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1 Parameter determination of the Compressible Packing Model (CPM)

2 for concrete application

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8

9 Abstract

10 The packing density of granular materials is in the centre of interest of many industries. For concrete 11 industry, the packing density of aggregates is the most representative physical parameter of the 12 granular mixtures: it combines the grain size and the morphology of grains. Compressible Packing 13 Model (CPM) accurately predicts packing density by involving three parameters: wall effect coefficient, 14 loosening effect coefficient and compaction index. The identification of these parameters has been 15 made on elementary granular classes (narrow particle size distributions) which can be time consuming 16 and difficult to achieve. In this research work, we have elaborate a strategy to determine CPM 17 parameters for wide particle size distribution of granular mixtures. This allows reducing the number of 18 tests to optimize packing density of granular mixtures. The results of the modelling show a good 19 agreement with the experiment. The work will be undertaken on two typologies of grains: crushed and 20 rolled aggregates.

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1 Keywords

- 2 Packing density
- 3 Interaction coefficients
- 4 Compaction index
- 5 Crushed aggregates
- 6 Rolled aggregates
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12 **1. Introduction**

13 In the field of civil engineering, concrete is an indispensable raw material which often offering the best 14 technical and economic choice for the construction of buildings and infrastructures. Aggregates in 15 concrete reach up to 80% of the total volume and come in different sizes and shapes. The remaining 16 volume is occupied by a cement paste. Aggregates can arise from natural, artificial or recycled 17 (industrial by-product and demolition source materials). The most important property in the granular 18 mixture of concrete is its packing density. Packing density optimization for the granular mixtures allows 19 not only minimizing the quantity of cement incorporated in the concrete but also enhances its 20 performance and durability [1], [2].

21

In conventional concrete mix design methods, granular mixtures are determined empirically, often from particle size distribution curves (ideal distribution curves), as for the works of Fuller & Thompson (1907), Andreasen & Andersen (1929), Faury (1958) and Dreux (1970) [3], [4]. These methods make it possible to determine the ideal proportions of each grain size to approach the maximum packing density of the mix, but do not allow to predict accurately the packing density [3] and may require several series of experiments for the optimization of the granular mix [2], [5].

With the emergence of modern concretes and special concretes in the 20th century (such as High Performance Concretes (HPC), Self-Compacting Concrete (SCC)...), concrete mix design by "ideal" particle size distribution curves proved to be difficult or unsuccessful [2], [5]. For these concretes, several objectives are aimed at the same time: obtaining high workability, without risk of segregation and good mechanical properties. Therefore, a wide variety of distributions are possible and there is no ideal curve that suits all requirements [3].

7

8 In order to appear to these difficulties, several models have been developed to predict the packing 9 density of a granular mixture. Modelling of packing density was developed first by highlighting the 10 interaction effects between grains of different sizes, in particular by the work of Caguot (1937), which 11 shows the major influence of the wall effect on granular mixtures [3]. It has prompted researchers to 12 study granular classes in pairs. Inspired by Mooney viscosity model (1950), Stovall studied binary 13 mixtures, with and without interaction. He has developed the linear packing model [6] for granular 14 mixtures with multiple classes, taking into account both the wall effects and the loosening effect 15 between the granular classes of different sizes. The loosening effect appears when a small grain is 16 inserted in a dominant large grains population and the wall effect appears when some quantities of 17 large grains isolated are immersed in fine grains agglomerate [7].

18

19 The linear packing model was refined to build the virtual packing model that predicted virtual packing 20 density (i.e. an orderly packing of grains with the least voids), including mixtures of grains of the same 21 size but of different shapes. The virtual packing density for a mixture of "n" granular fractions where 22 the class (i) is dominant, is given by Equation 1 [5]. It involves the volume proportions of each of the 23 granular classes (y_i), their packing density when they are arranged separately (β_i), the loosening effect 24 coefficient (aij) given by Equation 2 and the wall effect coefficient (bji) given by Equation 3 [5]. "e" is 25 the void ratio of the granular mixture defined as the ratio of void and solid fraction (given by Equation 4, \emptyset is the solid fraction). 26

27

$$\gamma = \gamma_i = \frac{\beta_i}{1 - \sum_{j=1}^{i-1} y_j \left(1 - \beta_i + \beta_i b_{ji} \left(1 - \frac{1}{\beta_j}\right)\right) - \sum_{j=i+1}^n y_j \left(1 - a_{ij} \frac{\beta_i}{\beta_j}\right)}$$
Equation 1

| 1 | $a_{ij} = \beta_j \left(\left \frac{\partial e}{\partial y_j} \right _{y_j = 0} + \frac{1}{\beta_i} \right)$ | Equation 2 |
|---|---|------------|
| 2 | $b_{ji} = \frac{\frac{1}{\beta_j} - 1 - \left \frac{\partial e}{\partial y_j} \right _{y_j = 1}}{\frac{1}{\beta_i} - 1}$ | Equation 3 |
| 3 | $e = \frac{1-\phi}{\phi}$ | Equation 4 |

4

5 A first attempt to predict the packing density by the Solid Suspension Model [8], [9] was limited by two 6 defects related to the notion of reference viscosity (considered as a description of the degree of 7 compaction of the system) and the interaction functions that were not satisfactory in their mathematical 8 form [10]. This compaction energy was introduced later in CPM model by the concept of the 9 compaction index "K". This model makes it possible to predict the real packing density of a mixture of 10 several granular classes from the knowledge of the compactness of each one-dimensional class and 11 the energy of the setting up. The real packing density depends on the compaction energy. The real 12 packing density "C" of a mixture of aggregates is connected to "K" by the expression given in Equation 13 5. Moreover, simplified formulas (Equation 6 and Equation 7) of the granular interaction coefficients (a 14 and b) was proposed by de Larrard [5] after calibration of the CPM on different series of experimental 15 data.

16

17
$$K = \sum_{i=1}^{n} K_{i} = \sum_{i=1}^{n} \frac{\frac{y_{i}}{\beta_{i}}}{\frac{1}{c} - \frac{1}{\gamma_{i}}}$$
Equation 5
18
$$a_{i,j} = \sqrt{1 - \left(1 - \frac{d_{j}}{d_{i}}\right)^{1,02}}$$
Equation 6
19
$$b_{j,i} = 1 - \left(1 - \frac{d_{i}}{d_{j}}\right)^{1,5}$$
Equation 7

20

In the framework of the CPM, the prediction of the packing density is therefore possible after determination of the coefficients a, b (by the calculation of the void index at the limits of coarse grains dominant and fine grains dominant) and K. These parameters are calibrated on binary mixtures of aggregates of different natures (crushed and rolled) [5], [11]. The error between measured and 4 calculated packing densities after calibration does not exceed 0.77% for rolled aggregates and 1.71%
 for crushed aggregates [5].

3

4 In order to improve the prediction of the packing density, several models and calculation approaches 5 have been proposed [12]-[18]. Some authors introduced new interaction parameters [15], [17]. 6 Roquier [17] introduced the interference effect that occurs when coarse grains, in a growing number, 7 become too close to each other by trapping some fine grains in small spaces between them [3], [17]. 8 However, despite these various attempts to improve the packing density prediction, the CPM remains 9 among the most accurate models and the most simplest to apply [19], [20]. Moutassem [19] had 10 compared 9 packing density models and he found that the CPM predict correctly the packing density 11 of granular mixtures used in concrete.

12

13 Other research works has focused on the CPM parameters in order to propose optimized values for 14 aij, bji and K [11], [20]. The work of Lecomte [11] has slightly improved the calculation of the interaction 15 coefficients through new simplified formulas of calculation. Indeed, the simplified formulas of the 16 interaction coefficients are probably valid only for one type of aggregates which justifies the 17 differences between the formulas proposed by de Larrard [2] and by Lecomte [11]. In fact, for the 18 CPM, the functions of aij and bij have been calibrated on elementary granular classes (d and D are the 19 minimum and maximum grain sizes respectively) while respecting on the one hand a ratio di/Di>0.1 (so 20 that the elementary granular classes are the more unimodal possible) [2], [5] and on the other hand a 21 ratio $d_i/d_i < 4$ [2], [5]. In the case of granular mixtures optimization for concrete, this work is long and 22 expensive to achieve because it requires a long process of material preparation and sieving 23 elementary granular classes.

24

In the present paper we will calibrate the CPM parameters in the case of large granular classes with d_i/D_i<0.1 (especially for sand) and for some ratios d_j/d_i>4. This study does not require any sieving work to obtain elementary granular classes that must have narrower particle size distributions. Our goal is to save time and materials in packing density studies of granular mixtures for concrete without affecting the accuracy of the model. First, we will apply the CPM as developed by de Larrard in order to qualify

the prediction of the model and determine the most influential parameters. After that, we will optimize the parameters of the CPM for wide particle size distributions of granular mixtures of crushed and rolled aggregates. The coefficients a, b and K will be calibrated on binary mixtures. Finally, we will evaluate the accuracy of the model applied to large granular classes of binary and ternary mixtures compared to previous studies.

6

7

2. Materials and methods

8 In Table 1 are listed the aggregates of the present study and few characteristics. The size, shape and 9 roughness of the grains are the three main parameters that affect the packing density [3], [5], [21] that 10 has influenced our choice. Rolled aggregates come from "Chevrières" quarry ("Hauts-de-France" 11 region) and "Decize" quarry ("Bourgogne-Franche-Comté" region). The crushed aggregates come 12 from the quarries of "Boulonnais" ("Nord Pas-de-Calais" region). The granular fractions adopted for 13 this study range from sand (0/4 mm) to gravel (4/10 and 10/20 mm). The absolute density of the 14 aggregates was measured according to the European standard NF EN 1097-6 [22] and the particle 15 size analysis was carried out according to the European standard NF EN 933-1 [23].

16 In order to measure the packing density of the mono-granular classes, the LPC procedure No. 61 was 17 followed [24]. After weighing 7 kg of one mono-granular material, the sample is placed in a cylindrical 18 mould in three equivalent layers. Each layer is subjected to 20 shocks in the shaking table before 19 putting the next layer. The sample is then subjected to 40 shocks under a pressure of 10 kPa 20 (equivalent weight of 20 kg can be placed above the sample). The energy transmitted by the shocks 21 allows the granular material to be into a more dense configuration, which improves its packing density. 22 The compaction index (K) of this process was calibrated after several tests. Using Equation 5, K = 923 was found [24].

24

The packing densities were measured on binary granular mixtures of crushed and rolled aggregates to determine the interaction coefficients for each type of aggregates [2], [11]. Different combinations between sand and gravels were analysed. As shown in Table 2, considering the maximum nominal diameter of each class, the effect of the size ratio D_1/D_2 and the grain shape on the packing density was experienced. Otherwise, the variation of the proportions of the smaller granular class in the binary

mixture equal to 5% in the extreme parts of the curve of packing density and to 10% in the centre of the curve. This is in order to measure the impact of the granular interaction effects that have defined previously (wall effect and loosening effect coefficients). Finally, packing density of ternary granular mixtures was measured for crushed and rolled aggregates. This is in order to evaluate the efficiency of the interaction parameters determined on the binary mixtures. The measurements were realized by increment of 10%. The different combinations of the experimental program on ternary mixtures are shown in Fig. 1.

8

9

3. Results and discussions

The main properties of the aggregates used in this study are listed in Table 3. Otherwise, the particle size distribution of the different materials are shown in Fig. 2. Particle size analysis showed a high fines content in crushed sand (5% of grains are smaller than 63 microns and 16% smaller than 125 microns).

14

15 Packing densities measurement tests were realised using the compaction table according to LPC 16 procedure No. 61 [24] on binary and ternary mixtures of crushed and rolled aggregates. The different 17 materials are used in their raw state without any prior sieving to remove the fine fraction. For each test, 18 two different measurements of compactness were made. In each case, the average packing density 19 and the variances are calculated. It has been observed through the experimental program that the 20 repeatability of the tests is ensured [25] (maximum standard deviation recorded is 0.017 for crushed 21 aggregates and 0.008 for rolled aggregates). The use of the aggregates in their raw state without any 22 prior sieving (fine particles content up to 7%) did not influence the packing density measurements by 23 compaction table [25].

24

The evolution curves of the packing density measured by the compaction table as a function of the proportion of the addition of fine material are shown in Fig. 3. From the curves in Fig. 3, it is observed a rapid increase of the packing density when the sizes ratio of the mixed materials is large $(D_1/D_2 = 5)$. The packing density curves exhibits an optimum before decreasing when wall effects occur in the area where the small grains material becomes dominant. Even for smaller ratio $(D_1/D_2=2.5)$, the evolution of

the packing density still important (Fig. 3-b). For $D_1/D_2=2$, the packing density curve evolution is almost nil (Fig. 3-c). These results show that the packing density optimum of the binary mixtures increases when D_1/D_2 ratio increases from 2 up to 5. This is in agreement with the results found by de Larrard [2], [5] and McGeary [26]. Finally, the size ratio $D_1/D_2=2$ seems to define the limit for materials which are in total interaction [2], [5].

6

Despite their low fines content, the rolled aggregates (Fig. 3. right) reaches the highest packing density in comparison to crushed aggregates. The packing of rolled aggregates is composed of grains of various sizes (wide particle size distribution) that their shape is close to the sphere. This packing approaches the Apollonian packing (packing of circles) which gives the highest packing density [27], [28]. The high fines content (particle content <63 microns) in the crushed aggregates (Fig. 3. left) was not able to counterbalance the shape effects in terms of packing density (Fig. 3a and Fig. 3b).

13

14 From these results, we deduce that the maximum packing density that can be achieved on binary 15 mixtures is influenced by the combination of the following parameters (regardless of the compaction 16 index): the packing density of the elementary granular classes, the shape and roughness of the grains 17 as well as the fines content and the size ratio. The packing density of binary mixtures can be 18 increased by improving the packing density of the elementary classes as found by de Larrad [2], [5]. 19 Similarly, some authors have shown that the packing density increases when the size ratio of 20 elementary classes increases [2], [17], [29]. The effect of the presence of ultra-fine particles (powders) 21 can also be related to the large size ratio between the powders and coarse grains.

22

For the ternary mixtures (of crushed aggregates) studied in this research work, the results are shown in Fig. 4. The packing density measured decreases in the area of low sand content (dark area) and increases with the introduction of sand (white area). The presence of intermediate gravel (G 4/10) with the coarser gravel (G 10/20) disturbs the granular mixture because of the observed interaction between these materials as seen in Fig. 3-c.

The evolution curves of the experimental packing density for binary and ternary mixtures allow to proceed to the modelling of these packing densities by the CPM in which we will analyse the different parameters of the model which are: the coefficients of granular interactions (a_{ij} and b_{ji}) and the compaction index (K). This work will be undertaken for large granular classes.

5

In order to determine the packing densities by the CPM, the virtual packing densities are calculated using Equation 1 where the coefficient of loosening effect (a_{ij}) and wall effect (b_{ji}) have to be determined. In the first method, these parameters are calculated using the simplified formulas (Equation 6 and Equation 7) proposed by de Larard [5]. The real packing density of the mixtures is subsequently calculated through the compaction index which is taken equal to 9 [24]. In the second method and for comparison, the parameters a_{ij} and b_{ji} are determined on the experimental curves and the compaction index value is maintained equal to 9 as in the first method.

13

The results in terms of experimental data in comparison to the model prediction following the firstmethod are shown in Fig. 5.

16

In this case the CPM allows to predict experimental packing densities with an average error of 3.2% for crushed aggregates and 1.7% for rolled aggregates. We note through the curves shown in Fig. 5 that the largest differences are found in the area of optimal packing density. In this zone, the packing density is overestimated by the CPM for crushed aggregate mixtures and underestimated in the case of rolled aggregate mixtures. This leads to say that the efficiency of the compaction mode by the compaction table is influenced not only by the particle size as found by Sadok et al. [16], [30], but also by the shape of the grains. The parameters a, b and K used in this first modelling are given in Table 4.

24

In the previous studies performed by de Larrard, an average error of 1.71% for crushed aggregates
and 0.77% for rolled aggregates is found [2], [5].

27

In the second method, the parameters of the CPM are determined from experimental data. According
to the basic formulas of interaction coefficients (Equation 2 and Equation 3), it could be seen that the

slope of the experimental data in terms of void ratio (given by Equation 4) versus the percentage of fine material at the origin of the curve and at the extremity of the curve are respectively proportional to the parameters a_{ij} and b_{ji} . These latter are calculated using Equation 6 and Equation 7 after calculating β_1 , β_2 and β_3 by Equation 5 (K is taken equal to 9) and evaluating graphically the slope of the curve evolution of the void ratio as a function of the percentage of fine grains. The parameter determination for binary mixtures is highlighted in Fig. 6.

7

As it could be seen in Fig. 6, due to the discrepancy of the data, for some curves it is difficult to define the slope at the origin or at the extremity of the curve. In this order, the slope is evaluated on 25% of the curve (at the origin and at the extremity) in area considered as linear part. The values of the coefficients a_{ij} and b_{ji} determined (given in Table 5) were compared to those obtained by de Larrard [2], [5] and Lecomte [11] as shown on Fig. 7. The results of the modelling are presented in Fig. 8.

13

14 As shown in Fig. 7, the coefficients an and bi obtained on large granular classes follow the same 15 evolution as those obtained by de Larrard [2], [5] and Lecomte [11]. They increase when the ratio 16 D_2/D_1 increases. This shows that the granular interactions are stronger when the granular classes are 17 of the same size (until total interaction as shown in Fig. 3-c). However, Fig. 7 shows some differences 18 between the interaction coefficients determined experimentally and the functions of the simplified 19 formulas proposed by de Larrard or Lecompte. This allows to say that there is no unique function 20 which makes it possible to calculate the interaction coefficients accurately for different type of 21 aggregates and without causing errors in the prediction of the packing density. Moreover, when we 22 analyse in more details the obtained results from our experimental data we can point out that the 23 values of ai measured on rolled aggregates are systematically lower than those measured on crushed 24 aggregates. This result could be observed for most data published by de Larard and is in line with the 25 published paper of Roquier [18]. In this later, the estimates of the loosening effect parameter calculated from two different models (4- parameter CPM as 3PPM) show clearly that the values 26 27 calculated on rolled aggregates are lower than those calculated for crushed aggregates. It is also 28 interesting to note that for small diameter ratios (D2/D1), the loosening effect parameter evolves very 29 quickly with increasing the diameter ratios.

For the wall effects parameter b_{ji}, in terms of aggregates shape effects, the same conclusions than those addressed for the loosening effect parameter could be drawn. The measured parameters values on rolled aggregates are systematically lower than those measured on crushed aggregates. This is also in line with most published data from de Larrad and other researchers.

5 The comparisons of absolute values of the wall effect parameter seems to be systematically lower that
6 the values of the loosening effect parameter in the whole range of the diameter ratios.

Finally the comparison of the measured values for both parameters with the calculated values using
the proposed relationships by de Larrard [2], [5] and Lecomte [11] shows that a big differences could
be induced.

10

11 Comparisons between the predictions using CPM with identified aij and bji on experimental data and 12 experimental results (Fig. 8) show better agreement than in the first method (modelling through 13 simplified formulas of aij and bji). The difference between the model prediction and the experimental 14 data decreases to 2.0% for crushed aggregates and 1.1% for rolled aggregates. These errors are 15 similar to those found in the work by de Larrard [2], [5]. From these results, we can deduce that 16 identification on elementary granular classes obtained by sieving is unnecessary to achieve the best 17 performances of the CPM. However, it seems necessary to define interaction coefficients on 18 experimental data.

19

In order to assess the effect of the compaction index (K) on the prediction of the CPM, in the last section of this study a back calculation of K on experimental data is undertaken. In this procedure, the difference between the model prediction and the experimental data is decreased by changing the value of K. The parameters a_{ij} and b_{ji} are kept as defined in the second method. The obtained results in terms of compaction index and errors between experimental data and prediction of the model is summarised in Table 6 and Fig. 9.

26

The compaction index obtained after calibration is lower than 9 (especially for crushed aggregates). Different values of the compaction index are obtained (given in Table 6) depending on the granular mixture. The compaction index depends to the experimental process, other values of K was recorded

with others experimental processes [31], [32]. But for compaction mode by shaking table, K must be constant because the energy used was constant [7]. We conclude that for crushed aggregates the compaction by the shaking table is not effective [25], [33]. This is due to their high fines content [5], [30]. With regard to the rolled aggregates, the compaction efficiency was better since we could reach a compaction index close to 9. However, the difference is not very significant in terms of packing density prediction, from which we retain the compaction index K = 9 for modelling of ternary mixtures.

7

8 In the case of ternary mixtures, the prediction of the experimental data is undertaken using CPM with 9 a_{ij} and b_{ji} as defined in method 2 on binary mixtures. The compaction index, following the observed 10 results on the binary mixtures and the results of several authors [2], [3], [30] the result is value is fixed 11 to 9. To ease the comparisons, the predicted results versus experiments are shown on Fig. 10.

12

The errors observed between the model and the experiments for ternary mixtures is about 2.4% for crushed aggregates and 1.6% for rolled aggregates. From the results shown on Fig. 10, we note that in the majority of ternary mixtures of crushed and rolled aggregates, CPM overestimates the packing density.

17

18

4. Conclusion and perspectives

In this paper, the prediction of packing density of binary and ternary mixtures of crushed and rolled
aggregates was investigated. From this study, we can draw the following conclusions:

21

For packing density measurements at the compaction table made with materials including the
 fine fraction (<63 microns), the repeatability of the tests is ensured (maximum standard
 deviation of 0.015 between two tests). The segregation effects of fine particles can be
 neglected for crushed and rolled aggregates with fine particles (less than 63 microns) content
 up to 7%.

The modelling of the packing density by the CPM have shown that the use of the granular
 interaction coefficients determined by the simplified formulas is insufficient to have a good
 prediction.

The back calculation of the compaction index (K) may vary from one mixture to another
 (especially for crushed aggregates).

- It has been demonstrated in this study that the CPM can be applied to large granular classes
 without creating elementary subclasses. This save time and materials in packing density
 studies, especially for concrete mix design.
- The identification of the model coefficients following the procedure suggested improved the
 prediction of the CPM.
- 8

9 The experimental program and the modelization realized allow to improve the precision of the CPM in 10 the determination of the packing density of granular mixtures for concrete. In the perspective of this 11 study, the packing density will be exploited to go back to the yield stress and the compressive strength 12 of concrete following the approaches of Chateau et al. [34] and de Larrard [2], [35] respectevly by 13 combining the results of this study with the study of the properties of the cement paste.

14

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18

19 References

- P. J. Andersen, "Particle packing and concrete properties," *Mater. Sci. Concr. II*, pp. 111–146,
 1995.
- 22 [2] F. de Larrard, Concrete mixture proportioning: a scientific approach. CRC Press, 1999.
- [3] G. Roquier, "Etude de la compacité optimale des mélanges granulaires binaires: classe
 granulaire dominante, effet de paroi, effet de desserrement," PhD Thesis, Paris Est, 2016.
- S. A. Fennis and J. C. Walraven, "Using particle packing technology for sustainable concrete
 mixture design," *Heron 57 2012 2*, 2012.
- F. de Larrard, "Structures granulaires et formulation des bétons," *Etudes Rech. Lab. Ponts Chaussées*, vol. OA 34, p. 414 p., 2000.

- [6] T. Stovall, F. de Larrard, and M. Buil, "Linear packing density model of grain mixtures," *Powder Technol.*, vol. 48, no. 1, pp. 1–12, Sep. 1986.
- M. Miraoui, R. Zentar, and N.-E. Abriak, "Road material basis in dredged sediment and basic
 oxygen furnace steel slag," *Constr. Build. Mater.*, vol. 30, pp. 309–319, 2012.
- 5 [8] T. Sedran, F. de Larrard, and D. Angot, "Prévision de la compacité des mélanges granulaires
 6 par le modèle de suspension solide -l- Fondements théoriques et étalonnage du modèle," Bull.
 7 Liaison Lab. Ponts Chaussées, no 194, 1994.
- 8 [9] T. Sedran, F. de Larrard, and D. Angot, "Prévision de la compacité des mélanges granulaires
 9 par le modèle de suspension solide -II- Validation, cas des mélanges confinés," Bull. Liaison
 10 Lab. Ponts Chaussées, no 194, 1994.
- [10] F. de Larrard and T. Sedran, "Une nouvelle approche de la formulation des bétons," in *Annales du BTP*, 1999, vol. 6, pp. 39–54.
- [11] A. Lecomte, A. Zennir, and F. de Larrard, "Modèle de suspension solide et formulation de bétons
 calcaires en Lorraine," *Bull.-Lab. Ponts Chaussées*, pp. 41–52, 1997.
- [12] W. Toufar, M. Born, and E. Klose, "Contribution of optimisation of components of different
 density in polydispersed particles systems," *Freib. Bookl. A*, vol. 558, pp. 29–44, 1976.
- 17 [13] J. D. Dewar, "Ready-mixed concrete mix design," *Munic. Eng.*, vol. 3, pp. 35–43, 1986.
- [14] M. Reisi and D. M. Nejad, "A numerical method to predict packing density of aggregates in
 concrete," in *Advanced Materials Research*, vol. 337, pp. 313–316, 2011.
- [15] A. K. H. Kwan, K. W. Chan, and V. Wong, "A 3-parameter particle packing model incorporating
 the wedging effect," *Powder Technol.*, vol. 237, pp. 172–179, Mar. 2013.
- [16] A. Sadok, R. Zentar, and N.-E. Abriak, "Genetic programming for granular compactness
 modelling," *Eur. J. Environ. Civ. Eng.*, pp. 1–13, 2016.
- [17] G. Roquier, "The 4-parameter Compressible Packing Model (CPM) including a new theory about
 wall effect and loosening effect for spheres," *Powder Technol.*, vol. 302, pp. 247–253, Nov.
 26 2016.
- [18] G. Roquier, "A Theoretical Packing Density Model (TPDM) for ordered and disordered packings,"
 Powder Technol., 2018.

| 1 | [19] | F. Moutassem, « Assessment of Packing Density Models and Optimizing Concrete Mixtures », |
|----|------|--|
| 2 | | Int. J. Civ. Mech. Energy Sci. (IJCMES), vol. 2, no 4, p. 29-36, 2016. |
| 3 | [20] | M. R. Jones, L. Zheng, and M. D. Newlands, "Comparison of particle packing models for |
| 4 | | proportioning concrete constitutents for minimum voids ratio," Mater. Struct., vol. 35, no. 5, pp. |
| 5 | | 301–309, 2002. |
| 6 | [21] | A. Lecomte and JM. Mechling, "Compacité des mélanges et propriétés des grains," BullLab. |
| 7 | | Ponts Chaussées, pp. 21–34, 1999. |
| 8 | [22] | Association Française de Normalisation (AFNOR), "Tests for mechanical and physical |
| 9 | | properties of aggregates - Part 6: determination of particle density and water absorption", |
| 10 | | Standard No. 1097-6, 2014. |
| 11 | [23] | Association Française de Normalisation (AFNOR), "Tests for geometrical properties of aggrega- |
| 12 | | tes - Part 1: determination of particle size distribution - Sieving method", Standard No. 933-1, |
| 13 | | 2012. |
| 14 | [24] | V. Ledee, F. de Larrard, T. Sedran, and F. Brochu, « Essai de compacité des fractions |
| 15 | | granulaires à la table à secousses: Mode opératoire », Tech. Méthodes Lab. Ponts Chaussées, |
| 16 | | 2004. |
| 17 | [25] | M. Bala, R. Zentar, and P. Boustingorry, "Parameter analysis of the compressible packing model |
| 18 | | for Concrete application," 12th fib International PhD Symposium in Civil Engineering, Prague, |
| 19 | | Czech Republic, Unpublished results, pp. 1–8. |
| 20 | [26] | R. K. McGeary, "Mechanical packing of spherical particles," J. Am. Ceram. Soc., vol. 44, no. 10, |
| 21 | | pp. 513–522, 1961. |
| 22 | [27] | G. W. Delaney, S. Hutzler, and T. Aste, "Relation between grain shape and fractal properties in |
| 23 | | random Apollonian packing with grain rotation," Phys. Rev. Lett., vol. 101, no. 12, p. 120602, |
| 24 | | 2008. |
| 25 | [28] | D. Weaire and T. Aste, The pursuit of perfect packing. CRC Press, 2008. |
| 26 | [29] | A. B. Yu and N. Standish, "Porosity calculations of multi-component mixtures of spherical |
| 27 | | particles," Powder Technol., vol. 52, no. 3, pp. 233–241, 1987. |
| 28 | [30] | S. Abdelfeteh, "Formulation de matériaux de construction à base de sous-produits industriels |
| 29 | | avec des méthodes issues de l'intelligence artificielle, "PhD Thesis, Univ. Lille 1, 2016. |

| 1 | [31] | A. Sadok, R. Zentar, and N. E. Abriak, "Modélisation de la compacité granulaire par une |
|----|------|--|
| 2 | | approche d'Apprentissage Automatique," in Rencontres Universitaires de Génie Civil, 2015. |
| 3 | [32] | T. N. Thanh, "Valorisation de sédiments marins et fluviaux en technique routière," PhD Thesis, |
| 4 | | Univ. d'Artois - Ecole des Mines de Douai, 2009. |
| 5 | [33] | M. Bala, R. Zentar, and P. Boustingorry, "Etude d'impact de la forme des granulats sur les |
| 6 | | paramètres du modèle d'empilement compressible," presented at the 36èmes Rencontres |
| 7 | | Universitaires de Génie Civil de l'AUGC, Saint-Etienne, France, 2018, pp. 1–4. |
| 8 | [34] | X. Chateau, G. Ovarlez, and K. L. Trung, "Homogenization approach to the behavior of |
| 9 | | suspensions of noncolloidal particles in yield stress fluids," J. Rheol., vol. 52, no. 2, pp. 489-506, |
| 10 | | 2008. |
| 11 | [35] | F. de Larrard and A. Belloc, "L'influence du granulat sur la résistance à la compression des |
| 12 | | bétons," Bull. Lab. Ponts Chaussées, no. 219, 1999. |



Fig. 1: Experimental program of ternary mixtures (Crushed aggregates)





(c) Fig. 3: Packing density measured by the compaction table of binary mixtures of crushed aggregates (left) and rolled aggregates (right) for: (a) $D_1/D_2=5$, (b) $D_1/D_2=2.5$ and (c) $D_1/D_2=2$



Fig. 4: Packing density measured by the compaction table of ternary mixtures of crushed aggregates



Fig. 5: Measured and modelled packing density of crushed (left) and rolled (right) binary Mixtures via simplified formulas of interaction coefficients for: (a) $D_1/D_2=5$, (b) $D_1/D_2=2.5$ and (c) $D_1/D_2=2$



Fig. 6 : Determination of the granular interaction coefficients a and b for crushed (left) and rolled (right) aggregates for: (a) D₁/D₂=5, (b) D₁/D₂=2.5 and (c) D₁/D₂=2



Fig. 7: Interaction coefficients vs. size ratio for crushed and rolled aggregates (comparison between experimental data of the present study and data from the literature): loosening effect coefficient "a" (left) and wall effect coefficient "b" (right)



Fig. 8: Measured and modelled packing density of crushed (left) and rolled (right) binary mixtures after calibration of interaction coefficients for: (a) $D_1/D_2=5$, (b) $D_1/D_2=2.5$ and (c) $D_1/D_2=2$



Fig. 9 : Measured and modelled packing density of crushed (left) and rolled (right) binary mixtures after calibration of a, b and K for: (a) D₁/D₂=5, (b) D₁/D₂=2.5 and (c) D₁/D₂=2



Fig. 10: Modelled vs. Measured packing density of ternary mixtures after calibration of a and b (K=9)

| Type of aggregates | Origin | Origin Granular class d/D | |
|-----------------------|------------|---------------------------|------|
| | | Sand 0/4 | 2.69 |
| Crushed | Boulonnais | Gravel 4/10 | 2.67 |
| | | Gravel 12/20 | 2.67 |
| | Chovrièree | Sand 0/4 | 2.55 |
| Rolled | Chevneles | Gravel 4/10 | 2.43 |
| | Decize | Gravel 11/22 | 2.54 |

Table 1: Origin and nature of the studied aggregates

Table 2: Experimental program of binary mixtures

| Type of aggregates | Granular class d/D | Binary mixtures | D ₁ /D ₂ ratio |
|-----------------------|-----------------------|--------------------|---|
| | Sand 0/4 | SC 0/4 + GC 12/20 | 5 |
| Crushed | Gravel 4/10 | SC 0/4 + GC 4/10 | 2.5 |
| | Gravel 12/20 | GC 4/10 + GC 12/20 | 2 |
| | Sand 0/4 | SR 0/4 + GR 11/22 | 5 |
| Rolled | Gravel 4/10 | SR 0/4 + GR 4/10 | 2.5 |
| | Gravel 11/22 | GR 4/10 + GR 11/22 | 2 |

Table 3: Characteristics of the aggregates of the study

| | Cru | shed aggreg | jates | Rolled aggregates | | | |
|--|------------------|---------------------------|----------------------|--|---------------------|----------------------|--|
| d/D | SC 0/4 (Sand) | GC 4/10 (Gravel) | GC 12/20 (Gravel) | SR 0/4 (Sand) | GR 4/10 (Gravel) | GR 11/22 (Gravel) | |
| Fine content < 63 µm [%] | 6.2 | 1.0 | 0.8 | 1.3 | 1.7 | 0.1 | |
| d ₅₀ [mm] | 0.7 | 7.2 | 15.2 | 0.4 | 7.0 | 16.0 | |
| Fineness modulus (for sands) | 3.2 | - | - | 3.4 | - | - | |
| Absolute density [g/cm ³] | 2.726 | 2.735 | 2.711 | 2.626 | 2.648 | 2.626 | |
| Water absorption [%] | 0.43 | 0.59 | 0.49 | 1.18 | 3.06 | 1.79 | |
| Shape and roughness | Angular ag | ggregates, fla surface | t and rough | Rounded shape, flat or elongated aggregates, smooth surface | | | |

| Type of | Р | Р | Mixturaa | ם, ם | a & b by simplified fo | | d formulas | |
|------------|------------|----------------|-------------------------|---|------------------------|--------|------------|---------------|
| aggregates | U 1 | \mathbf{D}_2 | D ₂ Mixtures | | а | b | Κ | Average error |
| | 10 | 4 | SC 0/4 + GC 4/10 | 0.40 | 0.64 | 0.54 | | 1.5% |
| | 20 | 4 | SC 0/4 + GC 10/20 | 0.20 | 0.45 | 0.28 | 9 | 2.8% |
| Crushed | 20 | 10 | GC 4/10 + GC 10/20 | 0.50 | 0.71 | 0.65 | | 5.2% |
| | | | | Average error for crushed 3.2% aggregates | | | | 3.2% |
| | 10 | 4 | SR + GR 4/10 | 0.40 | 0.64 | 0.54 | | 2.2% |
| | 22 | 4 | SR + GR 10/20 | 0.18 | 0.43 | 0.26 | ٩ | 2.1% |
| Rolled | 22 | 10 | GR 4/10 + GR 10/20 | 0.45 | 0.68 | 0.60 | 5 | 0.7% |
| | | | | Averag a | e error for | rolled | | 1.7% |

Table 4: Parameters used for modelling by simplified formulas of a and b (K = 9)

Table 5: Parameters of the modelling by calibrated formulas of a and b (K = 9)

| Type of | | _ | Mixturee | | a & b after calibration | | libration | |
|------------|------------|----------|-----------------------|---|-------------------------|---------------|-----------|---------------|
| aggregates | D 1 | D_2 | mixtures | $\mathbf{D}_2/\mathbf{D}_1$ | а | b | Κ | Average error |
| | 10 | 4 | SC 0/4 + GC 4/10 | 0.40 | 0.62 | 0.22 | | 1.1% |
| | 20 | 4 | SC 0/4 + GC 10/20 | 0.20 | 0.54 | 0.30 | 9 | 2.1% |
| Crushed | 20 | 10 | GC 4/10 + GC 10/20 | 0.50 | 0.90 | 0.83 | Ū | 2.9% |
| | | | | Average error for 2.0% crushed aggregates | | | | |
| | 10 | 4 | SR + GR 4/10 | 0.40 | 0.53 | 0.06 | | 1.2% |
| | 22 | 4 | SR + GR 10/20 | 0.18 | 0.32 | 0.01 | 9 | 0.6% |
| Rolled | 22 | 10 | GR 4/10 + GR 10/20 | 0.45 | 0.87 | 0.71 | Ū | 1.4% |
| | | | | Averag a | e error fo ggregate: | r rolled s | | 1.1% |

| Turna of | | | | | a, b and K aft | | after ca | alibration |
|------------|------------|----------------|-----------------------|---|-------------------------|---------------|----------|------------------|
| aggregates | D 1 | D ₂ | Mixtures | D_2/D_1 | а | b | К | Average error |
| | 10 | 4 | SC 0/4 + GC 4/10 | 0.40 | 0.62 | 0.22 | 5.5 | 0.8% |
| | 20 | 4 | SC 0/4 + GC 10/20 | 0.20 | 0.54 | 0.30 | 3.2 | 1.1% |
| Crushed | 20 | 10 | GC 4/10 + GC 10/20 | 0.50 | 0.90 | 0.83 | 0.3 | 1.1% |
| | | | | Average error for 1.0% crushed aggregates | | | | |
| | 10 | 4 | SR + GR 4/10 | 0.40 | 0.53 | 0.06 | 4.8 | 0.8% |
| | 22 | 4 | SR + GR 10/20 | 0.18 | 0.32 | 0.01 | 8.2 | 0.6% |
| Rolled | 22 | 10 | GR 4/10 + GR 10/20 | 0.45 | 0.87 | 0.71 | 2.0 | 0.7% |
| | | | | Average | e error fo ggregates | r rolled s | | 0.7% |

| Table 6: Parameters of the modelling after calibration of a, b and K |
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Graphical abstract



Effect of calibration of a and b on modeled packing density by CPM