fluid Mech.* 75, 193-207.

- [11] Huynh-Than S. and Temperville A., 1990. A numerical model of the rough turbulent boundary layer in combined wave and current interaction. Proc. of the 22th Coastal Eng. Conf. ASCE, 853-866.
- [12] Huynh-Than S., Tran Thu T. and Temperville A., 1994. A numerical model for suspended sediment in combined currents and waves. in Sediment transport mechanisms in coastal environments and rivers, EUROMECH 310. Ed. M. Belorgey, R.D. Rajaona and J.F.A. Sleath, World Scientific, Singapore, 122-130.
- [13] Janssen C.M., Hassan W.N., van der Wal R.J. and Ribberink J.S., 1997. Grainsize influence on sand transport mechanisms. *Proc. Coastal Dyn.* '97, Plymouth, 58-68.
- [14] Jensen B.L., Sumer B.M. and Fredsøe J., 1989. Turbulent oscillatory boundary layers at high Reynolds numbers. *J. Fluid Mech.* 206, 265-297.
- [15] Jones W.P. and Launder B.E., 1972. The prediction of laminarization with a two-equation model of turbulence. *Int. J. Heat Mass Transfer* 15, 301-314.
- [16] Justesen P., 1991. A note on turbulence calculations in the wave boundary layer. J. Hydr. Res. 29, 5, 699-711.
- [17] Katapodi I., Ribberink J.S., Ruol P. Koelewijn R., Lodhal C., Longo S., Crosato A. and Wallace H., 1994. Intra-wave sediment transport in an oscillatory flow superimposed on a mean current. *Report H-1684*, *Part III*. Delft (The Netherlands).
- [18] Lewellen W.S., 1977. Use of invariant modeling. In *Handbook of turbulence*, Plenum Publishing Corp., vol. 1.
- [19] Li Z. and Davies A.G., 1996. Towards predicting sediment transport in combined wave-current flow. J. Waterway, Port, Coastal and Ocean Eng. ASCE, 122, 4, 157-164.
- [20] Patankar S.V., 1980. Numerical heat transfer and fluid flows. Mac Graw Hill Book Co.
- [21] Ribberink J.S. and Al Salem A.A., 1994. Sediment transport in oscillatory boundary layers in cases of rippled beds and sheet flow. J. Geophys. Res. 99 (C6), 12,707-12,725.

- [22] Ribberink J.S. and Al Salem A.A., 1995. Sheet flow and suspension of sand in oscillatory boundary layers. *Coastal Eng.* 25, 205-225.
- [23] Richardson J.F. and Zaki W.N., 1954. Sedimentation and fluidisation: part I. Trans. Inst. Chem. Engrs. 32(1), 35-53.
- [24] Rodi W., 1980. Turbulence models and their application in Hydraulics. *IAHR* monograph. Delft, The Netherlands.
- [25] Saffman P.G., 1970. A model for in-homogeneous turbulent flow. *Proc. Roy. Soc. Lond.* A 317, 417-433.
- [26] Sana A. and Tanaka H., 2000. Review of $k \epsilon$ models to analyze oscillatory boundary layers. J. Hydr. Eng. vol. 126, No 9, 701-710.
- [27] Savioli J. and Justesen P., 1996. Sediment in oscillatory flows over a plane bed. J. Hydr. Res. 35, 2, 177-190.
- [28] Savioli J. and Justesen P., 1997. Towards predicting sediment transport in combined wave-current flow - Discussion. J. Waterway, Port, Coastal and Ocean Eng. ASCE, Nov./Dec., 362-364.
- [29] Sleath J.F.A., 1987. Turbulent oscillatory flow over rough bed. J. Fluid Mech., vol. 182, 369-409.
- [30] Spalart P.R. and Baldwin B.S, 1989. Direct simulation of a turbulent oscillating boundary layer. *Turbulent Shear Flow*, vol. 6, 417-440.
- [31] Thais L., Chapalain G. and Smaoui H., 1999. Reynolds number variation in oscillatory boundary layers. Part I. Purely oscillatory motion. *Coastal Eng.* 36, 111-146.
- [32] Thwaites B., 1949. Approximate calculations of the laminar boundary layer.

 Aeronaut. Quart. 1, 245-280.
- [33] Van Rijn L.C., 1984. Sediment pick-up functions. J. Hydr. Eng. 110(10), 1494-1502.
- [34] Vittori G. and Verzicco R., 1998. Direct simulation of transition in an oscillatory boundary layer. *J. Fluid Mech.* 371, 207-232.

- [35] White F.M., 1991. Viscous Fluid Flow. Mac Graw-Hill, Inc. 2nd Edition.
- [36] Wilcox D.C., 1988. Reassessment of the scale-determining equation for advanced turbulence models. AIAA Journal, 26(11), 1299-1310.
- [37] Wilcox D.C., 1992. The remarkable ability of turbulence model equations to describe transition. Fifth Symposium on Numerical and Physical aspects of Aero-dynamical Flows 13-15 January, California State University, Long Beach, CA.
- [38] Wilcox D.C., 1998. Turbulence modeling for CFD. *DCW Industries, Inc.* 2nd Edition.
- [39] Wu X., 1992. The nonlinear evolution of high-frequency resonant triad waves in an oscillatory Stokes layer at high Reynolds number. *J. Fluid Mech.* 245, 553-597.

т		c		1 1	
1	18t	ot	Πa	.h	es

1	Experimental conditions for Dohmen-Janssen clear water experi-	
	ments (1999, [5]) and Janssen et al. sand experiments (1997, [13]) 2	7

List of Figures

1	Phase φ definition along the oscillatory part of the outer flow velocity	
	(—) and corresponding pressure gradient ()	28
2	Phase-averaged Reynolds stress time series obtained using the new	
	$k-\omega$ model for (a) eta_{sep} ranging from 4 to 40 ($\omega_{vortex}=300$) and	
	(a) ω_{vortex} ranging from 30 to 3000 ($\beta_{sep}=20$) ($R_{\delta}=2179,~R_{e}=$	
	2.4×10^6 , $A/k_N = 3173$)	29
3	Velocity vertical profiles at different phases (a) and bottom shear stress	
	time evolution (b) computed by DNS (), the original Wilcox transi-	
	tional $k-\omega$ model () and the new $k-\omega$ model (—) for a sinusoidal	
	outer flow $U = U_0 sin(2\pi t/T)$ with $T = 4$ s, $U_0 = 1.1$ m/s over a	
	smooth bottom ($R_{\delta} = 1241$, $R_e = 7.7 \times 10^5$)	30
4	Reynolds stress vertical profiles at different phases: (a) $\varphi = -45^{\circ}$,	
	(b) $\varphi = 0^{\circ}$, (c) $\varphi = 45^{\circ}$ and (d) $\varphi = 90^{\circ}$ computed by DNS (),	
	the original Wilcox transitional $k-\omega$ model () and the new $k-\omega$	
	model (—) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$ with $T = 4$	
	s, $U_0 = 1.1 \text{ m/s}$ over a smooth bottom ($R_{\delta} = 1241$, $R_e = 7.7 \times 10^5$).	31
5	Turbulent kinetic energy vertical profiles at different phases: (a) $\varphi =$	
	-45° , (b) $\varphi = 0^{\circ}$, (c) $\varphi = 45^{\circ}$ and (d) $\varphi = 90^{\circ}$ computed by DNS (),	
	the original Wilcox transitional $k-\omega$ model () and the new $k-\omega$	
	model (—) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$ with $T = 4$	
	s, $U_0 = 1.1$ cm/s over a smooth bottom ($R_{\delta} = 1241$, $R_e = 7.7 \times 10^5$).	32

6	${\it Half-period\ bottom\ shear\ stress\ time\ series\ showing\ laminar-turbulent}$	
	transition for increasing Reynolds number predicted by the original	
	Wilcox transitional $k-\omega$ model () and the new $k-\omega$ model (—)	
	for a sinusoidal outer flow over a smooth bottom.	33
7	Bottom shear stress measured by Jensen et al. (1989, [14]) (), the	
	original Wilcox transitional $k-\omega$ model () and the new $k-\omega$ model	
	(—) for a sinusoidal outer flow $U=U_0 \sin(2\pi t/T)$ with $T=9.72~\mathrm{s}$	
	over a smooth bottom for different Reynolds number	34
8	Velocity vertical profiles at different phases during the accelerating	
	part of the cycle (a) and during the decelerating part of the cycle (b)	
	measured by Jensen et al. (1989, test 8, [14]) (), computed with the	
	original Wilcox transitional $k-\omega$ model () and the new $k-\omega$ model	
	(—) for a sinusoidal outer flow $U=U_0 \sin(2\pi t/T)$ with $T=9.72~\mathrm{s},$	
	$U_0=1.02~{ m m/s}$ over a smooth bottom ($R_\delta=1789,~R_e=1.6 imes10^6$)	35
9	Turbulent kinetic energy vertical profiles at different phases during the	
	accelerating part of the cycle (a) and during the decelerating part of the	
	cycle (b) measured by Jensen et al. (1989, test 8, [14]) (), computed	
	with the original Wilcox transitional $k-\omega$ model () and the new	
	$k-\omega \mod el$ (—) for a sinusoidal outer flow $U=U_0sin(2\pi t/T)$ with	
	$T=9.72~\mathrm{s},~U_0=1.02~\mathrm{m/s}$ over a smooth bottom ($R_\delta=1789,~R_e=$	
	1.6×10^6)	36
10	Reynolds stress vertical profiles at different phases during the accel-	
	erating part of the cycle (a) and during the decelerating part of the	
	cycle (b) measured by Jensen et al. (1989, test 8, [14]) (), com-	
	puted with the original Wilcox transitional $k-\omega$ model () and the	
	new $k - \omega$ model (—) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$	
	with $T = 9.72$ s, $U_0 = 1.02$ m/s over a smooth bottom ($R_{\delta} = 1789$,	
	$R_e = 1.6 \times 10^6)$	37
11	Phase-averaged Reynolds stress time series at different levels from the	
	bed measured by Dohmen-Janssen (1999, exp. G4, [5]) (), obtained	
	using the original Wilcox transitional $k-\omega$ model () and the new	
	$k - \omega \mod el \ () \ (R_{\delta} = 2179, \ R_e = 2.4 \times 10^6, \ A/k_N = 3173). \ . \ . \ . \ .$	38

12	Phase-averaged Reynolds stress time series at different levels from the	
	bed measured by Dohmen-Janssen (1999, exp. G5, [5]) (), obtained	
	using the original Wilcox transitional $k-\omega$ model () and the new	
	$k - \omega \mod (-) (R_{\delta} = 1453, R_e = 1.06 \times 10^6, A/k_N = 2115).$	39
13	Phase-averaged concentration time series measured by Janssen et al.	
	(1997, exp. E2 and H6, [13]) () and computed with the original	
	Wilcox transitional $k-\omega$ model () and the new $k-\omega$ model (—)	
	(in both models, turbulence damping by stratification is included) for	
	$d_{50}=0.13$ mm at $z=1$ mm (c), $z=3$ mm (b) and at $z=200$ mm	
	(a) from the bottom and for $d_{50}=0.21~\mathrm{mm}$ at $z=1.5~\mathrm{mm}$ (f), $z=2.5$	
	mm (e) and at $z=210$ mm (d) from the bottom. Hydrodynamical	
	conditions are given in table 1	40
14	Same legend as figure 13, but in both models, enhanced hindering for	
	high concentration in the sheet flow layer has been introduced	41

experiment	grain size	${\rm roughness}$	current at $z = 100 \text{ mm}$	wave velocity	period
name	$d_{50}~(\mathrm{mm})$	$k_N (\mathrm{m})$	$u_c \; (\mathrm{m/s})$	$u_w \text{ (m/s)}$	T(s)
G4	fixed bed	$5.25 \ 10^{-4}$	0.27	1.44	7.2
G5	fixed bed	$5.25 \ 10^{-4}$	0.46	0.96	7.2
H6	0.13	$3.25 \ 10^{-4}$	0.24	1.47	7.2
<i>E2</i>	0.21	$5.25 \ 10^{-4}$	0.24	1.47	7.2

Table 1: Experimental conditions for Dohmen-Janssen clear water experiments (1999, [5]) and Janssen et al. sand experiments (1997, [13]).

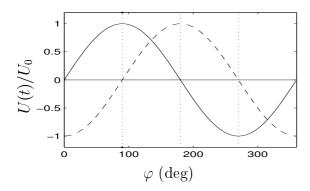
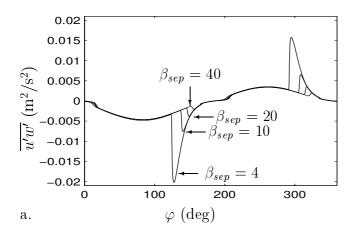


Figure 1: Phase φ definition along the oscillatory part of the outer flow velocity (—) and corresponding pressure gradient (- -).



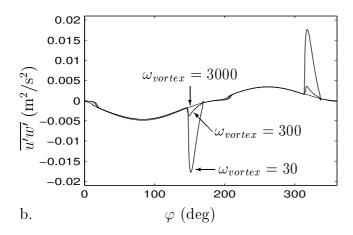
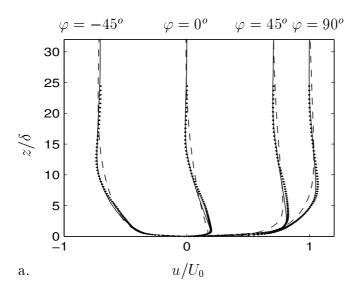


Figure 2: Phase-averaged Reynolds stress time series obtained using the new $k-\omega$ model for (a) β_{sep} ranging from 4 to 40 ($\omega_{vortex} = 300$) and (a) ω_{vortex} ranging from 30 to 3000 ($\beta_{sep} = 20$) ($R_{\delta} = 2179$, $R_{e} = 2.4 \times 10^{6}$, $A/k_{N} = 3173$).



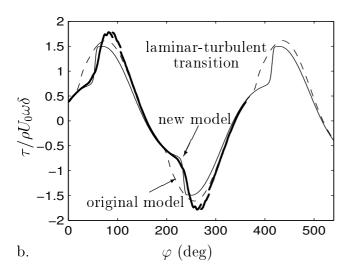


Figure 3: Velocity vertical profiles at different phases (a) and bottom shear stress time evolution (b) computed by DNS (...), the original Wilcox transitional $k-\omega$ model (-) and the new $k-\omega$ model (-) for a sinusoidal outer flow $U=U_0\sin(2\pi t/T)$ with T=4 s, $U_0=1.1$ m/s over a smooth bottom ($R_{\delta}=1241$, $R_e=7.7\times10^5$).

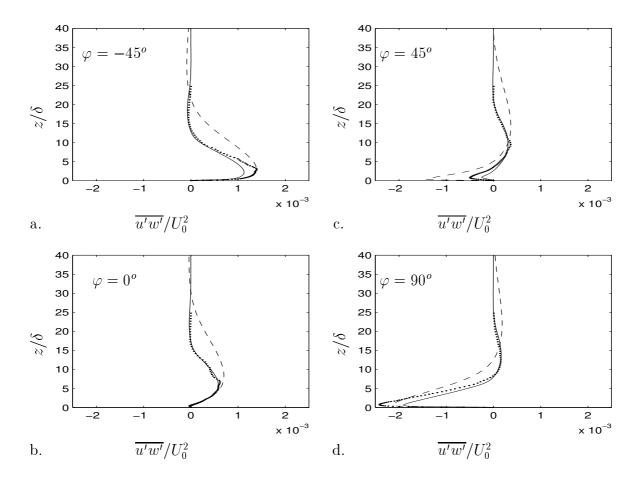


Figure 4: Reynolds stress vertical profiles at different phases: (a) $\varphi = -45^{\circ}$, (b) $\varphi = 0^{\circ}$, (c) $\varphi = 45^{\circ}$ and (d) $\varphi = 90^{\circ}$ computed by DNS (...), the original Wilcox transitional $k-\omega$ model (- -) and the new $k-\omega$ model (—) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$ with T = 4 s, $U_0 = 1.1$ m/s over a smooth bottom ($R_{\delta} = 1241$, $R_e = 7.7 \times 10^5$).

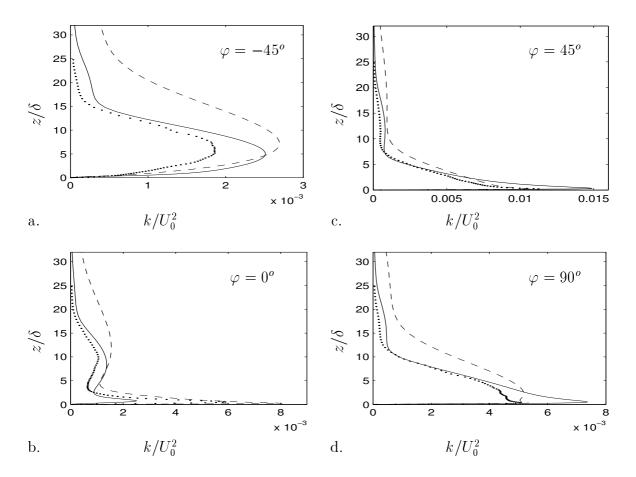


Figure 5: Turbulent kinetic energy vertical profiles at different phases: (a) $\varphi = -45^{\circ}$, (b) $\varphi = 0^{\circ}$, (c) $\varphi = 45^{\circ}$ and (d) $\varphi = 90^{\circ}$ computed by DNS (...), the original Wilcox transitional $k-\omega$ model (- -) and the new $k-\omega$ model (—) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$ with T = 4 s, $U_0 = 1.1$ cm/s over a smooth bottom ($R_{\delta} = 1241$, $R_e = 7.7 \times 10^5$).

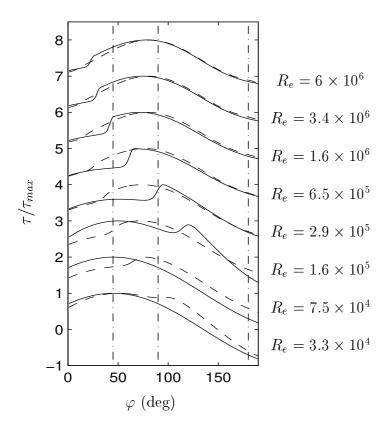


Figure 6: Half-period bottom shear stress time series showing laminar-turbulent transition for increasing Reynolds number predicted by the original Wilcox transitional $k-\omega$ model (- -) and the new $k-\omega$ model (—) for a sinusoidal outer flow over a smooth bottom.

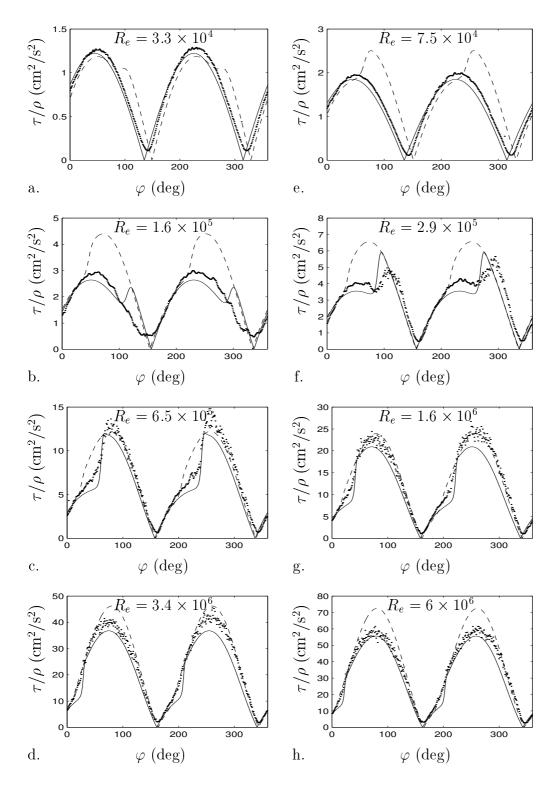


Figure 7: Bottom shear stress measured by Jensen et al. (1989, [14]) (...), the original Wilcox transitional $k - \omega$ model (- -) and the new $k - \omega$ model (--) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$ with T = 9.72 s over a smooth bottom for different Reynolds number.

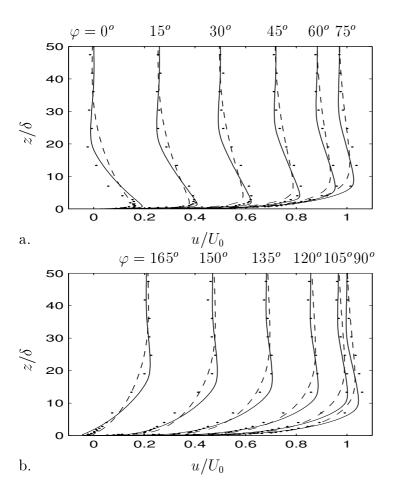


Figure 8: Velocity vertical profiles at different phases during the accelerating part of the cycle (a) and during the decelerating part of the cycle (b) measured by Jensen et al. (1989, test 8, [14]) (...), computed with the original Wilcox transitional $k - \omega$ model (- -) and the new $k - \omega$ model (-) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$ with T = 9.72 s, $U_0 = 1.02$ m/s over a smooth bottom $(R_{\delta} = 1789, R_e = 1.6 \times 10^6)$.

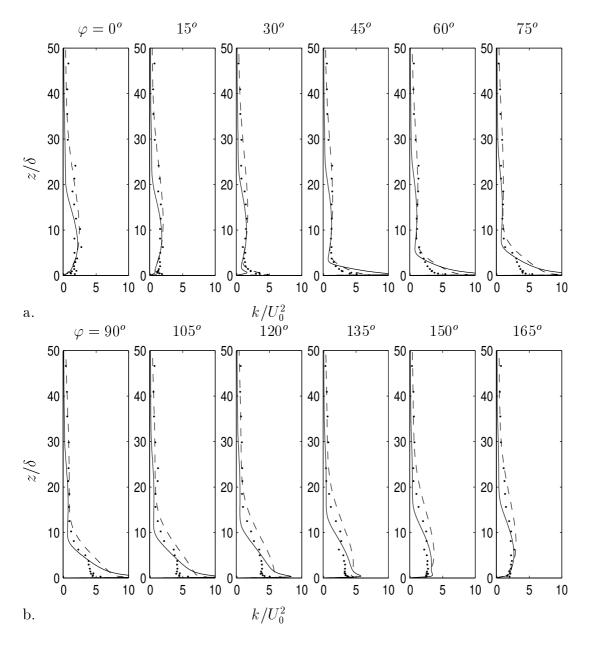


Figure 9: Turbulent kinetic energy vertical profiles at different phases during the accelerating part of the cycle (a) and during the decelerating part of the cycle (b) measured by Jensen et al. (1989, test 8, [14]) (...), computed with the original Wilcox transitional $k-\omega$ model (- -) and the new $k-\omega$ model (-) for a sinusoidal outer flow $U=U_0\sin(2\pi t/T)$ with T=9.72 s, $U_0=1.02$ m/s over a smooth bottom $(R_\delta=1789,\ R_e=1.6\times10^6)$.

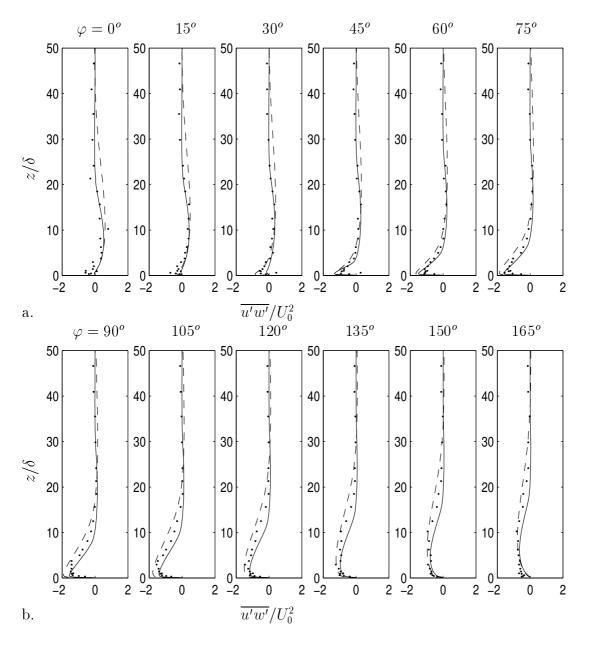


Figure 10: Reynolds stress vertical profiles at different phases during the accelerating part of the cycle (a) and during the decelerating part of the cycle (b) measured by Jensen et al. (1989, test 8, [14]) (...), computed with the original Wilcox transitional $k - \omega$ model (- -) and the new $k - \omega$ model (—) for a sinusoidal outer flow $U = U_0 \sin(2\pi t/T)$ with T = 9.72 s, $U_0 = 1.02$ m/s over a smooth bottom ($R_{\delta} = 1789$, $R_e = 1.6 \times 10^6$).

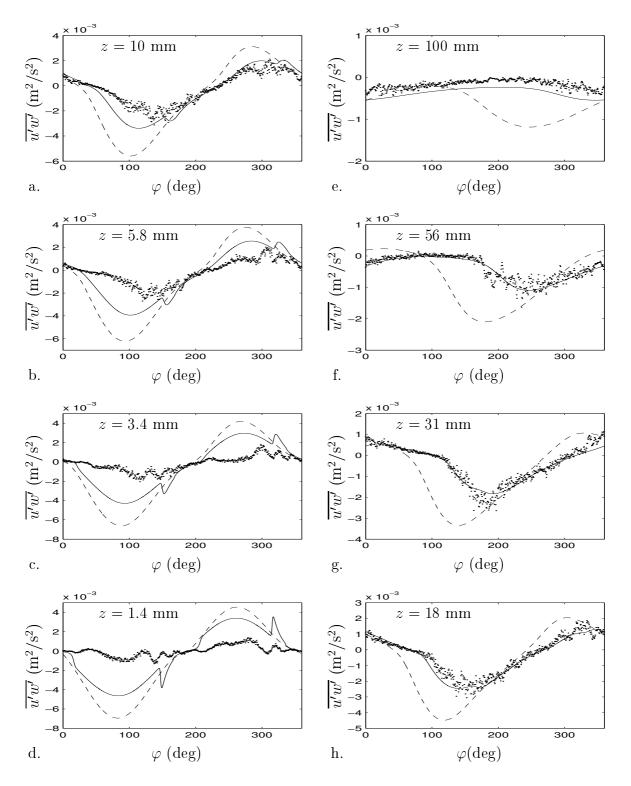


Figure 11: Phase-averaged Reynolds stress time series at different levels from the bed measured by Dohmen-Janssen (1999, exp. G4, [5]) (...), obtained using the original Wilcox transitional $k - \omega$ model (- -) and the new $k - \omega$ model (--) $(R_{\delta} = 2179, R_e = 2.4 \times 10^6, A/k_N = 3173)$.

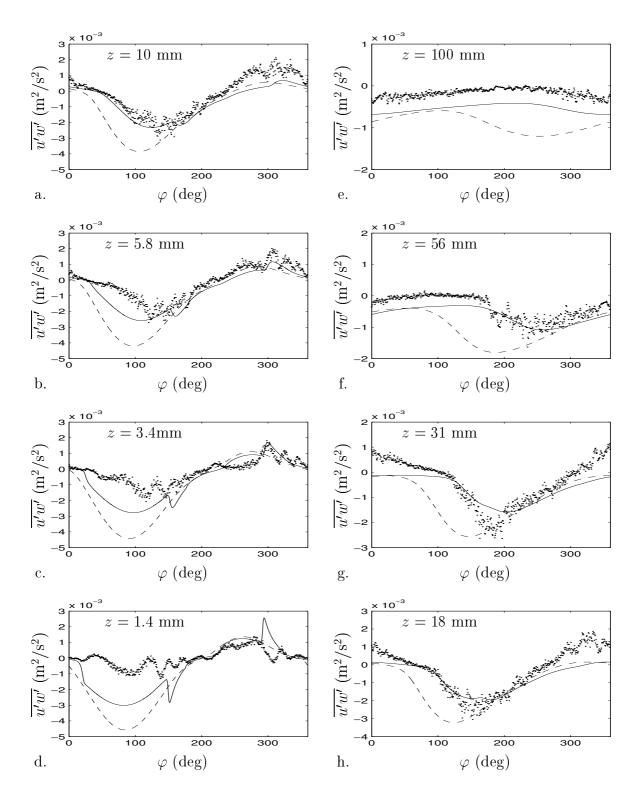


Figure 12: Phase-averaged Reynolds stress time series at different levels from the bed measured by Dohmen-Janssen (1999, exp. G5, [5]) (...), obtained using the original Wilcox transitional $k - \omega$ model (- -) and the new $k - \omega$ model (--) ($R_{\delta} = 1453$, $R_e = 1.06 \times 10^6$, $A/k_N = 2115$).

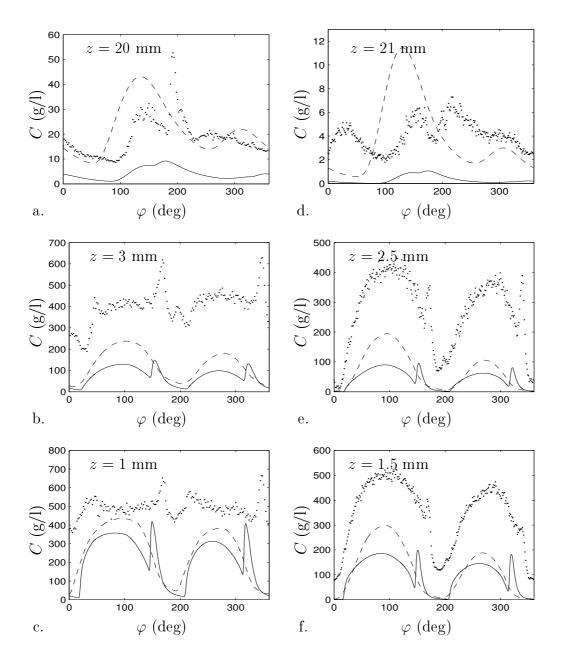


Figure 13: Phase-averaged concentration time series measured by Janssen et al. (1997, exp. E2 and H6, [13]) (...) and computed with the original Wilcox transitional $k-\omega$ model (- -) and the new $k-\omega$ model (—) (in both models, turbulence damping by stratification is included) for $d_{50}=0.13$ mm at z=1 mm (c), z=3 mm (b) and at z=200 mm (a) from the bottom and for $d_{50}=0.21$ mm at z=1.5 mm (f), z=2.5 mm (e) and at z=210 mm (d) from the bottom. Hydrodynamical conditions are given in table 1.

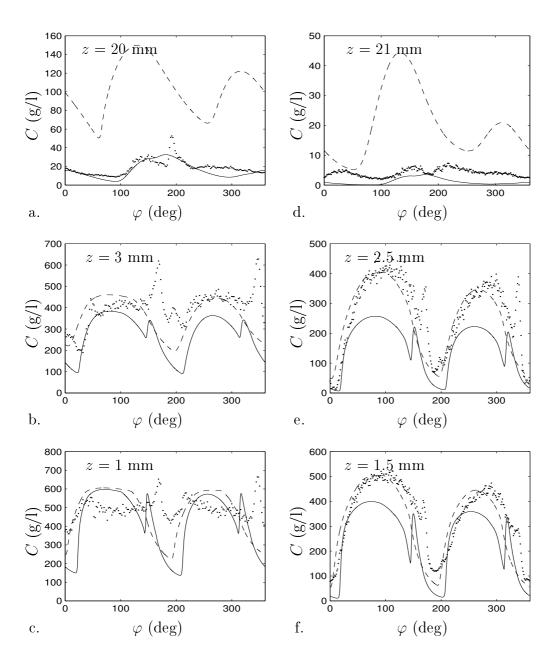


Figure 14: Same legend as figure 13, but in both models, enhanced hindering for high concentration in the sheet flow layer has been introduced.