



**HAL**  
open science

# Effects of Horizontal Seismic Band on Seismic Response in Masonry Structure: Behaviour Depending upon the Material Used

Santosh Yadav, Hala Damerji, Reinold Keco, Yannick Sieffert, Eugénie Créte, F. Vieux-Champagne, Philippe Garnier, Yann Malecot

## ► To cite this version:

Santosh Yadav, Hala Damerji, Reinold Keco, Yannick Sieffert, Eugénie Créte, et al.. Effects of Horizontal Seismic Band on Seismic Response in Masonry Structure: Behaviour Depending upon the Material Used. International Conference on Nepal's Reconstruction (ICNR - 2020), Aug 2020, Kathmandu, Nepal. hal-03136374

**HAL Id: hal-03136374**

**<https://hal.science/hal-03136374v1>**

Submitted on 10 Mar 2023

**HAL** is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.



Distributed under a Creative Commons Attribution - NonCommercial 4.0 International License

# Effects of Horizontal Seismic Band on Seismic Response in Masonry Structure: Application of DIC technique

Santosh Yadav<sup>a\*</sup>, Hala Damerji<sup>b</sup>, Reinold Keco<sup>c</sup>, Yannick Sieffert<sup>d</sup>, Eugénie Créte<sup>e</sup>, Florent Vieux-  
Champagne<sup>f</sup>, Philippe Garnier<sup>g</sup>, and Yann Malecot<sup>h</sup>

Corresponding author:

Email: [santosh.yadav@3sr-grenoble.fr](mailto:santosh.yadav@3sr-grenoble.fr)

<sup>a</sup> \*PhD Student, Univ. Grenoble Alpes, Grenoble INP, CNRS, 3SR, F-38000 Grenoble, France, [santosh.yadav@3sr-grenoble.fr](mailto:santosh.yadav@3sr-grenoble.fr)

<sup>b</sup> PhD, Univ. Grenoble Alpes, Grenoble INP, CNRS, 3SR, F-38000 Grenoble, France, [hala.damerji@3sr-grenoble.fr](mailto:hala.damerji@3sr-grenoble.fr)

<sup>c</sup> Bachelor student, Univ. Grenoble Alpes, Grenoble INP, CNRS, 3SR, F-38000 Grenoble, France, [reinold.keco@univ-grenoble-alpes.fr](mailto:reinold.keco@univ-grenoble-alpes.fr)

<sup>d</sup> Associate Professor, Univ. Grenoble Alpes, Grenoble INP, CNRS, 3SR, F-38000 Grenoble, France, [yannick.sieffert@3sr-grenoble.fr](mailto:yannick.sieffert@3sr-grenoble.fr)

<sup>e</sup> Research In-charge, CRAterre laboratory, Architecture, Environnement et Cultures Constructives research unit, École Nationale Supérieure d'Architecture de Grenoble, [crete.eugenie@gmail.com](mailto:crete.eugenie@gmail.com)

<sup>f</sup> Associate Professor, Univ. Grenoble Alpes, Grenoble INP, CNRS, 3SR, F-38000 Grenoble, France, [florent.vieux-champagne@univ-grenoble-alpes.fr](mailto:florent.vieux-champagne@univ-grenoble-alpes.fr)

<sup>g</sup> Architect-Researcher, CRAterre laboratory, Architecture, Environnement et Cultures Constructives research unit, École Nationale Supérieure d'Architecture de Grenoble, [craterre.pgarnier@club-internet.fr](mailto:craterre.pgarnier@club-internet.fr)

<sup>h</sup> Professor, Univ. Grenoble Alpes, Grenoble INP, CNRS, 3SR, F-38000 Grenoble, France, [yann.malecot@3sr-grenoble.fr](mailto:yann.malecot@3sr-grenoble.fr)

## Abstract

Masonry construction is one of the oldest building systems and practiced since the early stage of human civilization. Even though industrialized nations developed new building practices using steel and concrete materials, most developing countries still use unreinforced masonry buildings. Evidence from past events shows that masonry structures sometimes performed well and survived several earthquake events, particularly in the area where earthquake events struck regularly and enough to show how structures behave. However, an alteration from the traditional cultural building practice is seen due to the lack of proper scientific knowledge about such local construction cultures. Amongst traditional construction technics, one captured authorities' and scientists' attention as showing excellent properties: seismic bands that can be found in many parts of Nepal. After the 2015 Gorkha earthquake, reconstruction works started adopting the guidelines developed by DUDBC and National Building code that suggest using such seismic bands. The evidence of using such band dates to 2500 BC, and qualitative studies of those applications have proven to enhance the performance in the masonry structure. However, the experiment's quantitative parameters using various materials are needed to carry out the parametrical analysis to optimize seismic band's use. To provide quantitative results, we have started with two specimens, one without a band and one with a timber seismic band, and incorporate the digital image correlation (DIC) technique to obtain the localized information. The quasi-static cyclic test is carried out for our specimens to compare the behavior concerning the energy dissipation, and the surface area of the crack formed during the loading cyclic. This work clearly shows the seismic band's advantage in limiting the crack propagation and improving masonry structure integrity.

**Keywords:** *adobe masonry; traditional vernacular structures; seismic band; DIC application; shear test*

## 1. Introduction

Masonry structures constitute the oldest buildings constructed by human civilization and their technic evolved with cultural values and way of living. As per the world population statistics, approximately 86 % of the world population is present in the underdeveloped and developing nation [1]. According to the United Nations, half of these population live in building constructed using earth materials (rammed earth, kneaded earth, adobe, wattle, and daub, cob, etc.). [2] carried out a basic SWOT (Strengths, Weakness, Opportunities, Threats) analysis for the earthen structures and found several benefits of using such construction material. Such construction practices not only preserve the socio-cultural aspect but also has economic and environmental gain.

With the increasing construction trend using concrete and steel structures, people have started forgetting their traditional construction practices and adopting new materials without proper technique. When we look at the global seismic hazard map, most densely populated countries with weak financial conditions are located in the seismically active zone [3]. There is evidence showing that some traditional buildings survived several events of an earthquake. At the same time, sometimes modern RC structures collapsed [4]. Despite that, traditional cultural constructions seem to be fading out and being replaced by modern construction technics. Some questions are emerging- is the current construction practice sustainable, did we miss to understand better and study our traditional building practice?

To better understand the traditional cultural practice, several research works have been carried out. [5] has highlighted that the earthquake-resistant traditional construction is not an oxymoron with the examples of structures that survive during a disaster event in various parts of the world. Many works

have been carried out to enhance masonry structures' strength using retrofitting techniques [6–10]. Some of these techniques required skilled labor for an effective benefit from the application and could vary in cost depending upon the availability of materials.

The use of timber in masonry structure as a seismic band was present from the Minoan Crete 2500 years BC [11], and the finding observed after the 2017 Lesvos Earthquake in Greece by [12] showed that structures with timber reinforcement suffered only localized damage and prevented them from complete collapse. The materials for such a band can be timber, reinforced concrete, bamboo, etc. But there is a lack of quantitative data, which prevent the efficiency comparison of different reinforcement technique [13] and without scientific experimental results, the behavior and performance cannot be assessed.

In this paper, the main idea is to reveal the traditional practice using seismic band placed during construction of building rather than repairing it after the damage as adopted by many countries and observed the crack opening in the masonry wall using the Digital Image Correlation technique. The quantitative pieces of information are obtained through two experimental samples, which can be useful for comparison and validation for the numerical model to carry out a parametrical analysis. This work is a part of a multi-scale experimental campaign to characterize seismic bands' behavior and then validate the numerical model developed at our laboratory.

In the following subsection 1.1, the background about the use of the seismic band, their recommendation as per various guidelines, and codes are included. Subsection 1.2 allows one to understand the major failure modes in masonry and appreciate the advantages of having a seismic band (subsection 1.3) in the masonry structure.

## 1.1 Seismic band

Seismic bands are a constructive device-specific to traditional masonry constructions in seismic zones. The first traces come from the Minoan period - the bronze age - in Crete [14]. Such a device consists of placing at a level of several horizontal joints from the ground level regularly spaced pieces of wood forming a kind of ladder that is "laid" lengthwise on the masonry. Such bands may also be referred to as a ring beam in [15–19], but that is not always the case and depends on the connection between insertions. These elements can be continuous (above the openings) and look like a horizontal beam as shown in Table 1 (at lentil, roof, plinth level) or discontinuous (at the level of the openings) as shown in Table 1 (at sill level), even punctual (at the level of the angles). These inclusions are usually made of timber or reinforced concrete in fired or raw bricks (adobes) or stone masonry. Sometimes, one can also find longer stones used as a band in stone masonry. Nepal National Building Code [20] also mentioned using bamboo as a seismic band in NBC 203. Table 1 shows the recommendation for seismic band location and material that can be used for such band by different codes and guidelines for earthquake resistant design for masonry structures. As per our knowledge and finding, the various code's current provisions were prepared by the team of experts at International Association for Earthquake Engineering during 1980s [16] and consequently adopted by different countries.

Table 1 Guidelines with the recommendation for seismic band location and material

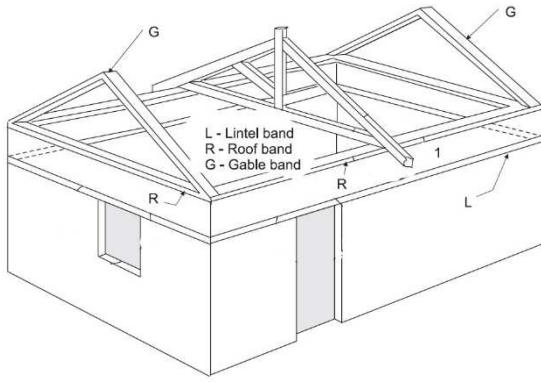
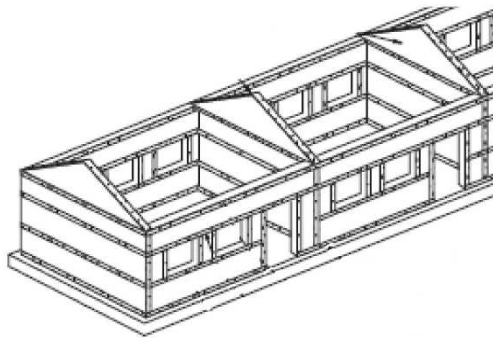
Guidelines and building codes	Seismic band material	Location
<p>Guidelines for earthquake resistant non-engineered construction[16]</p> <p>Indian standard [21]</p> <p>UNESCO Guideline [15]</p> <p>Earthquake resistant Guidelines-Afghanistan [22]</p>	<p>Reinforced concrete</p> <p>Timber</p>	 <p>Lintel, Roof, and Gable</p>
<p>NBC 203, National Building Code- Nepal [23]</p> <p>Earthquake resistant Guidelines-Pakistan [24]</p> <p>Building reconstruction guidelines for Nepal [25]</p>	<p>Reinforced concrete</p> <p>Timber</p> <p>Bamboo</p>	 <p>Plinth, sill, lintel, roof, and gable</p>

Table 2 shows that longitudinal timber members' dimension has some variation, which might be due to construction and material restriction in the region. For a reinforced concrete band, all the guidelines and reference have the same recommendation for the dimension.

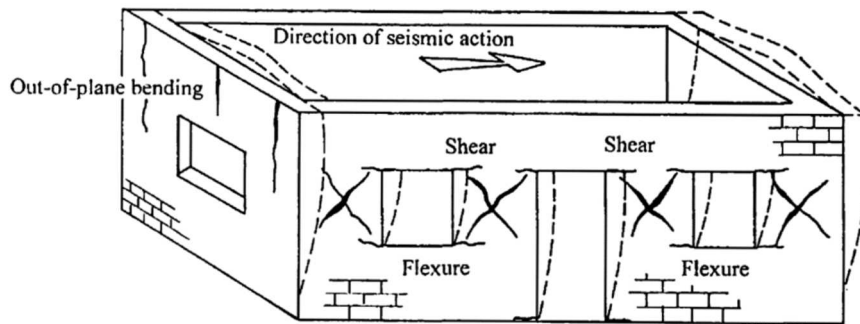
Table 2 Dimension specification for timber and reinforced concrete band

Timber seismic band	Reinforced concrete seismic band
<p>Longitudinal member Dimension:</p> <p><b>75 mm x 38 mm</b> [15,22,24,26]</p> <p><b>75 mm x 45 mm</b> [25]</p> <p><b>100 mm x 50 mm</b> [27]</p> <p>Transverse member spacing =<b>500 mm</b></p>	<p><b>75 mm</b> thick with two longitudinal rebars 12 mm diameter</p> <p><b>150 mm</b> thick with four longitudinal rebars at lintel level</p> <p>Tie spacing <b>150 mm c/c</b> using 6 or 8 mm diameter rebar</p> <p>[15,20,22,24–27]</p>

### 1.2 Failure modes

Usually, the interface is the weak bond between mortar and units in masonries. This low tensile strength creates a key reason for most of the damage [28]. Damage in the structure is directly influenced by the seismic event, which has its characteristics. Each seismic event is characterized by its horizontal and vertical ground motion components, which are influenced by the nature of

fault rupture, the distance between source to site, ground condition, and magnitude [29]. The structures' failure pattern can vary depending on the nature of the material used, bond strength, normal axial loading due to roof and other superstructure on the wall, placement of opening, and seismic loading. Fig. 1 shows the global failure mode in a masonry structure when submitted by seismic loading action. However, these entire failure modes are not necessarily occurring during any event. In the case of an out-of-plane wall, the failure can be due to flexural bending or corner separation or the wall's delamination. For the in-plane wall, it could be due to diagonal shear crack, shear sliding, or flexural bending/rocking. Suppose the crack formation and propagation can be limited by any means. In that case, the structural damage can be minimized and, most importantly, save the people's lives during a seismic disaster event. One such technique is the use of the horizontal seismic band.



*Fig. 1 Various failure mode in a masonry structure [30]*

### 1.3 Advantages of seismic band

Several experimental campaigns were conducted by submitting a masonry structure with a seismic band to dynamic loading using a shaking table [31–33]. They highlighted some advantages of inserting seismic bands. Their integration in masonry structures helps to improve integrity and ductility. If regularly spaced, bands prevent crack propagation and are limited within the region separated by the seismic band [34]. Moreover, the creation of a fused interface contributes to dissipating energy with minor damage. The dissipated energy depends on the material used and the connection between elements (as observed with timber band) [35].

Having past evidence from the ancient traditional structure and some of the qualitative performance behavior of the seismic band application highlights the benefit of using it. But, few more question arises, when we check the available guidelines and codes of some nations. What should be the suitable thickness of the band? Does the spacing between the band matter? Is there any major change in the behavior when the material used as a band is changed? Answer to such questions requires rigorous analysis, which becomes difficult with the only experiment, and hence, the need for numerical modeling to carry out parametrical analysis plays a key role. The accuracy of the numerical model depends upon the constitutive laws and validation with the experimental results. Thus, our experimental campaign's objective is to obtain reliable information that addresses the structural behavior in a localized region and can help validate the numerical model in the future.

## 2. Materials and methodology

As per the reconstruction guidelines of Nepal [25], there are suggested using a vertical reinforcement bar at the corner and opening. However, from the study of [8], internal placement of vertical reinforcement proved to create stress concentration and make the structure weak. Also, data from the site suggests difficulties in placing the vertical reinforcement bar. Therefore, we have considered a wallet-size sample with and without a band to determine in-plane behavior for this study. A quasi-



static cyclic loading test is used to determine the influence of a horizontal seismic band in a masonry wall. The shear resistance of the wall can be obtained by applying the in-plane loading [36], but for this study, we have compared the crack surface area with the corresponding force rather than the classical hysteresis curve. The loading pattern was selected based on the recommendation of [37] for the quasi-static loading cycle. The general experimental setup is shown in Fig. 2, where the loading actuator/jack, camera setup, and specimen location are indicated. With the help of a C-shaped metallic connected to the actuator (graphically indicated in Fig. 3), it is possible to apply cyclic loading by changing the loading direction. Two masonry walls (dimension 0.91 m in length, 0.86m in height, and thickness of 0.32m) are built with and without a seismic band. Timber was used as a seismic band with a cross-section dimension of 70 mm x 50 mm and arranged in the form of the ladder as shown in Fig. 4 using nail connection. Earth (adobe) bricks are used to construct the walls using mud mortar with a thickness of 20 mm. A reinforced concrete stud is cast around the first brick bed, and a metallic strip is added to the ends, which is fixed using nuts and bolts. The compressive strength of brick-and-mortar ranges from 1.2 MPa to 1.4 MPa when tested individually, and with the compression test on the wallet, the average compressive strength of about 1 MPa was obtained. These values are necessary for determining the loading conditions for the test. A speckle pattern on the wall surface, which is needed for carrying out digital image correlation (DIC), is created using black and white aerosol spray paint. DIC was quite useful in the past few decades to get information at the local level without the use of any sensor on the specimen. The digital image correlation technique allows for a continuous record of cracks formation all along the displacement cycle.

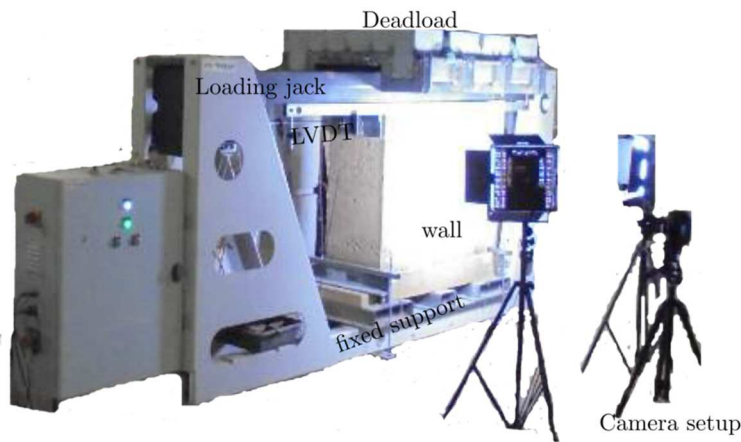


Fig. 2 General experimental set up for the quasi-static test

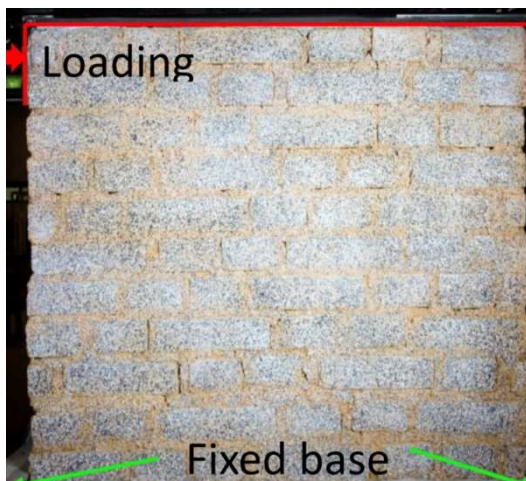


Fig. 3 Masonry wall without a band (left) and with timber seismic band (right)

A dead load of 9.8 kN is added on the top of the wall, representing that of a roof and any other structure supported by the wall. This dead mass is uniformly applied on the wall creating normal axial stress of 0.03 MPa. A cyclic displacement until a maximum amplitude of 80 mm is applied on the wall, as shown in Fig. 5, at a variable rate of displacement (see Fig. 6).

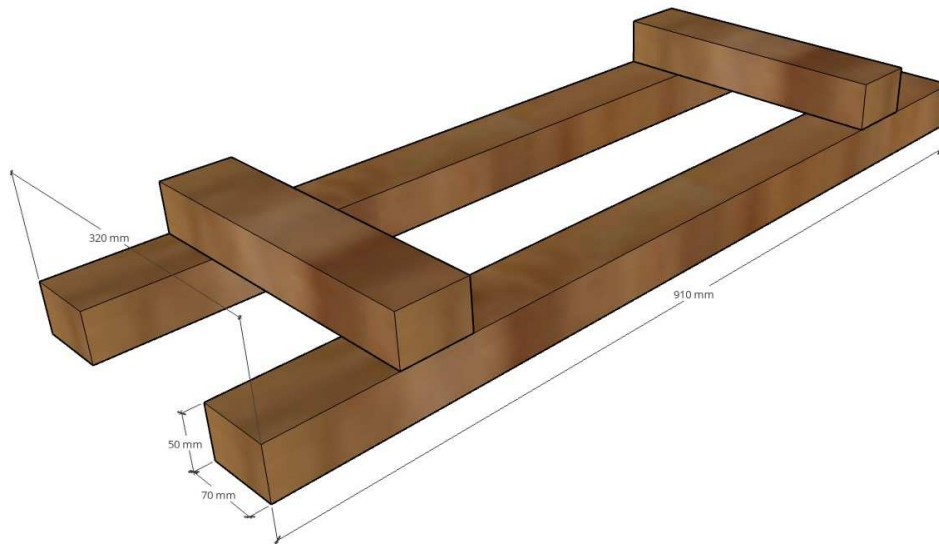


Fig. 4 Dimension of timber seismic band used

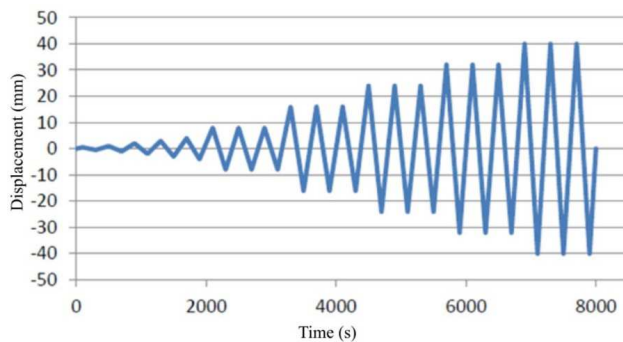


Fig. 5 Displacement sequence for the quasi-static cyclic tests applied to the jack

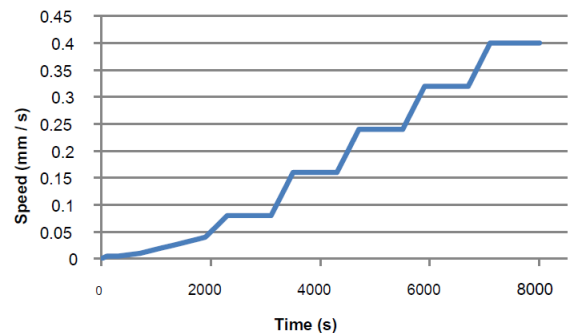


Fig. 6 Displacement loading rate

### 3. Results

Two masonry walls are quasi-statically loaded. The measurement of force recorded from the sensor associated with the jack and displacement recorded using the LVDT sensor at the top of the wall are used to generate a hysteresis curve, as shown in Fig. 7. From this plot, we observe that both walls' behavior is similar until 5 mm horizontal displacement, after which we notice changes in the nature of the curve. For the wall without the seismic band, the peak force value is increased more than the one with a timber band. However, the maximum displacement of the wall with the band is higher due to the wall's sliding behavior at the interface between the seismic band and the adobe brick.



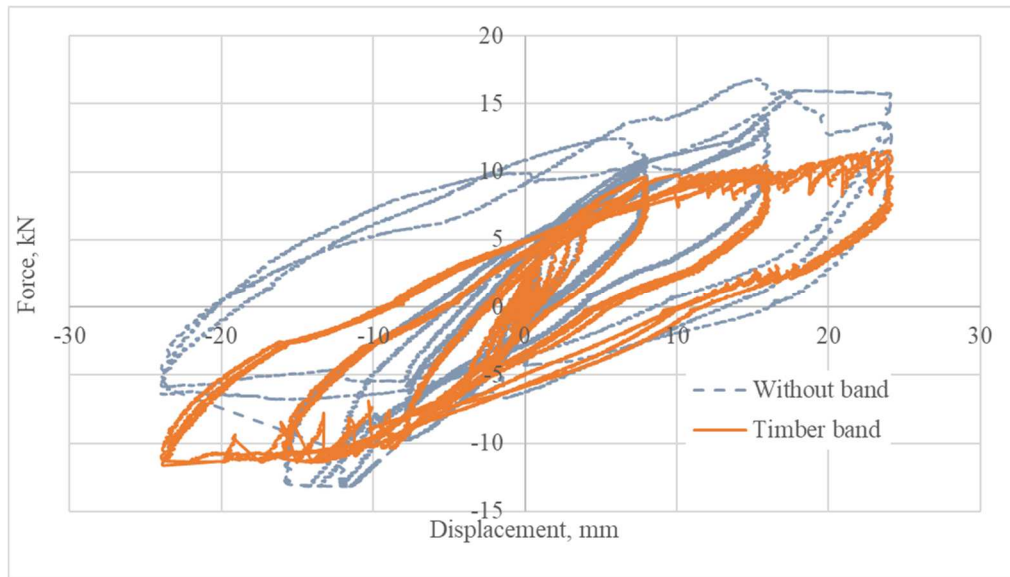


Fig. 7 force-displacement hysteresis curve

Fig. 8 shows the same tests' envelope curves, which are constructed by taking the ultimate force values during the first cycle of targeted amplitude cycles and their corresponding displacement point. From this plot, we observe that the force value increases in both loading directions for the wall without band because the changing direction develops new crack/fissures in the sample. Whereas, for the wall with timber band, the force is higher during the first cycle with an increasing amplitude indicated by the negative force-displacement part of the curve. This phenomenon could be due to the sliding behavior at the interface of the band and brick joint.

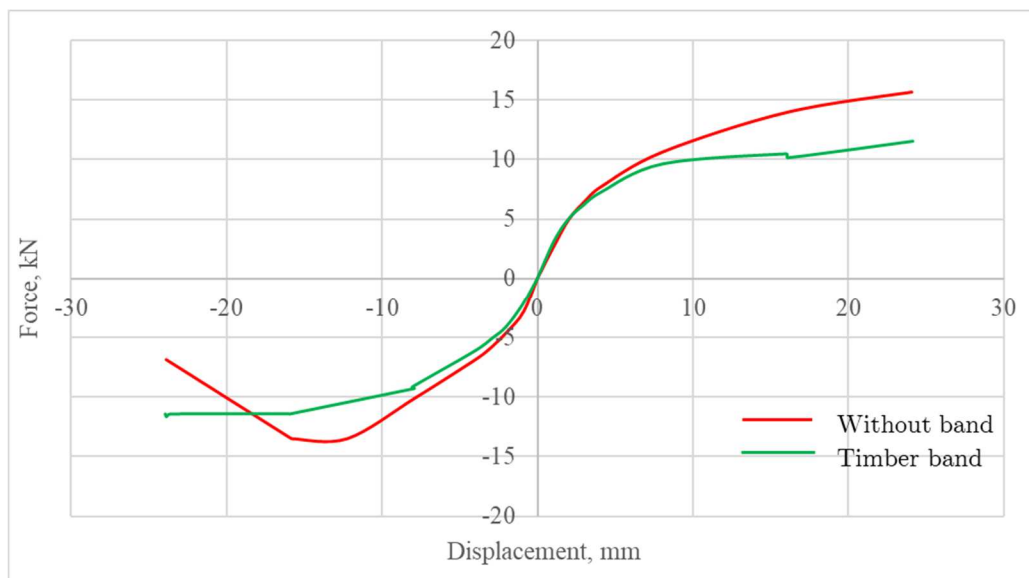


Fig. 8 Envelope curve for two wall samples

The energy dissipation is calculated from the hysteresis curve, taking the area under the curve for three classified cycles (Fig. 10) and tabulated in Table 3. The values indicated here are the total energy dissipated by the global behavior of the wall. The value of energy dissipation by the wall without band is more because of the more force required to reach the targeted displacement and more permanent damage on the wall.

Table 3 Energy dissipation from hysteresis curve

	Energy dissipation, kN-mm	
	Without band	Timber band
Cycle 1	303	241
Cycle 2	848	605
Cycle 3	1653	1154

The entire experiment is recorded with a high-resolution camera with images captured at an interval of 10 seconds. These images are later analyzed using the image-correlation software 'Tracker' [38]. The imaging was carried out for the only surface, but the crack propagation on both the surface was verified and observed uniformity regarding crack development. Each brick is associated with four points placed on its corners, forming quadrilateral, and the position of each brick is tracked image by image. The corner of the whole or fragmented brick was manually located by comparing the first image with the last one to avoid the error during DIC due to crack formation or missing part at the measurement point, which is the reason why the shape of the brick looks irregular, as seen in Fig. 9. The horizontal force's value applied on the wall at each stage is associated with the captured image in the analysis. Thus, results obtained with the two walls using DIC can be compared with the corresponding force applied at a particular instant. Fig. 9 shows the DIC output results plotted graphically using MATLAB.

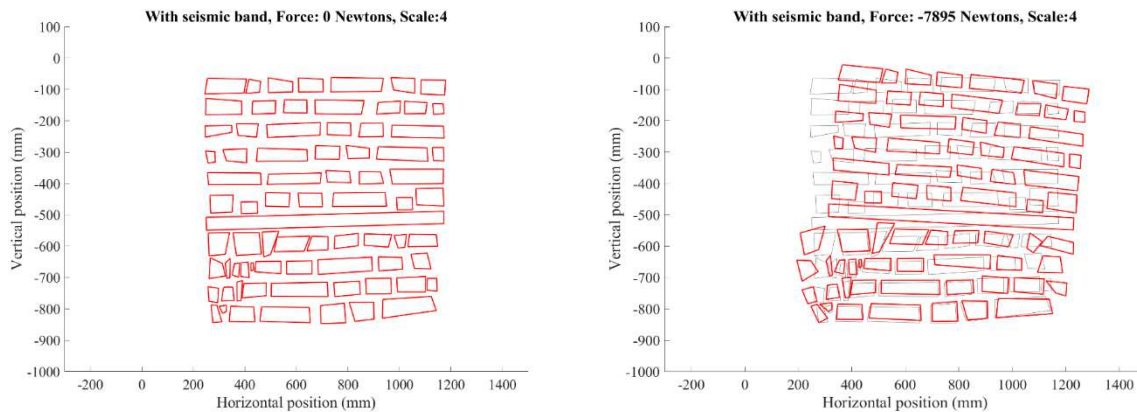


Fig. 9 DIC displacement output field for masonry wall with timber band at zero force (left) and 7.89 kN force (right)

As the loading in the horizontal direction is applied, the crack opening at header joints (vertical fissures) increases significantly. A process is established to evaluate the pattern of vertical cracks. Each crack is viewed as a quadrilateral made of four points, two from each side of adjoining bricks. Coordinates are calculated to enlarge the cracks only when there is a difference in the displacement values between two bricks. This displacement obtained from DIC is multiplied with the head or bed joint's length to calculate the surface area of fissures in a vertical or horizontal direction, respectively. For comparing the results of the two cases, it is important to consider deformation stages. For that, three cycles were chosen with constant displacement amplitudes, as shown in Fig. 10. The following analysis is carried out for each cycle using cracks surface area total variation in square millimeters extending upwards in the y-axis direction, dependent on the force value in the x-axis direction. The x-axis consists of the force applied by the loading actuator. The positive part indicates pulling force

(represented with positive force and displacement when the wall is moved toward the left), and the negative part indicated the pushing force (represented with negative force and displacement when the wall moves toward the right).

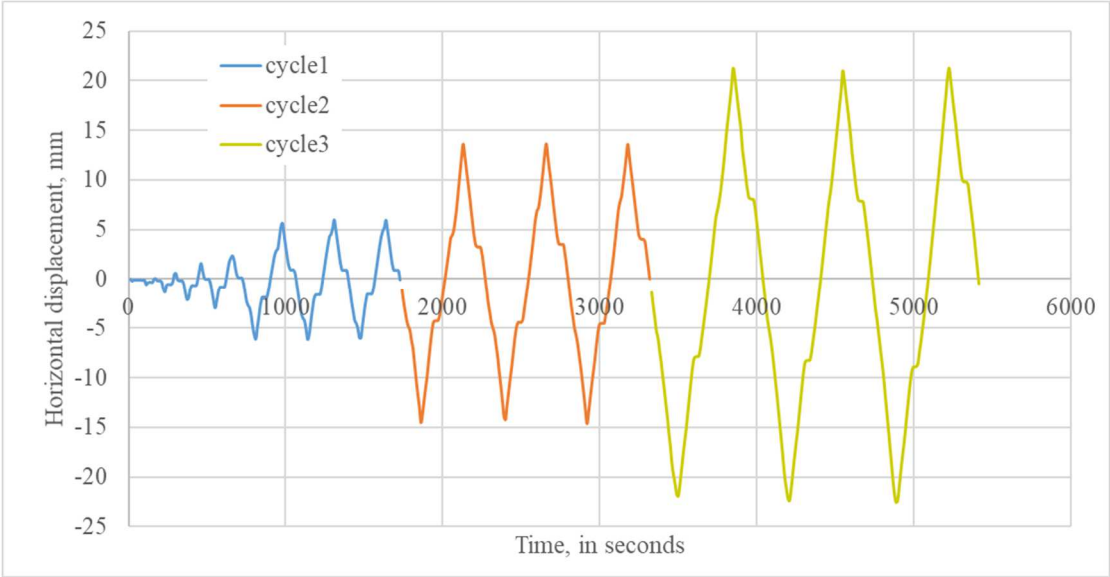


Fig. 10 Classification of displacement cycle for a wall with a band

Four main observations can be done throughout the first loading cycle that is shown in Fig. 11. Here, the cycles are classified by the peak horizontal displacement applied to the walls, and each cycle indicated has three periods of oscillation. First, in the beginning, both walls have similar behaviors. Crack opening increases at a slow pace, and deformation is characterized by almost perfect elasticity. Bonding between bricks is still intact, so walls behave like an entire unit. Second, the wall with a horizontal band suffers 8-shaped fluctuations in the crack opening, which is due to cracks below the band that are enlarged and then closed again while slightly ripping and drifting towards permanent deformation. Third, when a force pushes the wall without a seismic band towards the right (force is negative in this case), we can see an obvious increase in the crack opening. It reveals the appearance of the first major diagonal crack in the wall without any band. We can observe sorts of steps, or a stair-like shape, while fissures jump from 100 mm<sup>2</sup> to around 520 mm<sup>2</sup>. These sharp increases are the result of abrupt disconnections between certain bricks. Fourth, at the end of the first cycle, due to the newly formed crack, not much (negative) force is needed to impose displacement in the right direction. Then, the semi-detached block of bricks smoothly glides at every displacement cycle of constant amplitude.

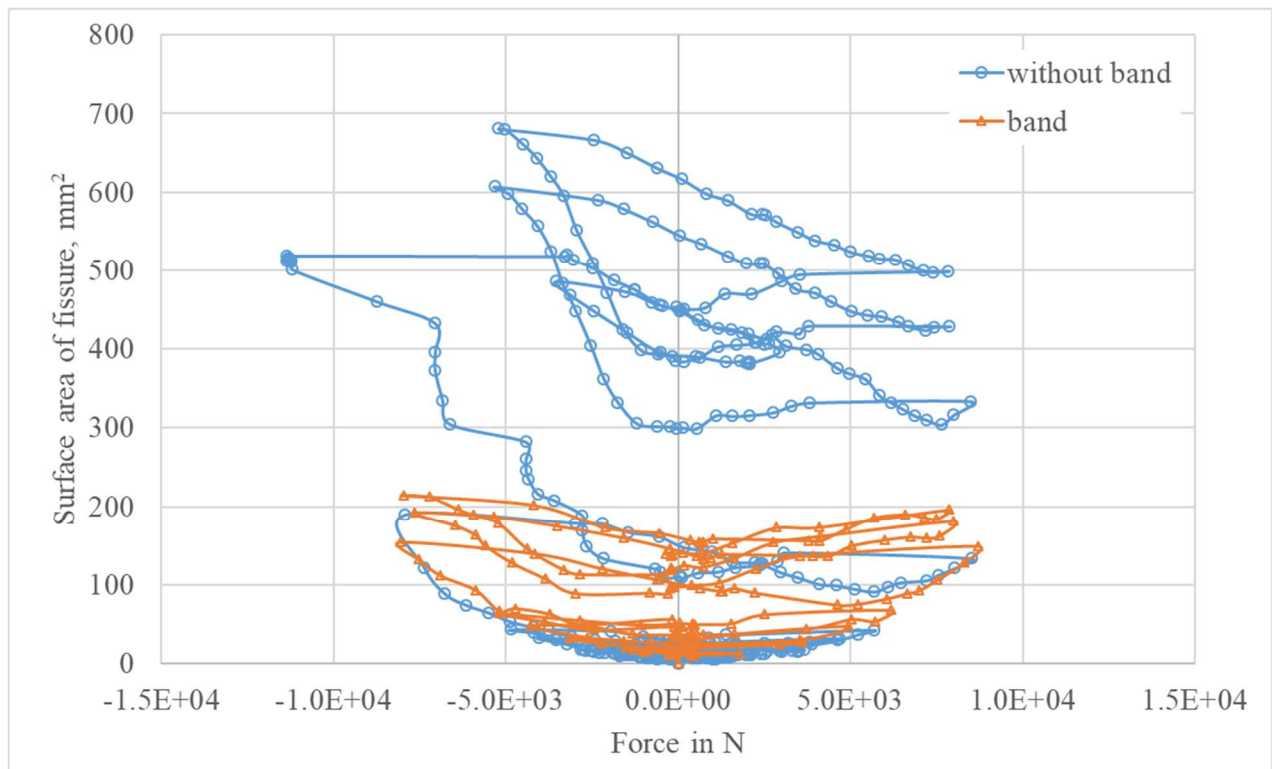


Fig. 11 Vertical fissure area in relation to the applied force during the first cycle

During the second cycle (Fig. 12), a more regular evolution pattern is observed. The phenomena previously described during the first cycle seem to predefine the behavior during the second cycle. The previous loading cycle remnants are immediately noted; the surface area is around 200 mm<sup>2</sup> for the wall equipped with a band and around 680 mm<sup>2</sup> for the wall without any band at the beginning of the second cycle. The orange curve is smoother and narrower than the blue one. This shows a very gradual and stable enlargement of cracks between the bricks. On the other hand, the blue curve shows a less stable pattern.

For both the wall, the surface area of cracks increases way faster at every round. Secondly, a steep decline in the surface area of crack is noticed in every direction change of the applied force. That is because separated blocks slide back to their initial positions. Since the first crack was seen on the right side of the wall when the force is negative, both increase and decrease of crack opening follow the same trend. However, when the force is positive, the wall is pushed on the left, the first left-side cracks appear on the wall without any band. Therefore, the blue curve on the top right side of the graph increases a lot more than the orange one. The maximum surface area of fissures obtained during the second cycle is approximately 800 mm<sup>2</sup> and 1620 mm<sup>2</sup> respectively, for the wall with a band and without a band.

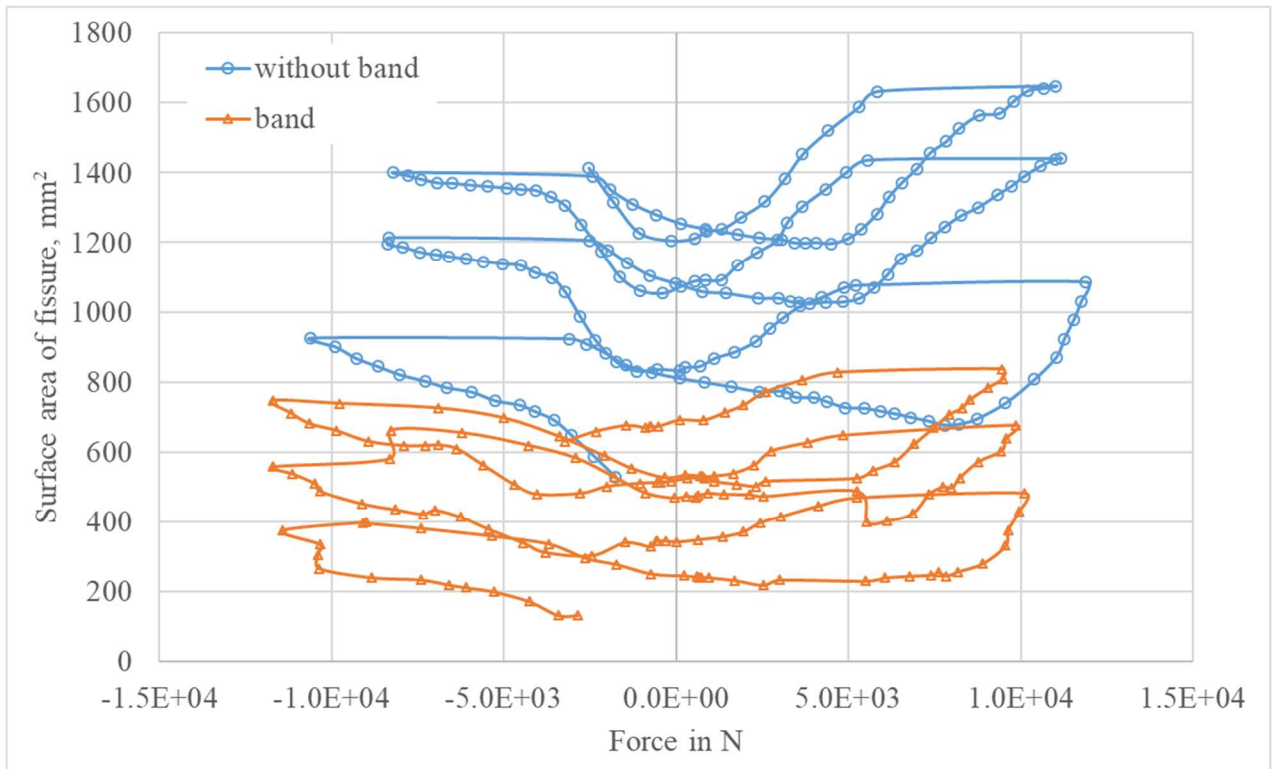


Fig. 12 Vertical fissure area in relation to the applied force during the second cycle

During the third cycle (Fig. 13), the behavior of the wall with the band is still the same, and the orange line follows the same trend as in Fig. 11 and Fig. 12. This means that integrity is preserved in a wall with the seismic band until the end of the third cycle of displacements. The same cannot be said about the behavior of the wall without any band. The blue line shows that the crack surface area seldom decreases in value. Older cracks expand, and new cracks in the wall's inner parts are created whenever the imposed force is at its highest. Another interesting thing to notice, in the case of the wall with a band, is that while force switches directions, the crack surface area remains almost constant. This happens because both the upper and lower part of the wall has been disconnected at the band level, so instead of expanding, cracks simply shapeshift from rectangles into parallelograms (when pushed or pulled from the initial location) while covering the same surface as shown in Fig. 14 and Fig. 15.



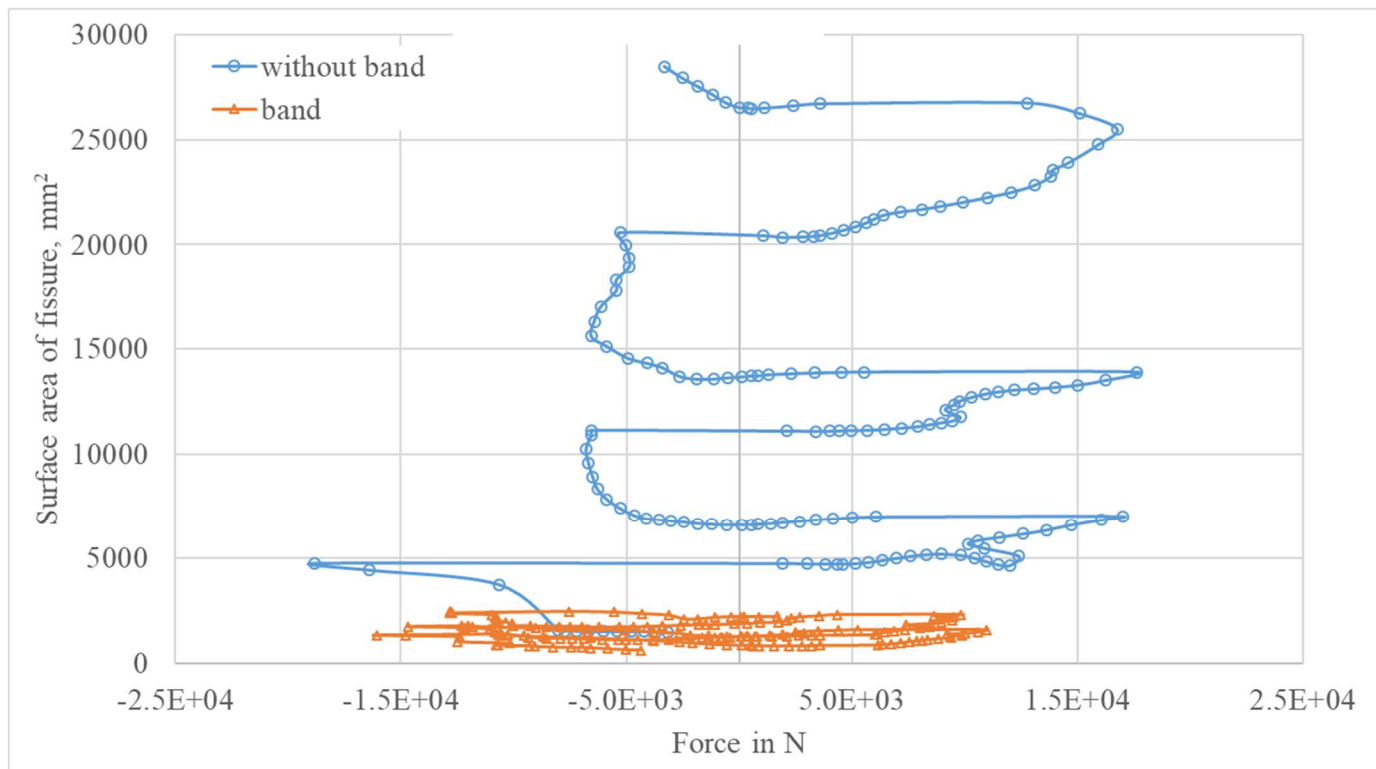


Fig. 13 Vertical fissure area in relation to the applied force during the third cycle



Fig. 14 Wall with timber band at 4560 seconds of loading at peak displacement towards left



Fig. 15 Wall with timber band at 4880 seconds of loading at peak displacement towards the right

#### 4. Discussion

The results presented in this paper show a correlation between the dissipation energy and the amount of crack opening area. Indeed, the wall without a seismic band has both dissipation energy and a quantity of crack opening area much stronger than the wall with the seismic band. This result may seem counter-intuitive but perfectly logical because the seismic band limits the masonry's damage and makes it possible to locate the slip at its interface. Without a seismic band, the slip between the bricks is stronger, leading to both an increase in dissipation energy and a loss of structural stability of the whole. The higher crack area shows more damage to the masonry structural integrity. From this, one could conclude that the wall without the band tends to have several major cracks, which might increase the cost for retrofitting, if possible, in the structure after a seismic event. However, when the



fissure surface area is small (as seen with the wall with a seismic band), the occupant's structural integrity and safety are maintained. Such structures can be easily retrofitted at a reasonable cost. The horizontal cracks have also been studied, but results are not conclusive, partly because the opening of horizontal cracks is low due to the axial loading caused by the dead mass on the wall's top. This phenomenon is also seen in the real structure where the gravitational load acting on the wall is high, and the seismic signal has significant horizontal components.

The failure occurred through the mortar joints and propagated along the horizontal and header joint. Some of the bricks were fragmented during the crack propagation, as seen in Fig. 14 and Fig. 15.

## 5. Conclusions

The crack opening size below 50% of ultimate displacement stays similar in both the walls (with or without timber band). After that limit, we have a bigger crack size for the wall without a band. Such cracks are not visible easily, but thanks to the DIC technique, we can capture the small variation in the displacement, which helps understand why there was a drop in the value of force after each cycle's first peak. For a wall sample with a timber band, the cracking opening change is gradual, contrary to that of the wall without a band. In the wall with a horizontal seismic band, the crack was formed in the lower part, which did not propagate to the upper part. This also highlights the benefit of a seismic band in limiting the crack, and once the crack reaches the interface with the band, it propagates horizontally, leading to a sliding mechanism between the interface of the band and the adjacent masonry layer, which helps in energy dissipation. The results of a comparison of two walls with respect to the crack opening and corresponding force are helpful to understand the shear behavior of the wall from a distinct perspective apart from the hysteresis curve, where we cannot have a detailed illustration of behavior and crack formation.

## 6. Perspectives

We studied the influence of the horizontal loading applied quasi-statically on the wall with and without a timber seismic band on crack opening size during this study. But further studies are needed with the use of the RC band and to figure out how such a band influences the structure's behavior. In design code, mostly horizontal seismic force is only considered, but there are some events where the vertical ground motion becomes significant. Therefore, the study on such band behavior needs to be made for both horizontal and vertical components. It would be interesting to extend the study to other types of interface material such as reinforced concrete and bamboo as per different guidelines. The preliminary study [35] shows that sliding and energy dissipation depends on the material and its surface roughness and connection type. Therefore, it is important to carry out a similar test with other materials to verify the benefits of having such a band. From this study, we saw the crack propagation, and damage in a wall with timber seismic band was limited in the bottom part (see Fig. 9). What will happen when several such sections are separated with multiple layers of the band must be investigated. Moreover, structural behavior varies during quasi-static and dynamic loading. In the real scenario, the earthquake loading is dynamic where the damping and inertial force play a key role. Therefore, it is also necessary to investigate the seismic band's performance when impacted by dynamic loading using shaking table tests and numerical modeling for parametrical analysis. In our multi-scale experimental campaign, we are going to test the reduced scale house model using seismic band under dynamic shake table test, more quasi-static test using wallet sample, shear behavior at the interface, along with material property characterization. Similarly, we are developing a numerical model developed using a macro element that will use our experimental results and then carry out parametrical analysis of the whole house structure.

## References

- [1] Population W. Total Population by Country 2019 2019. <http://worldpopulationreview.com/countries/> (accessed July 4, 2019).
- [2] Barbacci N. Earthen Architecture – Valorisation and Underestimation. ISPRS - Int Arch Photogramm Remote Sens Spat Inf Sci 2020;XLIV-M-1–2:1073–80. <https://doi.org/10.5194/isprs-archives-xxiv-m-1-2020-1073-2020>.
- [3] Hofmann M. Le facteur séisme dans l’architecture vernaculaire . Un décryptage entre déterminants culturels , types de structures et ressources cognitives parasismiques. 2015.
- [4] Langenbach R. Survivors in the Midst of Devastation: Traditional Timber and Masonry Construction in Seismic Areas. Seventh US Natl Conf Earthq Eng Urban Earthq Risk, July 21-25, 2002,Langenbach, R (2002) Surviv Midst Devastation Tradit Timber Mason Constr Seism Areas Seventh US Natl C 2002.
- [5] Langenbach R. “Earthquake Resistant Traditional Construction is NOT an oxymoron”:The resilience of timber and masonry structures in the Himalayan Region and Beyond and its relevance to heritage preservation in Bhutan. Int. Conf. Disaster Manag. Cult. Herit., The Royal Government of Bhutan; 2010, p. 1–25.
- [6] Wang C, Sarhosis V, Nikitas N. Strengthening/Retrofitting Techniques on Unreinforced Masonry Structure/Element Subjected to Seismic Loads: A Literature Review. Open Constr Build Technol J 2018;12:251–68. <https://doi.org/10.2174/1874836801812010251>.
- [7] Reyes JC, Rincon R, Yamin LE, Correal JF, Martinez JG, Sandoval JD, et al. Seismic retrofitting of existing earthen structures using steel plates. Constr Build Mater 2020;230. <https://doi.org/10.1016/j.conbuildmat.2019.117039>.
- [8] Dowling D. Seismic strengthening of Adobe mud brick houses. University of Technology, Sydney, 2006.
- [9] Bitar R, Saad G, Awwad E, El Khatib H, Mabsout M. Strengthening unreinforced masonry walls using natural hemp fibers. J Build Eng 2020;30:101253. <https://doi.org/10.1016/j.jobte.2020.101253>.
- [10] Sathiparan N, Mayorca P, Meguro K. Shake table tests on one-quarter scale models of masonry houses retrofitted with PP-band mesh. Earthq Spectra 2012;28:277–99. <https://doi.org/10.1193/1.3675357>.
- [11] Vintzileou E. Timber-reinforced structures in Greece: 2500 BC-1900 AD. Proc Inst Civ Eng Struct Build 2011;164:167–80. <https://doi.org/10.1680/stbu.9.00085>.
- [12] Vlachakis G, Kleanthi M. Learning from failure: Damage and Failures of Masonry Structures, after the 2017 Lesvos earthquake (Greece). Eng Fail Anal 2020. <https://doi.org/10.1016/j.engfailanal.2020.104803>.
- [13] De Filippi F, Pennacchio R, Restuccia L, Torres S. Towards a Sustainable and Context-Based Approach To Anti-Seismic Retrofitting Techniques for Vernacular Adobe Buildings in Colombia. ISPRS - Int Arch Photogramm Remote Sens Spat Inf Sci 2020;XLIV-M-1–2:1089–96. <https://doi.org/10.5194/isprs-archives-xxiv-m-1-2020-1089-2020>.
- [14] Ortega J, Vasconcelos G, Correia MR. Seismic-resistant building practices resulting from Local Seismic Culture. Seism Retrofit Learn from Vernac Archit 2015:17–22. <https://doi.org/10.1201/b18856-5>.
- [15] Arya AS, Boen T, Ishiyama Y. Guidelines for Earthquake Resistant Non-Engineered Construction. Paris, France: UNESCO; 2013.
- [16] Arya AS, Boen T, Ishiyama Y, Martemianov AI, Meli R, Scawthorn C, et al. Guidelines for Earthquake Resistant Non-Engineered Construction 2004.
- [17] Blondet; M, M. GVG, Brzev S, Rubinos A. Earthquake-Resistant Construction of Adobe Buildings: A Tutorial. vol. second. EERI/IAEE World Housing Encyclopedia; 2011. <https://doi.org/10.1227/01.NEU.0000255422.86054.51>.
- [18] Bothara J, Brzev S. A Tutorial: Improving the Seismic Performance of Stone Masonry Buildings. Oakland,CA, USA: Earthquake Engineering Research Institute; 2011.
- [19] IS4326. Earthquake resistant design and construction of buildings- Code of practice. India: 2013.

- [20] NBC203. Guidelines for Earthquake Resistant Building Construction-Low Strength Masonry. Nepal: Department of Urban Development and Building Construction; 1994.
- [21] IS13828. Improving earthquake resistance of low strength masonry building-Guidelines. India: 1993.
- [22] Arya AS. Guidelines for Earthquake Resistant Design, Construction, and Retrofitting of Buildings in Afghanistan. Hyogo: Ministry of Urban Development and Housing, Government of Afghanistan and United Nations Centre for Regional Development; 2003.
- [23] NBC203. Guidelines for earthquake resistant building construction: Low strength masonry. Nepal: Department of Urban Development and Building Construction; 2015.
- [24] UN-Habitat, L'urgence A de, NSET-Nepal. Guidelines for Earthquake-Resistant Construction of Non-Engineered Rural and Sub Urban Houses in Pakistan. Pakistan: 2006.
- [25] DUDBC. Design Catalogue for Reconstruction of Earthquake Resistant Houses. vol. 1. Kathmandu: 2015.
- [26] IS13828. Improving earthquake resistance of low strength masonry building-Guidelines. India: Bureau of Indian Standard; 1993.
- [27] Bothara J, Brzev S. A Tutorial: Improving the Seismic Performance of Stone Masonry Buildings. Oakland,CA, USA: Earthquake Engineering Research Institute; 2011.
- [28] Roca P, Lourenço PB, Gaetani A. Historic construction and conservation-Materials, systems and damage. New York: Taylor & Francis; 2019.
- [29] Kramer SL. Geotechnical Earthquake Engineering. New Jersey: Prentice Hall; 1996.
- [30] Tomažević M. Earthquake-Resistant Design of Masonry Buildings. vol. 1. London WC2H 9HE: Imperial College Press; 1999. [https://doi.org/doi:10.1142/9781848160835\\_fmatter](https://doi.org/doi:10.1142/9781848160835_fmatter).
- [31] Mouzakis C, Adami C, Karapitta L, Vintzileou E. Seismic behaviour of timber-laced stone masonry buildings before and after interventions : shaking table tests on a two-storey masonry model. Bull Earthq Eng 2017. <https://doi.org/10.1007/s10518-017-0220-9>.
- [32] Wang M, Liu K, Guragain R, Shrestha H, Ma X. Shake table tests on the two - storey dry - joint stone masonry structures reinforced with timber laces and steel wires. Bull Earthq Eng 2018. <https://doi.org/10.1007/s10518-018-00528-6>.
- [33] Bothara J, Ahmad N, Ingham J, Dizhur D. Experimental Seismic Testing of Semi- Reinforced Stone Masonry Building in Mud Mortar. Pacific Conf. Earthq. Eng., Auckland, New Zealand: New Zealand Society for Earthquake Engineering; 2019.
- [34] Aranguren J, Vieux-champagne F, Duriez M, Aubert J. Experimental analysis of timber inclusions effect on paraseismic behavior of earth masonry walls. Eng Struct 2020;212:110429. <https://doi.org/10.1016/j.engstruct.2020.110429>.
- [35] Yadav S, Sieffert Y, Créte E, Vieux-Champagne F, Garnier P. Mechanical behaviour of different type of shear band connections being used in reconstruction housing in Nepal. Constr Build Mater 2018;174:701–12. <https://doi.org/10.1016/j.conbuildmat.2018.04.121>.
- [36] Reyes JC, Smith-Pardo JP, Yamin LE, Galvis FA, Sandoval JD, Gonzalez CD, et al. In-plane seismic behavior of full-scale earthen walls with openings retrofitted with timber elements and vertical tensors. Bull Earthq Eng 2019;17:4193–215. <https://doi.org/10.1007/s10518-019-00601-8>.
- [37] ISO21581. Timber structures-static and cyclic lateral load test methods for shear walls. 2010.
- [38] Combe G, Richefeu V. Tracker : a Particle Image Tracking ( PIT ) Technique Dedicated to Nonsmooth Motions Involved in Granular Packings. AIP Conf. proceeding 1542, 461, UNSW, Sydney, Australia: 2013, p. 9–12. <https://doi.org/10.1063/1.4811967>.