

3D numerical simulation of seagrass movement under waves and currents with GPUSPH

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3D Numerical simulation of seagrass movement under waves and

currents with GPUSPH

Abstract

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The current study tries a new approach to simulating interactions between waves and seagrass through Smoothed Particle Hydrodynamics (SPH). In this model, the plants are defined as a solid that respects Hooke's law, and are assumed to have direct interaction with the fluid. Given the characteristics of the SPH method, especially in terms of computational time, the dimensions of the simulations were limited. The first goal of the current study was to optimize the approach to avoid reaching certain limits such as the rupture of the simulated plant. Plant movements under waves and/or currents have been studied by several authors in various in-situ, physical, and numerical experiments concerning various vegetation species, thus, proving that plant movements can be successfully reproduced by SPH 2D/3D. Manning's roughness coefficient, n, wa calculated to confirm that the results were in accordance with what had been measured in flume studies. Even though there is still room for improvement, it is shown that this method can be used to estimate Manning's coefficient for coastal vegetation (seagrass and saltmarsh vegetation) and to greatly improve the modeling and forecasting of coastal erosion and storm surge risks by including the effect of vegetation in integrated models.

Keywords:

Numerical seagrass movement, GPUSPH, Fluid structure interaction, Waves,

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1. Introduction

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Traditionally, to study fluid-structure interactions, structures usually are described with a Lagrangian formulation while fluids often are represented through an Eulerian formulation. The formulations generally are coupled using an Arbitrary Lagrangian-Eulerian formulation for the fluid. Numerous fluid-structure interactions have been studied according to such formulations, including valve springs (Rugonyi & Bathe,

& Zhang, 2004), and the absorption of hydro-elastic shocks (Le Tallec & Mouro, 2001). 56 The Lagrangian Smoothed Particle Hydrodynamics (SPH) method is a different 57 approach to modeling in which fluids and/or solids are represented by a set of particles. 58 Each solid particle has defined individual material properties and moves under the effect 59 of the fluid, itself represented by another set of particles. The use of a Lagrangian 60 61 formulation for fluids can be more convenient for certain types of applications, such as fluid-free surface studies or large movements along a fluid-solid interface. Indeed, the 62 SPH method does not require specifically processing the fluid-free surface (no surface 63 64 tension) and the two groups of particles (fluid and solid) can be tracked simultaneously. The SPH method has already been successfully used to study interactions between 65 fluids (Monaghan et al., 1999) and solids (Gotoh & Khayyer, 2018; Gray et al., 2001; 66 67 Liang et al., 2017; Van Liedekerke et al., 2013; Zhang et al., 2017). The SPH method was developed in 1977 (Gingold & Monaghan, 1977; Lucy, 1977) 68 for applications in astrophysics. J. Monaghan (an Australian researcher in applied 69 70 mathematics) contributed greatly to the development of the method. Since 1985, the method has been used for other applications. In the early 1990s, the SPH method was 71 72 used for fluid simulations including free surface flows, waves (Monaghan, 1994; 73 Randles & Libersky, 1996; Dalrymple & Rogers, 2006), multiphase fluids (Monaghan & Kocharyan, 1995) and weakly compressible flows (Morris et al., 1997). In the field of 74 solid mechanics, simulations have mainly focused on the study of hypo-elastic solids 75 76 (Gray et al., 2001). Recent developments in science and engineering have increased the 77 need for modeling Fluid-Structure Interactions (FSI) to analyze multi-physics processes like retroactive loops, in which the pressure and viscous stresses of the fluid create 78 deformation in the solid, which in turn affects stress, pressure, and velocity in the fluid. 79

2001), the interaction of compressible and incompressible fluids with structures (Bathe

Modeling FSI can, for instance, be useful for aerodynamics studies, biomechanics 80 81 simulations, or airbag design (Agamloh et al., 2008; Farhat et al., 2006). The earliest models of stimulant-elastic solid and fluid interactions using the SPH method appeared 82 83 a little more than ten years ago and focused on the laws of interaction (Antoci et al., 2007; Amini et al., 2011) and the definition of the solid interface (Ha et al., 2011). To 84 date, the SPH method has not often been applied to fluid-structure interactions in 85 86 environmental studies (Zhang et al., 2017). Here, it is proposed to use this method to 87 evaluate the influence of submerged aquatic vegetation on waves and currents in natural hydrosystems. 88 89 Seagrasses form underwater meadows in the coastal zone. These meadows are known to attenuate waves (Bradley & Houser, 2009; Fonseca & Cahalan, 1992; Koftis 90 et al., 2013; Paul & Amos, 2011; Kobayashi et al., 1993), and currents (Fonseca et al., 91 92 1982; Fonseca & Fisher, 1986; Peterson et al., 2004; Widdows et al., 2008), and to significantly influence sediment dynamics (Boscutti et al., 2015; Madsen et al., 2001). 93 94 Depending on the biometric characteristics of the species (shoot density, leaf area index, plant stiffness, submergence ratio) and the morphological characteristics of the 95 meadows (extension, fragmentation), the effect on hydrodynamics will vary (Fonseca & 96 Koehl, 2006; Koftis et al., 2013; Paul & Amos, 2011; Stratigaki et al., 2011). The non-97 linear interactions of these parameters with hydrodynamics variables (wave height, 98 wave period, water level, presence of a current) can also influence the attenuation of 99 waves, the modification of currents and sediment dynamics (Bradley & Houser, 2009; 100 101 Paquier et al., 2019; Paul & Amos, 2011). Large-scale hydrosystem models can integrate the role of vegetation through a 102 change in the roughness coefficient. This can be done empirically through the 103 modification of Manning's n value (for example, in the ADCIRC model (Luettich et al., 104

1992)). In other models, such as Xbeach (Roelvink et al., 2009) or Telemac (Hervouet, 2007), a specific vegetation roughness, as expressed by Mendez and Losada (2004), can be used. The calculation of this parameter is based on the geometric and physical characteristics of the vegetation meadow. This approach was very innovative in 2004, and is now widely used in modeling. However, it does not account for vegetation flexibility, which can be very important in subaquatic vegetation, especially when canopies are sparse (Paquier et al., 2019). As seagrass meadows are declining around the world (Waycott et al., 2009), it seems essential to better take into account vegetation flexibility in hydrodynamics modeling and to better understand the role of declining meadows or, more generally, sparse canopies. This could help to investigate seagrass meadows' limitations as coastal buffers protecting against erosion and storm surges, and could potentially enhance the protection of these declining ecosystems.

Sub-Aquatic Vegetation (SAV) is known to play a role in wave and storm surge mitigation, and, thus, in coastal protection. The impacts of different species under different conditions have already been studied (see introduction of Paquier et al. (2019) for a review of the different processes observed). SAV has the ability to attenuate waves and currents and to modify turbulence. However, no study to date has compared the impacts of the different existing species. Such studies are nearly impossible to do in the field considering the variety of environments in which the different species live. Furthermore, experiments in flumes with real marine plants often are forbidden to protect the marine plant species. A numerical study could, therefore, help remedy the lack of *in-situ* studies. However, the modification in hydrodynamics generated by the plants themselves is still difficult to estimate since plant movements under waves and currents are still not well simulated and no exact numerical model exists (Mendez & Losada, 2004; Sánchez-González et al., 2011). Generally, existing numerical studies use

rigid bodies to simulate plants with different roughness coefficients (Dalrymple et al.
1984; Wallace & Cox, 2000). However, among the numerical studies dealing with
plants, Beudin et al. (2017) describe a three dimensional (3D) modeling framework that
simulates the interactions between waves, flow, and flexible vegetation. Beudin et al
(2017) evaluated a plant drag model against field measurements in eelgrass canopies
and investigated the hydrodynamics in an idealized shallow-water setting.

The research presented herein aims to reproduce plant movement under waves and/or current using the SPH method. The method was tested by building numerical two dimensional (2D) and 3D flumes with the GPUSPH model (GPUSPH official website. www.gpusph.org. Accessed: 2017-03-31.; Gómez-Gesteira et al., 2010; Hérault et al., 2010, 2014), then comparing the simulations with the available data.

After a brief description of the SPH method (definition of the two numerical approaches used for plants and water), several tests are presented of the FSI simulations using the proposed model and these are compared with other simulations and with experimental data (the FSI was calibrated and evaluated in the open source GPUSPH code (Hérault et al., 2011).

The numerical simulation results are compared with experimental results based on real or plastic seagrass meadows. The Young modulus is evaluated for real or fake plants for both the experimental and numerical approaches. The wave attenuation due to the seagrass meadow in the simulation also compared with experimental data. Finally, a first simulation also is run with field data, collected in a bay occupied by a *Zostera Noltei* meadow (another species of marine plant) in a large lagoon in Southern France.

2. The SPH method and the FSI approach

The SPH method is based on the theory of interpolation (Benz, 1990; Liu, 2002; Monaghan, 1982; Monaghan, 1992; Monaghan, 2005). Its formulation often is divided into two parts: (i) the integral representation, and (ii) the particle approximation. The equation for the integral representation generally used in SPH is given in Appendix A.

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2.1. Equations for modeling the fluid

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- The fluid was assumed to be only slightly compressible, isothermal, and governed by the continuity equation:
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$$\frac{D\rho}{Dt} = -\rho \frac{\partial v_i}{\partial x_i} \tag{1}$$

and the motion equation:

$$\rho \frac{\mathrm{D}v_i}{\mathrm{D}t} = \frac{\partial \sigma_{ij}}{\partial x_i} + \rho b_i \tag{2}$$

with $\frac{D}{Dt}$ defined by the material derivative:

$$\frac{\mathrm{Du}}{\mathrm{Dt}} = \frac{\partial u}{\partial t} + v \cdot \nabla u \tag{3}$$

- where ρ , v_i , σ_{ij} , and b_i are the density, velocity in i direction, stress tensor in ij
- directions and force vector, respectively; t is time, x_i is the position in the i direction.
- 171 Stress tensor σ_{ij} is defined as:

$$\sigma_{ii} = -p\delta_{ii} + S_{ii} \tag{4}$$

- where p is pressure, δ_{ij} is the Kronecker tensor and S_{ij} is the deviatoric shear stress both
- in the ij directions.

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176 2.2. Equations for modeling the solid

The solids are governed by the same equations as the fluid. The stress tensor for a solid can similarly be decomposed into its isotropic pressure p and deviatoric shear stress S, as follows:

$$\sigma_{ij} = -p\delta_{ij} + S_{ij} \tag{5}$$

- where $p = -\sigma_{kk}/3$ is the isostatic pressure, δ_{ij} is the Kronecker tensor, and S_{ij} is the deviatoric shear stress (as for the fluid).
- 184 Equation 2 can, thus, be written as:

$$\frac{\mathrm{D}\mathbf{v}_{i}}{\mathrm{D}\mathbf{t}} = \frac{1}{\rho} \frac{\partial S_{ij}}{\partial x_{i}} - \frac{1}{\rho} \frac{\partial p}{\partial x_{i}} + b_{i} \tag{6}$$

The linear relation that connects the stress and strain tensors can be derived from the time equation, S_{ij} . This is done by applying the Jaumann rate and gives the following corrected Hooke's law:

$$\frac{\mathrm{DS_{ij}}}{\mathrm{Dt}} = 2\mu \left(D_{ij} - \frac{1}{3} D_{kk} \delta_{ij} \right) + S_{ik} \Omega_{jk} + \Omega_{ik} S_{kj} \tag{7}$$

where μ is the shear modulus, defined as:

$$\mu = \frac{E}{2(1+\nu)} \tag{8}$$

- and where E represents Young's modulus and ν represents Poisson's ratio. D_{ij} is the
- 193 strain rate tensor:

$$D_{ij} = \frac{1}{2} \left(\frac{\partial v_i}{\partial x_j} + \frac{\partial v_j}{\partial x_i} \right) \tag{9}$$

and Ω_{ij} is the rotation tensor:

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$$\Omega_{ij} = \frac{1}{2} \left(\frac{\partial v_i}{\partial x_j} - \frac{\partial v_j}{\partial x_i} \right) \tag{10}$$

In the case of large deformations, energy is not conserved. However, this method has
the advantage of describing both the fluid and the solid dynamics in terms of pressure
and velocity.

Pressure can be defined in the same way for both solids and fluid by applying the linearized equation of state:

$$p = c_0^2 (\rho - \rho_0) \tag{11}$$

- with the speed of sound being $c_0 = \sqrt{\frac{\varepsilon}{\rho_0}}$ for the fluid and $c_0 = \sqrt{\frac{K}{\rho_0}}$ for the solids.
- 204 ε is the modulus of the fluid studied, ρ_0 a reference density, and K is the modulus of
- elasticity of the solid, which can be expressed as a function of Young's modulus, as
- 206 follows:

$$K = \frac{1}{3} \frac{E}{(1-2\nu)} \tag{12}$$

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2.3. Approximation of the motion equations through the SPH method

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2.3.1. The fluid particles

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The following equation is obtained for the velocity of the fluid particles:

$$\frac{\mathrm{D}v_{iq}}{\mathrm{D}t} = -\sum_{b} m_{b} \left(\frac{p_{a}}{\rho_{a}^{2}} + \frac{p_{b}}{\rho_{b}^{2}}\right) \delta_{ij} \nabla_{a} W_{ab} + g_{i}$$
(13)

- with g_i = gravity, m_b the masse of the b particle, and the classical indices i, j, q, a,
- and b of the SPH method.

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2.3.2. *The solid particles*

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The following equation is obtained for the velocity of the solid particles:

$$\frac{\mathrm{Dv}_{i_q}}{\mathrm{Dt}} = \sum_b m_b \left(\frac{S_{ij_a}}{\rho_a^2} + \frac{S_{ij_b}}{\rho_b^2} - \left(\frac{p_a}{\rho_a^2} + \frac{p_b}{\rho_b^2} \right) \delta_{ij} \right) \cdot \nabla_a W_{ab} + g_i$$
 (14)

However, Eq. 14 suffers from numerical instability and requires two terms to be introduced: one term for artificial viscosity and another term to correct tensile instability in the solid traction.

Artificial viscosity, Π_{ab} , was proposed by Monaghan (Monaghan & Gingold, 1983; Monaghan, 1992) and is used to smooth the oscillations caused by speed and to prevent the particles from being non-uniformly distributed in space when they are too close to each other.

$$\frac{\mathrm{Dv}_{i_a}}{\mathrm{Dt}} = \sum_b m_b \left(-\left(\frac{p_a}{\rho_a^2} + \frac{p_b}{\rho_b^2}\right) \delta_{ij} + \frac{S_{ij_a}}{\rho_a^2} + \frac{S_{ij_b}}{\rho_b^2} + \Pi_{ab} \delta_{ij} \right) \cdot \frac{\partial W_{ab}}{\partial_a \partial x_i} + g_i \quad (15)$$

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$$\Pi_{ab} \begin{cases} \frac{-\alpha c'_{ab}\mu_{ab}}{\rho'_{ab}} & v_{ab}r_{ab} < 0\\ 0 & v_{ab}r_{ab} > 0 \end{cases}$$
 (16)

232 with

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$$\mu_{ab} = \frac{h(v_{ab}r_{ab})}{|r_{ab}|^2 + 0.1h}$$
, $r_{ab} = r_a - r_b$, $v_{ab} = v_a - v_b$, $c'_{ab} = \frac{1}{2}(c_a + c_b)$ (see Appendix

234 A).

This last term introduces a numerical dissipation, which must be minimized so as not to affect the solution. Antoci et al. (2007) proposed a compromise value between oscillation and dissipation.

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2.4. Correcting for speed

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To move the particles, the GPUSPH code uses the XSPH technique (Monaghan, 1989). For each particle, a term containing the average speed of the surrounding particles is added to the original equation. Particle velocities, \vec{v} , were corrected according to the XSPH scheme to smooth out unwanted numerical peaks.

$$\overrightarrow{v_a} = \overrightarrow{v_a} + \theta \frac{\sum_b \frac{m_b}{p_{ab}} (\overrightarrow{v_b} - \overrightarrow{v_a}) W_{ab}}{\sum_b \frac{m_b}{p_{ab}} W_{ab}} \tag{17}$$

The value of θ depends on the medium: $\theta = 0.08$ for the fluid which is increased to 1 to support high deformation for the solid.

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2.5. Validation of solid elasticity

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As proposed by Gray et al. (2001) and Antoci et al. (2007), the sensitivity of the artificial viscosity and the XSPH factor can be tested by simulating the free oscillations of an elastic plate having one end free and the other held in place. The plate is initially horizontal (Fig. 1a) and an initial velocity distribution is assigned according to the analytical expression of the free oscillations of a thin plate:

$$v_z = v_{l0}c_0 \frac{f(x)}{f(L)}$$
 (18)

- 258 where L is the length of the elastic plate and
- 259 f(x) =

$$260 \quad \left(\cos(kL) + \cosh(kL)\right)\left(\cosh(kx) - \cos(kx)\right) + \left(\sin(kL) - \sinh(kL)\right)\left(\sinh(kx) - \sin(kL)\right) + \left(\sin(kL) - \sinh(kL)\right)\left(\sinh(kx) - \sin(kL)\right) + \left(\sin(kL) - \sinh(kL)\right) + \left(\sin(kL) - \sinh(kL$$

$$261 \quad sin(kx)) \tag{19}$$

- while $v_{l0} = 0.01$ determines the initial velocity of the free end and kL = 1.875 is the
- fundamental mode. For a plate with the following features: L = 0.2 m, H = 0.02, m
- 264 $h = 0.2 \text{ m}, \rho = 1000 \text{ kg/m}^3, K = 3.25 \times 10^6 \text{ N/m}^2, \mu = 715000 \text{ N/m}^2$, the analytical
- non-dimensional amplitude (A) and period (T) of the first oscillation are A/L = 0.115
- 266 and $T c_0/L = 72.39$.
- 267 Gray et al. (2001) obtained A/L=0.125 and $T c_0/L = 82$ and Antoci et al. (2007)
- obtained A/L=0.124 and $Tc_0/L = 81.5$. For the proposed model the following was
- obtained A/L=0.116 and $T c_0/L = 73.34$ (Fig. 2). The small difference between the
- 270 results can be attributed to several factors, but it's difficult to compare 2D simulations

and the current 3D simulations, since the mesh is different. The difference between the current results and those presented by Antoci et al. (2007) and Gray et al. (2001) probably come from the current use of 3D simulation.

As proposed by Antoci et al. (2007), this experiment allowed the artificial viscosity coefficient, α , to be tested. In agreement with Antoci et al. (2007), the current numerical experiment showed that if $\alpha = 0$, the simulation is unstable; when $\alpha > 1$ the free oscillations damping reduces. Therefore, it was decided to use $\alpha = 1$, which caused some slight energy dissipation but guaranteed good stability.

Monaghan (1992) proposed a tensile instability corrective formula specifically for 2D simulations, but which does not seem to be needed for 3D simulations. It can be replaced by simply increasing the XSPH coefficient. For $0.5 < \theta < 2.5$, similar results always were obtained. For simulations with higher initial velocities of the free end of the plate ($v_{l0} = 0.05$ and $v_{l0} = 0.1$), good high oscillations without breakage were obtained for $\theta = 1$ but the plate broke or become unstable for lower θ values (see Fig. 1b), and permanent deformations occurred for higher θ values.

2.6. Fluid-Solid Interactions

To simulate seagrass motion under waves and wave induced-currents, solid particles must be defined to simulate the plants (a few millimetres thick) and the flume (a few meters long). It was decided to create different particle sizes depending on the nature of the element (water, interface, solid (see Fig. 1c)) and to adjust some interaction laws.

In order to use smaller particle sizes for the solid, interface particles were constructed from the solid particles. They were then initialized with the sum of the masses of the

neighboring solid particles. The interface particles interact with the fluid particles

according to the different laws previously presented. The stresses on fluid particles are directly transmitted to the interface particles, and for solid particles, the stresses are transmitted proportionally to the mass ratio between the interface particles and the solid particles. This treatment prevents a solid particle from being subjected to significant stress due to its low mass. It also makes it possible to smooth and evenly distribute the force on the solid particles.

Three different forces were tested to determine which one had the best fit for the current simulations. The computational methods developed for FSI simulations with deformable structures in the context of particle methods can be classified into several categories, including FSI solvers. The latter were developed by coupling particle methods with other computational methods or by integrating Lagrangian particles into solvers for both the fluid and the solid. Several examples can be found in the reviews on SPH methods done by Gotoh and Khayyer (2018) and Zhang et al. (2017).

Following Antoci et al. (2007), two types of boundary conditions were tested. First, the Lennard-Jones boundary condition as used by Monaghan (2005) was tested. The well-known Lennard-Jones formulation is a purely repulsive force principally used for molecule interactions. The force, f(r), per unit mass between a solid and a fluid particle separated by a distance r has the form:

$$f(r) = D\left(\left(\frac{r_0}{r}\right)^{p_1} - \left(\frac{r_0}{r}\right)^{p_2}\right) \cdot \frac{r}{r_0} \quad if \quad r > r_0$$

$$f(r) = 0 \quad if \quad r \le r_0 \tag{20}$$

The constants p_1 and p_2 (Eq. 20) must satisfy the condition $p_1 > p_2$ to be repulsive; we set $p_1 = 12$ and $p_2 = 6$, were selected as proposed by Monaghan (1992).

The mass of an interface particle is calculated as the sum of the masses of the nearest solid particles within its radius of action (a solid particle being bound to a single interface particle). The interaction is then calculated between the interface particles and

the fluid particles, and the force calculated on the interface particles then is transmitted to the linked solid particles, taking into account the mass ratios. The distance used in the interaction laws between the interface particles and the fluid particles depends on the size ratio between the interface particles and the solid particles at initialization so that the interaction volume equals the volume of the solid particles.

Coefficient D depends on the water depth, H, and it is usually taken as D = 5gH. D is applied to all the particles, and, therefore, does not generate any difference between particle types.

A formulation proposed by Kajtar and Monaghan (2008) also was tested, i.e., the same correction, as explained for (Eq. 20), was applied to the length scale. Both boundary forces yielded similar results in the calculation of solid and fluid interactions. Finally, the second boundary condition formulation that was tested concerned extending the fluid and solid momentum and continuity equations to all particles regardless of their nature. This formulation gave good results for the experiments of Antoci et al. (2007). However, in cases with large differences in density between the fluid and the solid, when impacts occurred, this formulation created local high pressure. Studies have questioned or perfected these different approaches to boundary conditions (Crespo et al., 2007; Ferrand et al., 2013). In the current case, the standard options in the GPUSPH code worked well enough for the current needs.

3. Calibration and comparison with experimental data

Antoci et al. (2007) did a laboratory experiment and compared the results with SPH simulations. The experiment consisted in deforming an elastic bundle acting as a dam in a water column. The configuration of the experiment is described in Antoci et al. (2007). Table 1 lists the size experiment parameters. The elastic properties of the beam

can be determined from Antoci et al. (2007). For the current simulations, a Young's modulus that provided the results closest to the experimental results was chosen. Liquid and elasticity properties are listed in Table 2.

To calibrate and validate the fluid-structure interaction (FSI) model used for plant movement, the current results were compared for the vertical (Fig. 3a) and horizontal movements (Fig. 3b) at the end of the plate with Antoci et al. (2007). The numerical results are consistent with the experimental results found by Antoci et al. (2007).

A fluid compressibility below the theoretical value was used to reduce the time step, as was done by Antoci et al. (2007). Despite this difference in compressibility, the displacements obtained were within the expected range.

In the current study, the Lennard-Jones (Monaghan, 2005), and Kajtar and Monaghan (2008) boundary conditions were used, and better results were found with the latter formulation. The proposed multi-scale approach also was validated with Kajtar and Monaghan (2008) boundary conditions for solid particles until they were ten times closer together than the fluid particles and with an interface particle similar to the one described in the previous section. A multi-scale approach such as this is needed to simulate small elastic solids in a fluid domain.

4. Results and discussion

The plant movements predicted with the proposed SPH simulations were compared with both experimental results (Le Bouteiller & Venditti, 2015; Luhar & Nepf, 2011) and field observations (Paquier et al., 2019).

Different species of seagrass colonize the shallow coastal waters around the world, often forming large underwater meadows. Seagrass meadows are known to reduce wave height and current velocity near the seabed, but the simulation meadow (see Fig. 4), could be also a problem for the simulations. The different datasets used for the current comparisons were collected on real or mimicked plants of different species. Luhar and Nepf (2011) and Stratigaki et al. (2011) worked on mimics of *Posidonia oceanica*, Le Bouteiller and Venditti (2015) workedon mimics of *Zostera marina* and Paquier et al. (2019) worked on real *Zostera noltei*.

4.1. Comparison using the initial 2D approach

The comparison was done on the basis of Luhar and Nepf's (2011) study. Plant modeling relies on solid knowledge of the physical properties of the plant (height and thickness but also density and elastic modulus). In addition, it is complicated to numerically simulate a very thin leaf. For the species *Posidonia Oceanica*, Folkard (2005) gives clear leaf properties (Table 3). Poisson's ratio is not given, but generally ranges from 0.3 to 0.4 for this type of solid.

Several limitations were encountered during the simulation process of the *Posidonia Oceanica* leaf thickness. First, the thickness of the plant leaf is naturally 0.1 mm and the particle size used for the simulations was 5 mm (a ratio of 1/150). To deal with this issue two choices were available: either divide the particle size by 150 or modify the plant properties while conserving a comparable flexibility. The 2D code as used originally (Oudart & Larroudé, 2012) does not accept multi-sized particles, which would induce a decrease in the water particle size. Furthermore, reducing the particle size would have dramatically increased the number of particles, and, thus, the computation time. The memory needed to run such a calculation is much too large to

allow the use of the SPH method. Therefore it was decided to modify the plant properties. Plant thickness, Young's modulus and fluid compressibility were modified.

The impact of density was not investigated; and another study should be done on this

point. The final modified parameters of the plants are listed in Table 3.

Some seagrass species' leaves (including *Posidonia Oceanica*) grow from shoots in small groups (the number of leaves per shoot depends on the species and can vary from one shoot to another). These shoots cannot be simulated in a 2D model. However, several leaves close to each other were simulated in order to represent a natural meadow as well as possible. Figure 5 shows the movement of three simulated leaves (as a first approach to meadow simulation) under a breaking wave. The numerical representation of the deformation of the plants in a meadow is therefore possible.

Luhar and Nepf (2011) did laboratory experiments to study the flow-induced reconfiguration of flexible subaquatic vegetation. The plant mimics used were inspired by *Posidonia Oceanica* plants; the drag and posture of the mimics were within the natural range for stiffness and buoyancy. Luhar and Nepf (2011) then compared the experimental results with a theoretical model. Two current velocities were tested: one lower and one higher than the restoring forces, the latter flow bending the leaves. The current simulation was compared to Luhar and Nepf's (2011) experiments from a qualitative point of view. Luhar and Nepf (2011) built two kinds of leaves:

- one made of silicone foam (Young's modulus, E=500 kPa; $\Delta \rho=330$ kg/m³ with a seawater density assumed to be 1025 kg/m³; thickness = 1.9 mm);
- one made of high-density polyethylene (HDPE) (Young's modulus, E = 0.93 GPa;
- $\Delta \rho = 50 \text{ kg/m}^3$; thickness = 0.4 mm).
- Figure 6a shows Luhar and Nepf's (2011) experimental set up and the comparison with the current simulations (Figs. 6b,c and Figs. 6d,e). For both current velocities, the

SPH simulation gives the correct inclination of the plant. For the low current velocity 419 (U = 0.16 m/s), an inclination angle of 45° was obtain versus 38° angle obtained in the 420 experiment with silicon foam leaf blades (see Figs. 6b,c). 421 For the higher current velocity (U = 0.32 m/s), Luhar and Nepf (2011) measured an 422 angle of 21° while an angle of 26° was simulated. The shape of the numerically 423 simulated blade is in reasonable agreement with the experimental results shown in black 424 425 on the picture (see Fig. 6e). 426 In the experiment, Luhar and Nepf (2011) also built artificial leaf blades from HDPE. Again, the SPH simulation showed results similar to the experimental results. The 427 difference Luhar and Nepf (2011) observed in the angle taken for the HDPE leaf 428 compared to the silicon leaf is well represented by the SPH modeling. 429 430 431 4.2. Comparing simulated 2D and 3D plant effects on waves with experimental results 432 433 434 The simulated 2D results were compared with the experimental results of Luhar and Nepf (2011) and Stratigaki et al. (2011); both researchteams worked on Posidonia 435 Oceanica mimics. The simulated 3D results were compared with those of Paquier et al. 436 (2019), who worked in the field on real Zostera noltei plants (a smaller species than 437 Posidonia Oceanica). 438 439 The 3D modeling approach used in the current study allowed simulation of plant movements under waves. The results for different h₀/h_{plant} ratios (where h₀ is the 440

reference water level in the channel and h_{plant} is the height of the plant) were compared

for one plant located at a distance of x = 1.2 m (Fig. 7) from the piston paddle. The

height of the wave generated by the piston paddle was influenced at x = 0.6 m from the

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paddle. This boundary condition is too close to the seagrass meadow and the computational domain had to be increased (Oudart & Larroudé, 2012; Oudart et al., 2013).

The wave heights at x = 1 m and x = 1.3 m were nearly identical for all the simulations: wave height increased at the approach to the plant and decreased significantly afterwards. However, when approaching the area of the channel slope (at x = 1.7 m), there was another increase in wave height, which varied according to the ratio h_0/h_{plant} . The presence of the plant generated a decrease in the wave height at this location, due to a loss of energy resulting from friction. Reflecting this situation numerically would require complementary simulations with the foot of the beach slope far from the end of the sea grass meadow to avoid any slope influence on wave attenuation.

Different scenarios were simulated: a channel without plants (Case 0), with one elastic plant (Case 1), with one rigid plant (Case 2), and with three elastic plants located at x = 1 m, x = 1.2 m, x = 1.4 m from the wave maker (Case 3). Plant height was set at 0.1 meter ($h_0/h_{plant}=0.5$ m, $h_0 = 0.2$ m); the changes in wave height relative to h_0 along the cross section is represented in Fig. 7. In the presence of a rigid plant (Case 2), wave height decreased then increased again after the plant due to the recirculation generated by the rigid obstacle. With a three-plant meadow (Case 3), the decrease in wave height was higher than with a single plant (Case 2) and significantly higher than without any plants (decrease 24% greater for Case 3 than for Case 0). Stratigaki et al. (2011) observed a decrease of 30 to 35% in wave height in their experimental study. These higher rates can be explained by a longer friction zone in the experiment than in the numerical simulations shown in Fig. 7.

2D simulations were appropriate for comparison with some experimental studies (Antoci et al., 2007; Luhar & Nepf, 2011; Stratigaki et al., 2011). These experiments are consistent with a 2D approach because very little flow can pass on each side of the seagrass. Indeed, the plant mimics in the foregoing studies were as wide as the channel or the meadow took up the whole width of the experimental set up. Obviously, to simulate experiments with sparser seagrass meadows or marine plants from coastal zones other than the ones studied by Stratigaki et al. (2011) and Luhar and Nepf (2011), a 3D simulation is necessary. The proposed multi-scale 3D FSI simulations with the SPH method are able to reproduce the shape and phasing of leaf movements resulting from the wave passing over the plant or plant meadow. This is the first step toward developing a more quantitative approach. It also was possible to simulate this movement for an entire 3D meadow of 200 plants (Fig. 8a) and for a 3D meadow with two separate zones of seagrass (Fig. 8b). With a 3D method, different particle sizes can be used for water and plants. This makes it possible to (i) create plants with a more realistic thickness, (ii) to simulate several plants (200 in the current case), and (iii) to simulate several densities of seagrass meadow with realistic bathymetry (see Fig. 9). Fig. 9d shows the numerical results with and without a plant meadow on a real bathymetry (the "Point de Berre" beach in Berre Lagoon, France, (Paquier et al., 2019)). This first qualitative approach with the 3D SPH method clearly shows the attenuation of the waves by the plants. The simulated plant movements (Figs. 9a, b and c) are also qualitatively consistent with those observed in the study area (video available in the supplementary material in Paquier et al. 2019). The next research step will be to calibrate the model with in-situ wave measurements.

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4.3. Comparison of simulated flow properties (Manning's coefficient) with experimental results

The aim of the TEC21 post-doc program at the University of Grenoble was to numerically reproduce the experiment of Le Bouteiller and Venditti (2015), which had been done on mimics of *Zostera marina* in an inclined flume (15 m long and 1 m wide) located in the River Dynamics Laboratory at Simon Fraser University in Canada (Figs. 10a,b). The central section of the flume between 5 and 11 m was covered with a vegetated bed (see Fig. 10a) consisting of staggered artificial blades. The blades were made of a low density polyethylene film (920 kg/m³) with a Young's modulus of 2x10⁸ Pa, a length of 25.5 cm, a width of 0.75 cm and a thickness of 0.2 mm (see Fig. 10b). The material and dimensions of the blades were chosen for their similarity to natural *Zostera marina* plants to maintain the ratio of flexural rigidity to buoyancy forces (Ghisalberti & Nepf, 2002). Two densities of plants were used in the experiments: 130 blades (low density) and 800 blades (high density) per square meter.

To simulate the interactions between the low-density meadow and hydrodynamics, several lines of plants were built. For the low-density meadow experiment, the lines were 6.25 cm from each other and the plant mimics were spaced 12.5 cm apart on each line. The alignment of the plants on one line was shifted by 6.25 cm from the previous line to form an alternating network. The nearest surrounding plants were, therefore, around 8.8 cm away. For the high-density meadow experiment, the lines were separated by 2.5 cm while the plant mimics on each line were affixed every 5 cm. Again, an alternating network was built. The nearest surrounding plants were, therefore, around 3.56 cm away. In the simulation presented here, the low-density meadow case was

considered (see Fig. 10c). The simulated movements of the plants correspond to what was observed during the actual flume experiment (see Figs. 10b,c, and d), even if a better quantitative evaluation of the simulated plants' movements is still needed. A Manning's n coefficient similar to the one found experimentally by Le Bouteiller and Venditti (2015) based on a uniform flow hypothesis was evaluated. For the low-density meadow, Le Bouteiller and Venditti (2015) found a Manning's n coefficient value of 0.023, which remained constant under increasing flow velocity. In the current study, a Manning's coefficient equal to 0.025 was obtained. A better parametrization of the Manning's n evaluation is still needed but it does seem appropriate to use the GPUSPH simulation to calculate the Manning's n coefficient. The evaluation of the Manning's n following the SPH simulation gave good results for the mesh (number of particles) used in the test cases. In future studies, the mesh should be increased to refine the results. What is pointed out in the current study is that it is feasible to use the same Manning's n evaluation methodology for 3D SPH simulations and real experiments.

5. Conclusions

The results obtained for the various FSI simulations with the SPH method were reasonable, even though improvements can be made in the future. First, in a deformation test case of an elastic plate under a column of water, it was shown that it is possible to model the movement of a solid submitted to water effects with the SPH method and FSI. Plant movements also were successfully simulated in a wave flume. The different curves taken by the plant under the passing waves show the role of the plant in wave height modification. In these initial simulations, the decrease in wave

height was significant. The influence of plant flexibility (in opposition to a rigid body) also was investigated. Further numerical experiments are needed to include larger and/or higher meadows that have been studied in physical experiments and in-situ in the literature.

Current modeling limitations include the sizing problems of the particles related to the cost in computation time but these limitations will undoubtedly be overcome in the future. Even so, the SPH method is still time-consuming in terms of computation for simulations over large areas. It is, however, a good way to evaluate an equivalent viscosity that can be applied in other methods and models. Once wave damping in-situ can be quantitatively simulated, researchers will be able to simulate the role of any plant and to determine an equivalent viscosity depending on each plant's properties. Improvements will likely be made to the methodology presented here for comparing SPH models with existing experiments and the computational time required for 3D calculations will tend to decrease with better computer technology. For the time being, however, it is difficult to do quantitative comparisons between simulations and existing experimental results due to the differences in the initial parameters themselves and to the limitations of the SPH method (especially regarding computational time). The next step in this research will be to build simpler theoretical cases (for which the dimensioning of the elements and the quantification of the fluid and solid movements are known) through SPH simulations and to evaluate the need to add a coefficient to the law of interaction.

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It was also possible to simulate long plastic blades with high flexibility based on the experimental flume data from Le Bouteiller and Venditti (2015). This allowed a Manning's n coefficient to be calculated consistent with the one experimentally

evaluated by Le Bouteiller and Venditti (2015). The next step in this study will be to ensure the quantification of this numerical reproduction and to improve the accuracy of the estimated Manning's n roughness coefficient for a variety of seagrass species. There is a real need for these values for the coastal management community's use in integrated models.

Interest in the role of nature-based defences is growing (Bridges et al., 2015; "Committee on Environment", 2015) and model users must respond to this need. The proposed method, applied to a wide range of vegetation species with various biometric parameters, will make it possible to evaluate Manning's n for a great number of plants; the proposed model, therefore, represents a breakthrough in modeling.

Another aspect of this research, within the framework of the CANOPé program, was to do 3D SPH simulations based on *in-situ* measurements collected from three coastal zones with seagrass meadows. This reflects another fundamental objective of the present feasibility study of 3D sea-plant/wave interaction simulations: applications in environmental engineering. Indeed, one future option in the context of coastal protection is to use seagrass meadows as protective barriers, either by extending existing seagrass beds or by creating artificial ones. In this case, large-scale digital studies can supplement *in-situ* studies. Channel experiments cannot be set up with real, often protected, marine plants; therefore, the authors hope that the virtual FSI channel will contribute to solving these environmental objectives. Finally, a layer of sediment (Ghaïtanellis et al., 2018) or gravel (Kazemi et al., 2020) could also be taken into account, which, when added to the plant/wave interaction we simulated in our study, would be able to numerically reproduce a complete coastal system.

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APPENDIX A:

The integral representation of an arbitrary function, F(r), is defined as follows:

$$F(r) = \int_{V} F(r') \,\delta(r - r') \mathrm{d}r' \tag{A.1}$$

where $\delta(r-r')$ represents the Dirac delta function, V is the area of integration (in two dimensions) and r is the position vector. For the SPH method an approximation to the

$$F(r) \approx \int_{V} F(r') W(r - r', h) dr'$$
(A.2)

where h is the smoothing length defined by the area of influence of the smoothing function (or weight function), W.

In the SPH method, the system is represented by a number of particles, each carrying a mass and other fluid parameters and defined in a specific area of space. The particle approximation is used to discretize the domain. If F(r') is known in N discrete points $r_1, r_2, ..., r_N$, then it can be approximated by:

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$$F_h(r) = \sum_{b=1}^{N} \frac{m_b}{\rho_b} F_b W(r - r', h)$$
 (A.3)

where *b* corresponds to a particle neighbor. The kernel function must satisfy (Benz, 1990; Liu, 2002; Monaghan, 1992):

$$\begin{cases} \int_{V} W(r-r',h) dr' = 1\\ \lim_{h \to 0} W(r-r',h) = \delta(r-r')\\ W(r-r',h) = 0 \ \overrightarrow{\text{when}} \ |r-r'| > \kappa h \end{cases}$$
(A.4)

The first condition is the *normalization condition*, the second is the *Delta function* property (observed when the smoothing length approaches zero), and the third is the compact condition where κ is a constant related to the length of smoothing and which defines the effective area of the smoothing function.

The stability of the SPH algorithm depends heavily on the second derivative of the kernel (Morris et al., 1997). There are different kernel functions, the most well-known being a Gaussian function. In the current study, the Wendland function primarily was used:

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$$W(r,h) = \frac{\Psi}{h^{\nu}} \begin{cases} 1 - 1.5s^{2} + 0.75s^{3} & 0 \le s \le 1\\ 0.25(2 - s^{3}) & 1 \le s \le 2\\ 0 & 2 \le s \end{cases}$$
 (A.5)

where $s = \frac{|r|}{h}$, and Ψ is the normalization constant, equal to $10/7 \pi$ for two dimensions.

The accuracy of the SPH method can be improved by increasing the number of neighbors considered, which also significantly increases computational time. In the case of 2D modeling, a number of neighboring particles between 20 and 30 is a good compromise between accuracy and computational time; this number needs to be increased to 80 for 3D modeling.

There are two forms used to calculate a gradient of a scalar *F* in SPH. One is mainly used when the particle density varies only slightly:

$$\frac{1}{\rho_a} \nabla_a F = \sum_b m_b \left(\frac{F_a}{\rho_a^2} + \frac{F_b}{\rho_b^2} \right) \nabla_a W_{ab} \tag{A.6}$$

where ∇_a represents the gradient of the particle and $\nabla_a W_{ab}$ is the gradient of the kernel function $W(|r_a-r_b,h|)$ with r_a the coordinates of the particle a. The second form is as follows:

$$\nabla_a W_{ab} = \nabla_a W(r_a - r_b, h) = \frac{\mathrm{dW}}{\mathrm{dr}_{ab}} \frac{1}{|r_{ab}|} \left(x_a^{\alpha} - x_b^{\beta} \right) \quad (A.7)$$

640	with $F_{ab} = F_a - F_b$, $r_{ab} = r_a - r_b$. More details about these two forms used to calculate
641	a gradient can be found in GPUSPH Theory Guide, version 5.0 (2016),
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916	Fig. 2. Comparison between the 3D numerical simulation and the theoretical vertical
917	displacement of the free end of the plate (case for a sinusoidal oscillation).

Fig. 3. Comparison between numerically simulated (sim) and experimental (exp) displacements (a) vertical, and (b) horizontal) of the free end of the plate: Antoci et al.'s (2007) experiment (black dots) and the current simulations with the Lennard-Jones (Monaghan, 2005) formulation (LJ, stars) and with the Kajtar and Monaghan (2008) formulation (MK, black squares), with an initial spacing between elastic particles 10 times smaller than the fluid particles with interface particles in the Kajtar and Monaghan (2008) formulation (MK10, shown as a solid line). Fig. 4. Seagrass meadow of *Posidonia Oceanica* (left) and (right) schematic SPH discretization of water (blue) and seagrass meadow (green).

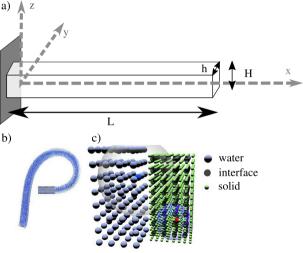
Fig. 5. Movement of three plants under a breaking wave using FSI simulated with SPH.

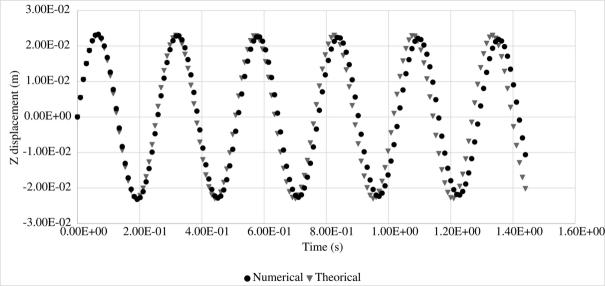
Fig. 6. Comparison between the experiment with artificial plants made of foam in Luhar and Nepf (2011) and the current simulation following the third FSI SPH approach presented in the study: a) set up of the experiment, b) simulation, and c) experiment with current velocity U=0.16 m/s, d) simulation, and e) experiment with current velocity U=0.32 m/s

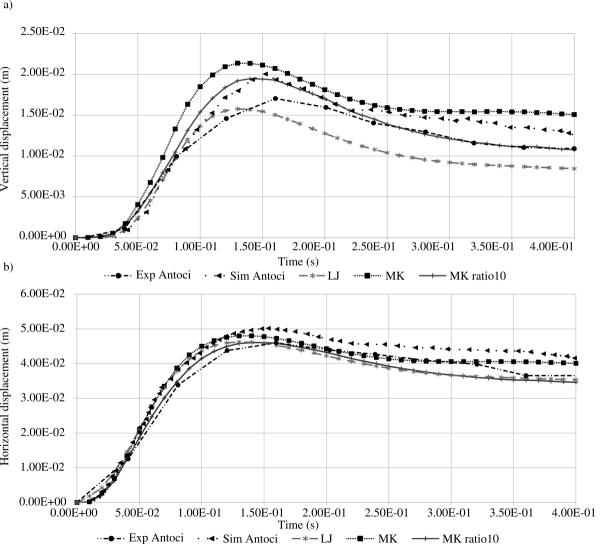
Fig. 7. Wave height relative to the reference water level in the channel ($h_0 = 0.2$ m) as a function of the cross-shore position for an h_0/h_{plant} ratio equal to 0.5. The vertical green line shows the plant's position at x = 1.2 m (one plant). In the simulation with three plants, they were located at x = 1, x = 1.2, and x = 1.4 m.

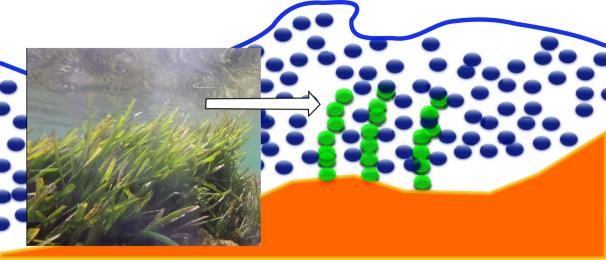
Fig. 8. Movement of the sea grass meadow under waves at different depths: a) with one 942 meadow of 200 plants, and b) with two separate meadows. 943 944 **Fig. 9.** GPUSPH 3D simulation of the real bathymetry in the Berre Lagoon: a), b) and c) 945 946 at 3 different times under one and the same wave, and d) simulated wave height with and without plants using real off-shore wave height, beach bathymetry, and seagrass 947 948 zones 949 Fig. 10. Flume experiment done at the River Dynamics Laboratory at Simon Fraser 950 University, courtesy C. Le Bouteiller a) sketch of the experimental set up (figure from 951 952 Le Bouteiller & Venditti, 2015) where Q_w is the water discharge, Q_s is the sediment transport rate, SMS is the Swath Mapping System, ADV is the Acoustic Doppler 953 954 Velocimetry, and SS sampling is the Seatek bed Sensors, b) photograph of the blades 955 during the experiment, c) close up of the numerical blades, and d) numerical flume at 956 the same dimensions as in the experiment, i.e. with a 4-m-long meadow

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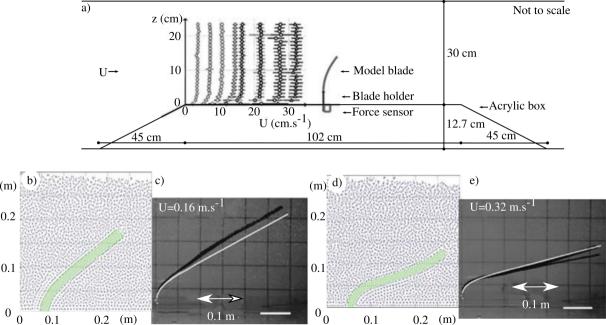


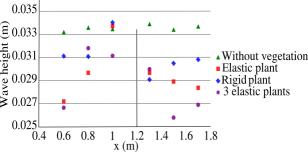


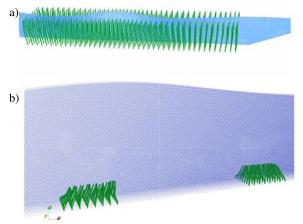


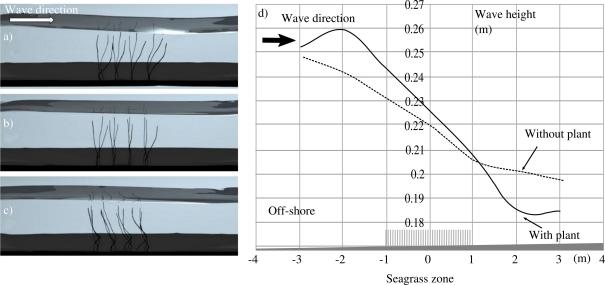












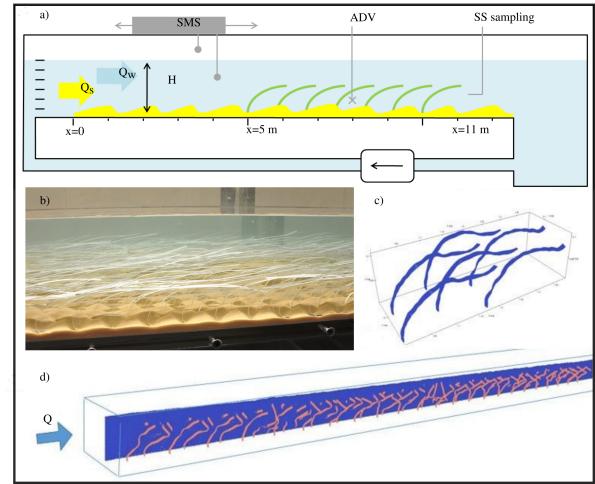


 Table 1. Dimensions in the experiment.

Dimensions in the experiment		
A(m)	0.1	
H(m)	0.14	
B(m)	0.1	
B*(m)	0.098	
L(m)	0.079	
s(m)	0.005	

Table 2. Fluid and solid properties and elastic modulus used

Fluid properties		
Density, $\rho(kg/m^3)$	1,000	
Compressibility modulus, ε (N/m ²)	$2x10^{6}$	
Solid properties		
Density, ρ_s (kg/m ³)	1,100	
Young's modulus, E (N/m²)	$1.2x10^{7}$	
Shear's modulus, μ (N/m²)	4.27×10^6	
Bulk modulus, K (N/m ²)	$2x10^7$	

Table 3. Properties of real *Posidonia Oceanica* and modified properties used for numerical simulations, from (Folkard, 2005)

Property	Real Plant	Simulated Plant
Density, ρ_s (kg/m ³)	$910 \pm 110 \text{ kg/m}^3$	910 kg/m ³
Young's modulus, E (N/m²)	$4.7 \pm 0.6 \times 10^8 \text{ N/m}^2$	$2.1 \times 10^4 \text{ N/m}^2$
Thickness (mm)	0.1 mm	15 mm
Bulk modulus, K (N/m ²)	$3.92 \pm 0.6 \times 10^8 \text{ N/m}^2$	$1.7 \times 10^4 \text{ N/m}^2$
Shear's modulus, μ (N/m²)	$1.81 \pm 0.5 \times 10^8 \text{ N/m}^2$	$8.7 \times 10^3 \text{ N/m}^2$