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1 **Coupling Spectral and Finite Element Methods for 3D physic-based seismic analysis**
2 **from fault to structure: application to the Cadarache site in France**

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26 **Abstract**

27 This paper presents the application of a coupling strategy between the Spectral (SEM) and Finite Element (FEM)
28 Methods to solve the soil-structure interaction (SSI) problem. The SEM-FEM coupling benefits from the mesh
29 refinement capabilities of the FEM in modeling the structure and the near soil, with the realism of the SEM for
30 regional scale earthquake simulations from the fault to the site. To this end, the Domain Reduction Method (DRM)
31 introduced by (Bielak et al., 2003) is herein employed. The DRM formulation solves the problem of multi-scale
32 earthquake simulations by subdividing the fault-to-structure problem into two simpler ones, namely: problem I-
33 containing the structure and (hereafter solved using the FEM) and problem II- containing the fault, the regional
34 geological and surface topography, the sedimentary basin (hereafter solved using the SEM). In this work, the
35 coupling between the FEM code CAST3M and the SEM code SEM3D is presented, verified and compared to
36 conventional 1D deconvolution methods for SSI analysis. A seismic simulation is performed, considering a
37 simplified virtual nuclear reactor building, artificially positioned on the Cadarache site in South-Eastern France.
38 The applicability of the SEM3D-CAST3M coupling is demonstrated, for SSI analysis in linear elastic regime and
39 with a 50% reduction of the model dimensions. Moreover, the coupling captures the amplified response for the
40 nuclear structure due to the 3D spatially variable input field and to the resulting surface wave propagation in the
41 SEM-FEM SSI analysis.

42 **Keywords:** Nuclear reactor building; Soil-structure interaction; seismic analysis; weak coupling; Finite Element
43 Method; Spectral Element Method.

44 1. INTRODUCTION

45 In recent years, a continuous effort to establish improved earthquake engineering modeling techniques has been
46 made, in order to improve the constitutive behavior models, to characterize the geotechnical and material
47 parameters at stake, to model the site effects and the soil-structure interaction (SSI) with efficient boundary
48 conditions.

49 One of the largest effort aims at developing more efficient numerical tools to render the complexity of wave
50 propagation phenomena in the Earth's crust and its interaction with the built environment. Despite being the most
51 commonly used tool for earthquake engineering applications (e.g. Fares et al., 2019; Lysmer et al., 1975; Santisi
52 d'Avila and Lenti, 2012), the classical Finite Element Method (FEM) is rarely used for large-scale three-
53 dimensional (3D) seismic wave propagation analyses, due to the high number of integration points per minimum
54 wavelength, with consequent high computation costs and high performance strategies are to be used in this case
55 (e.g. Dupros et al., 2010). Regarding nonlinear large-scale FEM simulations, Ichimura et al., (2014) computed the
56 seismic response of a $2\text{km} \times 2\text{km} \times 0.1\text{km}$ domain representative of a city with 13,275 structures using 10.7
57 billion degrees of freedom (DOF). On the contrary, methods such as the Spectral Element Method (SEM) (Faccioli
58 et al., 1997; Komatitsch and Vilotte, 1998; Mercerat et al., 2006; Guidotti et al., 2012; Gatti et al., 2018) or the
59 Finite Difference Method (FDM) (e.g. Boore, 1972; Virieux, 1986; Moczo et al., 2004; Maeda et al., 2017) are
60 more popular for seismological and geophysical studies, due to their high accuracy and convenience for parallel
61 implementation.

62 There are many possible approaches to take into account the SSI and the validity of these approaches strongly
63 depends on the considered assumptions. Following the Niigataken-Chuetsu-Oki earthquake of July 16th 2007 in
64 Japan, several studies investigated the applied methodologies for SSI analysis (IAEA, 2011, 2009; Wang and
65 Rambach, 2013). Since then, efforts have been made to propose methodologies for seismic design and beyond-
66 design bases of nuclear facilities (Coleman et al., 2013, 2016; Lo Frano et al., 2010; Van Nguyen et al., 2020).
67 Still, at present, the site topography, soil properties, spatial heterogeneity, fault front, and geometry of sedimentary
68 site are neglected and often considered in the signal motion proposed for the site based on nuclear norms
69 recommendations. Hence, the influence of site effects is considered indirectly in assessing seismic ground motions
70 at sites of interest and directly in the modeling of soil horizontally homogeneous stratigraphy. However, studies
71 have shown that the geometry and characterization of the sedimentary basin have a crucial effect on the seismic
72 response (Manakou et al., 2010; Raptakis et al., 2000; Semblat et al., 2005). Koufoudi et al., 2015 investigated the
73 spatial variation of the seismic motion at the ground surface, for the same seismic event, and registered differences
74 in recordings of 15m apart. This variation can significantly modify the internal forces induced in the structures
75 (Santisi d'Avila et al., 2022; Sextos and Kappos, 2008). On the other hand, Wang and Feau (2017) investigated
76 the impact of the choice of the control point for the input motion on the SSI in a fragility analysis using a reduced
77 model. The obtained results recommend defining the input at the outcrop for conventional SSI analysis, contrary
78 to the French nuclear authorities recommendations to define the input motion at the free field (FF) (Gupta and
79 Lacoste, 2006).

80 In order to model the earthquake phenomenon at large scale, from the fault to the structural components, there are
81 few examples of end-to-end simulations (McCallen et al., 2021, 2020). On site-city interaction, Lu et al. (2018)
82 proposed a nonlinear numerical coupling in SEM. An alternative, more efficient procedure suggests a coupling
83 method for physic-based large-scale numerical wave propagation solution. Bielak et al. (2003), proposed a domain
84 reduction method (DRM) to reduce the large computational costs of an end-to-end simulation and to apply to an
85 equivalent force field calculated from the solution of a simpler larger domain model to a smaller local domain.
86 This two-step approach consists of applying a weak coupling between two models, offering a simple resolution of
87 wave propagation from fault to structure with an acceptable computational cost (e.g. Jeremic et al., 2009; Russo
88 et al., 2017; Abell et al., 2018; Luo et al., 2019; Zhang et al., 2020; McCallen et al., 2021; Wang et al., 2021;
89 Korres et al., 2022). Most recently, this approach was adopted for SSI analysis by Wang et al. (2021) using
90 potential wave formulation to create a spatially varying inclined seismic wavefield and by Zhang et al. (2020)
91 using SEM and FEM coupling.

92 In this article, the DRM methodology for SEM-FEM coupling, to perform a SSI analysis from the fault to the
93 structure is proposed and verified, offering easier implementation and performance compared to the strong
94 coupling approach. The latter, on the other hand, consists in simultaneously solving and communicating the two
95 divided problems at each time steps (Brun et al., 2021; Zuchowski et al., 2018).

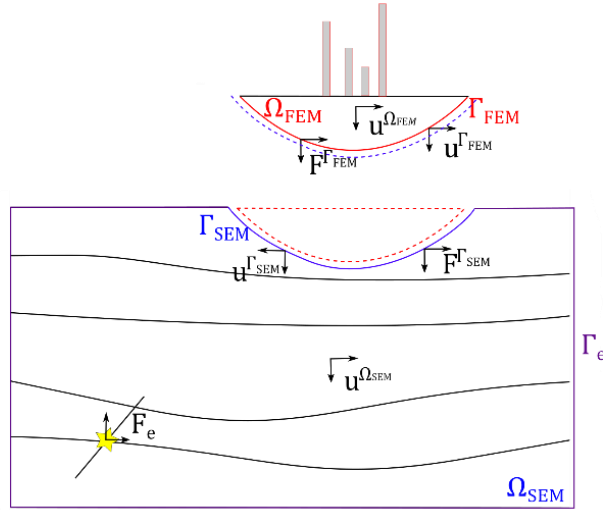
96 In the frame of the French research project SINAPS@ (Earthquake and Nuclear Installations: Ensuring and
97 Sustaining Safety (Berge-Thierry et al., 2017), we have applied the proposed SEM-FEM coupling on the CEA
98 (French Alternative Energies and Atomic Energy Commission) Cadarache site, located in the South-Eastern
99 France. We have also compared the SEM-FEM coupling analysis to the Full FEM analysis for SSI (one-step

100 approach for SSI analysis using FEM Saez et al., 2011) and Free Field (FF). Finally, the choice of the input motion
 101 implementation for the Full FEM analysis is discussed in a sensitivity study.

102 2. DOMAIN REDUCTION METHOD APPLIED FOR THE SEM-FEM DOMAINS COUPLING

103 The DRM method (Bielak et al., 2003) proposes a two-step analysis to solve the global problem of seismic wave
 104 propagation from the source to the site surface, which are often several tens or even hundreds of kilometers apart.
 105 Thus, the method divides the initial problem into two simpler ones (Figure 1). The first problem simulates the
 106 effects of the seismic source and wave propagation with a model that includes the source and the site domain by
 107 removing local effects, such as the site effect and the SSI (Ω_{SEM} in Figure 1). This domain is modeled using
 108 SEM3D, a 3D High-Performance SEM code for non-linear seismic wave propagation (CEA et al., 2017). The
 109 second problem (Ω_{FEM} in Figure 1) models the local site effects by considering a set of equivalent forces, derived
 110 from the first step, and imposed on the edges of the subdomain. This domain is modeled using Cast3M, a FEM
 111 code for structural and fluid mechanics (CEA, 2021). The discretized dynamic equilibrium equation of the wave
 112 propagation problem in the global assembled domain $\Omega = \Omega_{FEM} + \Omega_{SEM}$ is written in Equation (1), where \ddot{u} and
 113 u are the acceleration and displacement vectors, respectively, and F is the external loading vector. The matrices M
 114 and K represent the mass and stiffness matrices, respectively. The indices i , b and e denote the interior, the
 115 boundary and the exterior of the domain defined as the suffix.

$$\begin{aligned}
 & \begin{bmatrix} M_{ii}^{\Omega_{FEM}} & M_{ib}^{\Omega_{FEM}} & 0 \\ M_{bi}^{\Omega_{FEM}} & M_{bb}^{\Omega_{FEM}} + M_{bb}^{\Omega_{SEM}} & M_{be}^{\Omega_{SEM}} \\ 0 & M_{eb}^{\Omega_{SEM}} & M_{ee}^{\Omega_{SEM}} \end{bmatrix} \begin{bmatrix} \ddot{u}_i^{\Omega_{FEM}} \\ \ddot{u}_b^\Gamma \\ \ddot{u}_e^{\Omega_{SEM}} \end{bmatrix} \\
 + & \begin{bmatrix} K_{ii}^{\Omega_{FEM}} & K_{ib}^{\Omega_{FEM}} & 0 \\ K_{bi}^{\Omega_{FEM}} & K_{bb}^{\Omega_{FEM}} + K_{bb}^{\Omega_{SEM}} & K_{be}^{\Omega_{SEM}} \\ 0 & K_{eb}^{\Omega_{SEM}} & K_{ee}^{\Omega_{SEM}} \end{bmatrix} \begin{bmatrix} u_i^{\Omega_{FEM}} \\ u_b^\Gamma \\ u_e^{\Omega_{SEM}} \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ F_e \end{bmatrix} \quad (1)
 \end{aligned}$$



116
 117 *Figure 1. Overview of the forces and displacements applied to the boundaries of the domains Ω_{SEM} and Ω_{FEM} .*

118 In equation (1), the displacement is continuous at the boundaries Γ_{SEM} and Γ_{FEM} giving $u^\Gamma = u^{\Gamma_{SEM}} = u^{\Gamma_{FEM}}$ and
 119 $\ddot{u}^\Gamma = \ddot{u}^{\Gamma_{SEM}} = \ddot{u}^{\Gamma_{FEM}}$. Hence, Γ refers to the boundaries Γ_{SEM} or Γ_{FEM} . Moreover, according to the reciprocity
 120 theorem, the loading Γ_{FEM} is equal to the opposite of the forces calculated at Γ_{SEM} $F^{\Gamma_{FEM}} + F^{\Gamma_{SEM}} = 0$.

121 In order to solve the wave propagation problem in Ω_{FEM} , taking into account the effect of Ω_{SEM} , the equivalent
 122 force F_{eff} is calculated from the solution of the wave propagation in Ω_{SEM} as written in Equation (2). For sake of
 123 simplicity the full demonstration is not developed and readers are invited to refer to the paper of Bielak et al.,
 124 (2003) for a step by step solution.

$$F_{eff} = \begin{cases} 0 \\ -M_{be}^{\Omega_{SEM}} \ddot{u}_e^{\Omega_{SEM}} - K_{be}^{\Omega_{SEM}} u_e^{\Omega_{SEM}} \\ M_{eb}^{\Omega_{SEM}} \ddot{u}_b^\Gamma + K_{eb}^{\Omega_{SEM}} u_b^\Gamma \end{cases} \quad (2)$$

125 We can summarize the proposed coupling procedures in two steps:

- 126 - The first step consists of solving the wave propagation problem in SEM3D from the fault to the free
127 surface in Ω_{SEM} . The corresponding model considers the soil heterogeneity in x , y and z directions, the
128 topography of the medium, and the fault rupture mechanism. The displacements and constraints time
129 histories at the boundary Γ are logged.
- 130 - In the second step, the equivalent force F_{eff} hence calculated, according to Equation (2), is provided as
131 input loading for the wave propagation problem in Ω_{FEM} . The corresponding model considers the soil
132 heterogeneous in the vertical direction z and the constructed surface to take into account the SSI effect.

133 In this paper, the proposed coupling is restrained to the linear elastic regime.

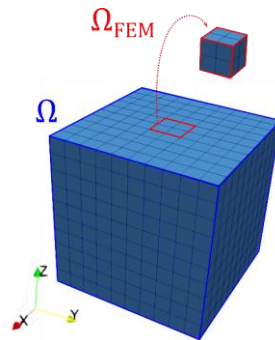
134 3. VERIFICATION OF THE PROPOSED SEM3D-CAST3M COUPLING

135 To be able to rely on the solution provided by the coupled computer codes, the quality of the solution needs to be
136 evaluated. Among developers, verification and validation procedures are used to benchmark the solution with
137 analytical or experimental outcomes (Schwer, 2009). In the field of soil dynamics, the analytical solution is not
138 available especially for elasto-plastic or nonlinear behavior of the soil and the experimental results are hard to
139 obtain. Hence, the V & V procedures are complicated to apply. However, there is also counter verification, where
140 the solution is benchmarked by using the results of a verified code or from a very fine discretization. This section
141 aims at verifying the employed procedure of coupling between both codes SEM3D and Cast3M, both validated
142 codes in soil and structural dynamics (Gatti, 2017; Touhami et al., 2022, 2022; Wang and Rambach, 2013),

143 Example 1: Counter verification of the coupling procedure with either Cast3M and SEM3D solutions

144 In order to verify the implementation of the DRM, the first example shows the comparison between a Full FEM
145 analysis (analysis for which the solution is calculated in the domain Ω) using the FEM code Cast3M and a FEM-
146 FEM analysis (the first FEM analysis solves the wave propagation in Ω and the second one in Ω_{FEM} , Figure 2)
147 using the same FEM code Cast3M. A homogeneous soil with a bulk density $\rho = 2000 \text{ kg/m}^3$, a shear wave
148 velocity $v_s = 300 \text{ m/s}$ and a body wave velocity $v_p = 700 \text{ m/s}$, is modeled. The soil domain Ω has larger
149 dimensions of $10\text{m} \times 10\text{m} \times 10\text{m}$. The domain Ω_{FEM} is totally embedded in the domain Ω , with which it shares
150 five boundary surfaces, and has a smaller dimension of $1\text{m} \times 1\text{m} \times 1\text{m}$ (Figure 2). A Ricker-type signal is applied
151 as a seismic input motion with a frequency equal to 5 Hz. The same time step $\Delta t = 0.01 \text{ s}$ is adopted for Full FEM
152 and FEM-FEM implicit analyses. When comparing the acceleration time histories registered at the center of the
153 top ground surface for both analyses (Figure 3), a match in time and frequency domains is obtained with a
154 difference in the peak ground acceleration (PGA) of less than 1%.

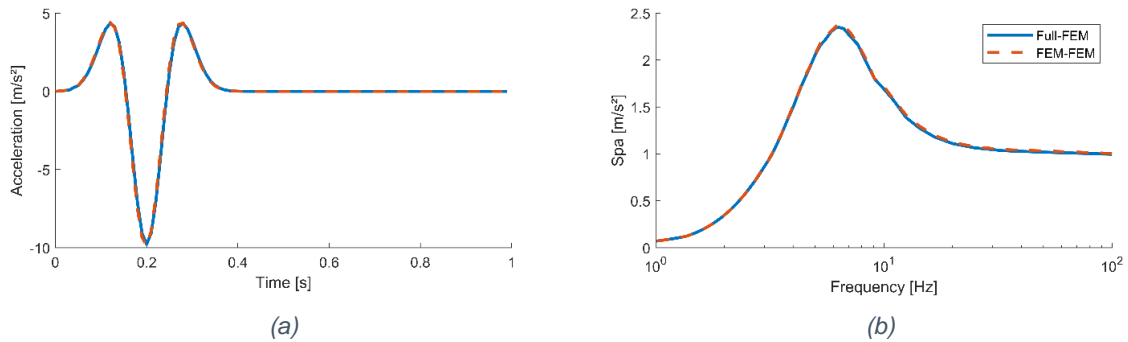
155 This same example is repeated to compare the Full SEM and the SEM-FEM coupling results. The input motion
156 for the Full SEM model in SEM3D is a sinusoidal point source of frequency equal to 5 Hz. When comparing the
157 acceleration time histories registered at the center of the top ground surface for both analyses (Figure 4), we obtain
158 a difference in PGA less than 8%, which may be due to the employed interpolation at the shared surfaces nodes
159 and the employed boundary conditions (BC). In fact, contrary to the previous Full FEM model, the Full SEM
160 model mesh does not conform to the mesh of the reduced FEM model. In addition, the soil boundaries (lateral and
161 at the base of the domain) damping used in Cast3M corresponds to a Lysmer-type absorbing layer (Lysmer and
162 Kuhlemeyer, 1969; Lysmer et al., 1975), whereas the one used in SEM3D is a Perfectly Matched Layer (PML)-
163 type absorbing layer applied (Festa and Vilotte, 2005). Finally, a Rayleigh numerical damping is used in Cast3M
164 according to (Clough and Penzien, 2003; Hughes, 1987), with a damping matrix in the form $C = \alpha M + \beta K$. The
165 Rayleigh damping is related to the inverse of the quality factor, according to the approximate relation $Q_s = Q_p =$
166 $v_s/10$, such that $Q^{-1} \approx 2\xi \approx \alpha/\omega + \beta\omega$ where $\omega = 2\pi f$ is the pulsation and ξ the damping factor (Semblat,
167 1997). In SEM3D on the other hand, no damping is applied.



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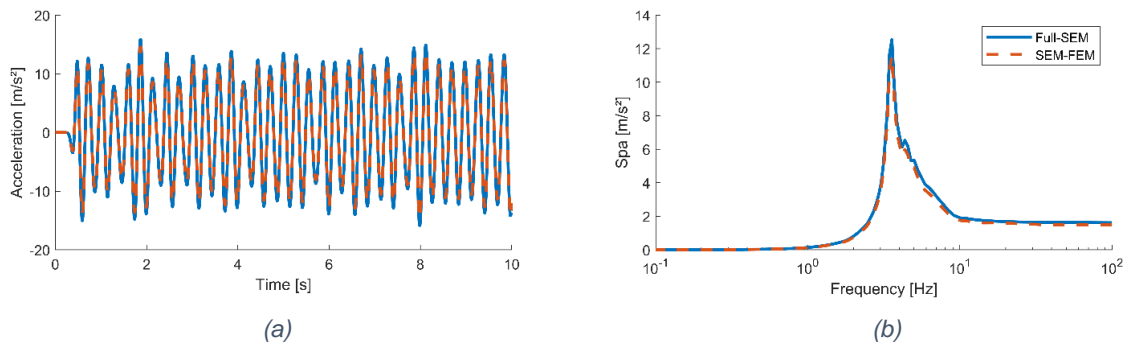
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Figure 2 Mesh of the Full FEM domain Ω and the reduced domain Ω_{FEM} .



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Figure 3 Comparison of the time history accelerations (a) and spectral responses (b) between Full FEM and FEM-FEM coupling analyses.



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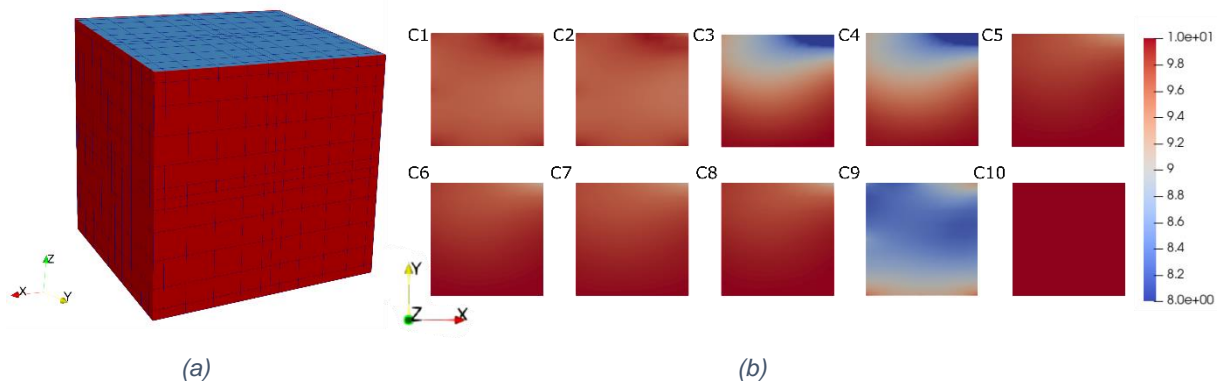
Figure 4 Comparison of the time history accelerations (a) and spectral responses (b) between Full SEM and SEM-FEM coupling analyses.

178 **Example 2: Counter verification of coupling procedure for PRENOLIN case study**

179 In order to verify the soil response solution, this example investigates the case study P1 in Régnier et al., 2016. A
180 homogeneous soil cubic medium of $10\text{m} \times 10\text{m} \times 10\text{m}$ is modeled with a density of $\rho = 1800 \text{ kg/m}^3$, a shear
181 wave velocity of $v_s = 300 \text{ m/s}$ and a body wave velocity of $v_p = 700 \text{ m/s}$. The solution is computed for a FF
182 assumption.

183 In order to quantify the differences between the results at each node of the top ground surface for the coupling
184 method and SEM method, Goodness of Fit (GoF) scores are calculated as proposed by Anderson, (2004). Figure
185 5-b shows the mapping of GoF scores at the top ground surface of Ω_{FEM} , where the results are compared according
186 to 9 criteria with a scoring scale ranging from 0 (null fit) to 10 (excellent fit): Arias duration (C1), energy duration
187 (C2), Arias intensity (C3), energy integral (C4), peak ground acceleration (C5), peak ground velocity (C6), peak
188 ground displacement (C7), response spectrum (C8), Fourier spectrum (C9), over the band frequency 0.1-20Hz,
189 and cross correlation ratio (C10). The lowest score is obtained for the Fourier spectrum $C9 = 8.1$, which
190 corresponds however to an excellent fit according to Anderson, (2004). Hence, the coupling procedure is
191 considered as verified.

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Figure 5 Mesh of the FEM model (a), mapping of the Goodness of Fit scores at the top ground surface of Ω_{FEM} (b).

197 4. APPLICATION TO THE CADARACHE SITE SIMULATION

198 The Cadarache site is a CEA center located in the South-Eastern France, with several basic nuclear facilities. It
199 has been chosen for this study, as it is a well-instrumented site with many data on the 3D geological model,
200 including the Middle Durance Fault, the sedimentary basin and the seismic activity. A careful seismic hazard
201 evaluation exists for this zone due to safety requirements for the existing nuclear facilities and the proper
202 functioning of their equipment against earthquakes.

203 In this study, the SEM3D model is used to solve the wave propagation from the fault to the free surface and to
204 provide the displacement field at each node of the interface with the Cast3M model. In a second step, the force
205 fields are calculated, according to the DRM (Equation (2)). The wave propagation is, hence, calculated from the
206 near soil to the structure by solving the dynamic equilibrium equation of the assembly.

207 This procedure allows for the SSI effect while providing more accurate estimation of site effects, including the
208 topography, the spatial variation of soil properties, and the sedimentary basin geometry, due to the variable force
209 field loading.

210 Hereafter, we present details of the SEM3D and Cast3M models for the Cadarache site and we discuss the impact
211 of the way to impose the input motion for the Full FEM analysis.

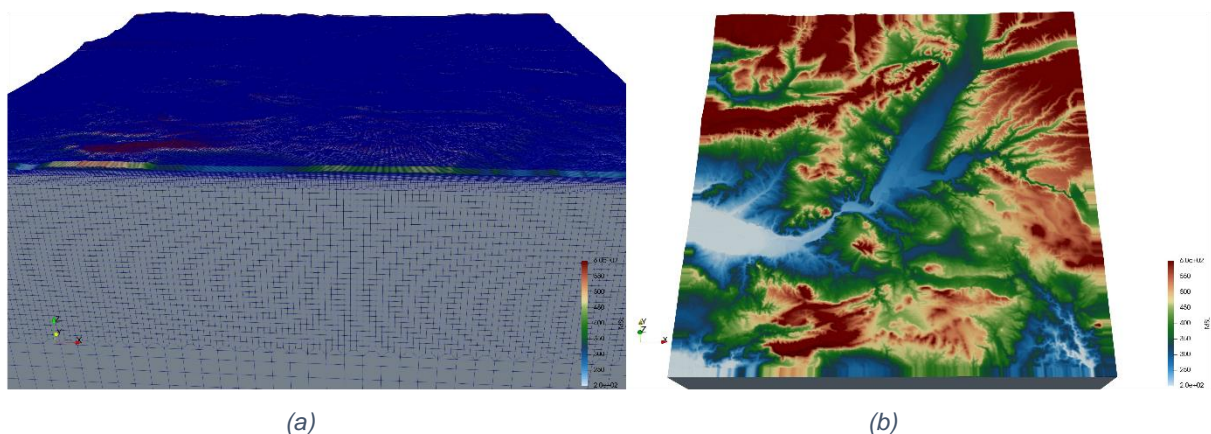
212 SEM3D MODEL DESCRIPTION

213 The numerical earthquake model is implemented in SEM3D. Using the same code, Castro-Cruz et al. (2021) have
214 proposed a hybrid approximation between a physic-based simulation and the Empirical Green function method for
215 the Cadarache site. The study in this paper uses the same geological and source information. The model mesh for
216 the Cadarache region contains ca. 5.9 million linear hexahedral elements for a total of 5.9 billion DOFs. PML
217 absorbing boundary conditions are applied at the model boundaries.

218 The mesh solves with high accuracy the wave-propagation problem up to a maximum frequency of 8Hz. The
219 elements have a minimal length of 105.8m and 45.7m inside the basin. Additionally, by using transition elements
220 (Figure 6), with irregular hexahedrons, the elements size is larger at deeper parts where soil mechanical properties
221 are stronger than those for the shallow layers.

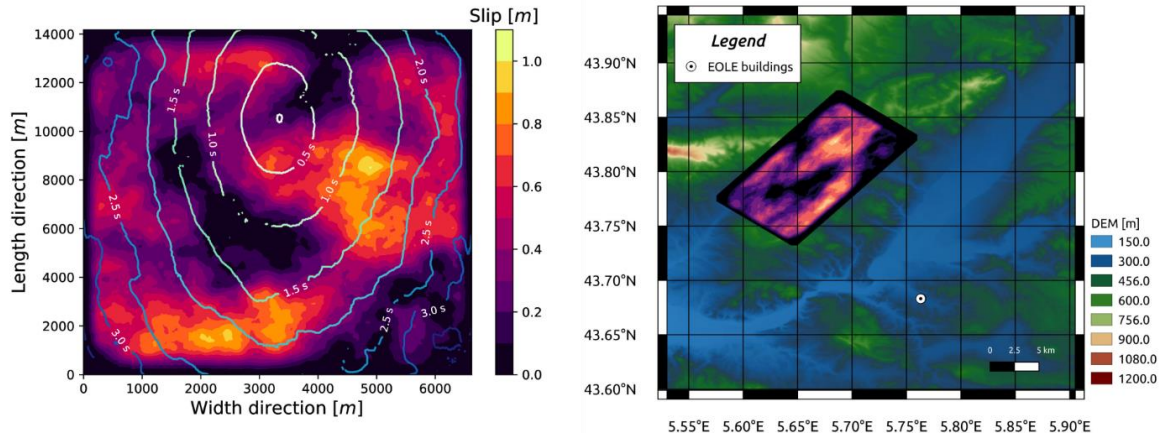
222 Figure 7 presents the kinematic source introduced in the model, as a double couple mechanism by displacement
223 imposition. This source starts around the center of the fault, and the rupture process takes around 3.8s. The
224 maximal slip in the rupture reaches 1.1m, and the maximal released seismic moment by second is $6 \times 10^{17} \text{ Nm/s}$.
225 The total seismic moment is equal to $1.04 \times 10^{18} (M_w = 6.0)$, which is the same magnitude as other studies in
226 the area (e.g. Dujardin et al., 2020), and corresponds to the maximal reported magnitude in the instrument database
227 of France (Baroux et al., 2003).

228 The Cadarache sedimentary basin model comes from the preliminary works of Guyonnet-Benaize, (2011), and has
229 been regularly updated by the Commissariat à l'énergie atomique et aux énergies alternatives (CEA) by integrating
230 the new data available. The basin model is constituted from thousands of boreholes investigations and numerous
231 H/V measurements. Further details are found in Perron et al., (2018). Castro-Cruz et al. (2021) present the
232 validation and a deeper description of the implemented geology in this model which is also used in this study. The
233 integration between SEM and FEM allows making specific analyses on the SEM simulation. Using SEM, we
234 predict the results at all the mesh nodes in FEM (Figure 8). The SEM model is much bigger, allowing for regional
235 analysis, and the FEM model contributes to a more refined structural analysis.



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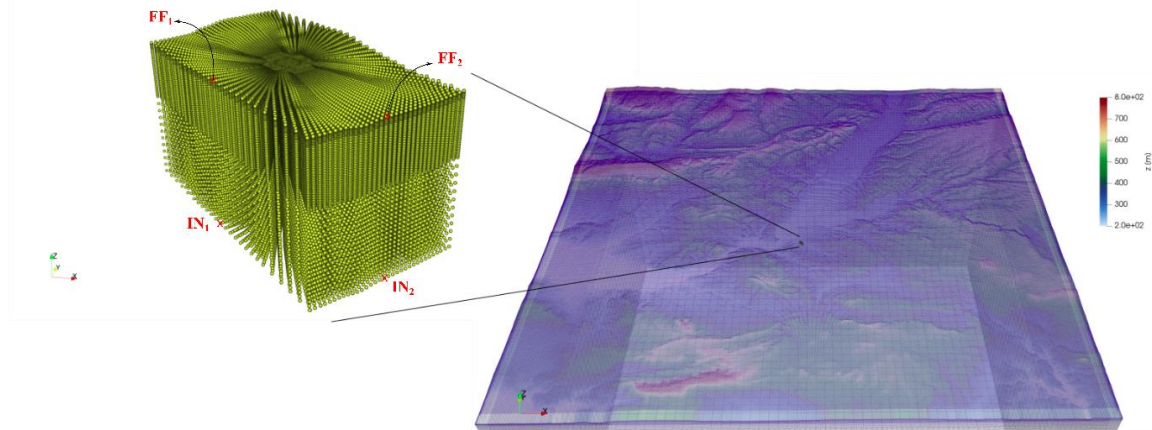
Figure 6 Mesh of the Cadarache site in SEM3D (a) and mapping of the geological model (b).



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240 Figure 7 Seismic source for an $M_w = 6$ earthquake, outline of the static slip distribution (a), Map of the region of
 241 interest, including the extended fault (b).

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Figure 8 Zoom on the receivers at which the results are predicted in the SEM3D model

245 CAST3M MODEL DESCRIPTION

246 *The soil model*

247 This study performs the coupling method using Cast3M. The soil domain modeled in Cast3M has a dimension of
 248 $250\text{m} \times 500\text{m} \times 250\text{m}$ in x , y and z directions respectively and is located inside the basin region (Figure 9). The
 249 soil domain is rotated 45° around the z axis to match the corresponding configuration in the Cadarache site (Figure
 250 9-a and b). The model uses a soil stratigraphy in agreement with the geological description of Cadarache site
 251 (Appendix A), with a horizontal homogenization of the soil stratigraphy to satisfy the vertical propagation
 252 assumption considered in the solution of the dynamic equilibrium equation in Cast3M. The adopted stratigraphy
 253 considers thin layers of $\Delta h \leq 5\text{ m}$ in the basin.

254 To realize SSI analysis higher frequency resolution is essential for engineering applications (Baker et al., 2014).
 255 The objective of the study herein is to extend the resolution to frequencies relevant to structures. Moreover, higher
 256 frequencies are observed as a result of wave scattering due to heterogeneous velocities in soil (Imperator and Mai,
 257 2015; Takemura et al., 2015). Hence the maximum frequency of interest in this study is set to $f = 20\text{ Hz}$. The soil
 258 model is meshed by hexahedral elements with quadratic interpolation for the displacement DOFs. The minimum
 259 number of elements is defined by the analytical relation $pfh/2v_s$, where h is the thickness of the soil layer and
 260 $p = 10$ is the minimum number of points per wavelength and v_s is the shear wave velocity of the soil layer (Fares,
 261 2018). At the mesh boundaries, the model has a Lysmer-like absorbing condition to ensure the attenuation of wave
 262 propagating towards infinity. The Newmark integration scheme is adopted for the time resolution of the dynamic
 263 equilibrium. A Rayleigh damping model is used, to introduce frequency dependent attenuation, and is calibrated
 264 on frequencies $f_1 = 2\text{ Hz}$ and $f_2 = 20\text{ Hz}$. The damping matrix is estimated by: $C = \alpha M + \beta K$ where $\alpha =$
 265 $2\zeta\omega_1\omega_2/(\omega_1 + \omega_2)$, $\beta = 2\zeta/(\omega_1 + \omega_2)$ and ω_1 and ω_2 are the angular frequencies associated with the
 266 frequencies f_1 and f_2 respectively. The simulations are performed in the time domain assuming linear behavior

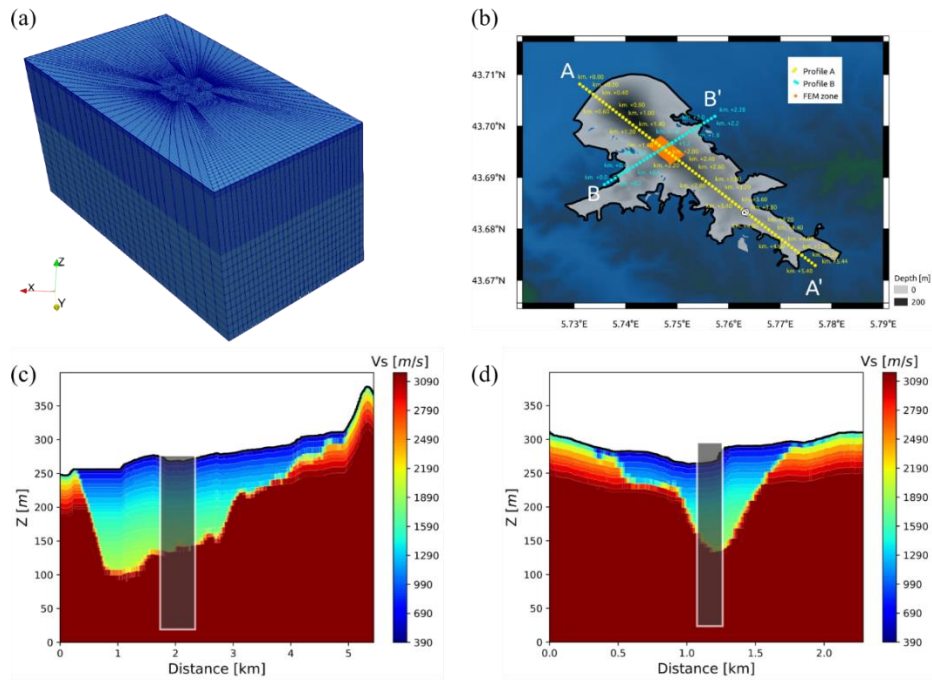


Figure 9 The soil mesh (a), the basin site (b), section AA' (c) and BB' (d) of the basin

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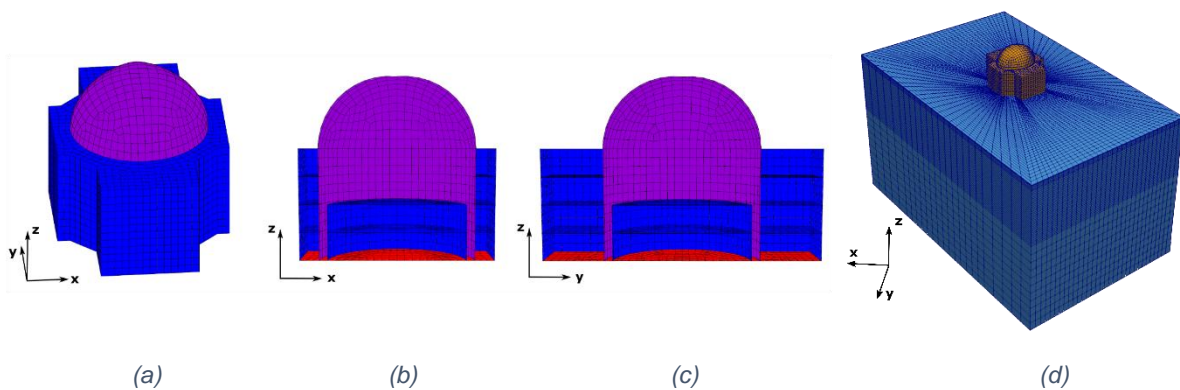
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269 for both the structure and the soil with a time step equal to $\Delta t = 1/(4 \times f_c) = 0.00125$ s, where f_c represents the
 270 signal cutoff frequency.

271 *The structure model*

272 A virtual structure - corresponding to a simplified Pressurized Water Reactor (PWR)-type building - is positioned
 273 on the Cadarache site. The used FEM model represents the detailed structure with median properties and it
 274 incorporates all the buildings of the nuclear island, i.e. the containment, the internal structures, as well as the
 275 backup buildings and the common basement.

276 The structure has two planes of symmetry along (Oxz) and (Oyz) (Figure 10). It is composed of a 3 m thick
 277 foundation with dimensions $70\text{m} \times 100\text{m} \times 65\text{m}$ and is modeled by 2D shell elements: the dome by 3-node
 278 shell elements (COQ3), the walls and floors by 4-node shell elements (COQ4) and the invert by 8-node shell
 279 elements (COQ8). The mechanical characteristics of the material (homogenized) composing the structure are
 280 presented in Appendix A. The frequency of the first natural mode in bending the structure is 3.8 Hz. The
 281 structure is connected to the ground by a rigid link at each node.



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(a)

(b)

(c)

(d)

284 Figure 10 The simplified reactor building mesh: (a) 3D view, (b) plan view (Oxz) and (c) plan view (Oyz) and the
 285 SSI model mesh (d).

286 *Investigation on input motions*

287 The choice of the input signal is critical for the accuracy of the Full FEM analysis in a nearfield domain. In the
 288 context of SEM-FEM coupling, the method employs a spatially variable field at the reduced FEM domain
 289 boundaries. However, in the Full FEM analysis, the method uses a uniform plane wave loading according to the

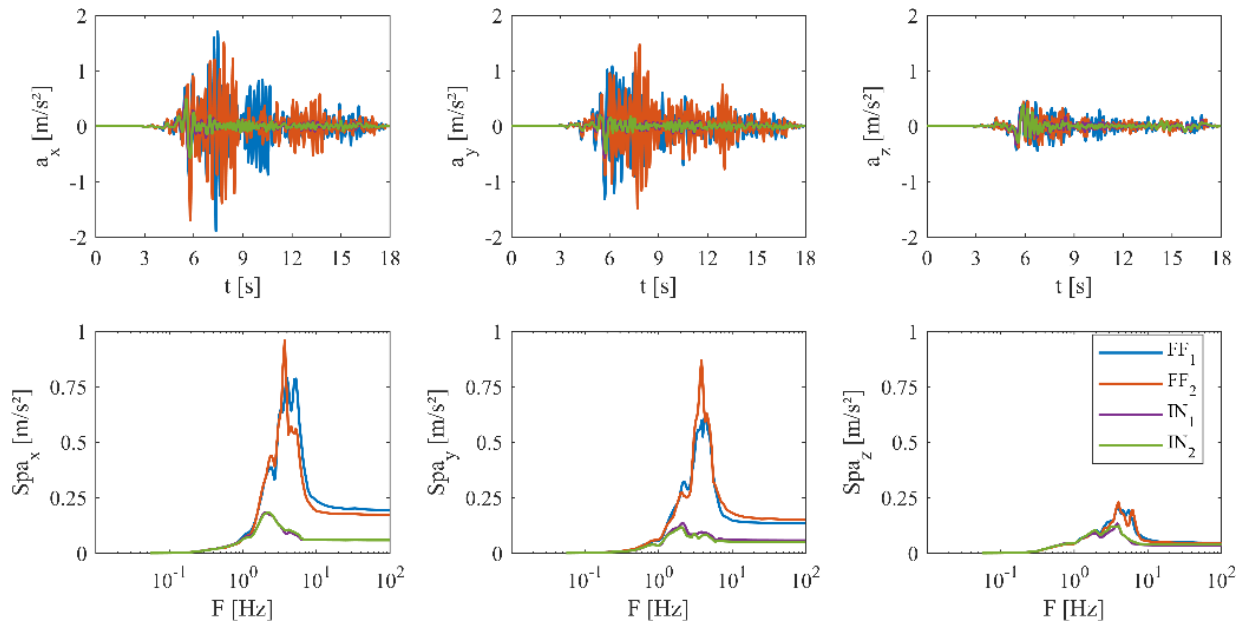
290 employed absorbing boundary condition (Lysmer and Kuhlemeyer, 1969). To generate this input field for the full
 291 FEM analysis, we perform a deconvolution process from a FF input signal and a convolution process from an
 292 inside bedrock (IN) input signal. The deconvolution is solved in the time domain with a transition to the frequency
 293 domain by the computation of the transfer functions. It consists of calculating over a column soil profile the
 294 dynamic response of each node to an impulsive horizontal input force applied at the bottom according to the
 295 Rayleigh damping model. The convolution process is identical to the deconvolution one, but the first one predicts
 296 the FF starting with a bedrock record and the other predicts the bedrock motion from a FF record. For simplicity,
 297 the term deconvolution is adopted hereafter for both processes unless specified.

298 In this study, 4 different input motions are investigated: two registered at the FF (FF_1 and FF_2) and two others at
 299 the corresponding bedrock (IN_1 and IN_2) (Figure 8 and Figure 11). Figure 12 shows a comparison between the
 300 deconvolution and the Full SEM results in terms of normalized acceleration time histories in the soil profile for
 301 different depths. The normalization is done according to the maximum between the maximum of absolute
 302 accelerations $\max|a_{z=h}^{FullSEM}|$ and $\max|a_{z=h}^{deconvolution}|$ at each depth h . A quantification of the differences is
 303 calculated in Table 1 computing the mean GoF coefficients at each node n in the soil profile between Full SEM
 304 and deconvolution time history acceleration results for x , y and z directions ($\mu_{x,y,z}(C1) = \mu$). The obtained results
 305 for the deconvolution of the FF input signals show an excellent score for all coefficients and a good score only for
 306 the Fourier spectrum coefficient C9. However, the results obtained for the convolution of bedrock input motions
 307 show less accuracy to reproduce the FF signal and register good results for all coefficients and very poor results
 308 for the Arias intensity and Fourier spectrum coefficients C8 and C9 respectively. This result may be due to the
 309 content of the FF input motions impacted by the wave propagation in the stratified soil profile. On the other hand,
 310 the convolution of the bedrock input motions predict amplified FF signals.

311 *Table 1 Mean GoF coefficients on results at each node in the soil profile between Full SEM and deconvolution /*
 312 *convolution time history acceleration results for x , y and z directions.*

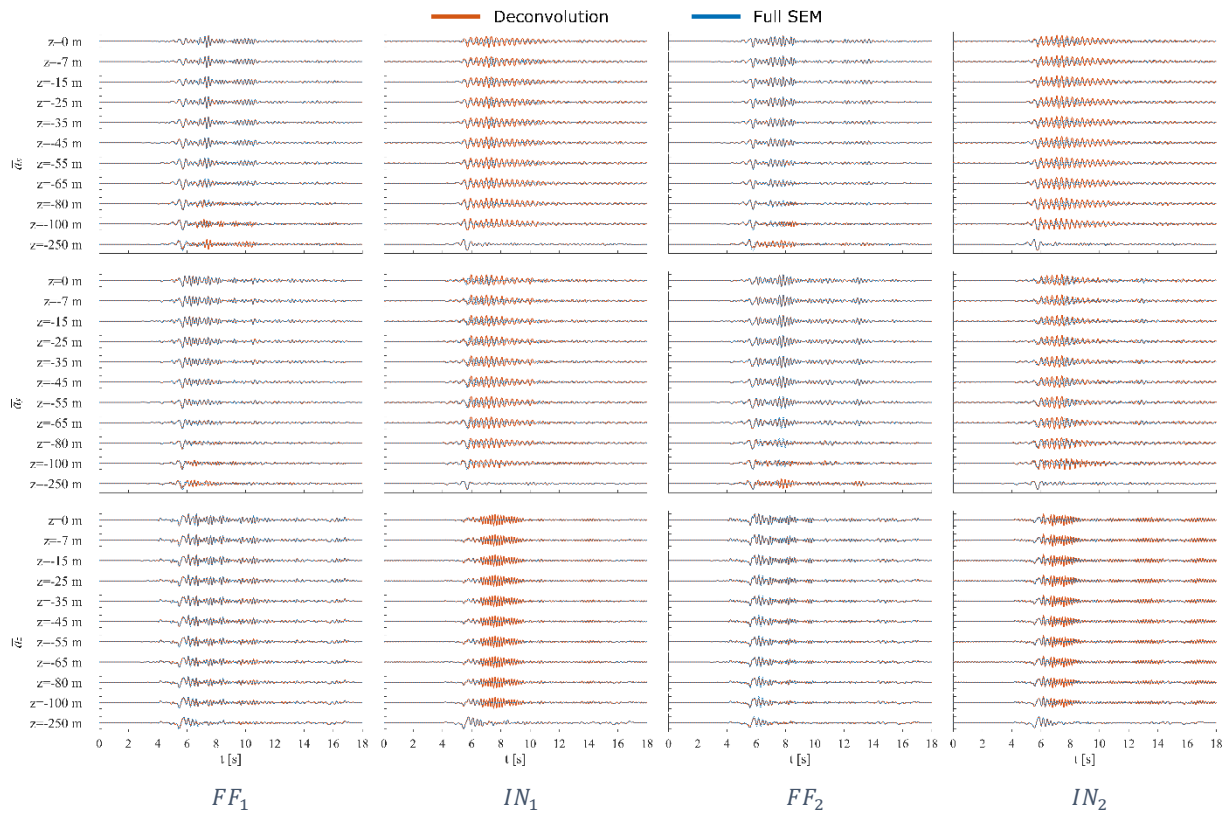
	$\mu_{x,y,z}(C1)$	$\mu_{x,y,z}(C2)$	$\mu_{x,y,z}(C3)$	$\mu_{x,y,z}(C4)$	$\mu_{x,y,z}(C5)$	$\mu_{x,y,z}(C6)$	$\mu_{x,y,z}(C7)$	$\mu_{x,y,z}(C8)$	$\mu_{x,y,z}(C9)$	$\mu_{x,y,z}(C10)$
FF_1	9.17	9.80	9.76	9.95	9.93	9.98	10.00	9.72	6.50	9.99
IN_1	7.15	7.09	0.47	3.80	5.90	9.25	7.08	6.09	0.43	9.15
FF_2	9.14	9.73	9.06	9.91	9.67	9.97	10.00	9.50	5.66	9.99
IN_2	7.45	7.48	0.77	4.50	7.65	9.58	5.54	7.45	0.43	9.44

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315 *Figure 11 Time history accelerations of the studied input signals (top) and their corresponding spectral responses*
 316 *(bottom) in x , y and z directions.*

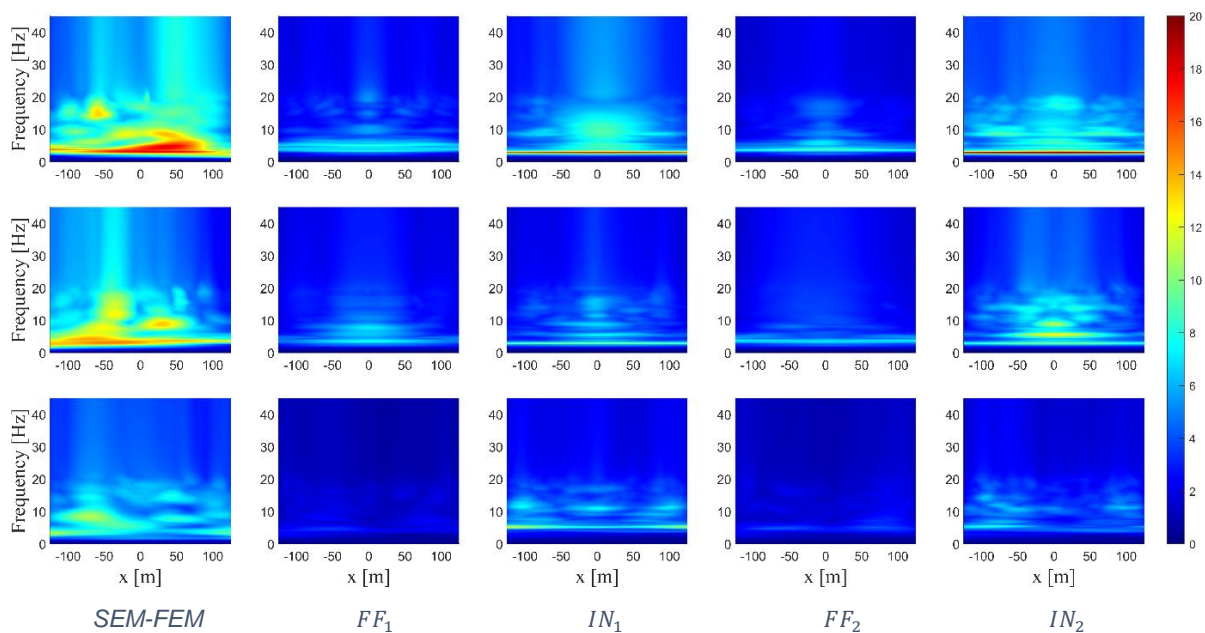


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319 *Figure 12 Comparison of the normalized time history accelerations along the soil profile depths for the Full SEM*
320 *and the deconvolution / convolution results in x, y and z direction from top to bottom respectively.*

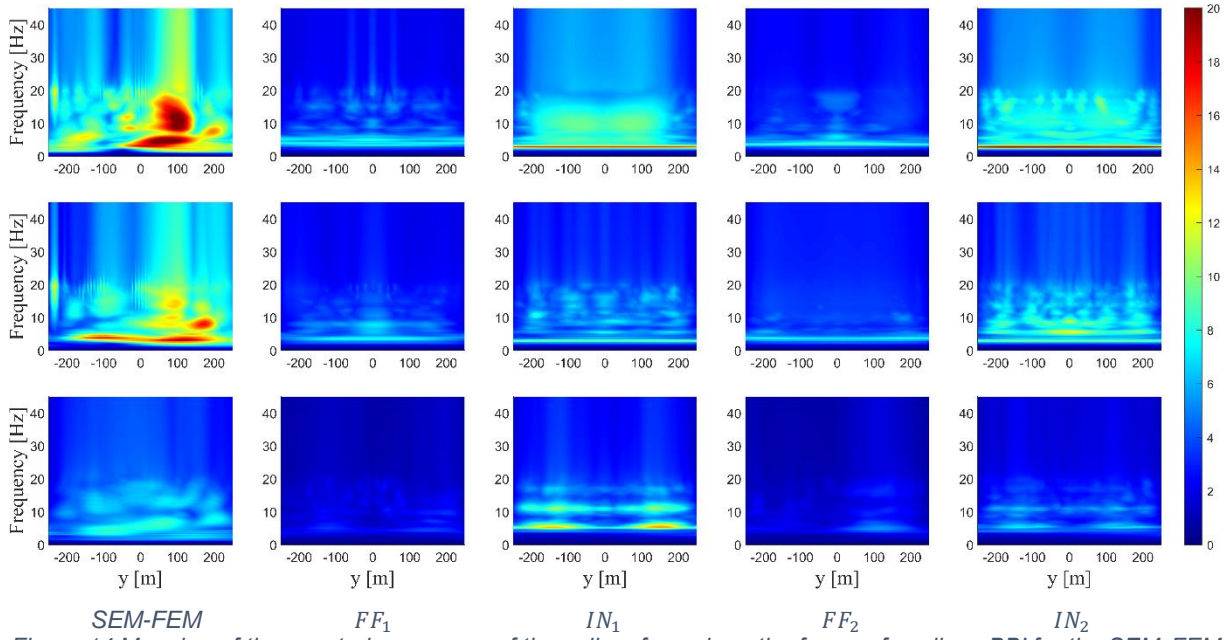
321 **FREE FIELD ANALYSIS**

322 This section presents the study of the choice in the input signal considering the site effects related to the
323 topography, the spatial heterogeneity of the mechanical properties of the soil, and the surface waves. For this
324 purpose, the Cadarache site is modeled in a 3D soil domain, as described previously. Contrary to the deconvolution
325 procedure, the Full FEM and SEM-FEM analyses solve the dynamic equilibrium equation for the wave
326 propagation in the 3D soil domain. Figure 13 and Figure 14 show a comparison between the mappings of the
327 spectral responses of the soil surface along with the free surface lines AA' and BB' respectively (Appendix B,
328 Figure B-1) for the SEM-FEM, and Full FEM analyses considering the input motions FF_1, IN_1, FF_2 and IN_2 .



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331 *Figure 13 Mapping of the spectral responses of the soil surface along the free surface lines AA' for the SEM-FEM*
332 *coupling and Full FEM analyses considering the input motions F_1, IN_1, FF_2 and IN_2 .*



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Figure 14 Mapping of the spectral responses of the soil surface along the free surface lines BB' for the SEM-FEM coupling and Full FEM analyses considering the input motions FF_1, IN_1, FF_2 and IN_2 .

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The Full FEM analysis results underestimate the soil response, compared to SEM-FEM analysis. In fact, the SEM-FEM analysis allows the surface wave propagation due to the spatially variable input field. Hence, it allows a better estimation of the site effects in the soil. Although the FF input signals lead to satisfying results for the deconvolution analysis, in a Full FEM 3D wave propagation approach, the bedrock input signals perform better. According to the results obtained for input motions IN_1 and IN_2 in Figure 13 and Figure 14, the predominant frequency and its associated amplification correspond to the solution in SEM-FEM coupling. Whereas results obtained for input motions FF_1 and FF_2 show a slightly higher frequency with an attenuated energy.

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SOIL-STRUCTURE INTERACTION

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Modeling the wave propagation in a Full FEM 3D domain of reasonable size requires introducing boundary conditions chosen with care. This attention allows the reproduction of the diffracted wave field at infinity and limits the parasitic edge reflections (Snell's law) that can strongly degrade the solution. This is why it is not possible to consider Dirichlet type boundary conditions (i.e. imposed displacements), unless the boundary is moved away considerably, at the cost of significantly increased computation time and memory consumption, due to the increase in the size of the mesh and the number of associated DOFs.

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In the case of SSI, it is preferable to use an absorbing boundary condition, which allows reproducing the far field solution (Hudson et al., 1994). Nevertheless, this type of boundary generally requires the structure to be moved away from the boundaries (lateral and at the base of the domain), in order to ensure a normal incidence of the waves at the edges, otherwise, the absorption conditions will be strongly degraded. Thus, the condition proposed by Lysmer and Kuhlemeyer (1969) pioneers in the study of the SSI, requires, for example, to move the boundaries away from the structure by a distance of $5 \times r$ from the center of its foundation, where r represents the radius of the equivalent circular foundation. All these limitations and conditions do not apply in the case of a SEM-FEM calculation.

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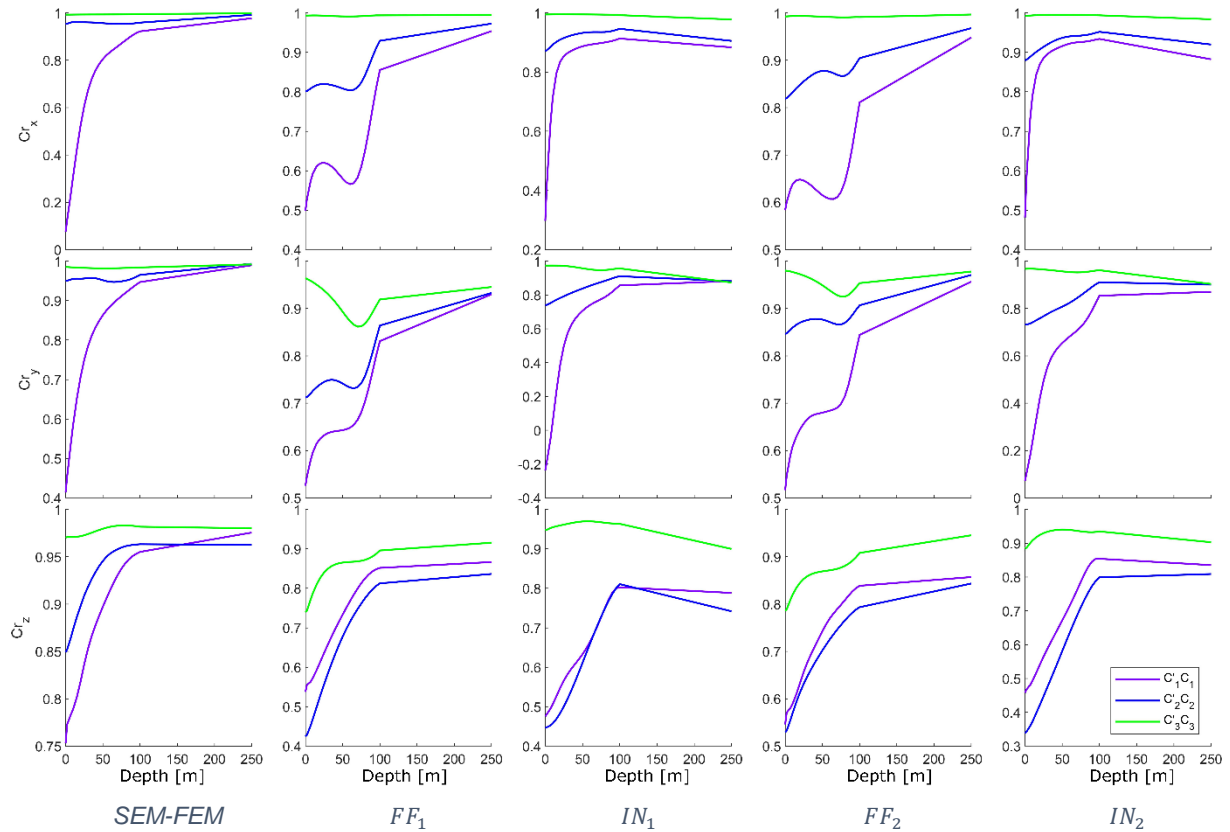
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Figure 15 shows a comparison of the recorded accelerations in x, y and z -directions, in terms of correlation (C_r between SSI and FF results, for the soil profiles C'_1C_1, C'_2C_2 and C'_3C_3 (see Appendix B, Figure B-1), for the SEM-FEM coupling and Full FEM analyses considering the input motions FF_1, IN_1, FF_2 and IN_2 . The correlation C_r is calculated according to the Equation (3) where n is the number of time steps, a_i is the corresponding acceleration for the time step index i , μ_a and σ_a are the mean and standard deviation of the acceleration a , respectively (Fisher, 1992). This correlation coefficient measures the linear dependence between the acceleration responses in the SSI and FF analysis and quantifies the SSI effect in the soil. A correlation $C_r = 1$ corresponds to results in the soil unaffected by the presence of the structure.

$$C_r = \frac{1}{n-1} \sum_{i=1}^n \left(\frac{a_{i,FF} - \mu_{a_{FF}}}{\sigma_{a_{FF}}} \right) \left(\frac{a_{i,SSI} - \mu_{a_{SSI}}}{\sigma_{a_{SSI}}} \right) \quad (3)$$



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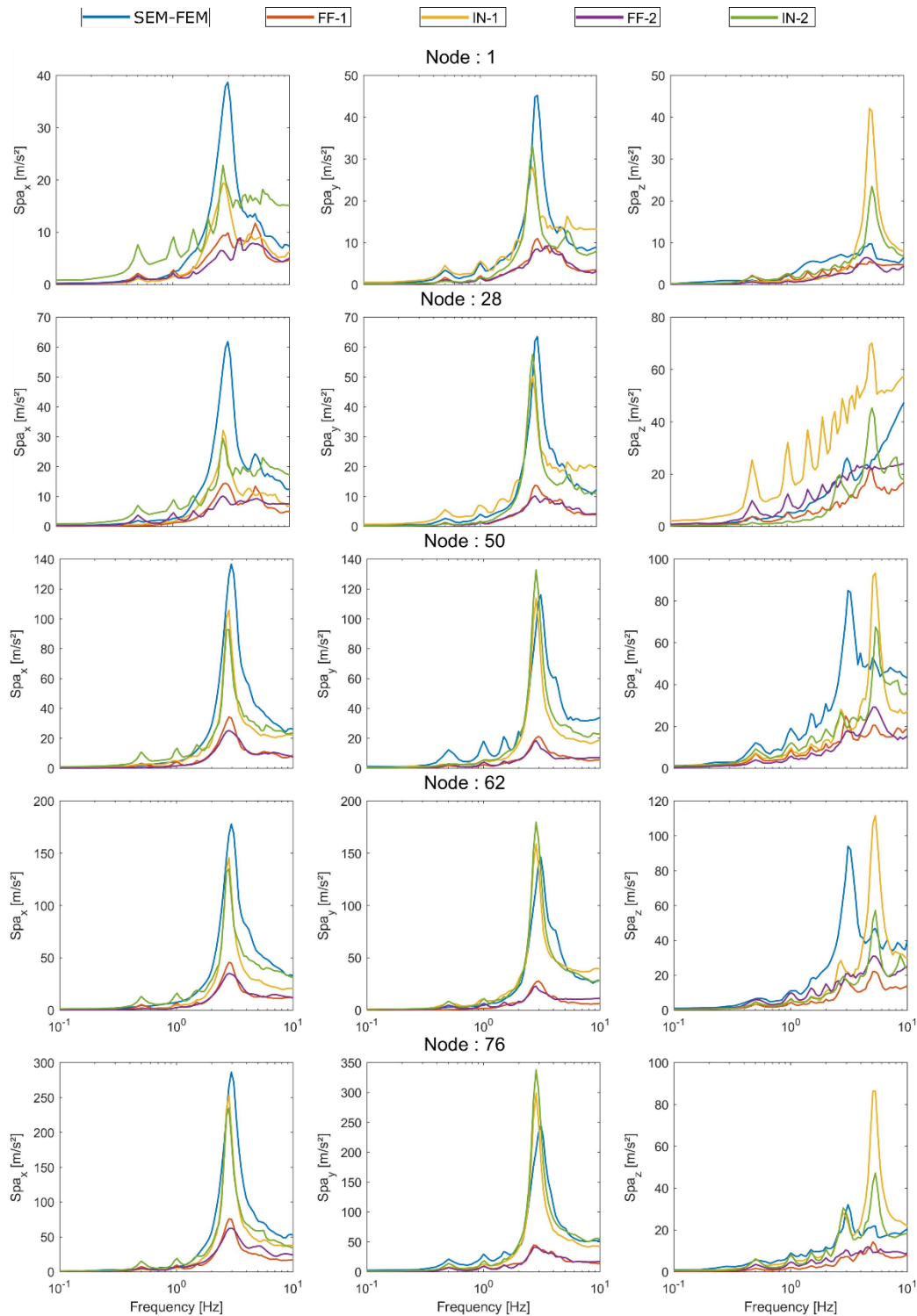
369 *Figure 15 Correlation between SSI and FF results along the line profiles $C_1^1C_1^1$, $C_2^2C_2^2$ and $C_3^3C_3^3$ for the SEM-FEM*
 370 *coupling and Full FEM analyses considering the input motions FF_1 , IN_1 , FF_2 and IN_2 .*

371 The results confirm that the domain dimensions considered for the Full FEM SSI calculation, and suggested by
 372 Lysmer and Kuhlemeyer (1969), is sufficiently large to satisfy the vertically incident wave and the FF conditions
 373 (correlation close to 1 at the base of the model $z = 250m$ and for $C_3^3C_3^3$). Nevertheless, it is admissible to reduce
 374 the domain size to at least 50% for a SEM-FEM SSI analysis coupled by the DRM. On the other hand, results also
 375 show the influence of the structure on the soil dynamic response, and hence the soil on the structure, depending
 376 on the analysis type and the input motion choice. In fact, for the SEM-FEM analysis, SSI is very important at the
 377 ground surface and decreases smoothly with depth. The SSI effect is observed for the Full FEM analysis with IN
 378 input motions. However, for the Full FEM analysis with FF input motions, the SSI effect is weaker in the soil and
 379 variable with depth.

380 The dynamic response of the Reactor Pressure Vessel (RPV) structure is examined for SEM-FEM and Full FEM
 381 analyses and presented in Figure 16. The comparison between the time history accelerations at different nodes in
 382 the structure (Appendix B, Figure B-2) for the SEM-FEM and Full FEM analyses show that the Full FEM analysis
 383 results underestimate, in most cases, the response of the structure. A shift of the structure response to lower
 384 frequencies is noticed for all Full FEM analyses. The results obtained with IN input motions tends to converge to
 385 the SEM-FEM results for higher elevations in structure and present better estimation of the structure response
 386 compared to analysis using FF input motions. Finally, the SEM-FEM analysis presents a physic-based solution for
 387 SSI analysis considering site effect and variable field input motion. The results show an amplification of the
 388 dynamic responses of the soil and the structure due to the additional site effect captured in the SEM-FEM coupling.

389 **5. CONCLUSIONS AND DISCUSSION**

390 In this article, the domain reduction method (DRM) is applied for the coupling between Spectral (SEM) and Finite
 391 Element (FEM) Methods, used to solve the wave propagation problem from the fault to the structure. The present
 392 study of SEM-FEM coupling leads to a more accurate solution w.r.t the SSI analysis, compared to the case of
 393 using a single (full FEM or SEM) method. The coupling method is implemented in Cast3M FEM code (CEA,
 394 2021), and counter-verified using Full FEM (Cast3M) and Full SEM (SEM3D, CEA et al., 2017) analyses. The
 395 verification analysis of the coupling using SEM3D and CAST3M codes gives satisfactory results and suggests to
 396 use the same time steps and conforming meshes for optimal results. In addition, satisfactory results are obtained
 397 for interpolated SEM elements corresponding to the FEM elements.



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399 *Figure 16 Comparison of the structure response spectra in x, y and z directions in a SSI analysis for the SEM-*
 400 *FEM coupling and Full FEM analyses considering the input motions FF_1, IN_1, FF_2 and IN_2 .*

401 The SEM-FEM coupling is adopted for the Cadarache site, located in the South-Eastern France. The SEM model
 402 considers the geology complexity of the site including the basin, the fault front and the topography. The FEM
 403 model considers the near soil, assumed to be horizontally stratified, and the nuclear structure corresponding to a
 404 simplified virtual PWR type building used for a SSI analysis.

405 A first analysis allows to compare the influence of two methods classically used to define the incident loading
 406 applied at the boundaries of the near-ground domain: (1) the approach consisting in deconvolving a free field (FF)
 407 accelerogram recorded at the ground surface of a soil column extracted from the site; (2) the approach consisting
 408 in convoluting a signal recorded at the base of a soil column (rock or other) also extracted from the site. For this

409 analysis, two FF input motions and two IN (inside bedrock) input motions are considered. The results obtained by
410 these two approaches show higher accuracy for the deconvolution of the FF input motions regarding the results
411 obtained in a Full SEM analysis. On the other hand, the results obtained by the convolution of IN input motions
412 are amplified and can be considered as conservative in an engineering point of view.

413 The four wavefield calculated by the deconvolution of FF and convolution of IN input motions are nevertheless
414 used here, for the 3D wave propagation in FF and SSI analyses. The results point out the effect of 3D wave
415 propagation comparing to the 1D deconvolution/convolution approach. Results obtained for a FF analysis with
416 wavefield calculated from the convolution of IN input motions show more accurate rendering of the frequency
417 content and amplification comparing to those with wavefields calculated from the convolution of FF input motions.
418 This result highlights the known differences between 1D and 3D wave propagation analyses. Moreover, the SEM-
419 FEM analysis shows amplified results compared to all four Full FEM analyses as well as surface waves
420 propagation. This is due to the additional site effect considering the topography and spatially variable field input
421 applied at all five surface boundaries in the coupling analysis.

422 The SSI analysis considers a simplified PWR-type building virtually located on the CEA Cadarache site. The
423 results show that the size of the soil domain to be considered in a SEM-FEM coupling analysis using the DRM
424 can be reduced by 50% compared to a conventional Full FEM model size recommended by Lysmer and
425 Kuhlemeyer (1969) to ensure an optimal absorbing boundary condition. A regular dissipation of the SSI effect as
426 a function of depth and a concentration of these effects near the structure is observed in the cases of SEM-FEM
427 coupling and Full FEM analyses with the IN input motions. The structure responses are amplified in SEM-FEM
428 coupling analysis due to the additional site effect captured in this analysis.

429 The DRM method for SSI from the fault to the structure provides a reliable tool for a better understanding of the
430 response of the structure in its environment. It overcomes the limitation of the SEM to model structural elements
431 and the Full FEM to model the wave propagation from the fault, with reasonable computation resources. However,
432 despite the advantages of the coupling some limitations of the coupled codes remains : as the limitation in the
433 maximum frequency considered in SEM3D or the limitation of Cast3M to model local heterogeneity. On the other
434 hand, the coupling method is only applicable in the elastic domain as it considers the superposition theorem.
435 Nonetheless, local nonlinearities of materials can be considered in the reduced FEM domain. Further studies are
436 necessary to define the applicability of the DRM method considering nonlinear material behavior.

437 Finally, the DRM's SEM-FEM coupling has proved to be a more realistic and physic-based solution for SSI
438 analysis, taking into account the geometric and mechanical heterogeneities of the site geology in the variable input
439 force field. However, access to geological data, seismological data, and numerical resources is mandatory for the
440 coupling analysis and can be pretty challenging.

441

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448

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628 **APPENDIX A**629 *Tableau A-1 Mechanical properties of the structure*

Module de Young $E[GP\grave{a}]$	Densité $\rho[Kg/m^3]$	Coef. d'amortissement ξ	Coef. de Poisson ν
36	5000	0.07	0.2

630

631 *Tableau A-2 Velocity and density model of the Cadarache site basin*

$z[m]$	$v_s(z)[m/s]$	$v_p(z)[m/s]$	$\rho(z)[kg/m^3]$
$0 \leq z \leftarrow 7m$	$400 + 130z^{0.5}$	900	2100
$z \leq -7m$	$400 + 130z^{0.5}$	$1500 + 190z^{0.5}$	2100

632

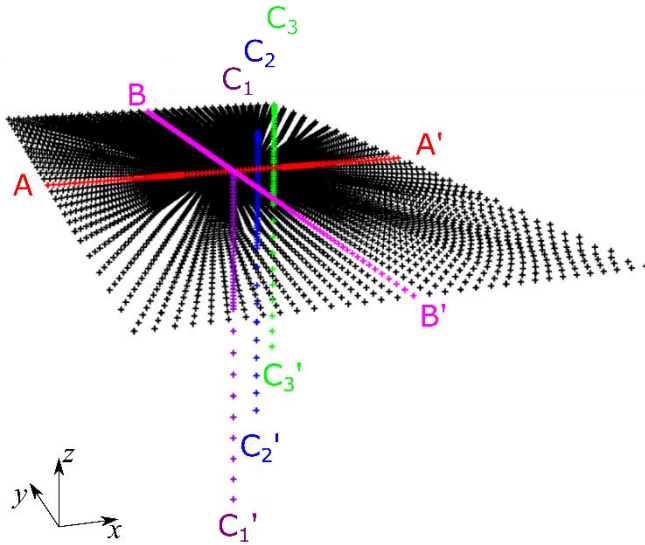
633 *Tableau A-3 Velocity and density model of the Cadarache bedrock*

$z[m]$	$v_s(z)[m/s]$	$v_p(z)[m/s]$	$\rho(z)[kg/m^3]$
$0 \leq z \leftarrow 60m$	$1000 + 427z^{0.4}$	$2000 + 1054z^{0.3}$	2500
$-60 \leq z \leftarrow 3000m$	3200	5600	2720

634

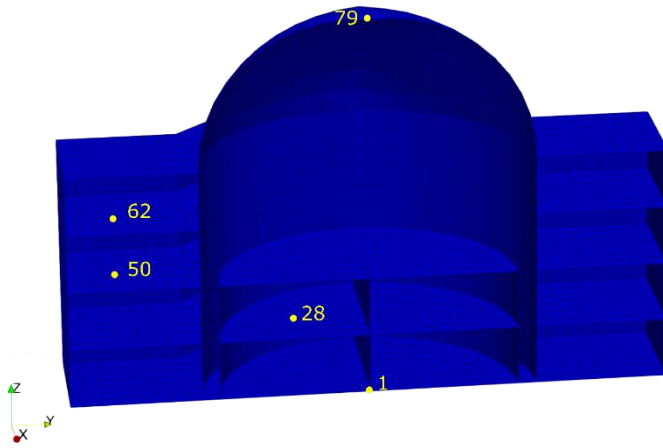
635 *Tableau A-4 Mechanical characteristics of the FEM soil profile model*

n°	$z[m]$	$h[m]$	$v_s(z)[m/s]$	$v_p(z)[m/s]$	$\rho(z)[kg/m^3]$	ν
1	2	2	583.848	900	2100	0.13668
2	7	5	743.948	2002.69	2100	0.41996
3	10	3	811.096	2100.83	2100	0.41241
4	15	5	903.488	2235.87	2100	0.40242
5	20	5	981.378	2349.71	2100	0.39435
6	25	5	1050	2450	2100	0.3875
7	30	5	1112.04	2540.67	2100	0.38151
8	35	5	1169.09	2624.06	2100	0.37617
9	40	5	1222.19	2701.67	2100	0.37134
10	45	5	1272.07	2774.56	2100	0.36693
11	50	5	1319.24	2843.5	2100	0.36286
12	55	5	1364.11	2909.08	2100	0.35907
13	60	5	1406.98	2971.73	2100	0.35554
14	65	5	1448.09	3031.83	2100	0.35222
15	70	5	1487.66	3089.65	2100	0.34909
16	75	5	1525.83	3145.45	2100	0.34614
17	80	5	1562.76	3199.41	2100	0.34333
18	85	5	1598.54	3251.71	2100	0.34066
19	90	5	1633.29	3302.5	2100	0.33811
20	95	5	1667.08	3351.89	2100	0.33567
21	100	5	1700	3400	2100	0.33333
22	250	150	3200	5600	2720	0.25758



637
 638 *Figure B-1 Nodes lines, in the soil model, along which the results history output are evaluated*

639



640
 641 *Figure B-2 Nodes in the structure model along which the results history outputs are evaluated*

642 *Table B-1 Coordinates of the observed structure nodes*

Node #	x	y	z
1	0.0	0.0	0.0
28	10	-12	10.0
50	12	-39	20.0
62	12	-39	30.0
79	0.0	0.0	65

643