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# Influence of shoreface morphological changes since the 19th century on nearshore hydrodynamics and shoreline evolution in Wissant bay (northern France)

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# Abstract

Located at the southern approaches to the North Sea, the Wissant bay has had the most rapidly eroding shoreline in mainland France in recent decades. Using historical bathymetries spanning the second half of the 19th century to the present, an analysis of the long-term coastal morphological changes and evolution of a prominent sand bank (Line bank) extending across the bay was carried out. A period of overall seabed erosion with a width-reduction of the Line bank; a deepening of the channel between the Line bank and the coast; and a lowering of the foreshore have been ongoing since the middle of the 20th century. This phase followed a period of channel infill in the western part of Wissant bay in the early 20th century. Numerical modelling of wave propagation and tidal circulation was performed using the TELEMAC suite of models to get some insights into the influence of these bathymetry changes on coastal hydrodynamics. Two seabed configurations were identified revealing a contrasting hydrodynamic circulation. An erosional one induces an acceleration of tidal currents and an increase in wave height. Conversely, accumulation leads to an hydrodynamic regime weakening with a decrease in current velocity and to

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more wave energy dissipation in the nearshore. This study highlights the role of morphological feedbacks between nearshore morphology and coastal hydrodynamics. The identification of these feedbacks mechanisms at a historical time scale is important to assess potential drivers of coastal changes.

*Keywords:* Sand banks, North Sea, France, Hydrodynamic, Tidal currents, Waves, Wissant bay

# 1 1. Introduction

Sand banks are large sedimentary features that occur in many coastal and
shelf seas where there is a large amount of sediment and strong enough currents
to transport this sediment (Caston, 1972; Kenyon et al., 1981; Dyer & Huntley,
1999; Deleu et al., 2004; Pan et al., 2007). There is a complex interaction
between hydrodynamic and seabed leading to a variety of shapes and sizes,
classified according to their formation and hydrodynamic settings (Huthnance,
1982; Dyer & Huntley, 1999; de Swart & Yuan, 2018). These sand banks are
controlled either by storms (storm-generated ridges) or by tidal currents (tidedominated banks) (Belderson, 1986)

The southern North Sea is characterized by a large number of tidal sand 11 banks and has been extensively studied (Houbolt, 1968; Caston, 1972; Davies, 12 1980; Lanckneus et al., 1992; Berné et al., 1994; Lanckneus et al., 1994; Tessier 13 et al., 1999) Sand banks play an important role in influencing shoreline evolution 14 by attenuating the incident wave energy through wave breaking and bed friction 15 processes (MacDonald & O'Connor, 1996; Héquette & Aernouts, 2010). Erosion 16 is widespread along the central and southern part of the North Sea coastline, in 17 France 20% of the coasts are under erosion (Hédou et al., 2018). In Wissant bay 18 (northern France) the average shoreline retreat can exceed 5 m. $an^{-1}$  (Aernouts 19 & Héquette, 2006; CEREMA, 2018). Previous studies have suggested strong 20 interactions between shoreline evolution and associated nearshore due to cross-21 shore and long-shore sediment transfers (Carter et al., 1982; Hanna & Cooper, 22 2002; Shaw et al., 2008; Héquette et al., 2009; Héquette & Aernouts, 2010; 23

<sup>24</sup> Thomas et al., 2011; Héquette et al., 2013; Sedrati & Anthony, 2014).

Long bathymetric time series (several decades to centuries) are rare and gen-25 erally localised in the vicinity of ports or in the context of dredging and aggregate 26 extraction operations. In northern France, topographic and hydrographic sur-27 veys of coastal and nearshore morphologies have been conducted since the begin-28 ning of the 19th century and provide a direct source of data for assessing coastal 29 and nearshore environments evolution (Latapy et al., 2019). Understanding the 30 long-term characteristics of the morphology and the association with natural 31 and anthropogenic forcing factors are important for coastal management. His-32 torical bathymetries have already been used to investigate coastal morphology 33 and hydrodynamic interactions (Cooper & Navas, 2004; Bertin, 2005; Bertin 34 & Chaumillon, 2005; Aernouts & Héquette, 2006; Horrillo-Caraballo & Reeve, 35 2008; Héquette & Aernouts, 2010). For example, Horrillo-Caraballo & Reeve 36 (2008) demonstrated that the 150 year-long changes in UK's eastern sand bank 37 morphology can be largely explained by changes in its tidal residual currents. 38 Cooper & Navas (2004) simulated wave propagation across the southeast coast 39 of Northern Ireland and found a marked change in nearshore patterns of energy 40 dispersal and related sediment transport pathways in one century. Neverthe-41 less, these investigations generally focused on wave or tide-induced processes 42 whereas natural systems are subjected to a combination of waves and tides. In-43 deed, tides change the mean water depth, which affects wave propagation over 44 the inner shelf and the shoreface (Wolf & Prandle, 1999; Osuna & Monbaliu, 45 2004). Tides are also responsible for variations in the nearshore flow field which 46 in turn modifies wave-current interaction (Wolf & Prandle, 1999), especially in 47 a macrotidal environment. In addition, interactions between wave generated 48 currents and tidal currents lead to an increase of bottom shear stress, particu-49 larly at the sand bank crests that can intensify sediment transport (Villaret & 50 Davies, 2004; Giardino et al., 2010; Brown et al., 2015). 51

The present investigation tries to quantify the relative importance of nearshore morphology changes on wave and tide interactions and analyzes the possible impacts on shoreline evolution since the 19th century. First, the coastal evolution of Wissant bay was assessed using historical bathymetry by comparison of digital bathymetric data. Then, effects of the tide-induced time-varying water depths and currents on wave height, and their possible changes depending of the sea bed evolution, were investigated. Finally, the links between documented shoreline changes and modelled modifications in tidal currents and wave regime in the coastal zone are also discussed.

# 61 2. Study area

# 62 2.1. Wissant bay's shoreface morphology

The North Sea coast of France generally consists of wide, gently sloping, 63 sandy barred beaches (Reichmüth & Anthony, 2007). Sand banks are particu-64 larly widespread in this area, where they form linear shore-parallel or slightly 65 oblique sand bodies about 10-30 km long and 1-3 km wide. These banks belong 66 to the Flemish Banks (Houbolt, 1968) and generally occur as groups of banks 67 from shallow coastal areas near beaches to depths of several tens of meters (Vi-68 caire, 1991; Anthony & Orford, 2002). Previous studies have revealed a strong 69 variability in nearshore evolution along large parts of the northern France's 70 coastal zone since the last few decades (Aernouts & Héquette, 2006; Héquette 71 et al., 2013) but also over a larger time-scale (Aernouts, 2005; Ruz et al., 2017; 72 Latapy et al., 2019). 73

Wissant bay is 8 km-long and located on the westernmost part of the French 74 North Sea coast (Figure 1). It forms a well defined, single longshore sediment 75 cell between Capes Gris Nez and Blanc Nez (Dewez et al., 1989; Beck et al., 76 1991; Anthony, 2000; Anthony et al., 2006). It is characterized by a 300 to 600 77 m-wide intertidal bar-trough beach dissected by drainage channels (Sedrati & 78 Anthony, 2007). The gently sloping shallow shoreface extending seaward of the 79 beach bars and trough is characterized by a prominent tidal sand bank named 80 the Line bank (Figure 1), oriented WSW-ENE, roughly parallel to the coastline. 81 The Line bank belongs to headlands banks based on the categorization by Dyer 82 & Huntley (1999) and particularly banner banks formed where the coast changes 83

direction and where headland are resistant to erosion. The sediment distribution consists of a remarkably homogeneous sand, with median diameter ( $D_{50}$ ) ranging from 0.17 to 0.21 mm (Anthony & Héquette, 2007; Sedrati & Anthony, 2007).

Massive aggregate extraction has been carried out in the past, especially 87 in the western sector of the Line bank, closest to the shore. A Senate report 88 mentions that during the 1970s, about 600  $000 \text{ m}^3$  of sediment was extracted 89 (Pintat, 1975). This practice was prohibited in 1983, but the damages caused 90 by this extraction, added to other anthropogenic causes such as the expansion of 91 the port of Boulogne-sur-mer in the 1930s, has resulted in a significant sediment 92 deficit in the bay. This recurrent sediment deficit now affects the beach which 93 contributes to the shoreline retreat (Crapoulet et al., 2017). Indeed Wissant bay 94 is presently one of the most strongly eroded coasts in mainland France, erosion 95 is strongest in the southwestern and central parts of the bay while the northeast 96 was characterized by a significant foredune growth during the second part of 97 the 20th century (Aernouts & Héquette, 2006; Chaverot et al., 2008; Sedrati & 98 Anthony, 2014) (Figure 1a). 99

## 100 2.2. Hydrodynamic context

As with adjacent coastal area, the Wissant bay is a typical mixed storm-wave 101 and tide-dominated environment. This bay is subject to a complex pattern of 102 time-varying influences of both tides and storms and even to wind forced flows 103 (Héquette et al., 2008). Winds are generally from southwest and northeast, 104 but the strongest winds mostly originate from west to southwest.Based on the 105 HOMERE wave hindcast database (Accensi & Maisondieu, 2015), offshore waves 106 are commonly less than 1 m (mean Hs = 0.87 m) but may exceed 3 m during 107 extreme events (max Hs = 4.5 m). Wave periods mostly range from 5 to 7 108 seconds (mean period 6.14 s) and the dominant waves are from the south-west 109 to west (from the English Channel) followed by waves from the northeast to 110 north (originated from the North Sea) (Figure 1b). Because depth-induced re-111 fraction and dissipation of waves occurs over the shoreface and over the Line 112 bank, at the coast waves are usually reduced and from a NW to NNE window. 113

The tidal regime is semi-diurnal and macrotidal (tidal range about 7 m during 114 spring tides (Shom, 2017)). The nearshore circulation during fair-weather con-115 ditions is characterized by a flood-dominated asymmetry responsible for strong 116 northeastward-directed currents (Héquette et al., 2008) (Figure 1c). Longshore 117 currents can become particularly strong during storms as a result of direct wind 118 and wave stress (Sedrati & Anthony, 2007). The net regional sediment trans-119 port in the coastal zone is directed eastward in response to the easterly-directed, 120 flood-dominated tidal circulation (Augris et al., 1990) but locally counter-drift 121 currents toward the west-southwest are also observed (EGIS, 2014). 122

#### <sup>123</sup> **3.** Methodology

#### 124 3.1. Bathymetry changes

Several bathymetry datasets covering the northern France coastline and ad-125 joining seabed are available dating back to the mid-19th century. Most surveys 126 were carried out over 2-3 years given the limited resources available at the time 127 and the extent of the area covered (Table 1). Historic hydrographic field sheets 128 from the French Hydrographic Service (SHOM) were scanned, digitized and 129 then interpolated using the "krigging" method to produce Digital Elevation 130 Models (DEMs). The coordinate system is the WGS84, and elevations are in 131 meters below chart datum (lowest low-water spring level). The uncertainty as-132 sociated with each survey was calculated by taking into account the sounding 133 tool and the positioning technique (Table 1). To quantify bathymetry changes, 134 differential DEMs, along-shore and cross-shore transects and sediment volume 135 calculation were made (more details on interpolation and computing methods 136 can be found in Latapy et al. (2019)). Morphological changes and computed 137 volumes are considered significant when they are not within the error margin. 138

#### 139 3.2. Numerical model

To investigate the influence of seabed morphological changes on coastal hydrodynamic, a numerical approach was developed. This study used the TELEMAC suite of models, consisting of TELEMAC-2D (Hervouet, 2007) and
TOMAWAC to simulate tides and waves respectively. As a first step, TELEMAC2D and TOMAWAC were run separately to study individually the impact of
bathymetric changes on tidal circulation and on wave propagation.

Subsequently, TELEMAC-2D and TOMAWAC were run in fully coupled 146 mode so that the wave propagation module integrates the effects of the time-147 varying water depth and currents computed by the circulation modules. TELEMAC 148 transfers to TOMAWAC values of current velocities and water depths, while 149 TOMAWAC solves the wave action density conservation equation with reference 150 to those current and water depth values and returns to TELEMAC the updated 151 values of the wave driving forces FX and FY (including surface, bottom and 152 radiation stresses) acting on the current. Water-depths variation modulates the 153 dissipation of waves energy by bottom friction and wave breaking. Opposing 154 tidal flows induce a steepening of the incident wave field thus increasing wave 155 height. Finally, the combination of the time-varying water depths and currents 156 leads to wave refraction and to a variability of the incident wave energy at 157 specific coastal locations. 158

TELEMAC-2D and TOMAWAC were set up on a domain covering northern France's coastal zone - from the Belgium border to the Dover Strait (Figure 9). The first three bathymetry surveys extended over sufficiently large areas were used for numerical modelling (2016 survey was restricted to Wissant bay) (Table 1). DEMs generated from the selected bathymetry surveys were used to obtain three grids (Table 2) with a mesh resolution ranging from 250 m at the coast and over sand banks, increasing to 2.5 km offshore (Figure 9).

We forced our model to simulate the most powerful currents and waves possible in each mesh in order to identify high energy areas and those that have potentially undergone the most significant changes. We focussed our analysis on extreme events because coastal changes (such as beach and coastal dune erosion) are largely due to high-energy processes (Masselink et al., 2014; Castelle et al., 2015). Forcing conditions are described in the following sections.

# 172 3.2.1. TELEMAC forcing

The model is forced at the offshore open boundary with specified elevation 173 for 143 tidal harmonics from the tidal model cstFRANCE developed and used 174 by the SHOM (Le Roy & Simon, 2003). The validation of the models was 175 carried out with tidal observations and predictions at the ports of Calais and 176 Dunkirk. Current observations also allowed the model to be adjusted and cal-177 ibrated (see the Appendix A for a detailed overview of the validation of the 178 model). The time step is set to 10 seconds and the bottom friction coefficient is 179 computed with Chezy's law. As the modelled area was spatially restricted, and 180 is composed of quasi-homogeneous sea-bed sediment (sandy-gravelly) (Anthony 181 & Héquette, 2007) a uniform friction coefficient was applied to the entire mesh 182 (64  $m^{1/2}$ .s). This coefficient is consistent with those obtained for equivalent 183 surficial sediment(Soulsby, 1983, 1997; Cooper et al., 2004). 184

The model was run for a period of 30 days during the highest astronomical tides of 2015 (03/01/2015 to 03/30/2015) with sea-level elevation over the whole domain at 10 min intervals. Residual tidal currents are computed over the whole simulated period, these maps were used to identify possible transport pathways. Since the maps represent flow and not sediment transport they are only indicative of the likely direction of sediment transport and should not be interpreted quantitatively.

## <sup>192</sup> 3.2.2. TOMAWAC parametrization

TOMAWAC model enables us to evaluate the effects of changing seabed 193 morphology on wave refraction and on the pattern of wave energy distribution 194 at the coast (see the Appendix B for a detailed overview of the validation of 195 the model). Significant wave height (Hs), peak wave period (Tpeak) and wave 196 direction (Dir) were calculated using TOMAWAC spectral wave model run on 197 the same mesh as the hydrodynamic calculation (Figure 9) with a time step of 198 10 seconds, 20 frequency bins and 20 wave directions. Observations from a wave 199 buoy located offshore (CEREMA, 1985) and analysis of hindcase database in this 200 area (Boudière et al., 2013; Accensi & Maisondieu, 2015) showed a strongly bi-201

model distribution of wave direction grouped around north-easterly and westerly directions (Figure1b). A set of two wave model runs were created by applying constant wave height, peak period and directions boundary forcing using the 99th percentile of wave height calculated from HOMERE database (Hs99%) for each of the two dominant wave directions. The model was run for these two wave conditions representative of the extreme wave regime of the region. (see Table 3).

#### 209 4. Results

#### 210 4.1. Evolution of the Wissant bay seabed

The bathymetric differential maps between 1878 and 2016 allowed us to 211 examine the evolution of the seabed morphology in the Wissant bay (Figure 2). 212 Significant bathymetry changes have occurred all over the study area since the 213 end of the 19th century. An extended seafloor lowering of more than 7 m has 214 taken place on both sides of the Line bank and also in the westernmost part 215 of the foreshore. Moreover, an erosion pattern is clearly identified offshore of 216 the Line bank in the northern part of the bay. Conversely, significant sediment 217 accumulation has only been observed on the NE of the Line bank. Based on the 218 bathymetry change observed and the error margin of  $\pm 2.7$  m between these two 219 dates, an estimate of the volume change was computed showing a significant 220 loss of more than  $28 \times 10^6 \text{ m}^3$  of sediment throughout the bay. Considering 221 only the Line bank, this loss is estimated at  $14 \ge 10^6 \text{ m}^3$ . In order to analyze 222 the Line bank and nearby morphological evolution, cross-shore and long-shore 223 profiles were derived from DEMs (Figure 3), which revealed two distinct periods 224 of evolution: 225

Between 1878 and 1910: on the westernmost and central part of the bay,
the Line bank underwent a lowering of its crest and a simultaneous widening (Profile A and B on Figure 3), especially towards the coast. This
coastward migration led to a infill of the channel between the coast and
the sand bank, which is well-identified on Profile A. On Profile C, the

onshore migration of the Line bank is visible, although the channel re-231 mained unchanged in the easternmost part of the Wissant bay. Profile D 232 confirms the lowering of the crest over almost the total bank area. The 233 foreshore rose (Profile A) or remained stable (Profiles B and C) between 234 the end of the 19th and the beginning of the 20th century. The associated 235 bathymetric differential map (Figure 2 inset map) shows that over almost 236 the whole bay, morphological changes were relatively minor. It indicates 237 a stability of the coastal zone to the east and sediment accumulation to 238 the west during this period. 230

• Between 1910 and 2016: the channel between the coast and the Line bank 240 was subject to significant erosion, with more than 5 m lowering on Profile 241 A. Conjointly, the Line bank experienced a decrease of its width of about 242 500-600 m (Profile B) and up to 700 m locally (Profile A). The most 243 remarkable changes are visible on Profile B, in 1974 the Line bank did 244 not exceed a width of 200 m. In 1974, the height of the bank was at its 245 lowest level (Profile A and D) while the eastern part of the bay was more 246 stable. Over the 1974-2016 period, even though the Line bank experienced 247 a slight increase of its width (Profile B), in most part of the Wissant bay, 248 the channel continued to deepen and reached the -5 m isobath on Profile 249 A and -12 m on Profile C. This had an impact on the foreshore which 250 lowered significantly during the 20th century in the western and central 251 part of the bay (Profile A and B) while Profile C indicates a more stable 252 morphology. 253

Therefore, until the beginning of the 20th century, Wissant bay had experienced an infilling of coastal channel, mostly due to the widening and the onshore migration of the Line bank in the western and central part of the bay. It was during the 20th century that a widespread erosion of the bay can be observed. It not only impacts on the Line bank's morphology, but also the channel and the foreshore - leading to a net landward retreat of the shoreline observed during the second half of the 20th century (Aernouts & Héquette, 2006; CEREMA, 261 2018).

Because wave energy distribution at the coast strongly depends on wave refraction and dissipation over the shoreface, any change in the nearshore morphology is expected to modify wave propagation patterns and therefore induce changes in wave height. In this macrotidal environment, tide must be taken into account in the model in order to also study the effects of water level variation on wave energy. Results of these simulations are presented in the following sections (Section 4.2 and 4.3).

# 269 4.2. Tidal residual currents changes

Figure 4 shows the computed tidally induced residual currents for Wissant bay over a 30-day simulation for three bathymetry configurations (1878, 1910, 1974). In Figure 5 the model integrates the wave propagation module suggesting a complex residual current flow field that is partly controlled by wave direction.

# 274 4.2.1. 1878

Considering the no-waves configuration, tidal residual currents are essentially directed to the NE which correspond to the flood-dominated direction and can reach 0.3 m.s<sup>-1</sup>on the seaward side of the Line bank (Figure 4a).

With waves from the N-NE direction, the intensity of residual currents is generally of the same amplitude as that of the no-waves configuration, although the distribution of these velocities changes: with weaker currents on the offshore side and more intense to the interior of the bay (Figure 5a). Locally, and mostly on the east of the bay, current direction is also reversed, with residual currents towards the SW, which correspond to a predominance of the ebb.

WSW waves tend to induce much stronger residual currents (Figure 5d) with speeds ranging from 0 to 0.9 m.s<sup>-1</sup>; the maximum residual currents being located on the seaward flank of the Line bank (residual current peak at 0.9 m.s<sup>-1</sup>). In this configuration, currents are mainly directed towards the NE, which would be expected given the initial wave direction that would drive and intensify the currents in Wissant bay. 290 4.2.2. 1910

By not taking into account any waves, an area of high current velocities (about 0.5 m.s<sup>-1</sup>) is noticeable on the western border of the model, probably due to boundary effect in the model (Figure 4b). Across Wissant bay, tidal residual currents are directed towards the NE, except in the westernmost part where onshore currents are apparent. Throughout the foreshore, residual currents are weaker than those computed in the 1878 mesh. In the NE part of the Line bank, there is also a decrease in the velocity of residual currents.

When waves are from the NE sector, as in the 1878 model, there is a residual current direction reversal in the eastern part of the foreshore leading to a local predominance of the ebb (Figure 5b). On the SW part of Wissant bay, currents are weaker, not only at the foreshore, but also at or near the crest of the Line bank (0.12 m.s<sup>-1</sup>).

For westerly waves, over the whole bay, currents are stronger than the two previous set-ups. However, residual currents are lower over the 1910 bathymetry compared to currents velocities observed with westerly waves over the 1878 bathymetry. This is clearly observable, even on the seaward side of the sand bank where currents are usually more intense, they do not exceed 0.75 m.s<sup>-1</sup> against 0.9 m.s<sup>-1</sup> in 1878 (Figure 5e).

309 4.2.3. 1974

With the no-waves set-up, residual currents mostly flow toward the NE (flood-direction) (Figure 4c). Weakest currents are located on the central part of the Line bank ( $\approx 0.05 \text{ m.s}^{-1}$ ), while strongest ones, are located in the SW part of the bay, particularly on the seaward side of the bank ( $\approx 0.3 \text{ m.s}^{-1}$ ).

For northeasterly waves, residual currents speeds are lower at the sand bank margins  $(< 0.1 \text{ m.s}^{-1})$  but they increase on the Line bank crest. At the shoreline, the linear area where currents are directed towards the SW is extended northeastward and now reaches the town of Wissant. Facing the hamlet of Wissant, these SW residual currents are opposed to those in the channel and may create shearing zones (Figure 5c). With westerly waves, residual current are still stronger than those obtained with the no-waves and NE-waves configurations (Figure 5f). However, compared to the 1910-W set-up, areas where currents exceed 0.4 m.s<sup>-1</sup> are more extensive, especially in the western part of the bay (on the top of the Line bank and also in the channel). Likewise, there is an increase in velocities on the Line bank seaward flank and at the shoreline in the central part of the bay.

#### 326 4.3. Impact on waves propagation

Wave height transects from the 1878, 1910 and 1974 models are presented in Figures 6 for northeasterly waves and in Figure 7 for westerly waves (the location of the wave height transects is the same as the bathymetric ones). In each transect, for same wave conditions, the wave heights obtained during high water springs and without considering tides are plotted in order to compare the effect of tides on wave height distribution and in turn energy dissipation in the nearshore.

Figures 6 and 7 clearly show the effect of tides on wave height with higher 334 wave heights occurring during high water springs. These results are consistent 335 with previous studies that investigated the contribution of tidal modulation on 336 incident wave power (Peregrine, 1976; Lugo-Fernández et al., 1998; Masselink, 337 1998; Davidson et al., 2008; Hedges, 2015). There is a strong wave energy 338 dissipation over the sand bank for both westerly and northeasterly waves, which 339 results in a similar Hs distribution from one configuration to another as shown 340 on Profile A, B and C (Figures 6 and 7). 341

On the top of the Line bank, wave height changed significantly between 1878 342 and 1974 (Profile D Figures 6 and 7). Without tides, in the 1878 model, there 343 is already a strong gradient, with an Hs of 1.5 m in the NE part of the bay 344 decreasing until 0 m to the SW. However, from 1878 onwards, waves exceed 0.5 345 m on the Profile D over a length of 4 km in 1878, increasing to 6 km in 1910, 346 and finally 7.5 km in 1975, which corresponds approximately to the length of 347 the Line bank (Profile D Figures 6 and 7). At the end of the 19th century, 348 the Line bank was locally above the hydrographic datum (Profile A Figure 3) 349

so it was often emerged inducing wave dissipation and breaking. This explain 350 why, without tides, in the 1878 and even in the 1910 model, Hs is equal to 0 351 m from the top of the bank to the coast (Profile A Figures 6 and 7). However, 352 during high water springs, the water depth is high enough to cover the Line 353 bank and waves can break farther landward (Profile A Figures 6 and 7). When 354 high water springs are considered, on the NE part of the Profile D, Hs slightly 355 increases in time but remained mostly stable for both western and northeastern 356 waves. However, significant changes occurred in the western and central part of 357 the Line bank where, for both waves direction, an increase Hs of around 0.5 m 358 is observed between the 1878 and the 1974 models (Profiles A and B Figures 6 359 and 7). 360

From 1878 to 1974, waves do not only become higher on the top of the sand 361 bank, but also in most part of the coastal channel. This increase is well-identified 362 in Profiles D and E (Figures 6 and 7) where Hs become larger particularly in 363 the central part of the profiles. At the end, in 1974, without tides, more than 4 364 km of channel recorded a wave height greater than 0.5 m (against 2 km in 1878 365 and 3 km in 1910). During high water springs, for all models, Hs exceeds 0.5366 m over the whole channel. In the same way, the portion of the profile greater 367 than 1 m represents 66% for 1878, 60% for 1910 and 83% for 1974 revealing a 368 small decrease of wave energy in the channel in 1910 and a strong rise in 1974 369 (Profile E Figures 6 and 7). 370

From 1878 to 1974, in the western and central part of Wissant bay, there is 371 an increase in wave height at the coast of 0.2-0.4 m for W waves and of 0.3-0.6 m 372 for NE waves for both non-tidal and high water springs configurations (Profile 373 A et B Figures 6 and 7). The highest wave heights are logically observed during 374 high water springs due to higher water levels over the bank and shoreface, this 375 being particularly visible on Profile B. With a lower water depth (no tide) the 376 wave height strongly decrease landward in 1878 and 1910 but when the water 377 level is higher (high water springs), wave height decreases gradually (Figures 378 6 and 7). On Profile B, this results in a strong reduction of Hs between the 379 channel and the coast (over less than 500 m) highlighting strong wave energy 380

<sup>381</sup> dissipation at the coast.

The easternmost part of Wissant bay is characterized by different wave 382 height changes, becoming more stable and even reducing locally between the 383 1878, 1910, and 1974 models (Profile C Figures 6 and 7). In Profile C, wave 384 height changes are less significant. It should be noted, that for the 1974 model, 385 with or without tides, the wave height is always lower than the ones obtained 386 with the 1878 and 1910 models. Considering the tide, however, in 1974, waves 387 are higher offshore up to X=2000 (isobath -5 m) and only lower from the fore-388 shore to the shoreline (Profile C Figures 6 and 7) 380

In summary, the results of our numerical modeling wave height over successive bathymetries since 1878 suggest that morphological changes and Line bank positions, have induced significant changes in wave height distribution (Table 7) along Wissant's coast. Accordingly, we can divide Wissant bay into three main areas (Table 7):

- The eastern part of the bay, characterized by a stability in wave heights through time, and even a decreasing trend in some areas.
- The central part of the bay where our results show a generally increased wave heights between the Line bank and the coast from 1878 to 1974.

• The western part of the bay, with a more complex evolution: between 1878 and 1910, wave height increased on the Line bank, but decreased on the foreshore and the shoreline, while an overall increasing trend of wave height is observed all along the nearshore for the period 1910-1974.

#### 403 5. Discussion

Based on historical bathymetric surveys, our study reveals a high variability in the Wissant bay seabed morphology. Using ancient nautical charts, Briquet's work (Briquet, 1930) pointed out that the Line bank was once located more offshore from Wissant Bay and has experienced a gradually on-shore migration since 1640 (Figure 8). In 1776, the bank was located 800 m seaward of the Gris

Nez Cape at low tide, but was at only 200 m from the cape in 1810. In 1835, the 409 area between the bank and the coast was largely exposed at low tide, although 410 a deep channel was still present. Briquet's work highlighted that the trough 411 was gradually filled in the 1870s, and in 1910 the Line bank was almost welded 412 to the beach, which is confirmed in the western part of the Wissant bay by our 413 DEMs (Profile A Figure 3). Previous studies and our historical bathymetry sets 414 enable us to assess the complex morphological evolution of this area: 415

• During the end of the 19th and the beginning of the 20th century, the 416 Line bank experienced a widening and a coastward migration resulting in 417 a gentler beach and foreshore profile in 1910; 418

• Since the 1970s, Wissant bay underwent a thinning of the Line bank, a 419 deepening of the coastal channel and a lowering of the foreshore associated 420 with an overall erosion of the seabed. It appears that the lowering of the 421 seabed in Wissant Bay is a fairly recent phenomenon following a phase of 422 sediment accumulation and coastward migration of the bank. 423

Nearshore changes are strongly linked to shoreline evolution as shown by sev-424 eral other studies carried out in a variety of coastal settings (Corbau et al., 1999; 425 McNinch, 2004; Aernouts & Héquette, 2006; Backstrom et al., 2007; Houser 426 et al., 2008) and the recent shoreline retreat observed in the western and cen-427 tral part of the bay and the shoreline advance in the eastern part can be partially 428 explained by this interaction (Aernouts & Héquette, 2006). 429

With a similar approach, Bertin (2005); Horrillo-Caraballo & Reeve (2008) 430 found that the pattern of residual currents is strongly linked to the configuration 431 of sand banks. In addition, observations and numerical results on sandy coast 432 revealed the essential role of wave energy dispersal in the nearshore sediment 433 distribution (Morton et al., 1995; Sedrati & Anthony, 2007) and its contribution 434 for controlling the development of erosion or accretion hot spots.(Barnard & 435 Hanes, 2006; Héquette & Aernouts, 2010) 436

437

The results of our numerical modeling highlight two types of shoreface/nearshore

configurations leading to two different types of hydrodynamic circulation pre-sented below.

#### 440 5.1. The eroding-configuration

The 1974 model can be assimilated as the **eroding-configuration**. For 441 this bathymetric configuration, stronger residual tidal currents, and higher wave 442 heights are observed in the western and central part of the Line bank, compared 443 to the two preceding seabed morphologies. This occurs for waves either from the 444 W or NE (Figures 5, 6 and 7). The last DEMs highlight a significant thinning of 445 the Line bank and an extensive lowering of the seabed in Wissant Bay between 446 1910 and 1974 (Figure 2). This contributes to increase the foreshore's exposure 447 to high energy waves by reducing the effects of wave dissipation and refraction 448 on the sea bed (Barnard & Hanes, 2006). This may have also modified wave-449 induced sand dynamics (due to wave-current interaction) and thus sand bank 450 equilibrium. Sediment transported from the nearshore to the upper shoreface is 451 more likely to be remobilized and distributed northeastward by currents than 452 to accumulate on the beach. This combined phenomenon may have induced 453 an increase in longshore sediment transport that could explain the pervasive 454 shoreline retreat observed in the second half of the 20th century (Aernouts & 455 Héquette, 2006; Sedrati & Anthony, 2014). Moreover, the wave-current shear-456 ing zones in front of Wissant town obtained with the NE-waves configuration 457 (Figure 5c) could accentuate the erosion of the channel and the beach in this 458 area (Jing & Ridd, 1996). In the eastern part of Wissant bay, the foreshore 459 experienced a decrease of Hs and also a reduction of the strength of residual 460 currents directed to the SW. This reduction in current speeds may result in less 461 sediment transport to the western and central parts of the Wissant bay that 462 could enhance the sediment deficit in these parts of the bay. On the other hand, 463 it may also result in more sediment deposition in the eastern part of the bay 464 and therefore generate the shoreline advance observed in this part of the bay 465 during the second half of the 20th century. 466

# 467 5.2. The accumulation-configuration

The accumulation-configuration (corresponding to the 1910 bathymetric 468 configuration), reveals significant changes in the central and western parts of the 469 bay. First, during NE-waves events, residual currents are weaker than in 1878 470 between the Line bank and the coast where current speeds do not exceed 0.15 471 m.s<sup>-1</sup> (Figure 5b). For western waves, residual currents remain strong on the 472 seaward side of the Line bank, but have decreased in the channel and on the 473 foreshore compared to 1878 (Figure 5e). Secondly, the increasing wave height 474 between 1878 and 1910 on the top of the Line bank (Profile D Figures 6 and 475 7) and the following decrease of Hs in the channel (Profile E Figures 6 and 476 7) indicate wave energy dissipation. As shown in several studies, gently slop-477 ing dissipative profiles favor onshore-directed transport (Hesp, 1988; Davidson-478 Arnott & N. Law, 1996; Aagaard et al., 2004; Cooper & Navas, 2004; Héquette 479 & Aernouts, 2010). Modelling of wave propagation revealed that the widen-480 ing and the onshore movement of the Line bank between 1878 and 1910 was 481 responsible for a decrease in wave energy associated with more dissipative con-482 ditions in the nearshore zone, which may have resulted in a net, wave-induced 483 shoreward sediment transport. In this configuration, as residual currents speeds 484 are particularly lower in the western and central part of the bay, this may have 485 favored sediment accumulation and channel infill. 486

Beach and nearshore morphologies are the result of the interaction between 487 physical processes controlling the cross-shore and longshore transport that even-488 tually cause morphological changes. The present study highlights positive feed-489 backs between hydrodynamic and coastal morphology. Indeed, both bathymet-490 ric configurations (eroding and accumulation) affect the hydrodynamic circula-491 tion in such a way that the morphodynamic response may be reinforced. How-492 ever, our analysis do not identify processes that can induce a switch between 493 both regimes. According to their dynamic and morphological characteristics, 494 sandy beaches can be classified into several morphodynamic types (Wright & 495 Short, 1984; Short, 1999). Scott et al. (2011) expanded this classification to 496 take into account the influence of tidal range. In their classification, Wissant 497

bay belongs to the MITB beach type (Multiple inter-tidal barred). Investigation 498 made on MITB beaches suggest that these beaches have a rapid morphologi-499 cal response under increasing energy conditions, while under decreasing energy 500 the adjustment is slower. Owever, because our study is essentially focussed on 501 extreme events, it does not examine relaxation time effects. Nevertheless, a 502 decrease or increase in storms frequency, variations in large-scale hydrodynamic 503 circulation and/or changes in sediment supply through time may have affected 504 the nearshore-shoreface equilibrium and then induced a morphodynamic rever-505 sal. 506

# 507 5.3. Combined effects of waves and tides

It appears from these different simulation runs that, residual currents ob-508 tained do not exceed 0.40 m.s<sup>-1</sup> and are mostly directed in the flood-direction 509 (*i.e.* to the NE) when tides only are considered. The interaction with the waves, 510 on the contrary, accelerate these currents (up to 0.9 m.s<sup>-1</sup> locally for western 511 waves) and sometimes even reverse their direction (with NE waves). These re-512 sults are consistent with King et al. (2019) study which investigate the relative 513 influence of tidal and wave forcing on potential sand transport in analogous 514 macrotidal environment (South West UK). Similarly, they found that extreme 515 waves are able to induce directional shifts and even full reversals which strongly 516 influence sand transport direction. Likewise, although the direction of the wave 517 has only a limited influence on the wave height at the coast, taking into ac-518 count the tides affects the water elevation and therefore, in the case of high 519 water springs, nearshore wave height may double or even triple. This shows the 520 potential importance and influence of these processes on shoreline dynamic. In-521 dividually, they already provide some preliminary explanations for the observed 522 shoreline changes, but when coupled together, a very energetic hydrodynamism 523 emerges in this sector, which has evolved considerably over the past century. 524

#### <sup>525</sup> 5.4. Limitations of the results and perspectives for future research

In this study, one of the main assumptions made was that past and present hydrodynamics remained the same. Accordingly, the only variable would be the

seabed morphology as the main task of this study was to highlight the influence 528 of nearshore morphology changes on wave and tide interactions. In fact, wave 529 climate has potentially changed since the beginning of the 20th century. Using a 530 100-year numerical wind-wave hindcast, Dodet et al. (2010); Bertin et al. (2013) 531 found an increase of wave height of 0.01 m.year<sup>-1</sup> in the North-East Atlantic 532 Ocean since 1900 due to a rise in wind speed of more than 20%. At a global 533 scale, with a 23-year database of altimeter measurements (1985-2009), Young 534 et al. (2011) investigated global changes in wave height. They suggested an 535 increase of more than 1%.year<sup>-1</sup> in the 99th-percentile wave height trends over 536 this period at high latitudes. The rate of increase is greater for extreme events 537 as compared to the mean condition. Moreover, MacDonald & O'Connor (1996) 538 predicted an average wave energy increasing by the order of 10% by 2130 under 539 a sea level rise scenario. On European shelf, Idier et al. (2017) investigated 540 the effect of sea level rise on tidal dynamics and observed a notable increase in 541 high tide levels in the Southern part of the North Sea. All these studies suggest 542 significant changes in hydrodynamic circulation. Sea level rise was not taken 543 into account in our numerical models, but added to the increase in wave height 544 during the 20th century, one can imagine that the hydrodynamic context at 545 the end of the 19th century was weaker and therefore that our 1878 and 1910 546 models partially overestimate currents and waves. Potentially, it is possible that 547 the differences in energy levels between the end of the 19th century and 1974 548 may be even greater than expected. Finally, in the present context of climate 549 change, even if coastal morphology remained hypothetically unchanged in the 550 following decades, it can be expected that there will be significant changes in 551 coastal hydrodynamics in this area with the increase in wave height with sea 552 level rise. 553

Hydrodynamic circulation changes have an important role in the coastal systems equilibrium. However, it is also important to assess the sediment in transit through this system, which should be included in any further work. It is frequently suggested that the recent trend reversal is not only due to natural causes but also to local factors related to anthropogenic development or con-

struction (Mattheus et al., 2018; Li et al., 2019), such as the expansion of the 559 port of Boulogne-sur-mer in the 1930s and the massive aggregate extraction in 560 the 1960s-1970s. The construction of the Boulogne-sur-mer dikes may have re-561 duced or even blocked the sedimentary transit from the English Channel coasts 562 to the North Sea, thus reducing the supply to the Bay of Wissant. In addition, 563 the extraction of marine aggregate in the western of the bay and on the Line 564 Bank, the sediment budget being in deficit, may have triggered a process of ero-565 sion that became self-maintained even following the interruption of extraction 566 (Sedrati & Anthony, 2014). 567

Other studies carried out in the northern France's coastal zone have revealed 568 that sand banks are actively migrating alongshore and onshore as a result of 569 shore-parallel tidal currents and onshore storm waves (Garlan, 1990; Corbau, 570 1995; Tessier et al., 1999; Héquette et al., 2009; Héquette & Aernouts, 2010; 571 Héquette et al., 2013). Other studies carried out in the inner part of some con-572 tinental shelves have highlighted significant variations in nearshore morpholo-573 gies due to sand banks dynamics (Parker et al., 1982; De Moor, 2002; Hanna 574 & Cooper, 2002; Schmitt & Mitchell, 2014). This numerical approach, being 575 based upon general principle of hydrodynamics is anticipated to have wider ap-576 plicability to others nearshore sandbank systems, as those previously mentioned. 577 This will contribute to a better understanding of the long-term dynamics of this 578 coastal zones, characterized by the presence of mobile sand banks, and provide 579 a better insight into the mechanisms involved in their evolution. 580

# 581 6. Conclusion

<sup>582</sup> Changes in the Wissant bay nearshore have been investigated directly from <sup>583</sup> a comparison of historical hydrographic surveys determined from seabed sound-<sup>584</sup> ings as well as with the results from an hydrodynamic model. This model, <sup>585</sup> coupling both tides and waves, for the northern France area (between the Dover <sup>586</sup> Strait and the Belgium border) has been calibrated and validated. In the second <sup>587</sup> half of the 20th century, the nearshore experienced a thinning of the Line bank, a deepening of the coastal channel and a lowering of the foreshore leading to an
overall erosion of the seabed. This phase followed a period of channel infill in
the western part of the bay in the early 1910s.

These recent morphological changes led to significant hydrodynamic shift 591 with residual tidal currents acceleration in most parts of the nearshore zone and 592 an increase of wave height in the western and central parts of Wissant bay where 593 one of the most rapid shoreline retreats has been recorded in France since the 594 1950s. However, at the beginning of the 20th century, seabed morphology led to 595 more dissipative conditions: it resulted in an overall decrease in wave height and 596 to lower residual currents speeds that could favor sediment deposition on the 597 foreshore and eventually on the beach. As the seabed morphology changes, this 598 affects tidal currents and wave dissipation; so there is a complex interaction 599 and feedback between hydrodynamic circulation and the seabed. Using his-600 torical bathymetry coupling with numerical models has provided a qualitative 601 explanation of observed bathymetry changes and shoreline evolution during the 602 last decades. This approach involves wave-current interactions and brings new 603 insight into the long-term coastal dynamics of this area and on the processes 604 controlling erosion and accumulation along shorelines affected by the movement 605 of nearshore sand banks. 606

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alexa.latapy@gmail.com

# 623 Appendix A. TELEMAC-2D validation

TELEMAC-2D (v6p3) solve the non-conservative form of the shallow water equations (Equations A.1, A.2 and A.3):

$$\frac{\partial h}{\partial t} + \mathbf{U}.\nabla(h) + h.div(\mathbf{U}) = S_h \tag{A.1}$$

$$\frac{\partial U}{\partial t} + \mathbf{U} \cdot \nabla(U) = -g \frac{\partial h}{\partial x} + S_x + \frac{1}{h} div(h\nu_t \nabla U)$$
(A.2)

$$\frac{\partial V}{\partial t} + \mathbf{U} \cdot \nabla(V) = -g \frac{\partial h}{\partial y} + S_y + \frac{1}{h} div(h\nu_t \nabla V)$$
(A.3)

where h is the water depth, t is the time, **U** is the depth-averaged velocity 626 vector with components U and V in the x and y Cartesian coordinate respec-627 tively.  $\nu_t$  is the diffusion coefficient (considered as constant), g is the gravity 628 acceleration,  $S_h$  is a source or sink term in the continuity equation. The source 629 or sink term  $S_x$  and  $S_y$  in the dynamic equations represent the wind and the 630 atmospheric pressure, the Coriolis force, the bottom friction and additional 631 sources or sink of momentum within the domain in the two directions x and 632 y. Model domains are shown in Figure 9 and integrate historical bathymetry 633 (1878-79, 1910-11, 1930-32 and 1974-76). To validate the tidal model, a range 634 of cases were studied. A set of tests was performed in which the bottom friction 635 coefficient was changed by varying the Chezy's law coefficient between 50 and 636 70  $m^{1/2}$ .s. The influence of the bottom friction coefficient on the models ability 637 to reproduce observed data was examined first. A set of tidal height observa-638 tions are available for Dunkirk and Calais harbors at each date and are used to 639 compare with outputs of our 4 models (Figure10b). Tidal height predictions are 640 also compared with results from numerical models to only evaluate astronomical 641 tides. Bests results are obtained using a coefficient of  $64 \text{ m}^{1/2}$ .s. Soulsby (1983, 642 1997)'s works on dynamics of marine sands linked the Chezy coefficient to the 643 nature of the seabed. For sandy-gravelly seabed (Anthony & Héquette, 2007), 644 Chezy coefficient ranges between 57 and 65 which is consistent with our results. 645

The Table 5 shows tidal height comparison at Calais and Dunkirk (tidal ob-646 servation are only available in 1878 and 1930); it appears that for each models, 647 simulated results and tide predictions give a good correlation ( $\mathbb{R}^2 > 0.85$ ). By 648 analysing observations and models,  $\mathbb{R}^2$  obtained are also statistically signifi-649 cant even if these values are lower than those computed with predictions. This 650 deviation is probably due to meteorological phenomena (atmospheric pressure, 651 wind effect) recorded in water levels but not taken into account in the model 652 parametrization. Only a few tidal current records are available in model areas 653 before the 1970s, however, in 1935, current observations were made at Calais 654 and are then used to improve the calibration our models. It appears that with 655 a coefficient of 64  $m^{1/2}$ .s, model currents amplitude and phase are consistent 656 with observations made in 1935 (Figure 10a). A statistically significant corre-657 lation is obtained with a  $R^2=0.95$ . Simulations were carried out over a winter 658 period (March 2015), *i.e.* when the water-column is well mixed, no thermocline 659 is present and therefore no vertical stratification is required. 660

## 661 Appendix B. TOMAWAC validation

TOMAWAC models the changes, both in time and in the spatial domain, of the power spectrum of wind-driven waves and wave agitation. The directional spectrum of wave action density is considered as a function of five variables (Marcos, 2003):

$$N(\overrightarrow{x}, \overrightarrow{k}, t) = N(x, y, k_x, k_y, t)$$
(B.1)

Where  $\vec{x} = (x, y)$  is the spatial location in a Cartesian coordinate system,  $\vec{k} = (k_x, k_y) = (k.sin\Theta, k.cos\Theta)$  is the wave number vector for directional spectrum discretization, $\Theta$  is the wave propagation direction and t the time. Hypotheses made on the wave representation, on the model application domain and on modelled physical processes enable to write the evolution of the directional spectrum of wave action as followed:

$$N(k_x, k_y, x, y, t) = \frac{CCg}{2\pi\sigma} \tilde{N}(x, y, f_r, \Theta, t)$$
(B.2)

Cg and C are respectively the group velocity and the phase velocity of waves 672 and  $\sigma$  is the angular frequency. The equation expresses that, in the general case 673 of waves propagating in a non-homogeneous, unsteady environment (currents 674 and/or sea levels varying in time and space), the wave action is preserved to 675 within the source and sink terms (designated by the term Q)(Marcos, 2003). 676 Our study area is mainly located in the coastal zone, so we configured our 677 model for the coastal domain. For shallow areas, some physical processes must 678 be included and specified in our parameter file such as: 679

Bottom friction-induced dissipation (Qbf), with Qbf=0.038 m<sup>2</sup>.s<sup>-3</sup> which
 improves the estimation of wave growth in shallow water (Zijlema et al.,
 2012).

• Non-linear triad interactions between waves, considering the LTA (Lumped Triad Approximation) which improves the computation time.

Bathymetric breaking-induced dissipation (Qbr) where we have chosen the
 Battjes and Janssen's model (1978) after performing a sensitivity test be tween the 4 proposed models in TOMAWAC (Battjes and Janssen, Thorn ton and Guya, Roelvink, and Izumiya and Horikawa).

The TOMAWAC wave model was first run independently. In the absence of 689 wave observations before the 1970s, the model was validated from another wave 690 model (WaveWatchIII). WaveWatchIII has been implemented on the MANGAS 691 grid (created by Michaud et al. (2015)), which is an unstructured grid with a 692 resolution ranging from 10 km at the open boundaries of the domain to about 693 200 m resolution at the coast. The mesh is based on the HOMONIM digital 694 elevation model (DEM) developed by SHOM as part of the HOMONIM project 695 (Biscara et al., 2014). In order to compare the same bathymetry, we created 696 a new grid based on the HOMONIM DEM and we simulated the two offshore 697 waves configuration (Table 3). Then, results obtained with TOMAWAC are 698 compared to those of Wavewatch with similar wave conditions (Table 6). 699

For westerly and northeasterly waves, Hs obtained with WaveWatchIII and 700 TOMAWAC were extracted at each node (Figure 11a-b). The distribution of Hs 701 was compared using Q-Q plot. Regression coefficient obtained are very close to 1 702 and error terms are close to 0, which confirms the model's ability to reproduce Hs 703 of WW3 (Figure 11c-d). Largest differences are observed for Hs between 0 and 1 704 m. These differences are mainly due to the fact that WW3 considers additional 705 forcing, such as wind, tide and currents in contrast to TOMAWAC. Overall, we 706 obtain for both simulations an RMSE of less than 0.15 m corresponding to an 707 error of less than 6% and an  $\mathbb{R}^2$  of 0.96-0.97 which is statistically significant 708 (Figure 11c-d). 709

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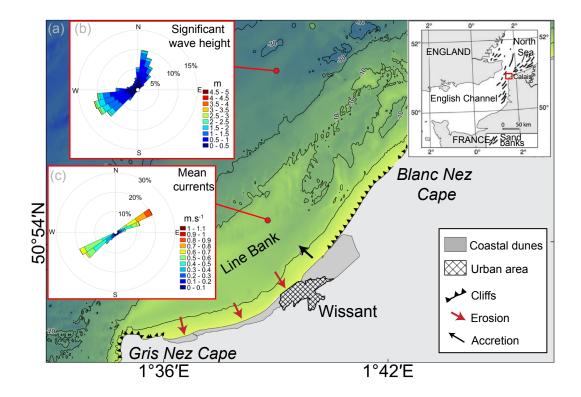


Figure 1: Wissant bay in the southern North Sea facing the Dover Strait (inset map) (a) Bathymetry from HOMONIM Digital Elevation Model (http://dx.doi.org/10.17183/MNT\_ATL100m\_HOMONIM\_WGS84) and synthesis of Wissant bay's shoreline evolution (Chaverot, 2006; Crapoulet, 2015), (b) Wave height from the HOMERE database (point E164N5094) and (c) direction and intensity of mean currents during the campaign between 05/17/2016 and 06/07/2016 (DDTM62)

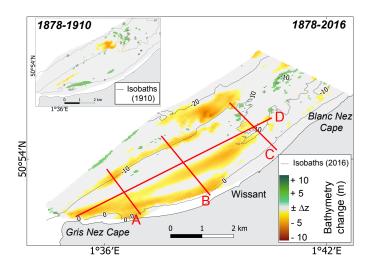


Figure 2: Bathymetric differential map of the Wissant bay between 1878 and 2016 and between 1878 and 1910 (inset map). Red-orange pattern indicates a seabed lowering; reciprocally blue-green areas correspond to vertical accretion; areas in grey correspond to the margin error  $(\pm \Delta z)$  and are considered as stable over the study period. Transects A, B, C and D correspond to the location of bathymetric profiles in Figure 3.

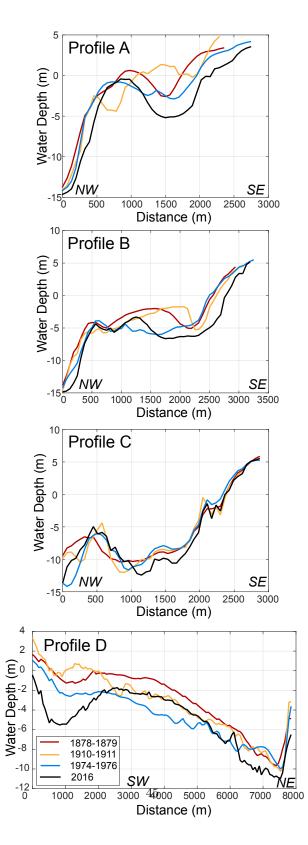


Figure 3: Bathymetry profiles evolution across the Wissant bay and the Line Bank from 1878 to 2016, (a-c) are cross-shore profiles and (d) is long-shore profile. See Figure 2 for profile location

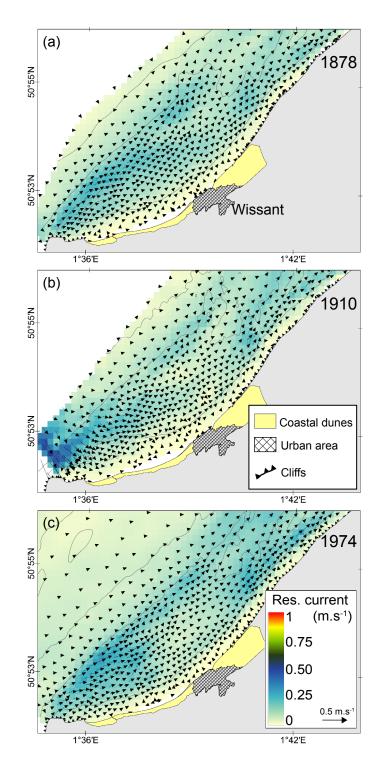


Figure 4: Simulations of changes in tidal residual current over 1878 (a), 1910 (b), and 1974 (c) bathymetries considering only the effect of tides. 46

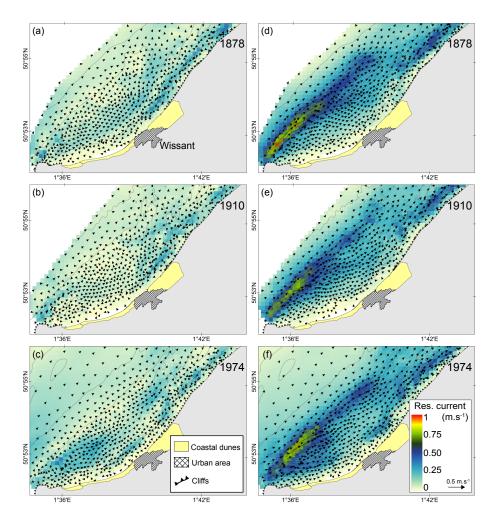


Figure 5: Simulations of changes in tidal residual current over 1878, 1910, and 1974 bathymetries considering the coupling effect of tides and waves: (a),(b), and (c) with north-easterly off-shore waves and (d), (e), and (f) with westerly off-shore waves

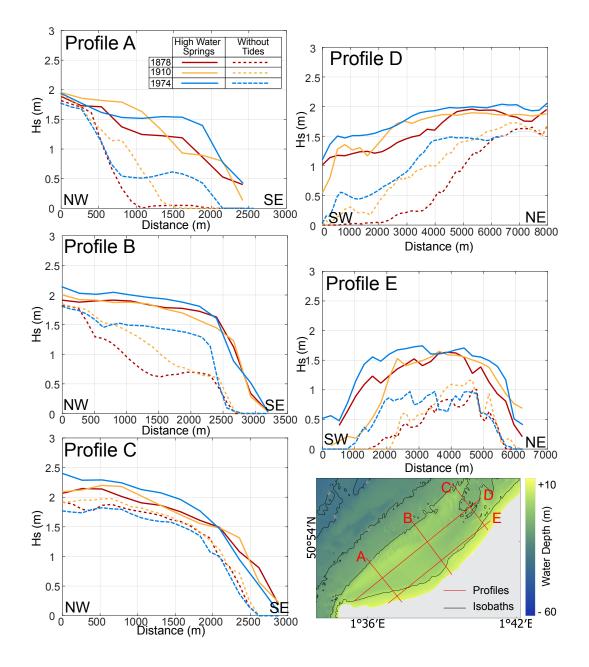


Figure 6: Significant wave height transects from the 1878, 1910 and 1974 models with north-easterly waves with and without considering tides

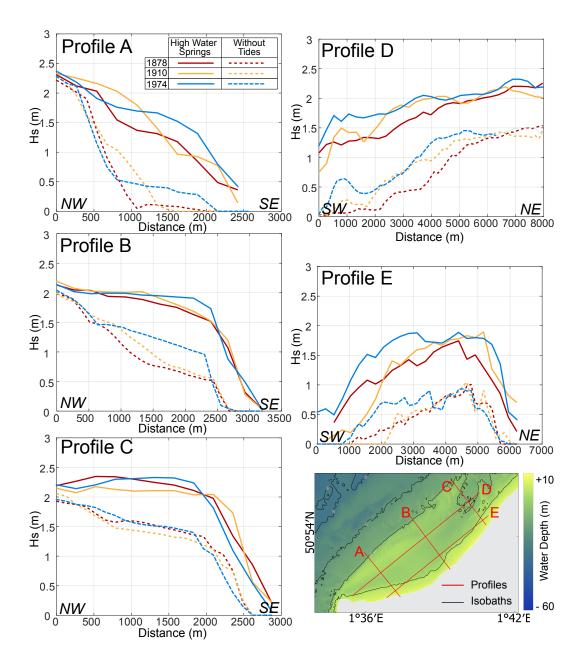


Figure 7: Significant wave height transects from the  $1878,\,1910$  and 1974 models with westerly waves with and without considering tides

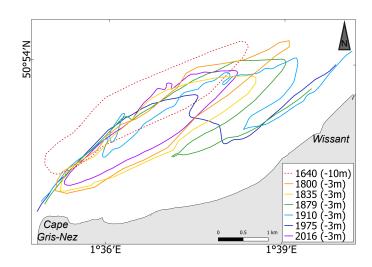


Figure 8: Evolution of the Line bank position and shape since 1640, adapted from the work of Briquet (1930)and completed with data digitized through this study. The -3m isobath was used as a reference for the position of the Line bank except for 1640 where the -10m isobath was chosen

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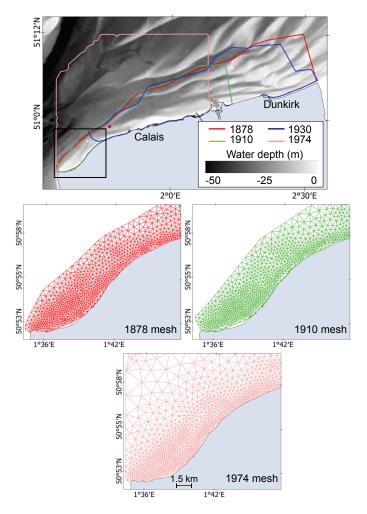


Figure 9: Above: Spatial extent of the 4 models used in this study on the HOMONIM Digital Elevation Model (http://dx.doi.org/10.17183/MNT\_ATL100m\_HOMONIM\_WGS84), the black box indicates the location of the Wissant bay and the red dot the position of the currentmeter used for current validation in the Figure 10. Bottom: zoom on meshes covering Wissant Bay (1878, 1910, 1974)

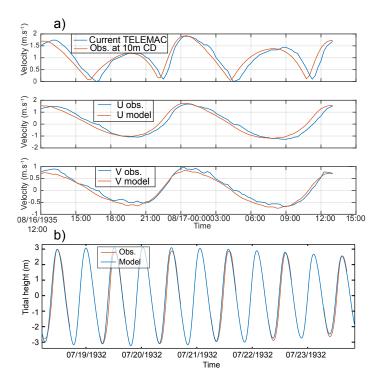


Figure 10: a) Tidal current observation made in 1935 and obtained with the 1930 model, offshore of Calais, at 10 m below Chart Datum (position of the currentmeter is indicated on the Figure 9). b) Comparison between tidal height observation at Calais in 1932 and tidal height obtained with the 1930 model at the same dates.

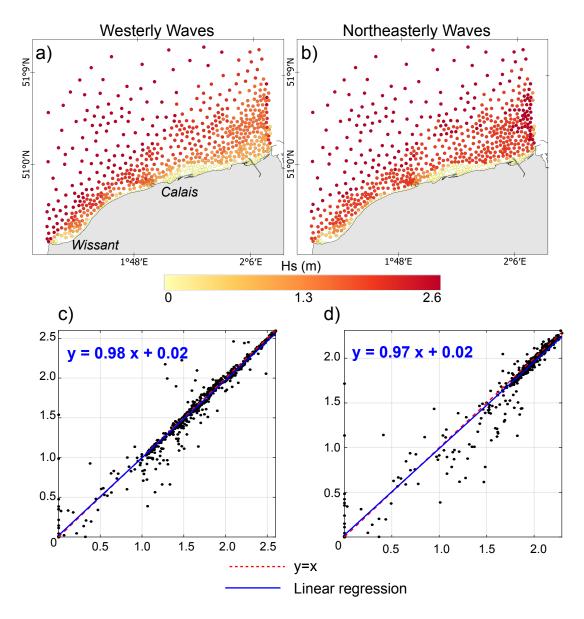


Figure 11: Wave height (Hs) obtained with the HOMONIM DEM with Tomawac for westerly (a) and northeasterly (b) off-shore waves. Q-Q plot comparing at each node Hs obtained with Tomawac and WaveWatch3 for westerly (c) et northeasterly waves (d). For both Q-Q plot linear regression (in blue) is plotted

Year	1878-79	1910 - 11	1974 - 76	2016
Surface $(km^2)$	691	400	1025	41
MHE (m)	15.5	15.5	10	3
MVE (m)	2.0	2.0	1.2	0.6

Table 1: Bathymetry surveys used in this study (source: SHOM, except for 2016 made by DDTM). MHE: Maximum Horizontal Error; MVE: Maximum Vertical Error. These first three bathymetry surveys were used for numerical modelling

Dates	Nodes	Finite elements
1878	8,058	15,698
1910	$5,\!690$	10,961
1974	4,408	8,559

Table 2: Computational mesh characteristics. Note that the year of each grid corresponds to the first year of the bathymetry surveys

NNE waves (Dir=N10)	WSW waves (Dir=N250)
$\mathrm{Hs99\%} = 2.25\mathrm{m}$	$\mathrm{Hs99\%} = 2.62\mathrm{m}$
Fpeak99%= $0,1364$ Hz	$Fpeak99\%{=}0{,}1336\mathrm{Hz}$
(Tpeak=7.33s)	(Tpeak=7.33s)

Table 3: 99th percentile of Hs and fpeak for WSW and NNE waves calculated from the HOMERE database (http://doi.org/10.12770/cf47e08d-1455-4254-955e-d66225c9dc90)

	1878-1910	1910-1974	1878-1974
Western foreshore	$\mathrm{Hs}\downarrow$	$\mathrm{Hs}\uparrow$	Overall increase
Central foreshore	$\mathrm{Hs}\uparrow$	$\mathrm{Hs}\uparrow$	Overall increase
Eastern foreshore	Hs $\downarrow$ or remained stable	Hs $\downarrow$ or remained stable	Stable
Line bank	Hs $\uparrow$ or remained stable	$\mathrm{Hs}\uparrow$	Increase
Coastal channel	Hs $\downarrow$ or remained stable	$\mathrm{Hs}\uparrow$	Increase

Table 4: Synthesis of wave height evolution depending on the location in the bay and the period considered

		1878	1910	1930	1974
Calais	Obs	0.87	0.92	0.92	0.83
$\mathbf{R}^2$	005	0.67	0.51	0.51	0.79
RMSE	Pred	0.92	0.92	0.93	0.88
RMSE	- Pred	0.50	0.54	0.48	0.68
Dunkirk	Obs	0.91		0.88	
$\mathbf{R}^2$	Obs	0.43	-	0.54	-
RMSE	Pred	0.92		0.88	
nmse	lited	0.43	_	0.55	_

Table 5: Statistic computed ( $R^2$  and RMSE) by comparing simulated tidal height at Calais and Dunkirk from each model with observations and predictions

NNE waves WW3	WSW waves WW3
$(Dir=10^{\circ}N)$	$(Dir=230^{\circ}N)$
21/03/2015 22:00	22/02/2015 23:00
Hs=2.2 m	Hs = 2.5 m
Fpeak99%=0,14 Hz	Fpeak99%=0,14Hz
(Tpeak=7.14s)	(Tpeak=7.14s)

Table 6: WaveWatch3 outputs used to validate TOMAWAC runs