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Effect of Fly Ash on microstructural and resistance characteristics of dredged sediment stabilized with lime and cement --Manuscript Draft--

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Abstract:	Contribution of fly ash (FA) as a complementary additive for dredged sediment (DS) stabilization was studied. The study is focused on definition of an efficient combination(s) to raise the DS properties. FA with lime and cement with both was used. Then, micro and macroscale investigations was performed. Test results demonstrate that FA additions promoted nucleation, formation of cementitious compounds and fabric modifications in sediment. DS stabilized using FA component show lower shrinkage and higher mechanical resistance than that stabilized using conventional binders. FA is found to be efficient in sediment-lime and sediment-cement mixtures since it accelerates cementation and strength gain.

1 Effect of Fly Ash on microstructural and resistance characteristics of dredged 2 sediment stabilized with lime and cement

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22 23 24 15 Highlights

- 25
26 16 - FA acts as filler when incorporated with sediment at early age
- 27 17 - FA promotes nucleation, cementitious compound growth resulting in porosity refinement
- 28 18 - FA reduces shrinkage and increases strength gain of mixtures, regardless curing time
- 29 19 - The effectiveness of FA addition is confirmed when used with sediment-lime and sediment-cement

30 31 32 33 34 21 Abstract

35
36 22 This study deals with the definition of an efficient combination of fly ash (FA) with lime or cement and
37 23 with both, to improve the dredged sediment (DS) properties. At early age, filler and nucleation effects
38 24 of FA lead to a refinement of the microstructure in addition to the macro porosity reduction induced
39 25 by lime and cement. At long term, the microstructure becomes denser due to the pozzolanic property
40 26 of FA. At macroscale, DS stabilized using FA show lower shrinkage and higher mechanical resistance
41 27 than that stabilized without FA, with more pronounced effects when FA is mixed with cement.

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46 29 **Key words:** dredged sediment, fly ash, soil stabilization, mechanical resistance, microstructure
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49 50 31 1. Background

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53 32 Selection of pavement materials is based on mechanical properties such as strength and stiffness. This
54
55 33 procedure is adequate in case of raw and inert materials are used. In stabilized materials, physical
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57 34 changes and chemical reactions take place [1][2]. Therefore, it is important to identify modifications
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2 35 which occur at different levels, in order to determine how the action mechanisms of different additives
3 36 affect soil resistance.
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5 37 Multiscale analysis leads to a comprehensive knowledge of improvement of stabilized soil using
6
7 38 hydraulic binders. In such a way that microscale analysis provides evidences of pozzolanic reactions,
8
9 39 cementing material formation and evolution of crystalline phases, and macroscale analysis explains
10
11 40 enhances in mechanical properties resulted from previous interaction highlighted at microscale level
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13 41 [3][4].
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17 42 At present time, Portland cement and lime are considered as the most convenient stabilizers for soil
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19 43 [2]. In soils stabilized using Portland cement, soil gradation changes since cement grains may fill a little
20
21 44 portion of the soil voids [5] [6]. Hence, cement hydration is responsible for the significant strength gain
22
23 45 over time. The mechanism of interaction between the different components can be explained as
24
25 46 follows: Portland cement hydrates using available water in the soil and form a stone-like material
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27 47 [3][7].
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32 48 Typical mineral components of Portland cement are calcium and silicon which are often in the form of
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34 49 oxides, such as CaO, SiO₂, Al₂O₃ and FeO₂. The main clinker constituents are tricalcium silicate (C₃S),
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36 50 dicalcium silicate (C₂S), tricalcium aluminate (C₃A), and calcium ferroaluminate (C₄AF). To control
37
38 51 reaction rate, gypsum (C \bar{S} H₂) might be added to cement.
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42 52 It is known that the hydration of clinker constituents produces cementitious compounds such as
43
44 53 calcium silicate hydrates (C-S-H), calcium hydroxide (CH). In pre induction of hydration, cement reacts
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46 54 with the available water and the most active phase C₃S produces calcium ions and OH⁻, SO₄²⁻, K⁺, Na⁺.
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48 55 In dormant stage, C₃S hydration continues and C₂S begins to be hydrated. CH separates from hydrolysis
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50 56 of C₃S and C₂S and may precipitate into empty voids. Ettringite (Ett) also forms due to the reaction of
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52 57 gypsum with C₃S and C₄AF [8].
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57 58 Over time (at early stage), C-S-H crystalline phases form an acicular morphology which branch, forming
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59 59 a honeycomb-shape structure. CH crystallizes in large crystals (~40μm), presenting hexagonal plate-
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60 shape, depending on the produced lime amount in early stages and the available free space. Ettringite

61 crystallizes in a needle-shape, with length up to 10 μ m and diameter of 0.25 μ m, which does not branch.

62 All aforementioned phases and processes have been observed in cement-based materials, included in
63 soil-cement mixtures [3][7][9].

64 At microscale level, when cement is mixed with soil, the soil-cement mixture presents a significant gain
65 of mechanical strength. Soil-cement strength results from cementation bonds and pore space
66 reduction [10][11]. Accordingly, cementitious compounds fill up the pores and connect cement grains
67 resulting in the increase in intra-aggregate pore volume [12].

68 When lime is used in soil stabilization, soil fabric changes since lime addition induces cation exchanges
69 and generates flocculation/agglomeration mechanism, in short term. In other words, water dissolves
70 some constituents of lime (CaO, CaSO₄, MgO and quartz) that react with soil and reduce double diffuse
71 layer (DDL), resulting in flocculation (agglomeration). This process reduces soil plasticity and improves
72 workability [13]. Over time, pozzolanic reactions take place at alkaline environment (pH=12), forming
73 cementitious compounds i.e. C-S-H and C-A-S-H, that are responsible for increasing of long-term
74 strength [3][10].

75 At microscale level, soil-lime strength gain is also explained based on the formation of cementitious
76 compounds from pozzolanic reactions. However, the increase of soil resistance generated by lime
77 addition is less significant than that observed in mixtures using cement; and the use of lime is
78 preferably recommended for clayey soils, in order to improve soil fabric, plasticity and workability [1]
79 [2][13]. It is also worth noting that the mechanical properties of lime-treated soils are affected by the
80 curing temperature. By studying the stiffness evolution of a silt soil stabilized with quicklime cured at
81 30 °C, Silva et al. [14] observed two different stages on the stiffness evolution suggesting the existence
82 of two different chemical phenomena involved. Evolution in the first stage seems to be mostly related
83 to the formation of calcium aluminate hydrates (CAH). However, the evolution in the second stage can
84 be more related to a structural rearrangement of CAH and the formation of calcium silicate hydrates

85 (CSH). These two distinct stages involved in the evolution of elastic modulus (E) with time suggest the
1
2 86 existence of two apparent activation energies (one for each process).
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5 87 To maximize the benefits of soil stabilization, a binary or ternary combination of hydraulic binder is
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7 88 proposed. Most of the time, lime and cement combination is recommended. Lime-cement stabilization
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10 89 combines workability enhancement from lime addition and resistance gain from cement addition. Both
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12 90 additives may be mixed without disturbing their own action mechanism. However, multicomponent
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14 91 mixed materials show differences in physical and chemical properties due to coexistence of cement
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16 92 hydration and mineral admixtures, changing hydration kinetics process and microstructure formation
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19 93 mechanisms [15]. According to [16] [17], the dosage of cement and the water content have a
20
21 94 significant impact on unconfined compression strength (UCS) values. By testing soil-cement mixtures
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24 95 with dosage of cement between 10 and 13%, Ribeiro et al. [17] observed that UCS is always larger for
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26 96 highest dosage of cement independently from the water to cement ratio, whereas independently from
27
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29 97 the dosage of cement, there is a clear optimum water content providing the maximum UCS value.
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31 98 Finally, compressive strength develops faster with time when larger water and cement ratios are
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33 99 adopted. For treatment using lime, the effects of molding water content on UCS values seems to be
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36 100 insignificant independently from the dosage of lime [18].
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39 101 For sustainable development purpose, the use of local soil, waste and industrial byproduct are
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41 102 encouraged to supply earthwork and earthen structure [6][19][20]. Two well-known industrial
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43 103 byproducts are often cited: bottom ash and fly ash. The benefits of use of bottom ash for road
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46 104 construction was been recently studied by different authors [21], [22], [23]. On the other hand, the
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48 105 use of fly ash still requires further investigation because different mechanisms of interaction need to
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51 106 be clarified.
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53 107 Researches in concrete technology has demonstrated that fly ash (FA) improves mechanical properties
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55 108 of concrete, reduces the costs of production and is ecologically beneficial. FA addition may enhance
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58 109 durability of the matrix and reduces the loss of heat energy during cement hydration [15][24].
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110 FA is a waste from coal-fired electrical power plants that is being studied as admixture in soil
111 stabilization [6][12][20]. Commonly, FA is composed by calcium, silicon and aluminium. The main
112 oxides are CaO, SiO₂ and Al₂O₃. Class C Fly ash contains about 20% of CaO which might induce cation
113 exchanges, flocculation and pozzolanic reactions, resulting in strength gain. Aluminum rich
114 composition of FA might promote a specific cementitious compounds formation, such as CAH and
115 CASH [3][7][25].

116 In concrete, FA addition stimulates reaction rate of cement hydration, promotes nucleation and
117 growth of cementitious compounds. However, FA may also retard the onset of acceleration (Stage III
118 of hydration) because of (i) inhibition of CH precipitation due to the formation of water containing
119 organic species and (b) slows formation of Ca rich surface layers on clinker phases, in the case of
120 aluminum rich FA [26].

121 The effects of FA action on soils are then physical and chemical. At physical point of view, finer particles
122 of FA would fill voids of soil particles. Chemically, pozzolanic products induced by FA presence would
123 fill pores. Both effects reduce porosity so that microstructure becomes denser. As results, strength and
124 stiffness are increased and compressibility is reduced [27].

125 Research findings suggest that FA disperses clusters of soil and strength development is controlled by
126 FA hydration [20][28][29], leading to an analogy with the aforementioned processes in cement pastes.

127 It is worth noting that water content of the fly ash stabilized soil mixture affects the strength [30]. The
128 maximum strength reached in soil-fly ash mixtures generally occurs at moisture contents below
129 optimum moisture content for density. For silt and clay soils the optimum moisture content for
130 strength is generally four to eight percent below optimum for maximum density while for granular
131 soils the optimum moisture content for maximum strength is generally one to three percent below
132 optimum moisture for density. Therefore, it is crucial that moisture content be controlled during
133 construction. Initial water content significantly affects the efficiency of soil stabilization.

134 In soil stabilization, FA addition alone is not sufficient to significantly increase strength to the design
135 allowable levels, then a combination with other additives is required [29]. Particular attention must be
136 paid when FA is added to soil-cement mixture. FA may compete with cement for the water available
137 in the soil. This competition may be detrimental to cement hydration and it is more problematic in soil-
138 cement because of the typical low water and cement ratio (w/c).

139 Interestingly, in binary combination involving lime and FA, stabilized soils present some benefits
140 resulting in strength gain. Notwithstanding, Sivapullaiah and Jha [29] indicate that there may be
141 particular FA contents to find greater resistance in the short and long term, respectively. Low contents
142 of FA increase strength in 28 days, whereas high contents of FA increase strength in 4-7 days. Generally,
143 FA in soil-lime changes the rate of strength gain and not the strength itself. Therefore, FA content up
144 to 15% is recommended. The effects of initial water content on the compressibility, strength,
145 microstructure, and composition of a lean clay soil stabilized by compound calcium-based stabilizer
146 composed of cement, lime and fly ash was investigated by Yin et al. [31]. It was observed that as the
147 initial water content increases in the range studied (from 11 to 19%), both the compaction energy and
148 the maximum compaction force decrease linearly and there are less soil aggregates or agglomerations,
149 and a smaller proportion of large pores in the compacted mixture structure. In addition, for specimens
150 cured with or without external water supply and under different compaction degrees, the variation
151 law of the unconfined compressive strength with initial water content is different and the highest
152 strength value is obtained at various initial water contents. Finally, with the increase of initial water
153 content, the percentage of the oxygen element tends to increase in the reaction products of the
154 calcium-based stabilizer, whereas the primary mineral composition of the soil-stabilizer mixture did
155 not change notably.

156 Regarding the microstructure organization, Furlan et al. [6] demonstrate that FA addition refines the
157 pore structure prior the development of pozzolanic reaction. This previous study demonstrates that
158 FA can be used as soil stabilization additive. However, some question related to the reactivity of FA
159 combined with other chemical additive requires further investigation. This paper aims to investigate

160 the contribution of FA in soil stabilization using hydraulic binders, with the purpose of finding: (i)
161 technically compatible material options, (ii) rational use of waste and byproducts and (iii) optimized
162 mix dosage.

163 **2. Experimental program**

164 Chemical stabilization of soils induces physicochemical changes of soils characteristics due to chemical
165 interactions between additives and soil. These changes imply improvements in the mechanical
166 behavior of the soil. Therefore, a multiscale analysis was selected in order to provide a holistic
167 knowledge of the stabilization mechanisms.

168 If, on the one hand, microscopic analysis can show the new cementing products resulting from
169 chemical reactions, on the other hand, it is worth quantifying their impacts on mechanical
170 performance improvement of stabilized soils. Then, measurements of design and mechanical
171 properties are necessary. Moreover, the combined use of these techniques allows to relate the
172 changes in different scales, in order to highlight advantages and disadvantages of chemical
173 stabilization.

174 Thus, the present experimental program aims to respond these questions, demonstrating the
175 beneficial of using fly ash as a complementary additive in soil stabilization, indeed, when combined
176 with conventional hydraulic binders.

177 In this study, dredged sediment from La Baule Le Pouliguen Harbor, France, and two cementitious
178 additives were used. Fly ash was added to mixtures containing lime, cement and lime and cement.
179 Only chemical stabilization was made (so without gradation correction) in order to find the best
180 combination of additives to improve the characteristics of this dredged soil considered as waste,
181 regarding economic and environmental aspects.

182 **2.1. Materials**

183 The geotechnical properties of the sediment were carried out in accordance with GTR guide [32]
1
2 184 dedicated for embankment and pavement applications. Table 1 shows results of soil characterization
3
4 185 based on results of maximum specific gravity, and organic material, pH and carbonates contents.
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6

7 186 **Table 1.** Geotechnical properties of sediment

Parameters	Value
Sand fraction > 63 μ m (%)	23
Silt fraction - 2 to 63 μ m (%)	41
Clay fraction < 2 μ m (%)	36
Specific gravity (g/cm ³)	2,66
Plasticity limit (%)	36.06
Liquidity Limit (%)	54.54
Plasticity Index	18.48
Organic matter content (%)	10.97
Carbonates content (%)	22.21
pH	8.5

187
188 Proviacial[®] ST quicklime was added to the sediment soil. The quicklime was provided by LHOIST from
189 Dugny-sur-Meuse, in Lorraine, France. This lime contains at least 90% of calcium oxide and at
190 maximum 2% of magnesium oxide. The lime content added to soil was 2%.

191 Portland cement was a CEM II/B-LL 32,5R CE CP2 (French Standard) whose short-term resistance is
192 32.5 MPa. Clinker content is between 65 and 79%, being its chemical constituents: tricalcium silicate
193 (66%), dicalcium silicate (10%) and tricalcium aluminate (7%). Limestone is the main natural
194 component of this cement, presenting a total organic material less than 0.20% in mass. The cement
195 content added to soil was 7% of the dry mass of the sediment.

196 Sodeline[®] Fly ash is manufactured and was provided by the central Emile Huchet in Saint Avold, France.
197 The main constituents of this fly ash are silicon dioxide (47.36%), aluminum oxide (21.63%) and calcium
198 oxide (8.52%). It is important to point out that sulfur content (4.02%) is higher than the conventional
199 ones. The fly ash content added to soil was 9%.

200 The particle size distribution of sediment, hydraulic binders and fly ash are summarized in Figure 1.

201 The particle size distributions help to understand the contribution of binders on the sediment particle
202 arrangement. The particle size distributions were carried out with Malvern mastersizer.

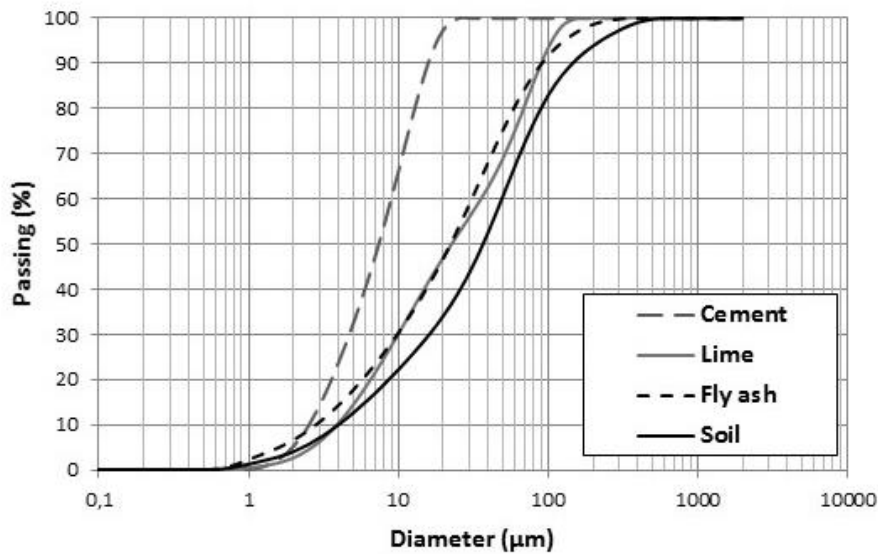


Figure 1: Grain size distribution of materials

Comparing the curves from Figure 1, it can be observed that the sediment is composed with coarsest material ($D_{90}=150\ \mu\text{m}$), with particle diameters between 500 and $1\ \mu\text{m}$; and cement is the finest binder compared to lime and FA ($D_{90}=15\ \mu\text{m}$), with particle diameters between 10 and $2\ \mu\text{m}$, that is also the most uniform distribution. Particle-size distributions of lime and FA are quite similar ($D_{90}=90\ \mu\text{m}$), with particle diameters between 200 and $0.8\ \mu\text{m}$.

2.2. Procedures of specimen preparation

Raw sediments have been oven-dried at 50°C for 48h prior to the treatment process. The sediment mixtures considered in this study are respectively formulated with 2% lime (S2L), 7% cement (S7C) or 2% lime and 7% cement (S2L7C). The percentages considered here correspond to the dry mass of sediment. A binary or ternary binder mixture has also been proposed for the mixture including fly ash (FA). The rate of fly ash addition, which is equal to 9%, represents the sum of the standard binder rates used, i.e. 2% lime and 7% cement: S2L9FA, S7C9FA, S2L7C9FA.

Proctor tests were performed according to French standard (NF P94-093) to get the optimum parameters of the mixtures, that is optimum water content and maximum dry density. All mixtures were prepared based on several precedent dosages [33]. In addition to mixture design, shrinkage tests

220 have been carried out to measure the effect of the binder on the volumetric strain of the compacted
221 mixtures. Mixtures were compacted at their optimum characteristics in an oedometric cell of 101 mm
222 in diameter and 7mm thick.

223 Specimens for UCS test were prepared according to Proctor optimum parameters. Mixing procedure
224 considered the addition of extra amount of water due to the binder presence, in order to preserve the
225 optimum Proctor parameters. Thus, water content was increased in 1% for each percentage of added
226 lime, 0.4% for each percentage of added cement, and 0.1% for each percentage of added fly ash.

227 Before compaction, the mixtures using lime underwent a previous period of 2 hour in a closed recipient
228 for the former reactions of lime and to avoid carbonation reactions. Cylindrical specimens
229 (76mmx38mm) were statically compacted. After compaction, specimens were packed in plastic film,
230 to prevent the loss of moisture during curing. Curing was made in a room at controlled temperature
231 ($20\pm 1^{\circ}\text{C}$) in different times, namely: 7, 28 and 90 days.

232 An extra specimen of each mixture was produced for the microstructure investigation. Cubic samples
233 (10mm^3) were sampled and freeze-dried from a compacted specimen at 7 and 28 days. To stop the
234 hydration and chemical reactions of the binders, cubic samples had the water removed by sublimation.

235 **2.3. Methods**

236 Unconfined compression strength (UCS) tests were performed according to French standard (NF P94-
237 420) under a constant strain rate of 1mm/min. After test, water content (w) was measured by oven
238 drying method at 105°C . In total, 3 specimens were tested and all results represent the average value
239 of UCS and w.

240 The volumetric deformation induced by shrinkage was followed until full stabilization within 9 days for
241 all mixture. Volumetric deformation of all samples was monitored by means of a digital caliper.

242 Microscale analysis was based on X-ray diffraction (XRD) analyses, Scanning Electron Microscopy (SEM)
243 observations and Mercury intrusion porosimetry (MIP) measurements.

244 XRD analyses were used not only to identify the crystallized component resulting from the hydration
245 of cement and pozzolanic reactions but also to characterize sediment matrix. The XRD analyses were
246 performed on powder using a Brucker diffractometer with a detector over the range 5° – $80^{\circ}2\theta$ using Cu-
247 $K\alpha_1$ radiation (1.55Å). Diffractometer operated with input voltage of 30 kV and current of 10mA.
248 Crystallography Open Database (COD) was used to identify constituents and new crystalline phases in
249 the patterns.

250 SEM images were used to investigate the mineral phases resulting from the interaction between the
251 sediment and the binder components. SEM observations were carried out using a FEI Inspect F-50 SEM
252 instrument coupled to an energy dispersive X-ray analyzer (EDX). To improve the image quality,
253 samples are coated with gold.

254 The impact of the mineral compound development on the sediment structure resulting from hydration
255 mechanism of the binder mixture was followed by the pore distribution. The pore size distributions
256 were plotted from MIP data obtained with Micromeritics Autopore IV.

257 **3. Results**

258 **3.1. Microstructure investigation**

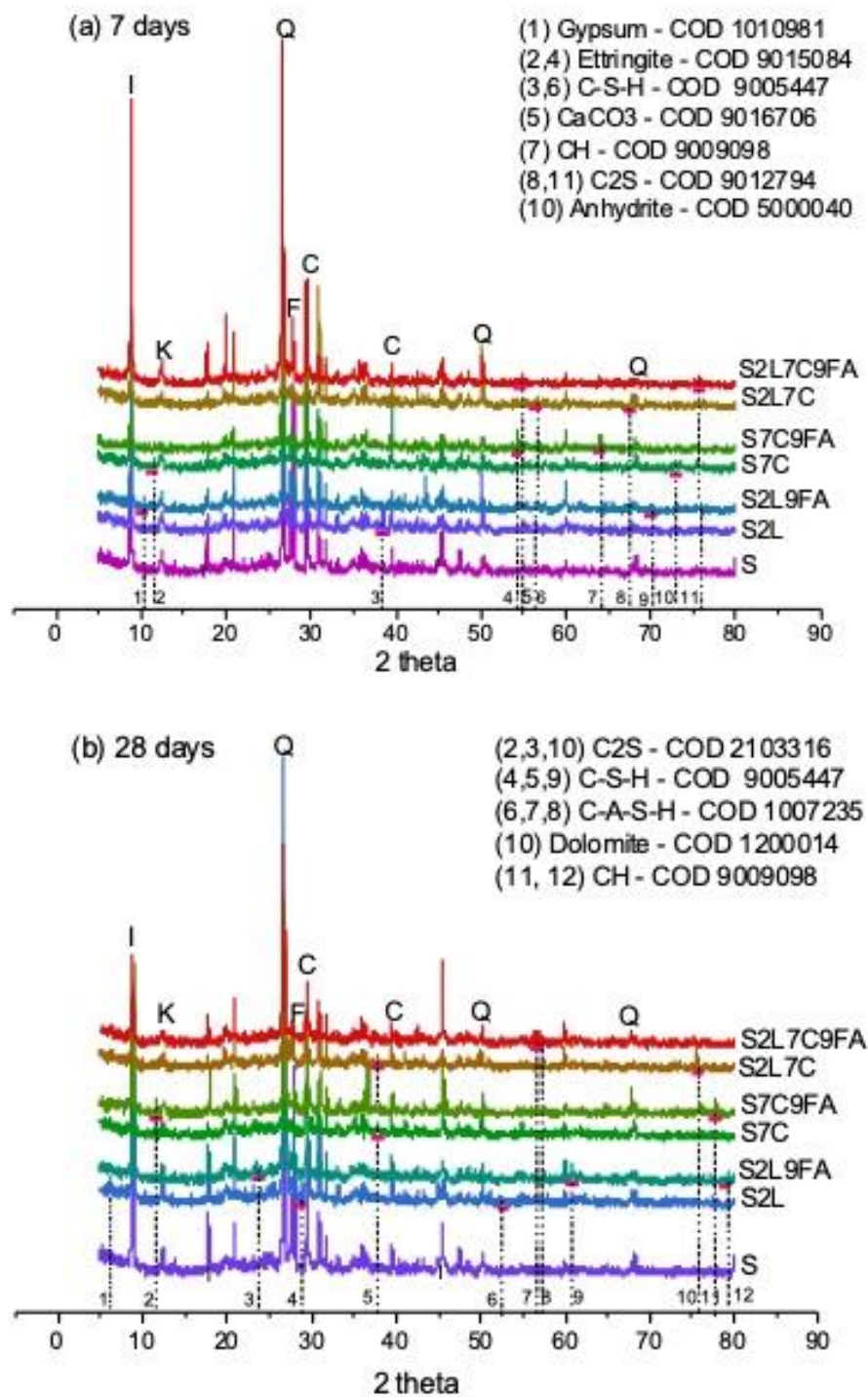
259 **3.1.1. Microstructure investigation by XRD**

260 The main strength gain in chemical stabilized soil is mostly attributed to cementitious compounds
261 formed from cement hydration and/or pozzolanic reactions. Figure 2 shows XRD patterns of all
262 mixtures at 7 and 28 days. These patterns compare mixtures with original soil, in order to identify new
263 crystalline phases addressed to chemical reactions. Different curing times are considered in attempt
264 to evidence the formation and growth of cementitious compounds. New peaks in patterns confirmed
265 not only the formation of cementitious compounds but also indicated the presence of non-hydrated
266 materials.

267 In Figure 2, it is observed that untreated sediment presents quartz (Q), feldspar (F), calcium carbonate
268 (C), Illite (I) and kaolinite (K) in its composition. Samples at 7 days (Figure 2a) showed the presence of

269 gypsum (S2L9FA), C₂S (S2L7C and S2L7C9FA) and anhydrite (S7C). It may indicate (a) insufficient water
1
2 270 to hydrate the binders, (b) delay in cement hydration due to FA addition, (c) restricted water
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4 271 availability of anhydrous grains surrounded by hydration products due to the formation of strong
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7 272 bounds between soil aggregates which can modify the porous structure and thus can slow down or
8
9 273 hinders the permeation and/or diffusion process of water, (d) a combination of previous assumptions.
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11
12 274 A remarkable peak of calcium carbonate (CaCO₃) was particularly observed in S2L7C9FA at 7 days. The
13
14 275 occurrence of CaCO₃ was associated to the presence of limestone in the cement (CEM II) and
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16 276 carbonates in the untreated sediment. The non-observation of such CaCO₃ peak in S7C or S2L7C could
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18
19 277 be explained by the heterogeneous distribution of binders in the sediment, the sediment itself and the
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21 278 low percentage of cement used. Since the specimens have been kept packed during curing,
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24 279 carbonation reactions were disregarded.
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27 280 Regarding hydration products, new peaks pointed out CH in S7C9FA, C-S-H in S2L and S2L7C, and
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29 281 ettringite in S7C and S7C9FA. The mixture with S2L9FA presented CASH peak. Even though some
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31 282 crystalized new phases from cement hydration have been identified, they did not occur in an evident
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34 283 way and this might be a consequence of the low water/cement ratio (w/c).

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 286 **Figure 2:** XRD results of mixtures at 7 days (a) and 28 days (b)

287 Figure 2(b) shows the pattern of mixtures at 28 days. Belite is still identified in S7C9FA and S2L7C.
 288 Other new peaks refer to C-S-H in S2L, S7C, S2L9FA and S2L7C, CH in S2L9FA and S7C9FA, and CASH in
 289 S2L and S2L7C9FA. The presence of non-hydrated material suggests hydration delay or water

1
2 291 insufficiency. This might be related to (i) lime addition, because the change of texture (agglomeration)
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4 292 would interfere with the amount of available water for hydration, since water may be trapped in the
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6 293 soil cluster; and/or (ii) FA addition, because water demand for FA is generally important due to its
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8 294 appreciable porosity, it might create a competition for available water. Besides, it is important to
9
10 295 remember that cementitious compounds in early ages are poorly crystalized and they are not well
11
12 296 detected by diffractometer.

13
14 297 Regarding cementitious compounds, one may observe that all treated samples had produced them at
15
16 298 28 days, unlike what happened at 7 days. Curing time was preponderant to development of
17
18 299 cementitious compounds, since time allows binders to hydrate according to their own reaction rates
19
20 300 (pozzolanic reactions) and allows also the transport of water necessary to the reactions of anhydrous
21
22 301 grains surrounded by hydration products. This implies that it would be recommended at least 28 days
23
24 302 to verify strength gains, at macroscale.

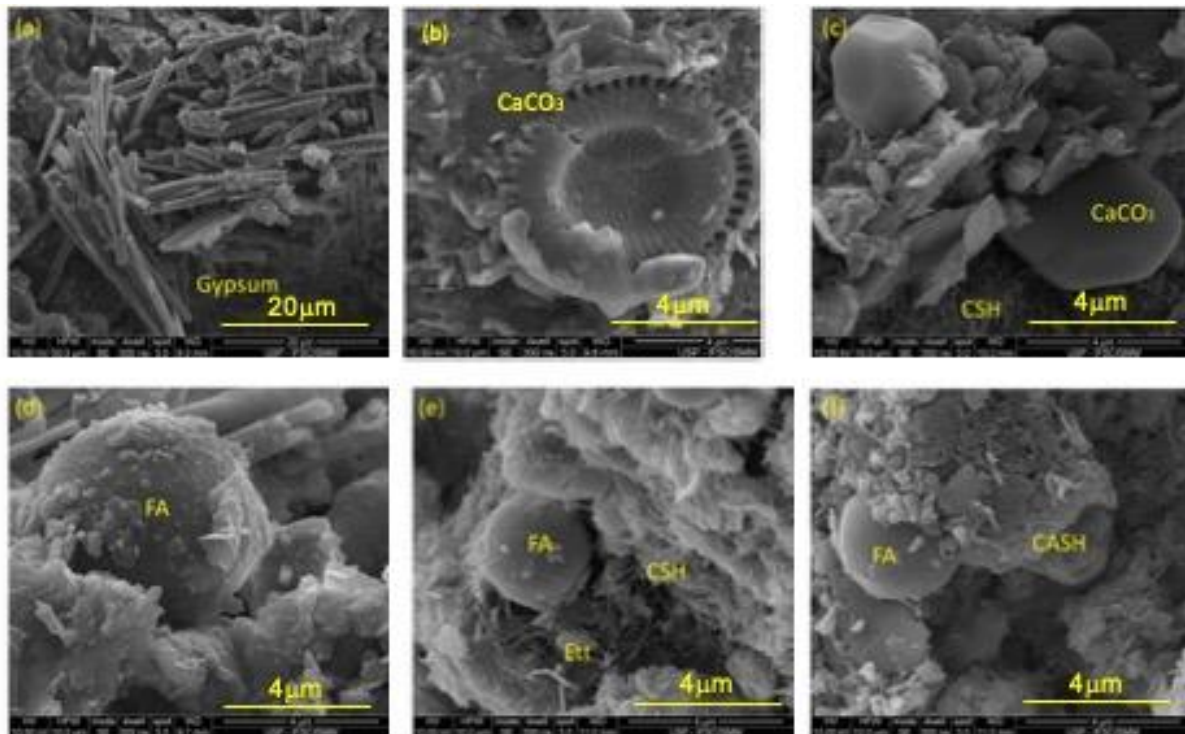
25
26 303 Hydrated silicates exhibit different morphologies and it might be more problematic for chemically
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28 304 stabilized soil due to the several factors, such as the type of additive, the low additive contents, the
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30 305 low water content, the complexity of hydration mechanisms and cementitious material formation of
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32 306 multicomponent mixed materials [15][25].
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35 307 In other words, since C-S-H are amorphous and/or poorly crystallized, it may be advisable to be careful
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37 308 when reading XRD patterns because of the disturbing factors (producing a heterogeneous sample) and
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39 309 to combine other microstructural data sources to help to support the results analyses.
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42 310 **3.1.2. Microstructure observation by Scanning Electron Microscopy (SEM)**

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44 311 SEM images are often used to provide evidences of cementitious compounds development and
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46 312 mixture changes. Firstly, Figure 3 attempts to demonstrate visually constituents and compounds,
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48 313 which were previously identified in the mixtures using XRD tests (Figures 2 and 3). SEM Images are
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50 presented in appropriate magnificence to highlight details of the materials. As it can be seen in figure
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314 3, almost all compounds identified with the XRD patterns are observed and confirmed, such as gypsum,
315 CaCO_3 (found in sediment and in mixtures with cement), hydrated silicates and FA as well.



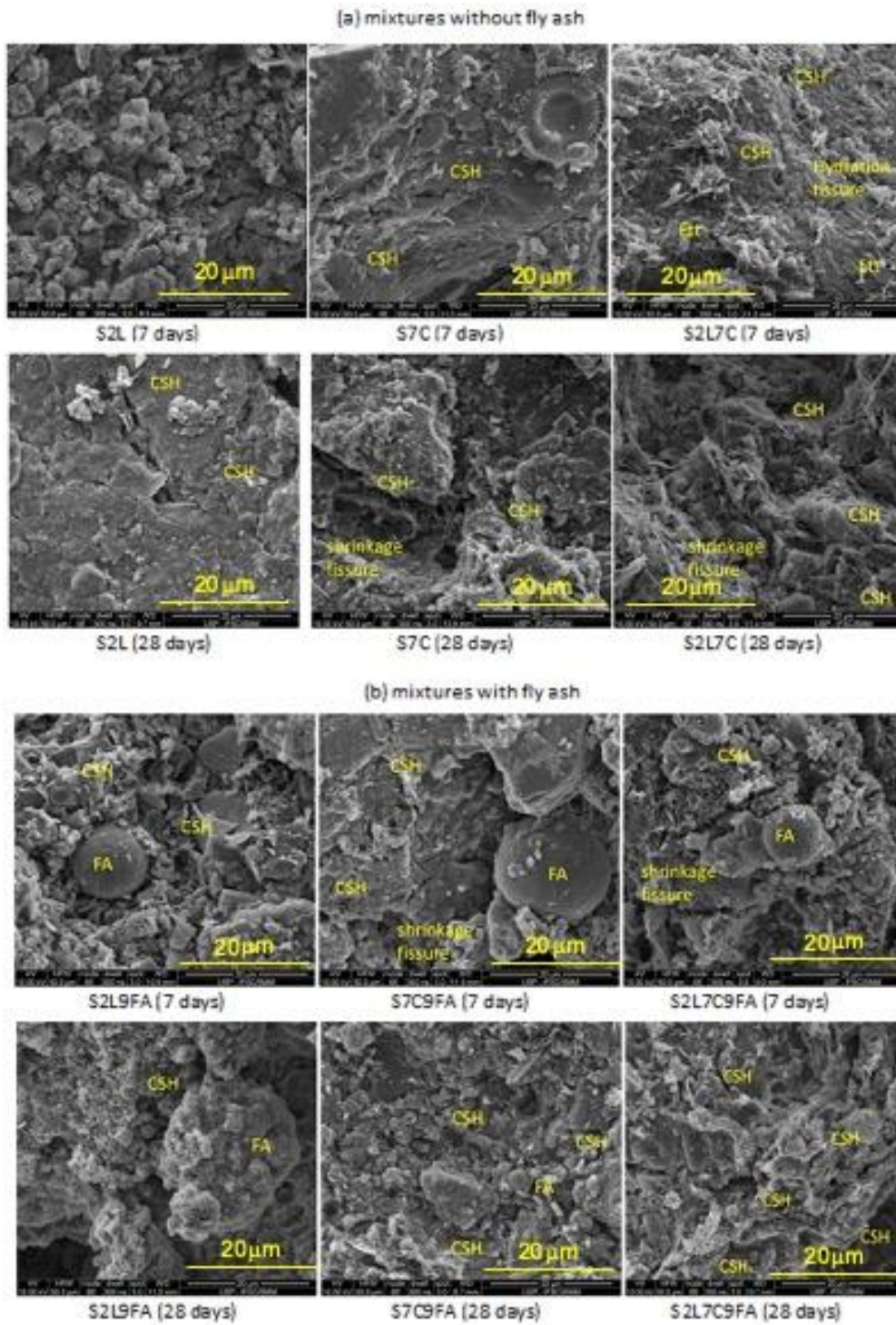
316
317 **Figure 3:** Identification of constituents and cementitious compounds of mixtures from (a)S2L9FA-7d,
318 (b) S7C, (c)S2L7C9FA – 7d, (d) S7L9FA – 7d, (e)S2L7C9FA-7d,(f) S2L9FA-28d

319 Secondly, Figures 4 shows SEM images obtained from sediment mixtures without and with FA (4a and
320 4b), in order to observe structural changes and cementitious compound presence from 7 to 28 days.

321 Figure 4(a) shows images from mixtures using conventional binders. Mixture with lime (S2L) at 7 days
322 exhibits a flaked-like structure, as a result of cation exchange and flocculation mechanism. At 28 days,
323 a block-like structure may be observed, indicating pozzolanic reactions, as demonstrated by
324 identification of needle-shape C-S-H.

325 Regarding mixture with cement (S7C), block-like structure may be observed already at 7 days. Besides
326 it might be identified the formation needle-shape C-S-H. Shrinkage fissure (due to cement hydration)
327 are observed at 28 days.

328 The mixture with binary binder, lime and cement (S2L7C), exhibited a combination of the
 1
 2 329 aforementioned observations related to structure, i.e. flaked and block-like. Notwithstanding, in this
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 4 330 mixture, C-S-H and ettringite are identified and as well as some shrinkage fissures at 7 and 28 days.
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 333 **Figure 4:** SEM images from mixtures without FA (a) and with FA (b)

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334 The observation of cementation products and the resulting block-like structure lead to infer some
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2 335 increase in strength at macroscale, for instance. Again, that is in accordance with the assumption that
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4 336 cementation enhances the mixture cohesion and controls strength gain of mixtures using cement [12].
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7 337 Figure 4(b) presents images of binary and ternary mixtures using FA, in attempt to observe its effect
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9 338 when combined with conventional binders. Generally, mixtures using FA revealed significant
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11 339 occurrence of pozzolanic and hydration products. These phenomena are more effective surrounding
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13 340 FA spheres. This finding is consistent to nucleation process and reaction stimulation reported in
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15 341 technical literature [3][12][25].
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19 342 Concerning visual aspect, at 7 days, it can be observed that mixture with lime and FA shows a flaked-
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21 343 like structure. At 28 days, mixture revealed a blocky structure. The change in structure is remarkable
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23 344 and is attributed to the development of cementation between the soil clusters and also covering FA,
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25 345 as seen in S2L9FA mixture at 28 days.
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29 346 For mixture with cement and FA (S7C9FA), blocky structure is observed at 7 and 28 days. C-S-H gel
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31 347 covers the surface of soil and FA and shrinkage fissures were identified. For the mixture with lime,
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33 348 cement and FA (S2L9FA), once again the flocculated structure was observed (at 7 days) as well as its
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35 349 evolution to a blockier structure (at 28 days).
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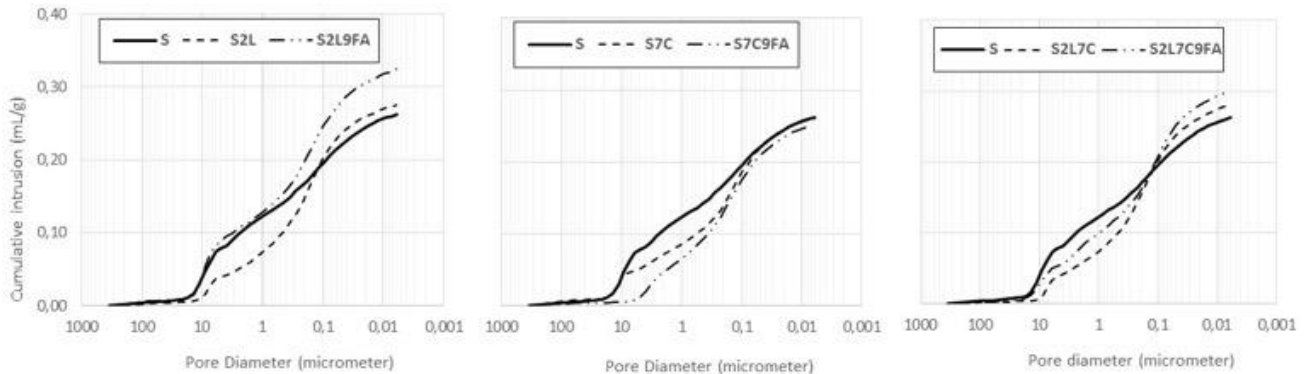
39 350 Cementitious compounds and hydration fissures seem to occur more frequently in mixtures using FA
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41 351 for both curing time. The increase of CaO content due to addition of FA as long as the nucleation
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43 352 process would promote formation of cementitious materials [3][4][26].
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47 353 **3.1.3 MIP results**

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50 354 The XRD patterns (Figure 2) showed the occurrence of cementation from new crystalline phases in the
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52 355 mixtures such as hydrated silicates. The SEM images (Figures 4) confirmed the presence of the
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54 356 cementitious compound and qualified the changes of the structure caused by the additives. MIP tests
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56 357 were carried out, in attempt to clarify the mechanism of interaction between FA and conventional
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58 358 binder which govern the alteration of sediment fabric.
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359 Figure 5 presents cumulative intrusion versus pore diameter curves. In general, additives reduced the
 360 pore diameter of original soil. In mixtures S2L and S2L9FA, for large pores (from 10 to 0.1 μm), it is
 361 observed that there are reductions of diameter of pore. Whereas, for small pores (from 0.1 to 0.001
 362 μm) there is an increase of cumulative intrusion, i.e. an increase of percentage of small pores. Curves
 363 are approximately parallels, but it can be seen that FA tends to change further large pores.



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 365
 366 **Figure 5: Pore distributions of all mixtures (7 days)**

367 Mixtures S7C and S7C9FA presented significant changes mostly in large pores. This finding suggests
 368 that changes in the pore distribution depend on the type and amount of binder added, and particularly
 369 the gradation of binders.

370 Pore distributions revealed the soil fabric changes resulting from addition of binders. For instance, at
 371 7 days, it is expected that:

372 (i) Lime generates a formation of aggregates/flocculates with finer pore width resulted from the
 373 exothermic reaction which take place during lime hydration [10][34].

374 (ii) Cement hydration produces cementitious compounds that fill large pores and also causes
 375 shrinkage reducing original soil pores diameter. No significant evolution of small pores domain is
 376 observed for both treatments using lime and cement. This is due to the fact that small pores are
 377 mainly associated with intra-aggregate porosity which is less affected by the treatment.

378 (iii) both lime and cement combine these two mentioned effects, and

379 (iv) in addition to the role of filler played by FA, it also promotes nucleation and formation of
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 2 380 cementitious compounds around FA, increasing the percentage of small pores of the original soil
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 4 381 [12].
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7 382 Since total pore volume (i.e. the highest cumulative intrusion value) is an indicator of material
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 9 383 densification, one could assume that, at macroscale, the resistance of mixtures using FA would be
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 11 384 higher than that of mixtures without FA. Nevertheless, accordingly [28], cementation controls
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 13 385 predominantly the resistance development of soil-cement mixtures.
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16 386 3.2. Macroscale analysis

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 20 387 Macroscale analysis was done in two steps. The first consisted of immediate property assessment of
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 22 388 the mixtures based on Proctor, shrinkage, and UCS test results. The latter was based on UCS results
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 24 389 throughout curing time. First and second steps focused on distinguishing the filler effect from curing
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 27 390 effect, respectively, in an attempt to remark the most adequate stabilization process, in terms of
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 29 391 additive content and strength gain.
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32 392 3.2.1. Design parameters and immediate strength

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 35 393 Binder addition and content may change soil gradation. Since additives are finer than the soil (Figure
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 37 394 1), they shall fill partially empty voids of soil and increase the initial strength values, due to the filler
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 40 395 effect. Table 2 present design parameters and immediate strength. Comparing to untreated sediment,
 41
 42 396 it is observed that binder addition promoted (a) the reduction of maximum dry densities (ρ_{dmax}) and
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 44 397 the increase of optimum water content (w_{opt}), (b) the reduction of the volumetric variation (dv) and
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 46 398 (c) the increase of immediate strength (UCS_{0d}). Nevertheless, the intensity of changes depends on the
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 49 399 type and the content of binder.
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 51

52 400 **Table 2.** Design parameters and immediate strength

Type of mixture	Legend	% filler	Proctor Parameters		Shrinkage Parameters			UCS _{0d} (MPa)
			w _{opt} (%)	ρ_{dmax} (g/cm ³)	Time (days)	dv (%)	dv/T (%/hours)	
Soil	S	0,0	27.8	1.48	3,0	27.0	0.44	0.28

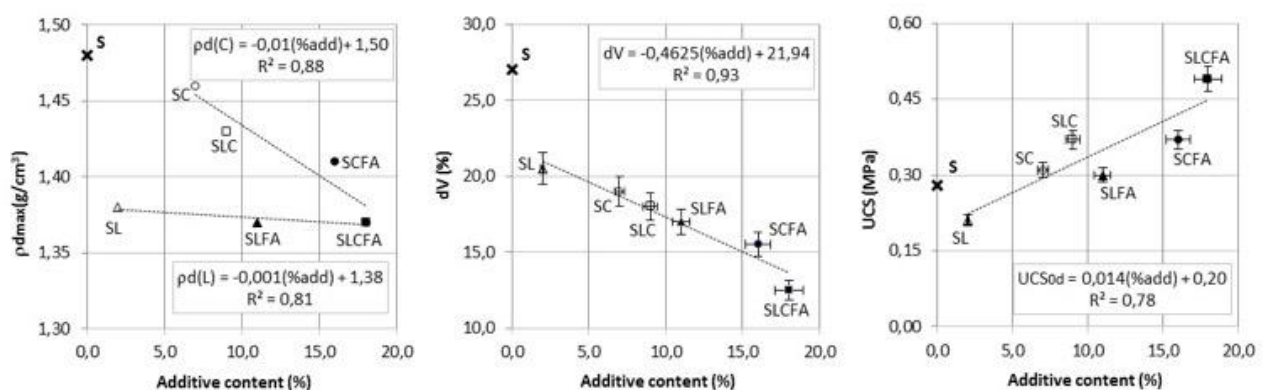
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Soil+ 2% lime	S2L	2.0	32.2	1.38	2.6	20.5	0.50	0.21
Soil+ 2% lime + 9% fly ash	S2L9FA	11.0	32.0	1.37	2.8	17.0	0.39	0.30
Soil+ 7% cement	S7C	7.0	28.1	1.46	2.4	19.0	0.32	0.31
Soil+ 7% cement + 9% FA	S7C9FA	16.0	30.3	1.41	3.0	15.5	0.33	0.37
Soil+ 2% lime+ 7% cement	S2L7C	9.0	30.7	1.43	2.2	18.0	0.25	0.37
Soil+ 2% lime+ 7% cement+9% FA	S2L7C9FA	18.0	31.6	1.37	2.0	12.5	0.17	0.49

To illustrate filler effect, Figure 6 shows correlations between Proctor and shrinkage parameters, and immediate UCS values in function of additive content. Figure 6(a) shows that the reduction of p_{dmax} is a result of the increase of binder content, but this behavior depends on the type of binder considered. It is observed that the evolution of the p_{dmax} follow a linear regression law with high degree of accuracy (R^2). R^2 values for mixtures containing lime (L) and cement (C) are 0.81 and 0.88, respectively. FA addition reduced p_{dmax} in mixtures with cement. In mixtures with lime this behavior was not observed.

Linear law demonstrates that cement addition reduces maximum dry density less than lime addition. Both curves converge to the p_{dmax} value of the SLCFA mixture, which was the minimum one.

Volumetric deformation is often consequence of shrinkage which occurs in cemented materials due to the water consumption for clinker component, generating fissures. Shrinkage test reports the variation of volume (dV) that a soil-cement may present and gives an idea about material durability in dosage phase. Figure 6(b) shows a strong correlation between dV and additive content ($R^2=0.93$), indicating that dV decreases with increasing additive content. In other words, partial filling of the voids would inhibit shrinkage.



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Figure 6: Immediate Properties and characteristics of mixtures

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419 It would be reasonable to indicate the addition of inactive fillers in order to reduce shrinkage in soil-
420 cement type mixtures. Figure 6(b) led to infer that FA plays this role in this phase, because its addition
421 in the mixtures further reduced dV. In mixtures without FA, the reduction of dV was approximately
422 equals to 30% in relation to untreated soil. When FA was added, this reduction was about 40% for SL
423 and SC mixtures, and 55% for SLC mixture.

424 Regarding the filler effect on immediate strength, Figure 6(c) shows a good correlation ($R^2=0.79$)
425 between additive content and UCS of the mixtures, where the higher the additive content the greater
426 the UCS_{0d}. It is important to point out that immediate UCS_{0d} of SL mixture was lower than the
427 untreated sediment. This behavior is attributed to flocculation reactions of lime that increase of voids
428 in the mixture, decreasing p_{dmax} and increasing water content (as seen in Table 2).

429 It is worth emphasizing that the combination of additives changes gradation of untreated sediment.
430 As additives are finer than the sediment soil, they may fill empty voids, increasing the sediment
431 strength. As a rule, the initial strength values gradually improve as binder percentages increase.

432 In spite of the good correlation between immediate UCS and additive content, the strength gain may
433 be also related to the type of additive, or combination between them, since the greatest immediate
434 UCS were observed in mixtures SLC, SCFA and SLCFA. The common point of these mixtures is the
435 presence of cement associated to other(s) additive(s). It seems to be a contribution of additive
436 gradations to the particle size arrangement of mixtures, resulting in the increase of their immediate
437 UCS. Immediate UCS is a reference point that might lead to dosage optimization attempts. Accordingly,
438 from this point on, it shall be observed the strength gain over time due to chemical reactions.

3.2.2. UCS and water content results over time

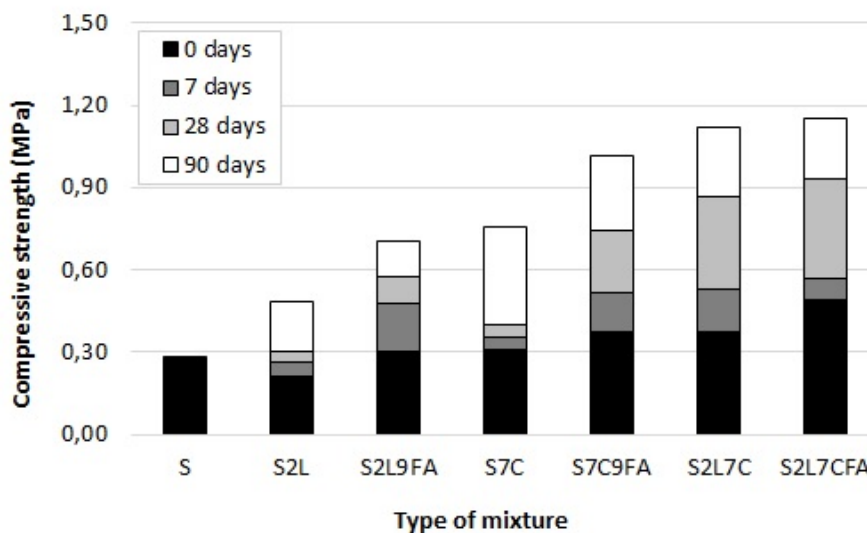
440 In order to highlight the effects of additives on the mechanical resistance of mixtures, Figure 7 presents
441 the kinetics of the UCS over time. Regarding compressive strength values (UCS), one may observe that
442 additives increased initial property of untreated sediment (0,27 MPa), even though some mixtures

443 required more time to present significant strength gains, such as mixture S2L that presented reduction
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 2 444 of UCS value at 7 days. All other additives combination increased UCS values and these increases are
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 4 445 more expressive when FA is added.
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7 446 From 0 to 7 days, the strength gain of S2L and S7C was about 0.05 MPa that represents respectively
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 9 447 20% and 16% of their initial UCS values (0.21 and 0.31 MPa). For conventional binders, the mixture
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 11 448 with lime and cement (S2L7C) had the higher strength gain (~0.15 MPa), that is 40% of its initial
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 13 449 strength (0.37 MPa).
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 17 450 For mixtures containing FA, strength gains were in average of 0.15 MPa. It is worth emphasizing the
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 19 451 strength gain of the mixture S2L9FA that was about 70%. In contrast, the strength gain at 7 days of the
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 21 452 mixture S2L7C9FA was the lowest one (0.10MPa) that represents about 20% of initial UCS value.
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 25 453 At 28 days, the UCS value of S2L was 0.31 MPa, S7C was 0.40 MPa, and S2L7C was 0.87 MPa. From 7
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 27 454 to 28 days, the gain of UCS remains about 0.05 MPa for S2L and S7C. Interestingly, FA addition
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 29 455 doubled the strength gains in these mixtures. The faster strength gain was also observed by Kang et al.
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 31 456 [27].
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457
 458 **Figure 7: UCS over time**
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459 For the mixture S2L7C9, the strength gain was 0.35 MPa. In this case, FA addition did significantly not
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 59 460 increase strength gain of SLC mixture. In general, mixtures with FA presented the higher UCS values.
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2 462 In order to observe the tendency of strength evolution, Figure 7 also presents UCS at 90 days. As it can
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4 463 be noticed, UCS values continued increasing, with emphasis on the highest strength gains of mixture
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6 464 S7C and S2L. Another point to remark is that mixtures using FA showed slower strength gains compared
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8 465 to mixtures with conventional binders, unlike what was observed for other curing time (7 and 28 days),
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10 suggesting a deceleration of these gains at 90 days.

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12 466 UCS Kinetics confirmed the additive mechanism-based assumptions. Regarding mixture with lime,
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14 467 cation exchanges and flocculation did not improve soil resistance. Over time, pozzolanic reactions were
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16 468 not effective to increase significantly the strength even in long term (from 28 days). In the mixture with
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18 469 cement, the occurrence of cementitious compounds increased strength at early age (7 days).
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20 470 Interestingly, it continued to increase up 90 days but at lower intensities, in contrast to expectations
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22 471 of fast reacting cements. This delay is probably associated to the salt presence and organic matter in
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24 472 the marine sediment that alters the kinetics of hydration cement, as mentioned by Horpibulsuk et al.
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26 473 [35].

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29 474 On the other hand, FA addition has led to the higher strength gains in all tested mixtures. This
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31 475 observation is consistent with the hypotheses of nucleation and growth of cementitious materials
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33 476 promoted by FA [15][26][27], which here were observed and discussed based on the results of
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35 477 microscale test (Figures 2 to 4).

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38 478 Assuming that the gain of resistance over time results from the cementing products of the additive
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40 479 components [28], it is also necessary to understand how the hydric state of the mixtures evolves.
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42 480 Figure 8 presents the evolution of water content over time. The plotted curves indicate that water
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44 481 content reduces over time and the intervals of variation are significant over long periods, as can be
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46 482 seen for all mixtures using cement.

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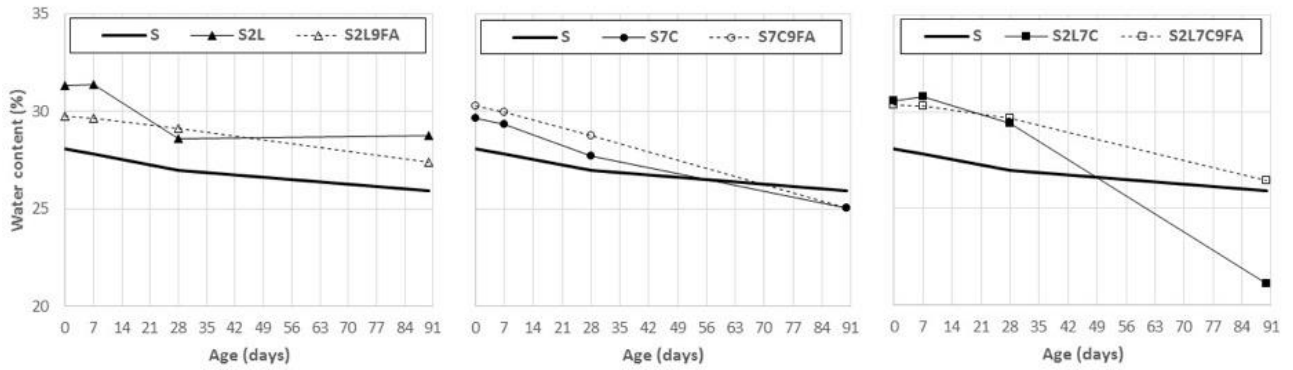


Figure 8: Variation of water content over time

In early age (7 days), water contents change slightly, maintaining practically the initial values, i.e. close to the w_{opt} (Table 4). However, for intermediary age (28 days), these changes evidence the reduction of water content, the average reduction was 5%. For longer period (90 days), the average reduction of water content is 12%, except for SLC, which was 31%.

FA addition also alters hydric conditions of stabilized mixtures over time but at lower intensity. As a rule, one may suppose that the greater the decrease in moisture content the greater the gain in resistance. This hypothesis is true since water is consumed in formation and/or crystallization of cementing products, becoming the structure denser and more resistant than the original soil.

4. Discussion

The decision for a given product takes into account technical, economic and environmental parameters. This study has shown that combining FA with stabilized soil with lime and cement (or both) is promising because of the FA action mechanisms promoted an acceleration of strength gain regardless of the curing time. Besides, mixtures using FA have always shown higher resistance, either through physical interactions or chemical reactions. These findings support technically the choice for combining FA with chemical soil stabilization using conventional binders.

For instance, UCS value at 90 days of mixture using cement (S7C) is compatible with that of the mixture using cement and FA (S7C9FA) at 28 days, or with mixture using lime and FA (S2L9FA) at 90 days. These mechanical compatibilities indicate alternative material options that might save time and economy,

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2 504 since it is possible to select mixtures that present greater resistance ahead of time, use cheaper
3 additives or smaller quantities of conventional binders.

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5 506 These benefits have a positive impact on environmental indicators because they promote the rational
6 use of waste and byproducts (marine sediment and fly ash). In this sense, it is worth mentioning that
7 507 the mixture with lime, cement and FA (S2L7C9FA) presented resistance compatible with the mixture
8 without FA (SLC), leading to question the advantages of this mixture (S2L7C9FA) as well as to think
9 508 alternatively about the possibility of reducing the cement content in order to optimize the mixture
10 design in terms of economic and environmental.
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20 512 **5. Conclusions**

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22 513 The effects of FA addition on microstructural and resistance characteristics of mixtures stabilized with
23 conventional binders were investigated. From this study the conclusions are made focusing the
24 interrelations between microstructural and macrostructural analyses.
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30 516 Regarding microstructural analyses, it was identified new crystalline phases in DRX patterns.
31 Cementitious materials, such as ettringite, portlandite and hydrated silicates, are produced over time
32 by cement hydration and pozzolanic reactions. The occurrence of these products depends on the
33 curing time due to particular binder reaction rate. These findings are supported by SEM images, that
34 confirmed the presence of hydrated silicates in different morphologies (gel, acicular C-S-H for
35 instance), mainly in mixtures using FA. The SEM images also show physical modification of the soil
36 fabric over time, which was also confirmed by analysis of pore distributions. The results of the pore
37 distribution measurements show a structural densification of the sediment matrix at early age,
38 demonstrating the contribution of finer gradations of the additives on the sediment pore
39 rearrangement, i.e. filler effect.
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54 526 Macroscale analyses led also to observe a prevalent filler effect on gain of immediate resistance and
55 reduction of volumetric variation by shrinkage (dV). These physical responses are especially
56 remarkable in mixtures using FA. These findings led to recommend more studies in dosage parameters
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529 and properties in order to identify the best combination of binder blend for chemical stabilization
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2 530 purposes and to optimize the mixture design with several binders, i.e. the choice and the content of
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4 531 binder(s). UCS values increase and water contents decrease over curing time, with emphasis on the
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7 532 significant contribution of FA to the better mechanical performance. Based on microstructural analysis,
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9 533 it can be stated that available water was consumed for cementing and pozzolanic reactions over time.
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11 534 **6. Acknowledgement**

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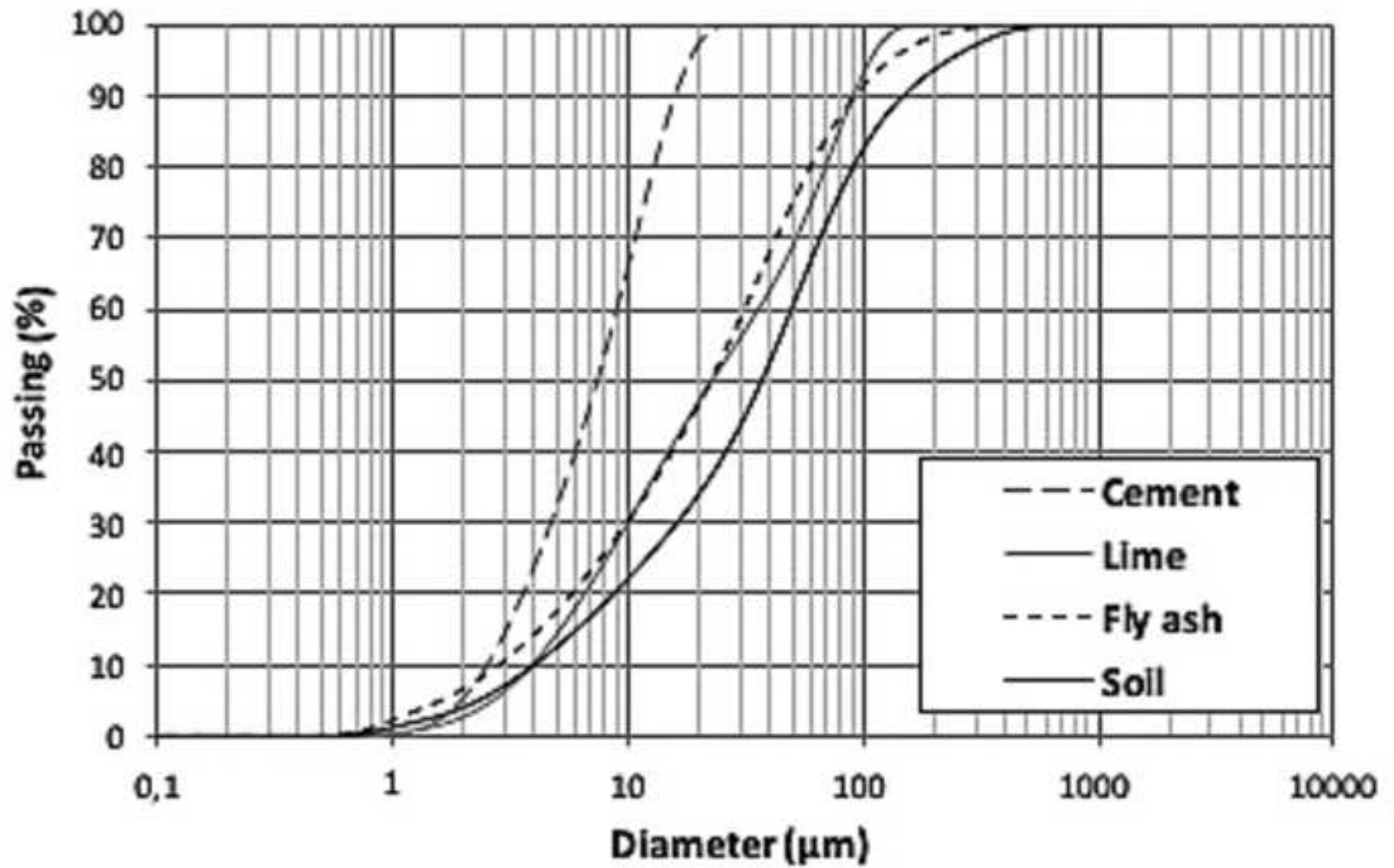
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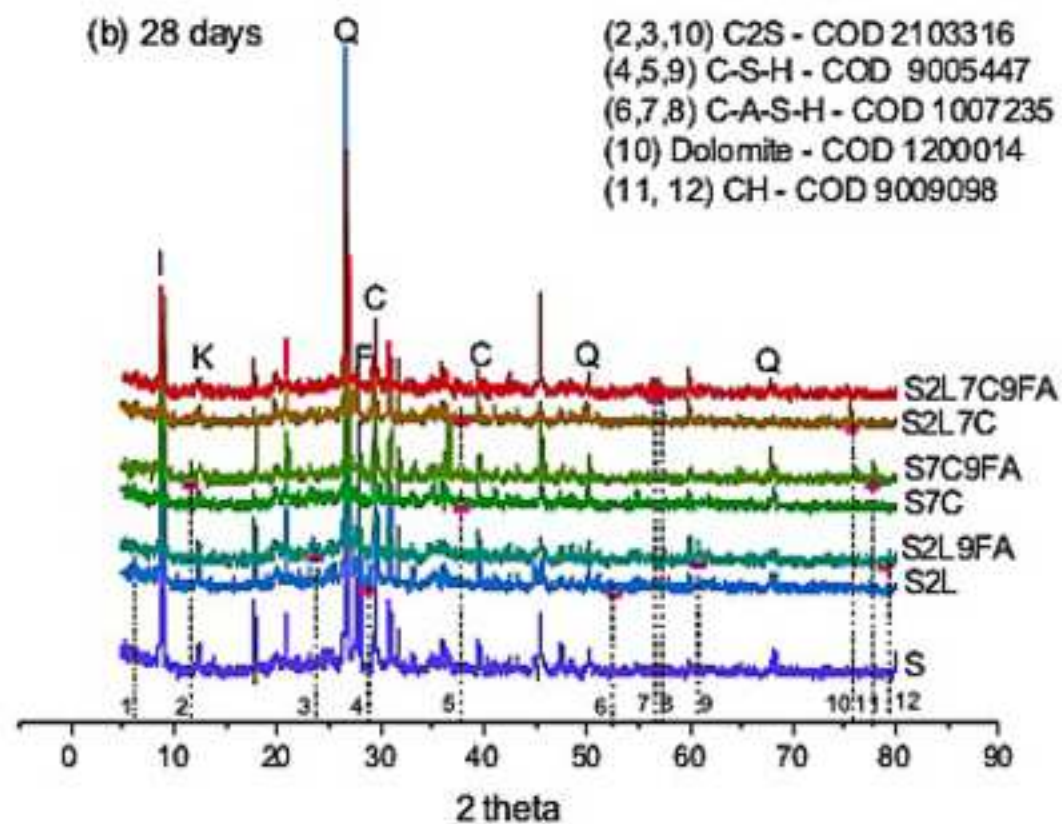
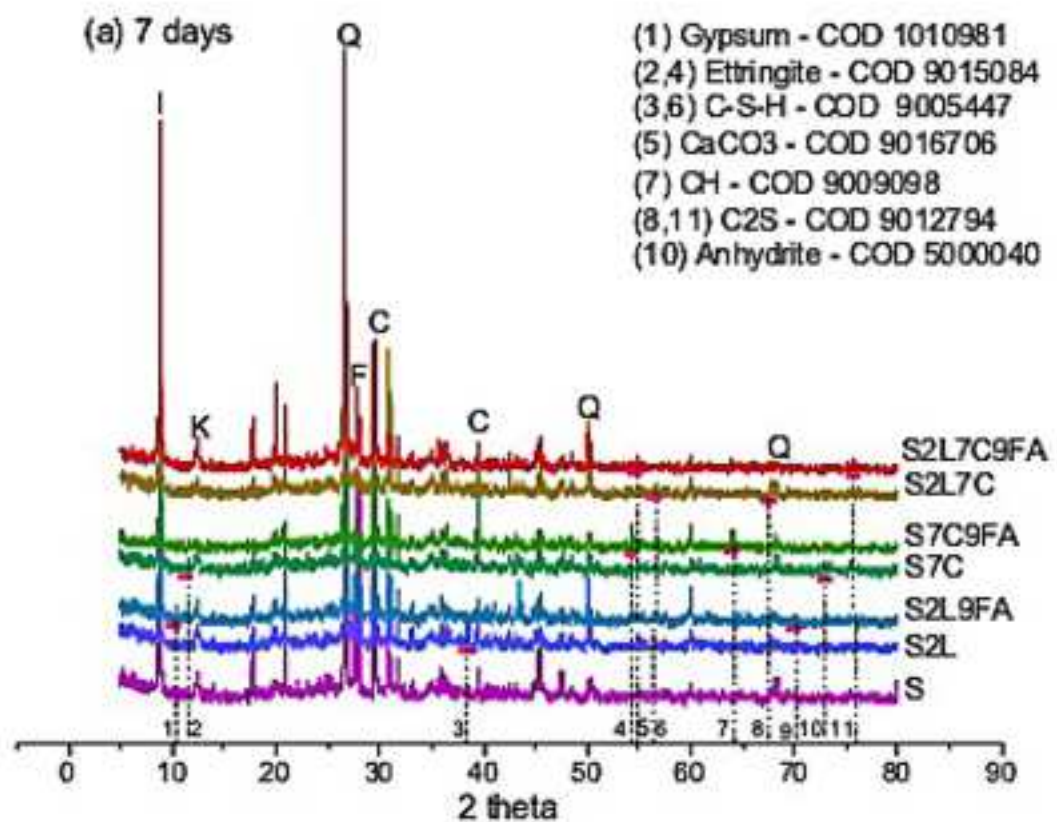
Parameters	Value
Sand fraction > 63 μ m (%)	23
Silt fraction - 2 to 63 μ m(%)	41
Clay fraction < 2 μ m (%)	36
Specific gravity (g/cm ³)	2.66
Plasticity limit (%)	36.06
Liquidity Limit (%)	54.54
Plasticity Index	18.48
Organic matter content (%)	10.97
Carbonates content (%)	22.21
pH	8.5

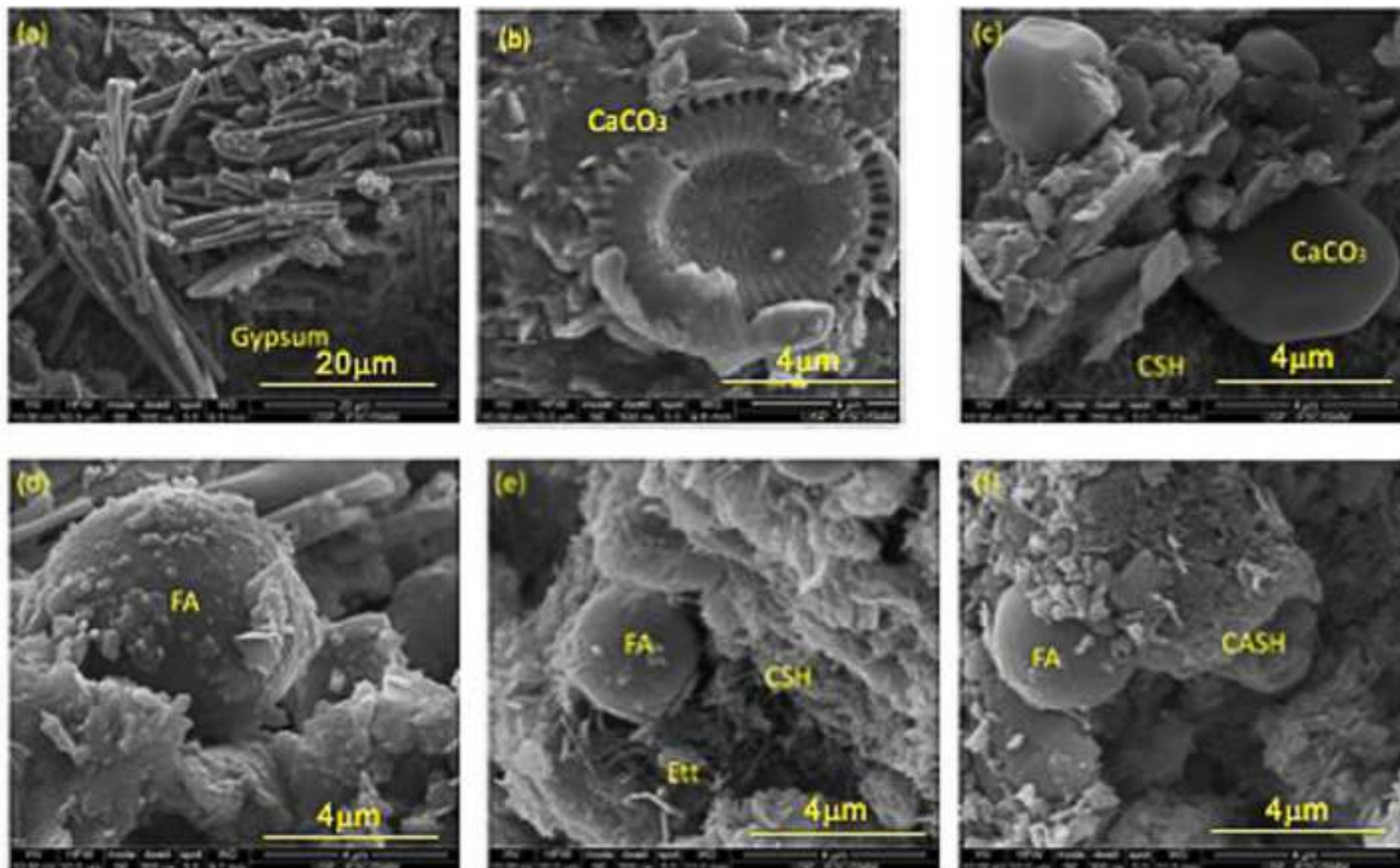
Table 1: Geotechnical properties of sediment

Type of mixture	Legend	% filler	Proctor Parameters		Shrinkage Parameters			UCS _{0d} (MPa)
			W _{opt} (%)	ρ_{dmax} (g/cm ³)	Time (days)	dv (%)	dv/T (%/hours)	
Soil	S	0,0	27.8	1.48	3,0	27.0	0.44	0.28
Soil+ 2% lime	S2L	2.0	32.2	1.38	2.6	20.5	0.50	0.21
Soil+ 2% lime + 9% fly ash	S2L9FA	11.0	32.0	1.37	2.8	17.0	0.39	0.30
Soil+ 7% cement	S7C	7.0	28.1	1.46	2.4	19.0	0.32	0.31
Soil+ 7% cement + 9% FA	S7C9FA	16.0	30.3	1.41	3.0	15.5	0.33	0.37
Soil+ 2% lime+ 7% cement	S2L7C	9.0	30.7	1.43	2.2	18.0	0.25	0.37
Soil+ 2% lime+ 7% cement+9% FA	S2L7C9FA	18.0	31.6	1.37	2.0	12.5	0.17	0.49

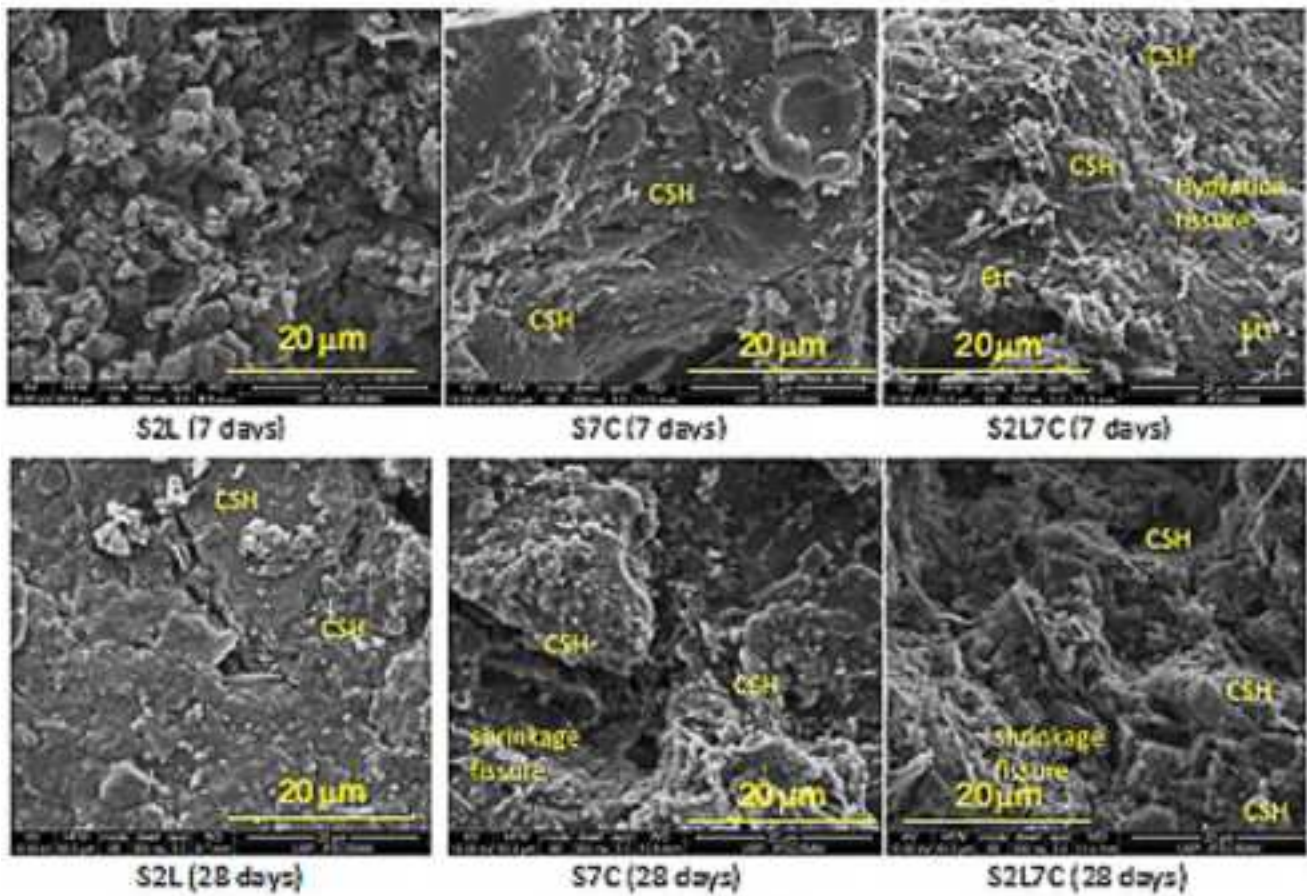
Table 2: Design parameters and immediate strength



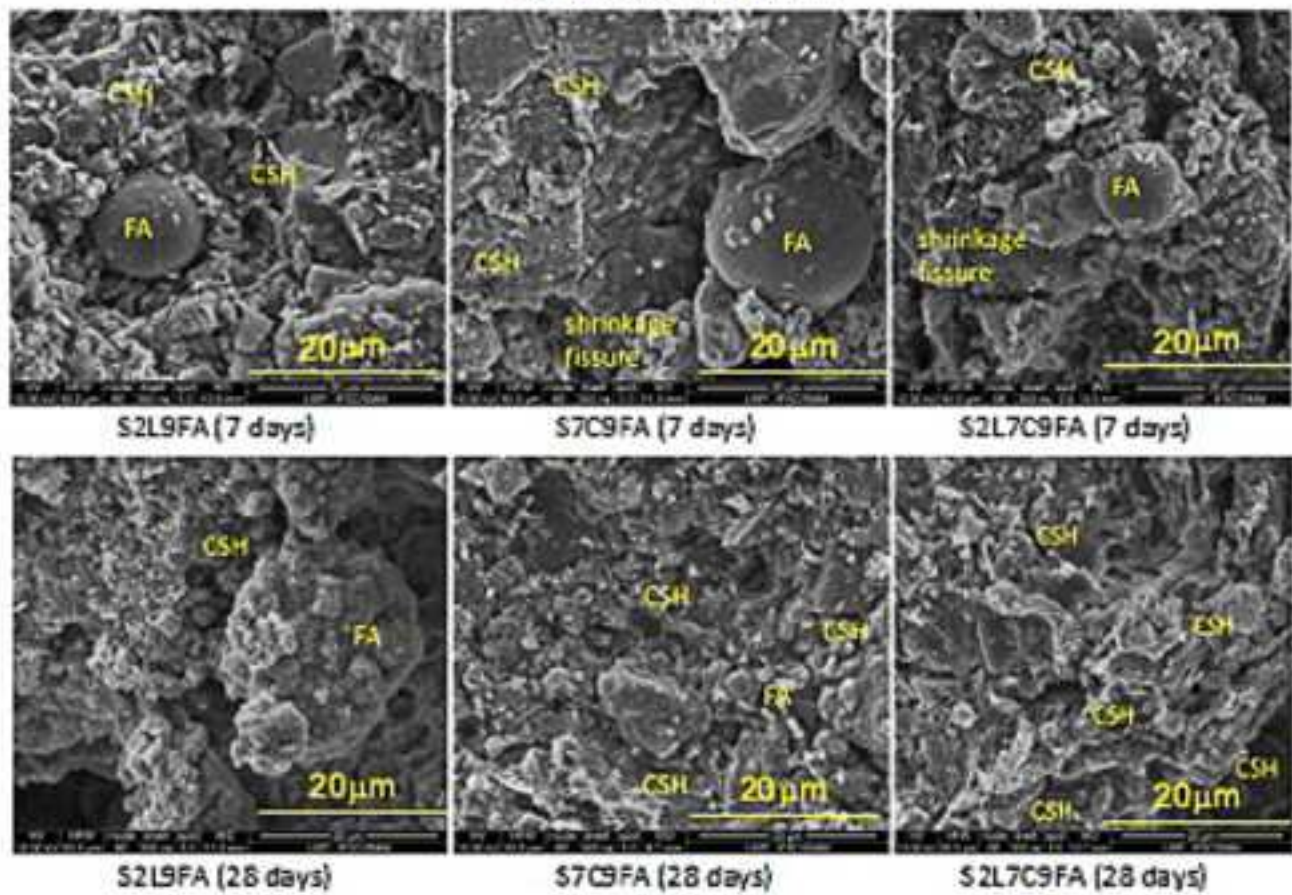


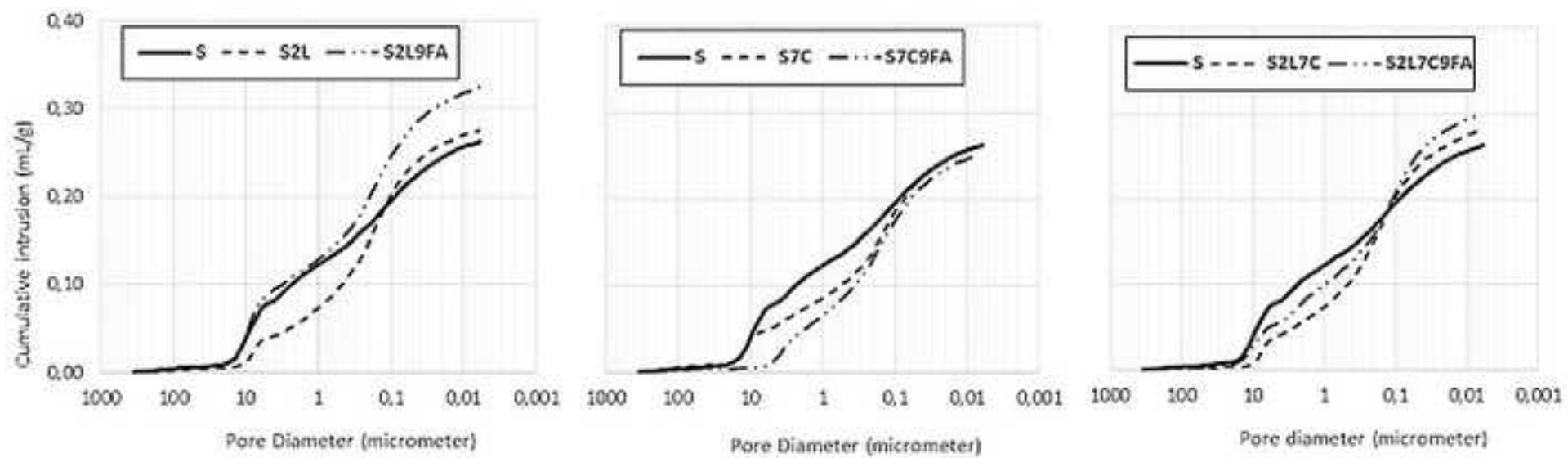


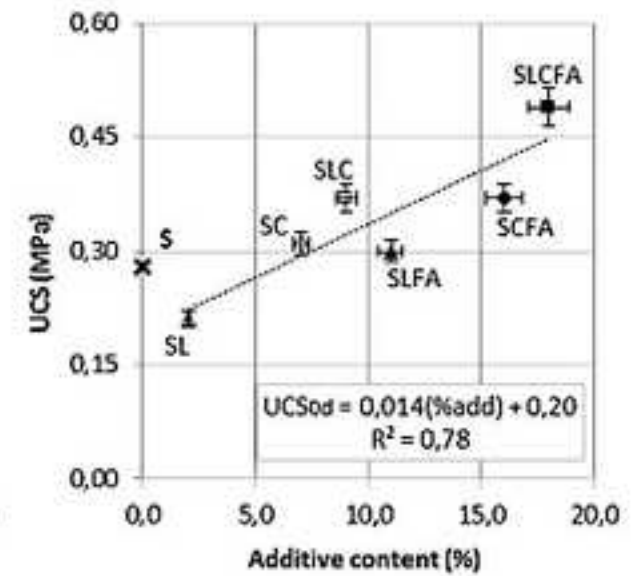
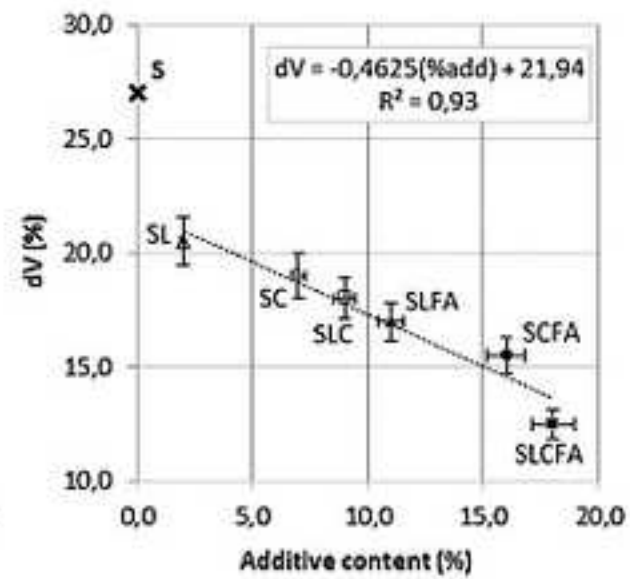
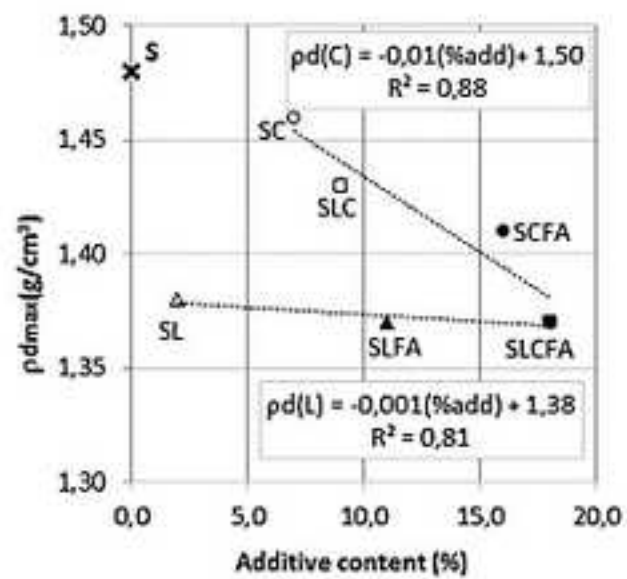
(a) mixtures without fly ash

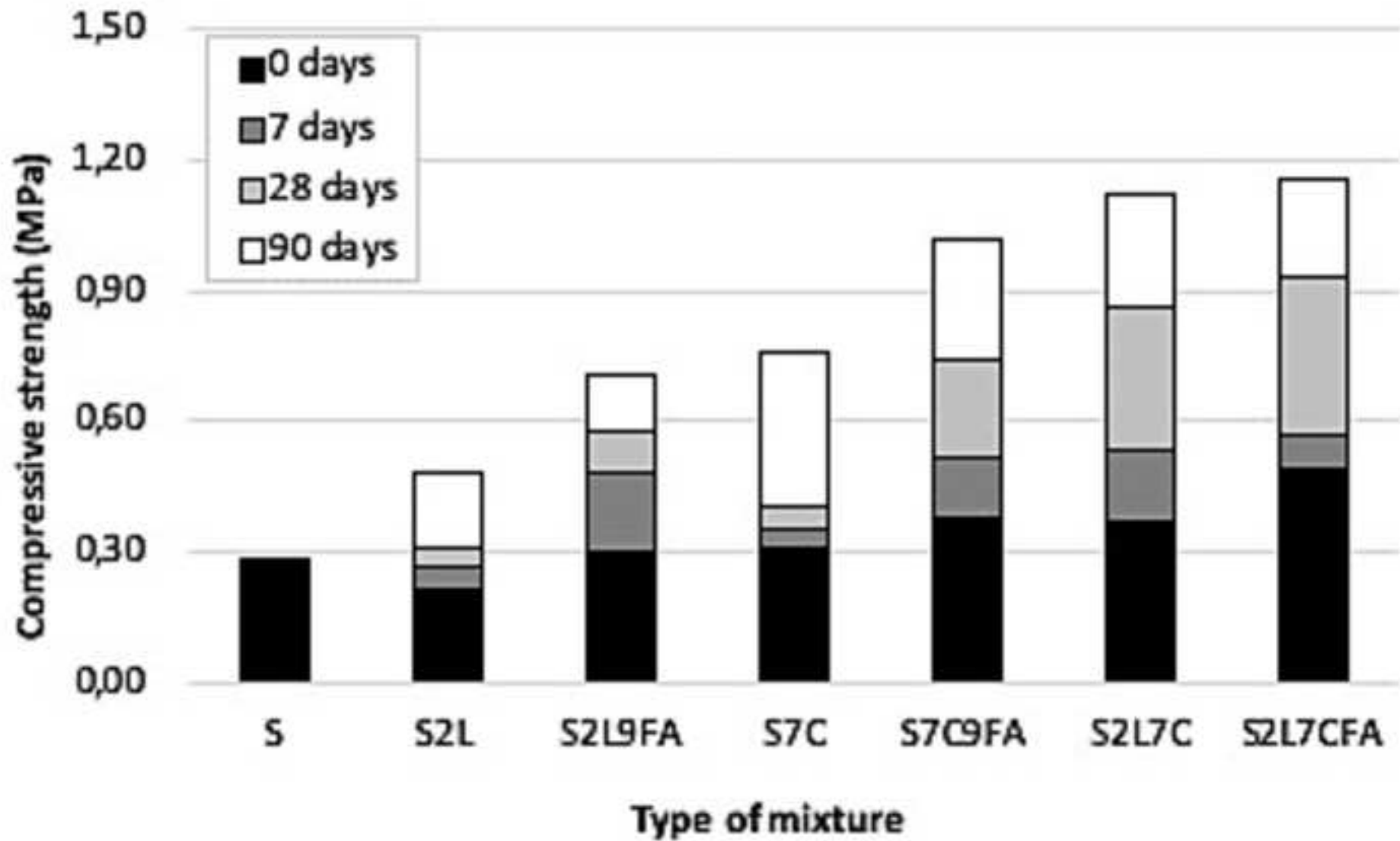


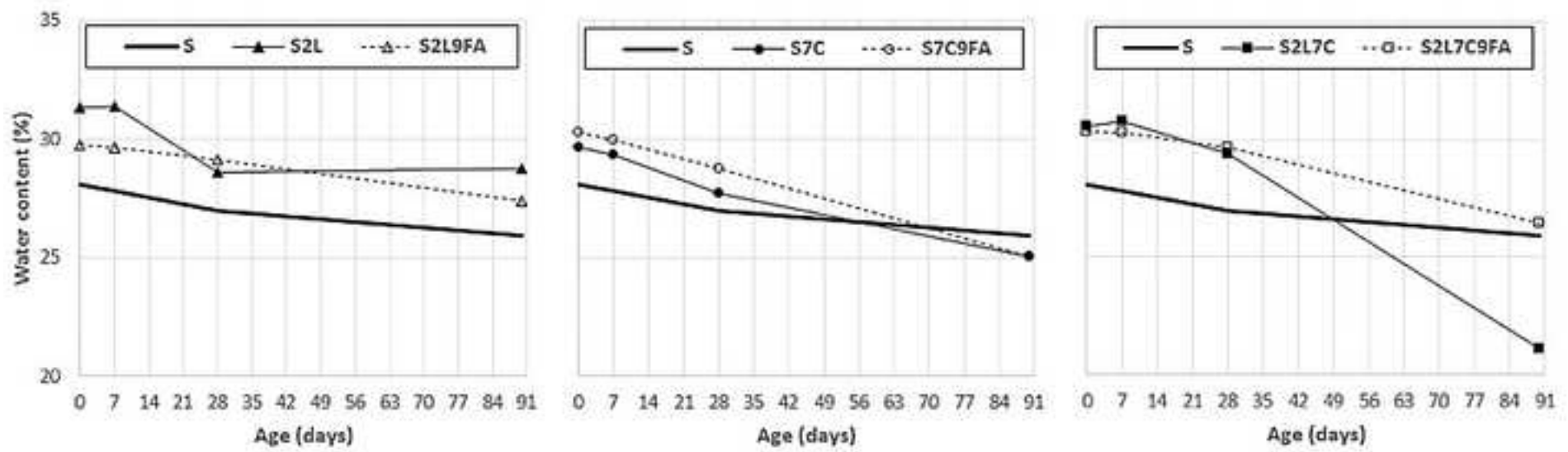
(b) mixtures with fly ash











Effect of FA addition on microstructural and resistance characteristics of a stabilized soil using lime and cement

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Graphic Abstract

