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Article

Effect of scour on the natural frequency responses of bridge piers: development of a scour depth sensor

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Abstract: Local scour is the removal of soil around bridge foundations under the erosive action of flowing water. This hydraulic risk has raised awareness to the need of developing continuous monitoring techniques to estimate scour depth around bridge piers and abutments. One of the emerging techniques is based on monitoring the vibration frequency of either bridge piers or a driven sensor in the riverbed. The sensor proposed in this study falls into the second category. Some unresolved issues are investigated: the effect of the geometry and material of the sensor, the effect of the embedded length and the effect of soil type. To this end, extensive laboratory tests are performed using rods of different materials, with various geometries and lengths. These tests are conducted in both dry sand and a soft clayey soil. Since the sensor will be placed in the riverbed, it is crucial to evaluate the effect of immersed conditions on its response. A numerical 3D finite-element model was developed and compared against experimental data. This model was then used to compute the 'wet' frequencies of the sensor. Finally, based on both the experimental and numerical results, an equivalent cantilever model is proposed to correlate the variation of the frequency of the sensor to the scour depth.

Keywords: Bridge scour; Vibration; Soil-structure interaction; Equivalent cantilever; Monitoring.

1. Introduction

Scour is considered as the main cause of bridge damages [1] and accounts for nearly half of all bridge collapses in the USA [2]. In France, the collapses of the Wilson Bridge in Tours (1978) and the St Louis Bridge on Reunion Island (2007) serve as national examples of damages caused by scour [3]. In order to anticipate this risk, it is important to measure the current scour depth at bridge supports, namely the piers and abutments. On one hand, many empirical formulas are proposed in literature [4–7]. However, most of them usually lead to an overestimation of its value [5] due to different factors including: scale effect since most of the equations are derived from flume test results, the simplifying hypothesis assumed for both bed material and flow and the difficulty of accurately measuring field data [8]. On the other hand, several monitoring devices already exist and are used in the field such as: float-out devices [9], radar [10,11], sonar [12], time domain reflectometry [13,14], magnetic sliding collar [15,16], electrical conductivity devices [17] and fiber optic [18,19]. However, those methods have several limitations such as: high sensitivity to noise, difficulties in result interpretations and not being suitable to high sediment concentration conditions. Therefore, recent studies attempt to suggest more accurate and practical monitoring techniques to evaluate scour at bridge foundations. An emergent technique based on the dynamic response of the structure is the main method proposed in this paper.

The principal of this monitoring technique is that scour causes an increase of the exposed length of the scoured structure. Consequently, based on the inverse relation between the fundamental frequency

34 and the length of a cantilever beam, a decrease of the frequency can be correlated to an increase of
35 scour depth [19,20]. Based on this result, two applications are generally proposed.

36 Zarafshan et al. [19] proposed to monitor bridge scour by means of rods embedded in the riverbed.
37 Each rod is equipped with a fiber-optic Bragg grating sensor that uses the strain response history in
38 the time domain to identify the fundamental frequency. In order to correlate the first frequency of the
39 sensor to scour depth, a numerical model was developed based on the Winkler model of the soil. Once
40 the rod is placed in the soil, its frequency is used to calculate the stiffness of the springs k used in the
41 model. Then the model can be used to measure the first frequency for different scour depths.

42 Prendergast et al. [20] proposed a direct approach, the effect of scour on the first frequency of
43 the pile itself was studied. The experimental laboratory set-up consisted on a pile placed in a block
44 of sand. Scour was simulated with the progressive extraction of a layer of the soil. For every scour
45 depth, an impact was applied and the dynamic response of the pile recorded with an accelerometer
46 placed on the top. The test showed that the first natural frequency decreases with the increase of the
47 depth of the scour hole. The same experimental protocol was applied in situ to a 8.76 m in length pile
48 and showed the same results. To establish a relation between the first frequency and the scour depth,
49 a spring-beam finite element model was developed and validated. Unlike Zarafshan et al. [19] who
50 used the vibration response of the sensor to determine the stiffness of the springs k , Prendergast et al.
51 [20] used two geotechnical methods: the first one uses the small strain shear modulus G_o determined
52 with in-situ test with Multi-channel analysis of surface waves (MASW) or Cone Penetration Test (CPT)
53 [21] and the second one uses the American Petroleum Institute design code (API).

54 Both studies show that the first frequency of piles or sensors decreases with the increase of scour
55 depth. However, the correlation between frequency and scour depth is not direct and requires the
56 use of both a numerical model and experimental data to calibrate the spring stiffness. The present
57 study focuses on the effect of scour on the dynamic response of sensor-rods partially embedded in soil,
58 specifically on the correlation between the variation of the first frequency and the current scour depth.
59 Some unsolved issues are also addressed such as: the effect of the sensor geometry and material, the
60 effect of soil type and the effect of the embedded length.

61 The paper starts in Section 2 with the description of the laboratory tests performed to assess the
62 effect of scour on the first frequency of different rods in two type of soils. The repeatability of the
63 measurement is evaluated and three important aspects are investigated: the sensitivity to the sensor
64 material and geometry, the sensitivity to the embedded length and the effect of the soil. Then, in
65 Section 3, a 3D numerical model is developed and validated. This model is then used to assess the
66 effect of immersed conditions on the response of the sensor. In Section 4, the main results of this study
67 are outlined and a simple method is proposed to correlate scour depth to the first frequency of an
68 equivalent cantilever. Finally, in Section 5, conclusions and future use of the findings of this study are
69 outlined.

70 2. Experimental program

71 The experimental study conducted in the laboratory aims to investigate the feasibility of
72 monitoring scour with rod-sensors. To this end, extensive tests are performed and the following
73 issues are addressed: the effect of scour, the repeatability of the measurement, the effect of the sensor
74 geometry and material, the effect of the embedded length and the effect of soil type.

75 2.1. Materials and cross-sections of the rod-sensor

76 In order to assess the effect of scour on the first frequency of the sensor, extensive laboratory tests
77 are performed. Five rods having various geometries, lengths and material properties are tested: two
78 circular aluminium rods of 800 mm and 600 mm length named CA-80 and CA-60 respectively, two
79 rectangular aluminium rods of 800 mm and 600 mm length named RA-80 and RA-60 respectively and
80 a circular PVC rod of 800 mm named CP-80. The geometrical and mechanical properties of each rod

81 are summarized in Table 1. The tests are conducted in two type of soils: dry sand and a soft clayey soil.
 82 The experimental process in each soil is detailed in section 2.2.

Table 1. Geometric and mechanical characteristics of the tested rods

Tested rods	Outer diameter/ Width(mm)	Thickness (mm)	Young modulus (GPa)	Bulk density (kg/m ³)	Flexural rigidity (N.m ²)
CA-80, CA-60	12	1	59	2700	31.1
RA-80, RA-60	19	2	59	2700	0.8
CP-80	20	2	3.5	1425	11.0

83 2.2. Experimental procedures

84 2.2.1. Sandy soil

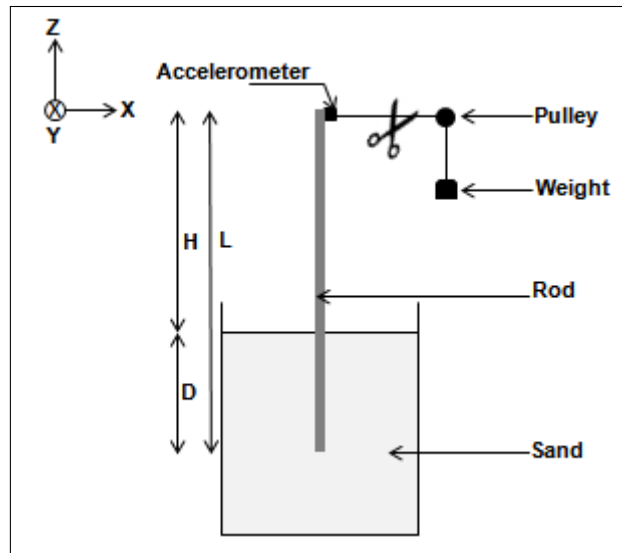


Figure 1. Laboratory set-up in dry sand, where the tank at the bottom has a volume of 1 m x 1 m x 1 m and can be filled up to 0.7m by sand.

The experimental set-up is presented in Figure 1. A tank of 1 m x 1 m x 1 m in width, depth and height respectively is progressively filled with dry sand of Seine until it reaches a height of 0.7 m. Since the Young modulus of the soil E_s is one of the most influencing parameters on the natural frequency of the soil-rod system [22], its value is measured and is used afterward in the numerical model. For this purpose, a mini-pressurimeter test [23] is conducted to determine the average value of the Ménard modulus E_m . The Young modulus E_s is then calculated using Equation (1) [24]:

$$E_s = \frac{E_m}{\alpha}, \quad (1)$$

85 where α a rheological parameter ($\alpha=1/3$ for sand). The properties of the dry sand used in this study
 86 are summarized in Table 2.

Table 2. Dry sand properties

D_{50} (mm)	ρ_s (Kg/m ³)	ν_s	E_m (MPa)	E_s (MPa)
0.7	1700	0.3	0.5	1.5

87 The sensor is then placed in the soil volume. A thread connects the top of the rod, through a pulley,
 88 to a known dead weight. To generate an impulse force of a known amplitude, ensuring similar testing
 89 conditions, the thread is cut inducing the vibration of the sensor in the X direction. An accelerometer
 90 having a mass of $m = 41$ g, is placed on the top of the rod since it has been proved to be the optimal
 91 location [25]. The accelerometer records the transient dynamic response of the sensor corresponding to
 92 its first bending mode of vibration.

93 The scour process is taken into account by the progressive increase of the exposed length H of
 94 the sensor. Table 3 summarizes the range of the exposed length H for each rod. This length is limited
 95 on the one hand by the tank dimensions, and on the other hand by the stability of the rod-sensor.
 96 The scour depth is increased step by step with increments of 50 mm. For each exposed length H ,
 97 the impulse force is applied and the vibratory response of the rod is recorded. The data samples are
 98 recorded with a sampling frequency of 512 Hz. The transient response of the system is then post
 99 processed using SCILAB to measure the first natural frequency from the Fast Fourier Transform (FFT).
 To evaluate the accuracy of the measurement, each test is repeated three times.

Table 3. Range of exposed length H of the rods in dry sand

Tested rods	Min H (cm)	Max H (cm)
CA-80	35.0	65.0
CA-60	15.0	45.0
RA-80	35.0	70.0
RA-60	15.0	50.0
CP-80	35.0	70.0

100

101 2.2.2. Clayey soil

102 A soft saturated clayey soil mixture is prepared with of 50% sand of Fontainebleau, 50% of
 103 Armoricaine Kaolinite clay and 25% water [26,27]. A Plexiglas cylinder of 400 mm diameter and 400
 104 mm height is progressively filled with the mixture. To ensure a uniform density, the soil specimen is
 105 manually compacted into five layers of equal thickness. The soil mixture is matured during 48 hours.
 106 The experimental protocol used in sand is adapted to the clayey soil. Due to the high plasticity of the
 107 soil mixture (Figure 2), an impact is applied to generate the impulse in order to avoid the deformation
 108 of the soil induced by the previous protocol before the beginning of the testing.

109 The aging of clayey soils often improves their mechanical properties [28] which can induce a
 110 variation of the rods frequencies. To make sure that the changes of the frequency are caused only by
 111 scour, vibration tests of the rod RA-60 are conducted after three days, ten days and forty-five days.
 112 Table 4 summarizes the range of the exposed length H for each rod in the clayey soil.

113 It should be noted that tests of the circular aluminum rods CA-80 and CA-60 showed no vibratory
 114 response due to their high flexural rigidity (see Table 1). In fact, if the structure is more rigid than the
 115 soil, the response of the structure is restricted to rigid body modes [29]. Consequently, no results are
 116 available for the rods CA-80 and CA-60 in the clayey soil.

Table 4. Range of exposed length H of the rods in clayey soil

Tested rods	Min H (cm)	Max H (cm)
RA-80	40.0	60.0
RA-60	20.0	50.0
CP-80	40.0	65.0

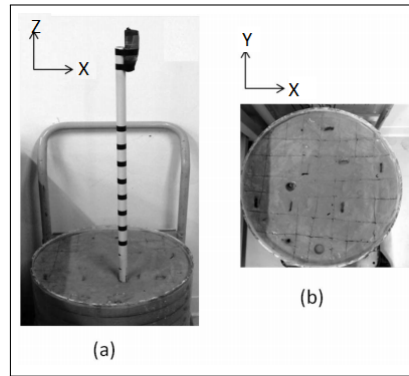


Figure 2. Photographs of (a) laboratory setup in clayey soil; (b) plastic deformation of the soil

117 3. Numerical model

118 A numerical model is created to validate and understand the experimental results.

119 Moreover, in practice, the sensor will be placed in the riverbed and immersed in water. Therefore,
120 it is crucial to assess the effect of water on the response of the sensor. To this end, a finite element model
121 is developed.

122 3.1. Theoretical formulation

The evolution of multiple-degree-of-freedom system is expressed by the following equation:

$$[\mathbf{M}]\{\ddot{\mathbf{u}}\} + [\mathbf{C}]\{\dot{\mathbf{u}}\} + [\mathbf{K}]\{\mathbf{u}\} = \{\mathbf{F}\} \quad (2)$$

123 where $[\mathbf{M}]$, $[\mathbf{C}]$ and $[\mathbf{K}]$ are respectively the mass, the damping and the stiffness matrices; $\{\ddot{\mathbf{u}}\}$, $\{\dot{\mathbf{u}}\}$ and
124 $\{\mathbf{u}\}$ are respectively the acceleration, the velocity and the displacement and $\{\mathbf{F}\}$ the external vector
125 force applied to the system. The dimension of the matrices is $N \times N$, where N is the number of degrees
126 of freedom of the system.

In the absence of damping, the free vibrations of the structure are described with the eigenvalue problem :

$$[\mathbf{M}]\{\ddot{\mathbf{u}}\} + [\mathbf{K}]\{\mathbf{u}\} = 0 \quad (3)$$

The solution of Equation (3) can be written $\{\mathbf{u}\} = \{\mathbf{U}\} \exp^{i\omega t}$, which leads to :

$$(-[\mathbf{M}]\omega^2 + [\mathbf{K}])\{\mathbf{U}\} = (-[\mathbf{M}]\lambda + [\mathbf{K}])\{\mathbf{U}\} = 0 \quad (4)$$

with $\lambda = \omega^2 = (2\pi f)^2$, f the natural frequency and $\{\mathbf{U}\}$ the mode shape. This linear system has N
non trivial solutions $(U_i, \lambda_i)[i = 1, 2, 3, \dots, N]$ [30] that verify the theoretical condition:

$$\det([\mathbf{K}] - \lambda_i[\mathbf{M}]) = 0 \quad (5)$$

127 Since only the first frequency is needed, a subspace iteration method [31] is used to solve the system.

128 3.2. Model description

129 A 3D finite elements model is developed using the finite element software Code-Aster [32]. The
130 proposed model is based on the following hypothesis: 1) the soil medium and the rod-sensor are elastic,
131 2) all displacements and strains remains small and 3) the soil and the sensor are perfectly bounded at
132 the interface. For the boundary conditions, the lateral faces of the soil are fixed against displacement in
133 the normal direction and the base is fixed against displacement in all directions. The weight of the
134 accelerometer is not negligible and is modeled as a nodal mass placed at the top of the rod-sensor. In
135 this model, the soil and the sensor are meshed with 10 nodes tetrahedron elements. The mesh was

136 refined near the sensor with a progressive transition to a coarser mesh away from the sensor. The
 137 average number of mesh nodes was fixed to 80000 after conducting a mesh convergence analysis for
 138 each tested rod. Figure 3a shows the three-dimensional numerical model of the rod-soil system.

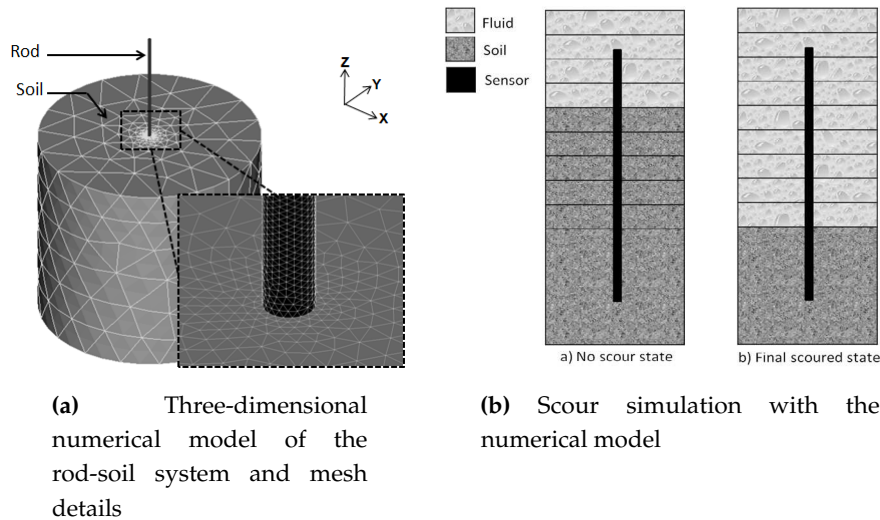


Figure 3. Finite elements scour model

139 In order to simulate scour process in immersed conditions, the numerical model is partitioned to
 140 several layers of 50 mm thickness. The initial scour state is presented in Figure 3b. As scour increases,
 141 the soil layers are progressively replaced by fluid layers, mimicking the natural phenomenon. This
 142 substitution is achieved by modifying the material properties of the given layer. This approach is
 143 therefore only valid if water does not change the general behavior of the rod compared to the case
 144 without water [33].

145 The material properties used in the model are those of dry sand and the rods presented in Tables
 146 1 and 2. No readjustments of the parameters is performed afterwards.

147 First, the model is used to compute the dry frequencies of all tested rods (without the fluid). The
 148 first numerical frequency corresponding to the bending mode of the rods is compared to experimental
 149 data to validate the model for each exposed length.

150 Then, in order to assess the effect of the immersed condition on the sensor response, the wet
 151 frequencies (with the fluid) of the circular aluminum rods (CA-60 and CA-80) are computed following
 152 the procedure described previously as shown in Figure ??.

153 4. Results and discussion

154 4.1. Experimental results

155 4.1.1. Effect of soil aging

156 The results of the vibration tests of the rod RA-60 at different dates are presented in Figure 4.
 157 There is no clear tendency of the evolution of the frequency with the aging of the clayey soil
 158 mixture. Consequently, the variation of the frequency, during the testing period, is not a result
 159 of the improvement of the mechanical characteristics of the soil.

160 4.1.2. Repeatability analysis

161 The accuracy of the sensor is evaluated in dry sand for the rods CA-80, RA-80 and CP-80. The
 162 three measured frequencies for the minimum exposed length $H = 35$ cm and the maximum exposed

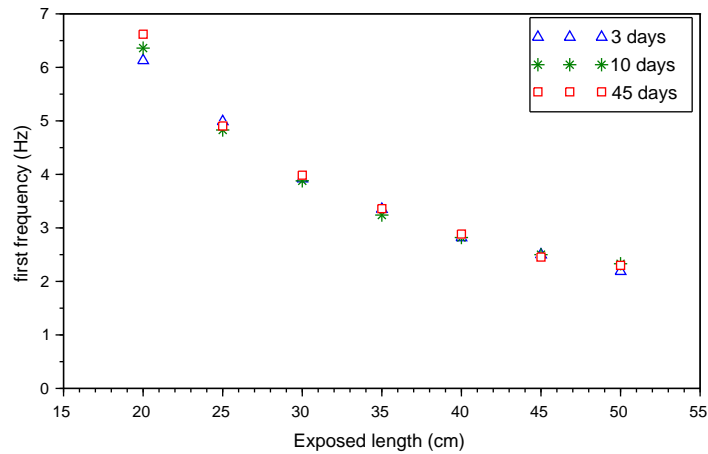


Figure 4. Variation of first frequency with exposed length H in the clayey soil after 3, 10 and 45 resting days

length $H = 65$ cm are summarized in Table 5. The results indicates that the standard deviation, for the three tested rods, at each exposed length is less than 0.50 Hz. This demonstrates the accuracy of the sensor measurement.

Table 5. Repeatability analysis in dry sand

Exposed length	CA-80		RA-80		CP-80	
	Measured Freq.(Hz)	Average Freq. (Hz)	Measured Freq.(Hz)	Average Freq. (Hz)	Measured Freq.(Hz)	Average Freq. (Hz)
h=65 cm	9.52	9.64 ± 0.30	1.55	1.55 ± 0.01	6.58	6.51 ± 0.07
	9.98		6.50			
	9.42		6.45			
h=35 cm	25.33	25.71 ± 0.50	4.21	4.14 ± 0.06	16.94	16.58 ± 0.38
	26.28		16.18			
	25.53		16.61			

165

166 4.1.3. Effect of scour

Figure 5 displays the experimental results in dry sand. The first frequency of the sensors decreases with the increase of the exposed length H . This trend is in full agreement with the results of [19,20]. As shown in Figure 6, a similar tendency is noticed in the clayey soil.

Table 6 shows the frequency of the rods in both soils for the exposed lengths $H=60$ cm and $H=40$ cm, representing 20 cm scour. To compare the sensitivity of the tested rods to scour, a frequency change rate p is defined with Equation (6):

$$p = \frac{f_{(H=40)} - f_{(H=60)}}{f_{(H=40)}} \quad (6)$$

It can be seen that the frequency change rate p increases with the flexural rigidity of the tested rod in both sand and clayey soil. The sensitivity of the rods is also affected by the soil. For instance, the frequency of the rod CA-80 varies by 46% in sand and 39% in soft clayey soil for the same scour depth.

Table 6. Sensitivity of the frequencies of the tested rods to scour in sand and clayey soil

Tested rods	Flexural rigidity $N.m^2$	Frequencies in sand			Frequencies in clayey soil		
		H=60 cm	H=40 cm	Change rate p (%)	H=60 cm	H=40 cm	Change rate p (%)
CA-80	31.1	11.31	19.50	42	-	-	-
CP-80	11.0	7.50	13.90	46	6.6	10.78	39
RA-80	0.8	1.78	3.45	48	1.53	2.8	45

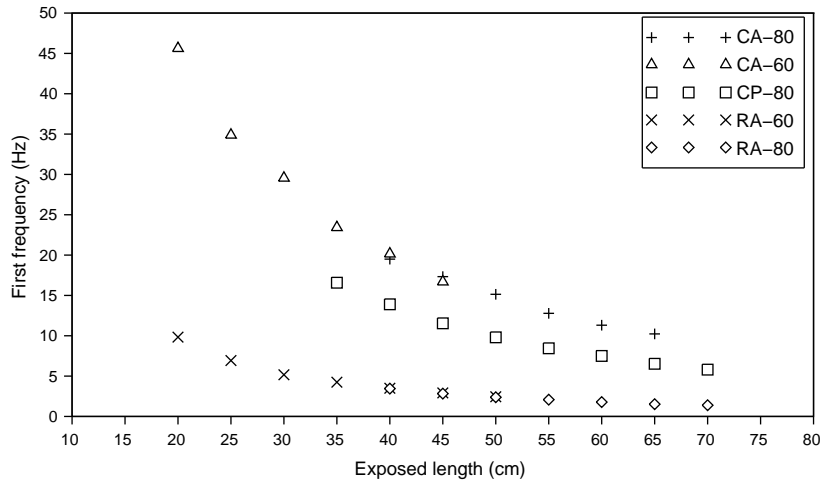


Figure 5. Variation of first frequency with exposed length in sandy soil

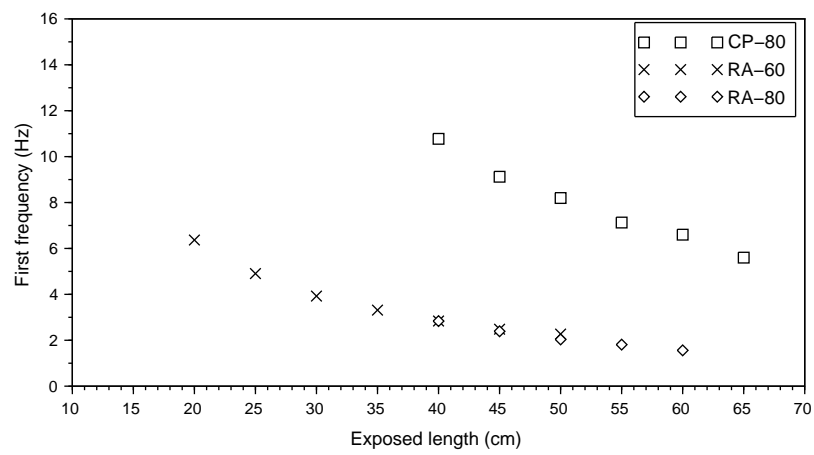


Figure 6. Variation of first frequency with exposed length in clayey soil

173 4.1.4. Effect of the embedded length

174 The effect of the embedded length D is investigated. Figures 5 and 6 show that the rods CA-80
 175 and CA-60 have the same frequency when their exposed length H is equal, even if their embedded
 176 length D are different. Similar results are observed for the rods RA-80 and RA-60 in both soil types.
 177 This means that the frequency is more influenced by the exposed length H rather than the embedded
 178 length D in our experimental conditions.

179 4.1.5. The effect of soil type

180 In order to highlight the effect of the soil type on the frequency of the sensor, the variation of the
 181 first frequency with the embedded ratio of the rods RA-80, RA-60 and CP-80 is shown in Figure 7 for
 182 sand and clayey soil.

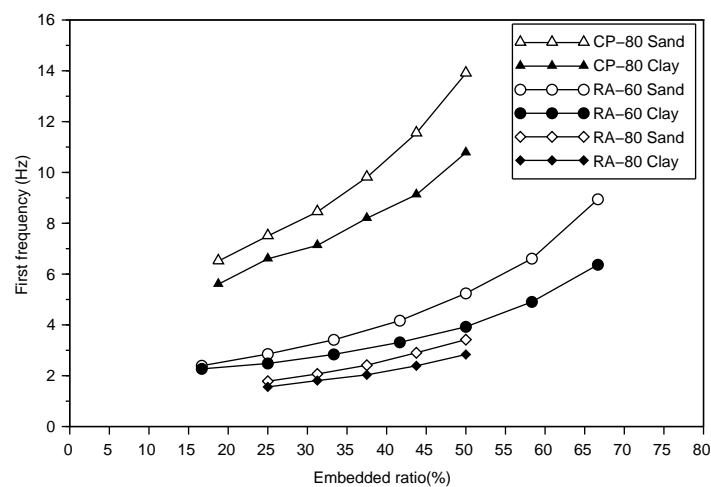


Figure 7. Variation of first frequency with embedded ratio in sand and clayey soil

183 The results show that the frequencies in sand are higher than the frequencies in the clayey soil.
 184 This is mainly due to the higher stiffness of sand compared to the lower stiffness of the clayey soil
 185 with high plasticity used in this study. For low embedded ratios, the first frequency of the rods in both
 186 soils is almost similar. But as the embedded ratio increases, the gap between the frequency curves in
 187 sand and in clayey soil increases. This implies that for high embedded ratio, the frequency of the rod
 188 is significantly influenced by the stiffness of the soil it is embedded in.

189 4.2. Numerical results

190 4.2.1. Model validation

191 The numerical frequencies of all tested rods are compared to the experimental frequencies in dry
 192 sand. Figure 8 shows that the numerical and experimental results are in good agreement without any
 193 readjustment of parameters.

194 4.2.2. The effect of water

195 The effect of water on the frequency of the sensor is investigated. Following the numerical
 196 procedure described in Section 3.2, the wet frequencies of the rods are calculated for each exposed
 197 length H . As shown in Table 7, water decreases the frequency of the sensor. As scour increases, the
 198 effect of water becomes more significant with changes from 4% for $H=25$ cm to 9% for $H=55$ cm.

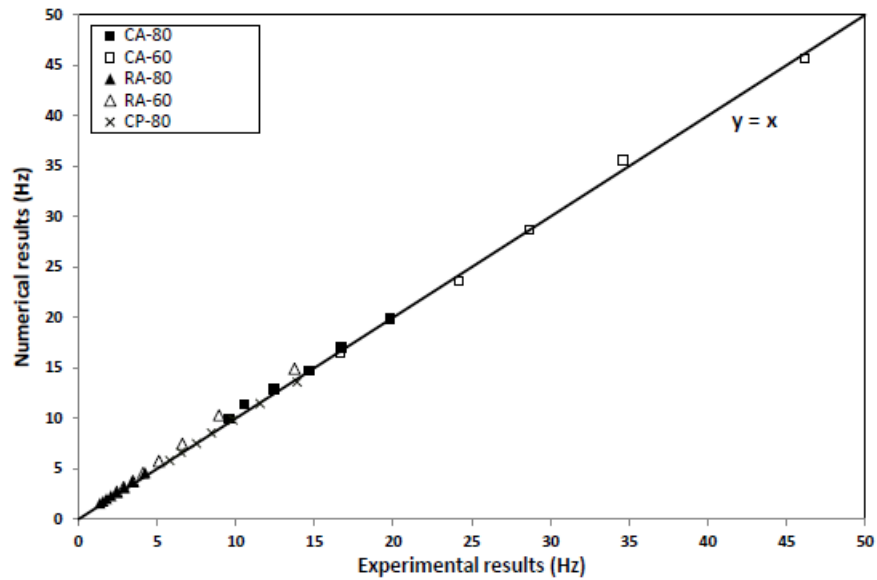


Figure 8. Comparison of experimental and numerical first frequencies of the tested rods (correlation coefficient of $R^2 = 0.9905$).

Table 7. Comparison between wet and dry frequencies of the CA rods

Exposed length (cm)	Dry frequency (Hz)	Wet Frequency (Hz)	Percentage change of the frequency between air and water (%)
55	12.8	11.6	9
50	14.6	13.3	9
45	16.8	15.5	8
40	19.7	18.3	7
35	23.4	21.9	6
30	28.3	26.8	5
25	35.2	33.6	4

199 4.3. Proposed calibration technique of the sensor

200 4.3.1. Equivalent cantilever beam

The variation of the experimental first frequencies of the tested rods, in sand and clayey soil, is compared to the response of a cantilever beam with a punctual mass attached at its free end to take into account the accelerometer. The free length of the cantilever is called H_c , the total mass of the cantilever M and the mass of the accelerometer m . The theoretical frequencies of the cantilever are calculated using Equation (7) [34,35] and are plotted with a continuous line in Figures 9, 10 and 11.

$$f_{th} = \frac{1}{2\pi} \times \sqrt{\frac{3EI}{H_c^3(0.24M + m)}} \quad (7)$$

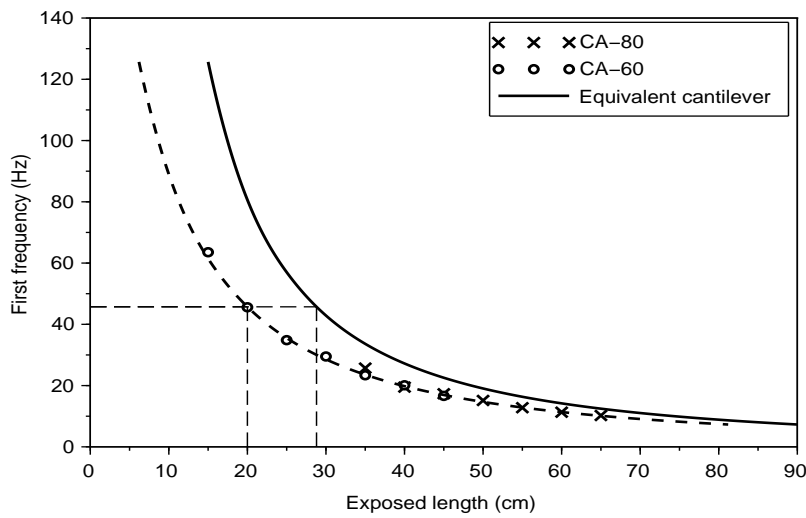


Figure 9. Equivalent cantilever of CA rod-sensors in sand: experimental first frequency for road-sensors CA-80 and CA-60, and first frequency of equivalent cantilever beam $f_{th} = \frac{1}{2\pi} \times \sqrt{\frac{3EI}{H_c^3(0.24M + m)}}$.

201

202 It appears that all tested rods and in both soil types, the theoretical results of the cantilever are
203 horizontally translated against the experimental results with a constant H' . This adjustment length H'
204 varies with the sensor and the soil characteristics.

The physical meaning of H' is related to the soil-rod interaction which does not correspond to a perfect cantilever, but may match this assumption from a distance H' under the soil surface. Therefore, for each value of the exposed length H , the first natural frequency of the rods in each soil is equal to the frequency of an equivalent cantilever with a free length $H_c = H + H'$ (Figure 12). For instance, to estimate the experimental first frequency of the circular rod in the sand with an exposed length $H = 20\text{cm}$, Equation (8) is used:

$$f_{exp}(H = 20) = f_{the}(H_c = 20 + 8.8 = 28.8) = 45.7 \text{ Hz} \quad (8)$$

The inverse relationship between the first frequency and the free length (9) derived from Equation (7) can be used to determine the scour depth y_s , see Equation (9):

$$H = \sqrt[3]{(2\pi f_{exp})^2 \times \frac{0.24M + m}{3EI}} - H' \Rightarrow y_s = H - H_0 \quad (9)$$

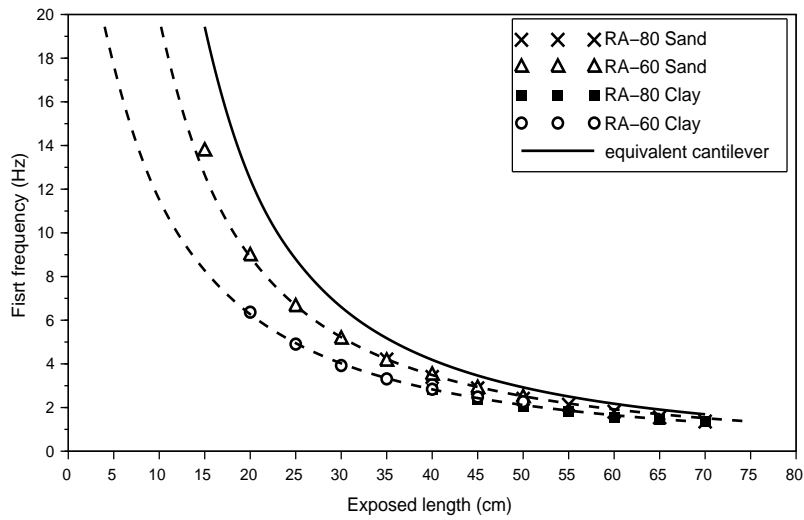


Figure 10. Equivalent cantilever of RA rod-sensors in sand and clayey soil: experimental first frequency for road-sensor RA-80 and RA-60 in both sand and clayey soil, and first frequency of equivalent cantilever beam $f_{th} = \frac{1}{2\pi} \times \sqrt{\frac{3EI}{H_c^3(0.24M + m)}}$.

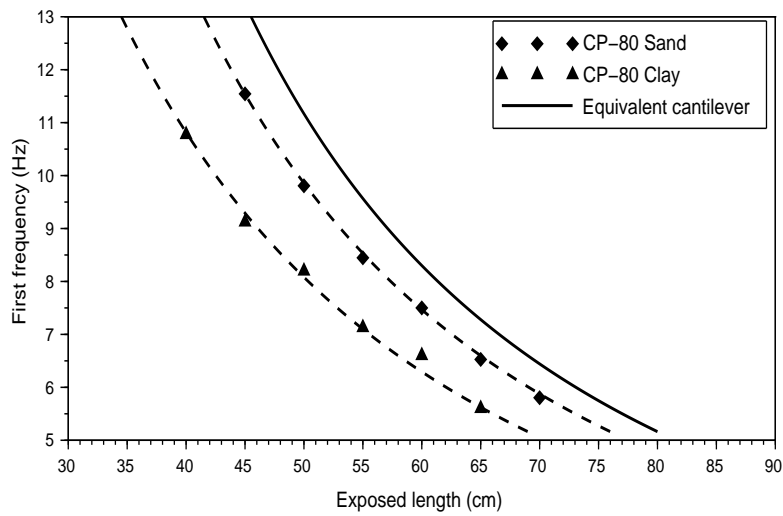


Figure 11. Equivalent cantilever of CP rod-sensor in sand and clayey soil: experimental first frequency for road-sensor CP-80 and RA-60 in both sand and clayey soil, and first frequency of cantilever beam $f_{th} = \frac{1}{2\pi} \times \sqrt{\frac{3EI}{H_c^3(0.24M + m)}}$.

Table 8. Value of the equivalent length of the tested rods in both soils

Tested rod	Corrected length in sand (cm)	Corrected length in clayey soil (cm)
CA-80, CA-60	8.8	No results
RA-80, RA-60	4	11
CP-80	4.6	11

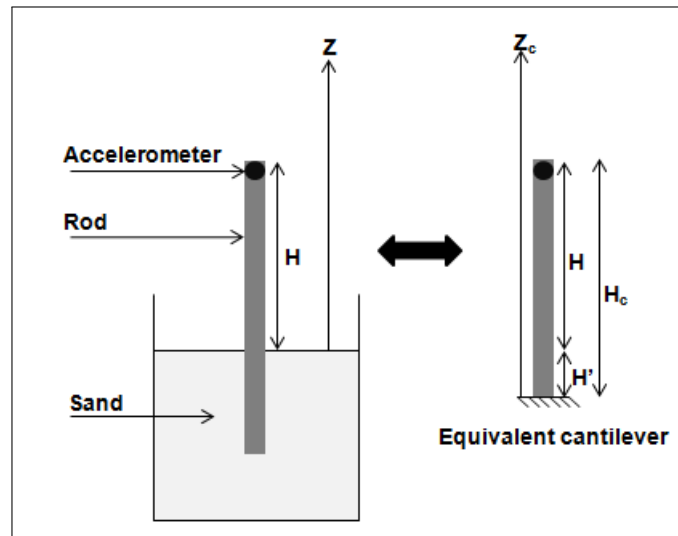


Figure 12. Equivalent cantilever of the dynamic soil structure interaction

205 The values of this adjustment length for all tested rods and soils are summarized in Table 8.

206 4.3.2. Wet frequencies and equivalent cantilever beam

In practice, the sensor will be placed in the riverbed and immersed in water. Therefore, it is crucial to verify the validity of the proposed calibration technique when the sensor is completely immersed. The calculated wet frequencies in Section 4.2.2 are first compared to the cantilever frequencies derived from Equation (7) but an adjustment length could not be derived. It seems more appropriate to compare the wet frequencies of the sensor to the wet frequencies of a cantilever derived from Equation (10):

$$f_{wet} = \frac{1}{2\pi} \times \sqrt{\frac{3EI}{H_c^3 [0.24(M + M_a) + m]}} \quad (10)$$

207 where M_a is the added mass of the fluid. Figure 13 shows that the wet frequencies of the sensor are
 208 indeed translated against the theoretical frequencies of the immersed cantilever. However, the value of
 the adjustment length in immersed conditions is $H_{wet} = 8$ cm against $H_{dry} = 8.8$ cm in dry condition.

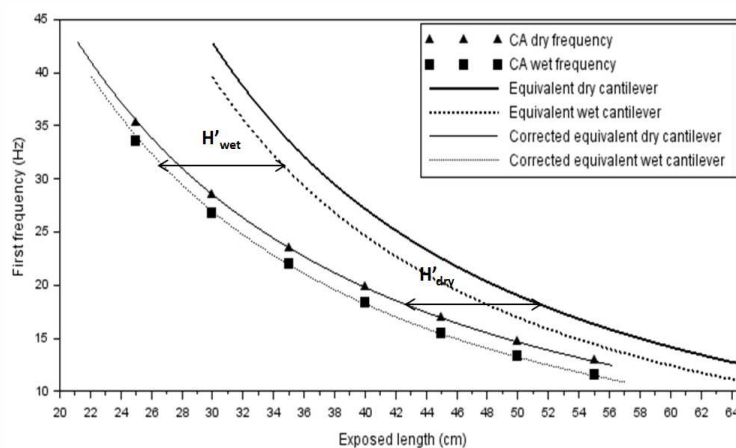


Figure 13. Equivalent cantilever of the circular rod in dry and wet conditions

210 4.4. Sensitivity study

211 The experimental tests show that the adjustment length H' vary with the geometry of the
 212 sensor and the soil type. In order to identify the parameters influencing this length, the numerical
 213 model detailed in Section 3b is used to perform a sensitivity study. The effect of three parameters is
 214 investigated: the Young modulus of the rod E_r , the bulk density of the rod ρ_r and the Young modulus
 215 of the soil E_s . The results of the numerical simulation which are presented here, have been performed
 216 using the rod CA-80.

217 4.4.1. The effect of the elasticity of the rod-sensor E_r

218 The frequencies of the rod are calculated for three values of E_r : 100 GPa, 59 GPa and 5.9 GPa. The
 219 results are then compared to the frequencies derived from Equation (7) of an equivalent cantilever
 220 with a similar Young modulus. The value of the adjustment length H' can then be estimated. Figure 14
 shows that the adjustment length H' increases with the increase of the Young modulus of the rod E_r .

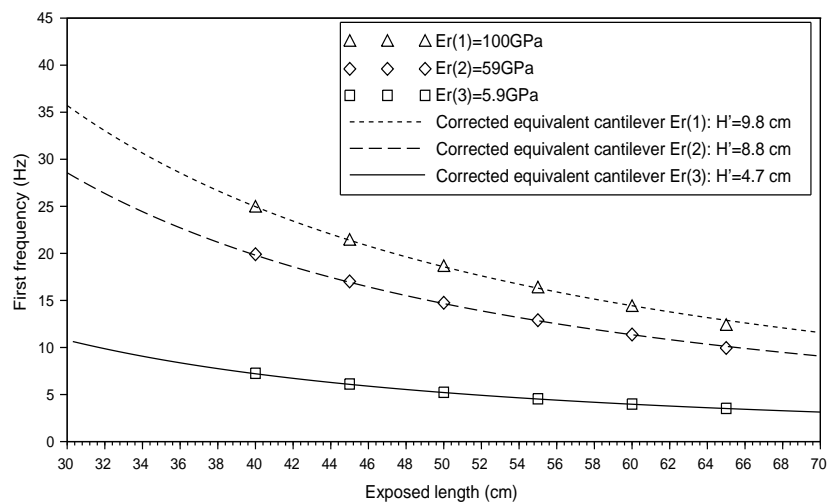


Figure 14. Variation of the adjustment length H' with the Young modulus of the rod E_r

221 Once again, the experimental frequencies and the first frequency of the equivalent beam are in
 222 good agreement.
 223

224 4.4.2. The effect of the density of the rod-sensor ρ_r

225 The frequencies of the rod are calculated for three values of ρ_r : 1300 kg.m⁻³, 2700 kg.m⁻³ and
 226 7500 kg.m⁻³. The results are then compared to the corresponding equivalent cantilever to estimate the
 227 value of the adjustment length H' . Figure 15 shows that the adjustment length H' is independent of
 228 the bulk density of the rod ρ_r .

229 4.4.3. The effect of the elasticity of the soil E_s

230 The frequencies of the rod are calculated for three values of E_s : 1.5 MPa, 15 MPa and 150 MPa.
 231 The results are then compared to the equivalent cantilever to estimate the value of the adjustment
 232 length H' . Figure 16 provides the value of H' for each value of E_s .

233 As it can be seen, H' decreases from 8.8 cm to 2.4 cm when the stiffness of the soil varies between
 234 1.5 MPa and 150 MPa. This result was predictable since the equivalent cantilever is the specific case
 235 where the soil has an infinite stiffness. In that case, the rod is completely fixed at the ground surface
 236 and $H' = 0$. As the stiffness of the soil decreases, the rod should be embedded deeply to insure a
 237 similar constraint as the clamped condition.

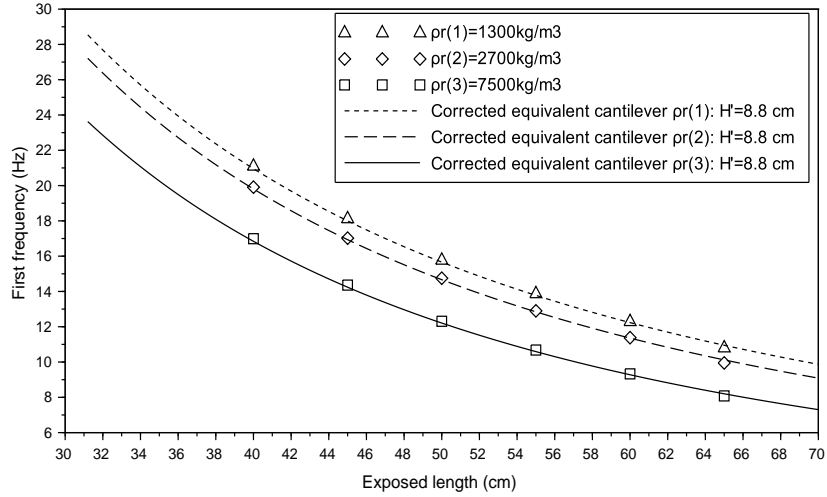


Figure 15. Variation of the adjustment length H' with the bulk density of the rod ρ_r

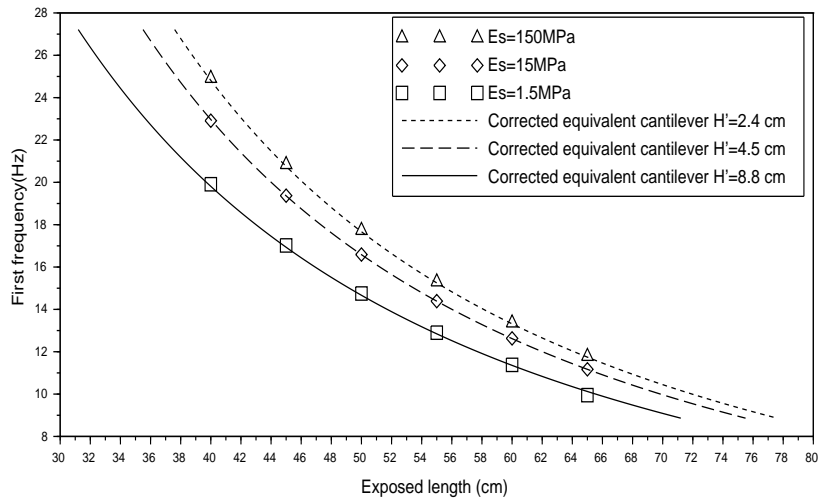


Figure 16. Variation of the adjustment length H' with the Young modulus of the soil E_s

238 The results of the sensitivity study have a practical interest. In fact, in the field, the only variable
239 input is the sensor, which means its geometry and its material characteristics. It would be wise to try
240 reducing the adjustment length H' of the sensor so that the cantilever model can be used to predict
241 the frequencies of the sensor with reasonable accuracy. The results show that this can be achieved by
242 varying the geometry and decreasing the Young modulus of the sensor.

243 4.5. General discussion about the findings

244 Zarafshan et al. [19] proposed the concept of fiber optic instrumented rods as scour-depth sensors.
245 In that case, the scour depth is obtained by the deformation of the rod. Here, it has been shown that
246 this approach is also valid for rods instrumented by accelerometres. The existence of scour is proven by
247 changes in the first frequency of the rod itself, and the depth can be assessed through this innovative
248 equivalent cantilever beam approach.

249 This brings about a low cost sensor, which may be complementary to direct bridge monitoring as
250 proposed by Prendergast et al. [20].

251 5. Concluding remarks and perspectives

252 Scour is one of the major risks threatening the stability of bridges across rivers and in coastal
253 areas. Therefore, it is paramount to evaluate the current scour depth around piers and abutments. The
254 reported study proposes a continuous monitoring technique of scour by means of rods embedded
255 in the riverbed. Extensive experimental tests were performed in the laboratory using various rods
256 and two types of soil: dry sand and a soft clayey soil. Some uncovered issues were investigated: the
257 effect of the geometry and material of the sensor, the effect of its embedded length and the effect of
258 soil type. The results showed that the sensitivity of the sensors decreases with their flexural rigidity.
259 Furthermore, when the flexural rigidity of the sensor is high in respect to the soil stiffness, no vibratory
260 response was recorded since the response of the sensor was limited to rigid body motion. Thus, it
261 is necessary to select the sensor material and geometry carefully depending on the stiffness of the
262 soil it will be placed in. The tests also showed that the effect of soil type is less significant when the
263 embedded ration of the rod decreases, in other words, when scour increases. Since the sensor will be
264 immersed in water around the pier, the effect of water on the response of the sensor was investigated
265 using a finite element model, and by assuming that the water does not change the behavior of the
266 rod. The numerical results indicate that the effect of water should not be neglected. Indeed, as scour
267 increases, the effect of water becomes more significant.

268 Finally, based on the experimental and numerical results, a simplified cantilever model with an
269 increased exposed length was proposed to correlate the exposed length of the sensor to the measured
270 frequency. This 'correction' of the free length of the cantilever varies with both soil and sensor
271 characteristics. This correction length can be estimated while installing the sensor by calculating the
272 frequencies of different exposed lengths. The proposed cantilever model is of practical interest since
273 it is easier and quicker to implement to estimate scour depth with acceptable accuracy compared to
274 the use of a beam-spring numerical model. Future research will focus on developing equations to
275 calculate the 'correction' of the cantilever model for different sensor materials and soils and on large
276 scale implementations of this monitoring technique.

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279 Abbreviations

280 The following abbreviations are used in this manuscript:

281

- D = Embedded length of the rod (m);
 D_{50} = Average grain diameter (mm);
 E = Young modulus of the cantilever (MPa);
 E_m = Ménard modulus of the soil (MPa);
 E_r = Young modulus of the rods (MPa);
 E_s = Young modulus of the soil (MPa);
 f = First frequency (Hz);
 f_{dry} = First frequency in air (Hz);
 f_{wet} = First frequency in water (Hz);
 f = First frequency (Hz);
 H = Exposed length of the rod (m);
 H' = Adjustment length (m);
 H_C = Free length of the cantilever (m);
 I = Inertia of the rod in the vibration direction (m^4);
 L = Total length of the rod (m);
 M = Mass of the rod (kg);
 M_a = Added mass of water (kg);
 m = Mass of the accelerometer (kg);
 S = Section of the rod (m^2);
 α = Rheological parameter of the soil (-);
 ρ_s = Bulk density of the soil ($kg.m^{-3}$);
 ρ_r = Bulk density of the rods ($kg.m^{-3}$);
 ρ = Bulk density of the cantilever ($kg.m^{-3}$).

References

1. Melville, B.; Coleman, S. *Bridge scour*; Water Resources Publications, Highlands Ranch, Colorado, 2000.
2. Wardhana, K.; Hadipriono, F.C. Analysis of recent bridge failures in the United States. *Journal of Performance of Constructed Facilities* **2003**, *17*, 144–150.
3. Chevalier, C.; Pham Van Bang, D.; Durand, E.; Charles, I.; Herrier, G. Scour and erosion phenomena occurring in waterways—recent advances (Keynote lecture). Scour and Erosion: Proceedings of the 7th International Conference on Scour and Erosion, Perth, Australia, 2-4 December 2014. CRC Press, 2014, p. p:33.
4. Breusers, H.; Raudkivi, A. *Scouring: Hydraulic Structures Design Manual Series*; IAHR Design Manual, Taylor & Francis, 1991.
5. Richardson, E.; Davis, S. *Evaluating Scour at Bridges, Fourth Edition*; Fourth Edition. Hydraulic Engineering Circular No. 18 (HEC-18), Federal Highway Administration, Washington, D.C., 2001.
6. Sheppard, D.M.; Jr., W.M. Live-Bed Local Pier Scour Experiments. *Journal of Hydraulic Engineering* **2006**, *132*, 635–642, [[http://dx.doi.org/10.1061/\(ASCE\)0733-9429\(2006\)132:7\(635\)](http://dx.doi.org/10.1061/(ASCE)0733-9429(2006)132:7(635))]. doi:10.1061/(ASCE)0733-9429(2006)132:7(635).
7. Brunner, G.W. HEC-RAS River Analysis System User's Manual Version 4.1. *US Army Corps of Engineers, Institute for Water Resources, Hydrologic Engineering Center* **2010**, *609*, 95616–4687.
8. Wang, C.; Yu, X.; Liang, F. A review of bridge scour: mechanism, estimation, monitoring and countermeasures. *Natural Hazards* **2017**, pp. 1–26.
9. Briaud, J.L.; Hurlebaus, S.; Chang, K.A.; Yao, C.; Sharma, H.; Yu, O.Y.; Darby, C.; Hunt, B.E.; Price, G.R. Realtime monitoring of bridge scour using remote monitoring technology. Technical report, Texas Department of Transportation, 2011.
10. Millard, S.; Bungey, J.; Thomas, C.; Soutsos, M.; Shaw, M.; Patterson, A. Assessing bridge pier scour by radar. *NDT & E International* **1998**, *31*, 251–258.
11. Anderson, N.; Ismael, A.; Thitimakorn, T. Ground-penetrating radar: A tool for monitoring bridge scour. *Environmental and Engineering Geoscience* **2007**, *13*, pp. 1–10.
12. Falco, F.D.; Mele, R. The monitoring of bridges for scour by sonar and sediment. *{NDT} & E International* **2002**, *35*, 117 – 123. doi:[http://dx.doi.org/10.1016/S0963-8695\(01\)00031-7](http://dx.doi.org/10.1016/S0963-8695(01)00031-7).

- 311 13. Lin, C.; Tang, S. Development and calibration of a TDR extensometer for geotechnical monitoring.
312 *Geotechnical Testing Journal* **2005**.
- 313 14. Yu, X.; Yu, X. Time domain reflectometry automatic bridge scour measurement system: principles and
314 potentials. *Structural Health Monitoring*, 8 (6) (2009), pp. 463–476 **2009**.
- 315 15. Cooper, T.; Chen, H.; Lyn, D.; Rao, A.; Altschaeffl, A. A Field Study of Scour Monitoring Devices for
316 Indiana Streams. *Joint Transportation Research Program* **2000**, p. 2.
- 317 16. Lu, J.Y.; Hong, J.H.; Su, C.C.; Wang, C.Y.; Lai, J.S. Field measurements and simulation of bridge scour
318 depth variations during floods. *Journal of Hydraulic Engineering* **2008**, 134, 810–821.
- 319 17. Hayes, D.; Drummond, F. Use of fathometers and electrical-conductivity probes to monitor riverbed scour
320 at bridge piers. Technical report, US Geological Survey; USGS Earth Science Information Center, Open-File
321 Reports Section [distributor], 1995.
- 322 18. Lin, Y.B.; Lai, J.S.; Chang, K.C.; Li, L.S. Flood scour monitoring system using fiber Bragg grating sensors.
323 *Smart Materials and Structures* **2006**, 15, pp.1950.
- 324 19. Zarafshan, A.; Iranmanesh, A.; Ansari, F. Vibration-Based Method and Sensor
325 for Monitoring of Bridge Scour. *Journal of Bridge Engineering* **2012**, 17, 829–838,
326 [[http://dx.doi.org/10.1061/\(ASCE\)BE.1943-5592.0000362](http://dx.doi.org/10.1061/(ASCE)BE.1943-5592.0000362)]. doi:10.1061/(ASCE)BE.1943-5592.0000362.
- 327 20. Prendergast, L.; Hester, D.; Gavin, K.; O’Sullivan, J. An investigation of the changes in the
328 natural frequency of a pile affected by scour. *Journal of Sound and Vibration* **2013**, 332, 6685 – 6702.
329 doi:<http://dx.doi.org/10.1016/j.jsv.2013.08.020>.
- 330 21. Lunne, T.; Robertson, P.; Powell, J. Cone penetration testing. *Geotechnical Practice* **1997**.
- 331 22. Boujia, N.; Schmidt, F.; Siegert, D.; Bang, D.P.V.; Chevalier, C. Modelling of a bridge pier subjected to
332 scour. *Procedia Engineering* **2017**, 199, 2925 – 2930. X International Conference on Structural Dynamics,
333 EURODDYN 2017, doi:<https://doi.org/10.1016/j.proeng.2017.09.343>.
- 334 23. Baguelin, F. *The pressuremeter and foundation engineering*; Trans Tech public., 1978.
- 335 24. Terzaghi, K.; Peck, R.B.; Mesri, G. *Soil mechanics in engineering practice*; John Wiley & Sons, 1996.
- 336 25. Bao, T.; Swartz, R.A.; Vitton, S.; Sun, Y.; Zhang, C.; Liu, Z. Critical insights for advanced bridge scour
337 detection using the natural frequency. *Journal of Sound and Vibration* **2017**, 386, p.p:116 – 133.
- 338 26. Haghghi, I.; Chevalier, C.; Duc, M.; Guédon, S.; Reiffsteck, P. Improvement of hole erosion test and results
339 on reference soils. *Journal of Geotechnical and Geoenvironmental Engineering* **2012**, 139, 330–339.
- 340 27. Haghghi, I. Characterization of erosion and dispersion : test development and practical applications.
341 Theses, Université Paris-Est, Champs-sur-Marne, 2012.
- 342 28. Schmertmann, J.H. The mechanical aging of soils. *Journal of Geotechnical Engineering* **1991**, 117, 1288–1330.
- 343 29. François, S.; Pyl, L.; Masoumi, H.; Degrande, G. The influence of dynamic soil–structure interaction on
344 traffic induced vibrations in buildings. *Soil Dynamics and Earthquake Engineering* **2007**, 27, 655–674.
- 345 30. Gmur, T. *Dynamique des structures: analyse modale numerique*, [Dynamics of structures: numerical modal
346 analysis]; Presses polytechniques et universitaires romandes, 1997.
- 347 31. Sorensen, D.C. Implicitly Restarted Arnoldi/Lanczos Methods for Large Scale Eigenvalue Calculations.
348 Technical report, Department of Computational and Applied Mathematics – Rice University, 1996.
- 349 32. www.code-aster.org. *Code Aster: Open Source general FEA software*.
- 350 33. White, F. *Fluid Mechanics*, 7th ed ed.; McGraw-Hill, 2009.
- 351 34. Rayleigh, J. *The theory of sound*; Dover, 1945.
- 352 35. Turhan, O. On the fundamental frequency of beams carrying a point mass: Rayleigh approximations
353 versus exact solutions. *Journal of Sound and Vibration* **2000**, 230, 449 – 459.