



**HAL**  
open science

# Simulating Hydraulic Fracturing: Failure in Soft Versus Hard Rocks

Janis Aleksans, Daniel Koehn, Renaud Toussaint, G. Daniel

► **To cite this version:**

Janis Aleksans, Daniel Koehn, Renaud Toussaint, G. Daniel. Simulating Hydraulic Fracturing: Failure in Soft Versus Hard Rocks. *Pure and Applied Geophysics*, inPress, 10.1007/s00024-019-02376-0 . hal-02388745

**HAL Id: hal-02388745**

**<https://hal.science/hal-02388745>**

Submitted on 21 Jan 2020

**HAL** is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.

# SIMULATING HYDRAULIC FRACTURING: FAILURE IN SOFT VERSUS HARD ROCKS

J. Aleksans<sup>1,2,3</sup>, D. Koehn<sup>1,4</sup>, R. Toussaint<sup>5,6</sup>, G. Daniel<sup>7</sup>

<sup>1</sup> School of Geographical and Earth Sciences, University of Glasgow, Glasgow UK.

<sup>2</sup> Fault Analysis Group, UCD School of Earth Sciences, University College Dublin, Belfield, Dublin 4, Ireland

<sup>3</sup> iCRAG (Irish Centre for Research in Applied Geosciences), UCD School of Earth Sciences, University College Dublin, Belfield, Dublin 4, Ireland

<sup>4</sup> GeoZentrum Nordbayern, University of Erlangen-Nuremberg, Schlossgarten 5, 91054, Erlangen, Germany

<sup>5</sup> Institut de Physique du Globe de Strasbourg, UMR 7516, Université de Strasbourg/EOST, CNRS Strasbourg Cedex France.

<sup>6</sup> PoreLab, Department of Physics, University of Oslo, Oslo, Norway.

<sup>7</sup> EDF, DI, Paris, France.

Corresponding author: Janis Aleksans (email: [janis.aleksans@ucd.ie](mailto:janis.aleksans@ucd.ie); [j.aleksans@gmail.com](mailto:j.aleksans@gmail.com); telephone: +447591985220)

ORCID Janis Aleksans: 0000-0002-8233-0016

ORCID Daniel Koehn: 0000-0002-2276-2626

Key words: hydrofracturing, numerical modelling, Young's modulus, microseismicity

## Abstract

In this contribution we discuss the dynamic development of hydraulic fractures, their evolution and the resulting seismicity during fluid injection in a coupled numerical model. The model describes coupling between a solid that can fracture dynamically and a compressible fluid that can push back at the rock and open fractures. With a series of numerical simulations we show how the fracture pattern and seismicity change depending on changes in depth, injection rate, Young's Modulus and breaking strength. Our simulations indicate that the Young's Modulus has the largest influence on the fracture dynamics and the related seismicity. Simulations of rocks with a Young's modulus smaller than 10 GPa show dominant mode I failure and a growth of fracture aperture with a decrease in Young's modulus. Simulations of rocks with a Young's modulus higher than 10 GPa show fractures with a constant aperture and fracture growth that is mainly governed by a growth in crack length and an increasing amount of mode II failure. These results are very important for the prediction of fracture dynamics and seismicity during fluid injection, especially since we see a transition from one failure regime to another at around 10 GPa, a Young's modulus that lies in the middle of possible values for natural shale rocks.

## Acknowledgments

We would like to extend our gratitude to Natural Environment Research Council (NERC) and especially to NERC Center for Doctoral Training in Oil & Gas for provided funding of the project within which this research has been carried out.

## 1 Introduction

Hydraulic fracturing is the process of creating cracks within a rock volume by increasing the fluid pressure until the rock fractures (Hubbert & Willis, 1959; Nordgren, 1972; Valko & Ecomides, 1995). It plays an important role in the geophysical, geomechanical and structural mechanics of the Earth's crust in a wide variety of geological settings (Fyfe, 2012) and has a growing interest in geotechnical and industrial applications (Baria et al., 1999; Urbancic et al., 1999; Warpinski et al., 1999). A hydrofracture develops if the fluid overpressure exceeds the sum of the least compressive stress and the tensile strength of the rock (Cobbold & Rodrigues, 2007). During industrial hydrofracturing large quantities of fluid are pumped into a finite pore volume generating an overpressure and an outward push, which will lead to brittle failure and the development of fractures (Pearson, 1981; Zoback & Harjes, 1997; Niebling et al., 2012).

Fractures typically grow once the critical stress intensity factor is reached for materials (Inglis, 1913; Griffith, 1921, Irwin 1953). The stress intensity factor can be viewed as the prefactor of the stress singularity around a crack tip in an elastic rock, which is dependent on the crack length and its opening width or aperture (Anderson, 2005). At this point the rock properties such as the Young's modulus and Poisson ratio become important because they will influence the elastic response of the rock to the stress around the crack (Ohta et al., 1992; Tandaiya et al., 2008). Especially the Young's modulus is very variable depending on the rock type. It can vary from around 6 GPa for clay minerals (Prasad et al., 2002), to up to 8 GPa to 50 GPa for shale (Sayers, 2013) and up to 70 GPa for limestones (Sachpazis, 1990). The Young's modulus is the main component, which defines the compressibility of the material and thus has a major role in controlling the shape of the fracture. If the rock is very soft the crack will become elliptical with a large aperture, whereas it remains thin when the rock is hard.

Fracturing usually occurs in extensional or shear mode depending on the setting and balance between the differential stress  $\tau = \sigma^1 - \sigma^3$  as well as the least compressive stress in a rock if it becomes tensile (Cobbold & Rodrigues, 2007). In most cases in the Earth's crust, a triaxial stress distribution due to the gravitational loading prompts shear opening and for tensile fractures to develop an additional stress is required. This stress is thought to come from fluid overpressure that builds up in the pore space and pre-existing fractures in the rock (Sachau et al., 2015), or is increased during hydraulic pumping.

Stresses that build up in a fluid saturated rock must be divided into stresses that act on the solid and fluid and those that act only on the solid (Von Terzaghi, 1925). Stresses acting only on the solid are termed effective stress ( $\sigma'_{ij}$ ) and can be described by Terzhagi's law:

$$\sigma'_{ij} = \sigma_{ij} - P_f \delta_{ij} \quad (1)$$

where  $\sigma_{ij}$  is the stress tensor,  $P_f$  is fluid pressure and  $\delta_{ij}$  is Kronecker delta with positive sign conversion. The most common way to illustrate the 2D stress state within a rock is by using a Mohr diagram plotting shear against normal effective stress, with  $\sigma^1$  and  $\sigma^3$  at the right hand side of the diagram in the compressive state.  $P_f$  reduces the value of the effective stress and

1 pushes the Mohr circle towards the left hand side. Once the circle transects the Mohr-  
2 Coloumb failure envelope in the tensile regime, the rock will fail (Fig. 1).

3 However, this representation is an oversimplification and Cobbold and Rodriguez (2007) and  
4 Ghani et al. (2015) have shown with experiments and simulations that an increase in fluid  
5 overpressure in a simple sedimentary basin will lead to a local decrease of the differential  
6 stress of the effective stress tensor due to seepage forces. In this case the fluid overpressure  
7 has an anisotropic effect on the effective stresses. The Mohr circle for the effective stress  
8 state as such is still moving towards the left hand side into the regime of tensile failure,  
9 however the circle is shrinking at the same time.

10 Hydrofracturing is a complex interaction process and consists of at least a two way feedback  
11 between the fluid and the solid. Typically rocks are porous and thus the fluid overpressure  
12 can diffuse into the rock. According to Biot's poroelastic theory (Detournay & Cheng, 1993)  
13 this pressure diffusion influences the effective stress, since the fluid overpressure is reduced.  
14 Ghani et al. (2013) have shown that the feedback between fluid and solid leads to an initial  
15 build-up of fluid overpressure and diffusion of fluid into the rock, an increase in pressure  
16 gradient and final fracturing. The fractures in turn enhance the permeability and allow the  
17 fluid overpressure to be reduced. Existing fractures may even be pushed open by the  
18 developing fluid pressure gradients leading to a more dynamic system of overpressure release  
19 (Sachau et al., 2015). It is important to understand the full scope of this dynamic feedback in  
20 order to understand hydrofracture dynamics.

21  
22 [Figure 1]

23 **Fig. 1** a) Standard Mohr circle diagram of effective stress showing how an increase in fluid pressure  
24 shifts the Mohr circle from its initial compressive stress state at the right hand side to the left into the  
25 tensile regime leading to failure; Note that the differential stress remains constant. b) Mohr circle  
26 diagram of effective stress in the model illustrating the anisotropy of stress changes when fluid  
27 overpressure or pressure gradients are taken into account. Locally the system will fluidize, the  
28 differential stress goes to zero or becomes very small leading to mainly mode I failure

29  
30 When studying active hydrofractures in the Earth's crust, microseismic activity has been used  
31 to map the fracture's height, extent, orientation as well as magnitude of the fracturing event  
32 in order to monitor fractures during geothermal energy extraction (Baria et al., 1999),  
33 reservoir stimulation (Urbancic et al., 1999), and waste reinjection (Warpinski et al., 1999).  
34 Laboratory experiments of acoustic emission observation during hydrofracturing (Scott et al.,  
35 2000; Groenenboom & van Dam, 2000) as well as modelling of such emissions (Guest and  
36 Settari, 2010; Murphy et al., 2013; Rutqvist et al., 2013) can also be used to better understand  
37 the fracturing process and its aspects such as direction of fracture propagation and fluid  
38 injection induced static stress perturbation. Inversion of seismic data allows to determine the  
39 moment tensor during hydrofracturing and thus the movement of the fracture walls relative to  
40 each other (Hazzard & Young, 2002; Guest & Settari, 2010; Vavryčuk, 2011). It is of vital  
41 importance to understand how different rock types break during hydrofracturing, what  
42 seismic signals are emitted during this process and in turn how microseismicity can be used  
43 to monitor fluid movement and hydrofracturing.

1 In order to understand the solid-fluid feedback and associated seismic signals during  
 2 hydrofracturing in a variety of rock types we are using a numerical model (Ghani et al., 2013,  
 3 2015) to analyse seismic aspects associated with fracture opening, mode of fracturing as well  
 4 as magnitude of fracture events under variable extrinsic (overburden load, fluid flux) as well  
 5 as intrinsic (Young's Modulus, breaking strength) parameters. A more detailed account of the  
 6 initial model design can be gained from Ghani et al. (2013, 2015). In this work we equipped  
 7 the given model with a code module designed to track individual particles and thus analyse  
 8 dynamics of the fracture opening and associated seismicity.

## 10 2 Methodology

11 In order to numerically simulate the development of hydrofractures we employ a discrete  
 12 element model (Latte), which is built upon a 2D triangular setup coupled with a square grid  
 13 representing the fluid (Fig. 2) and is part of the "Elle" package (Bons et al., 2007).

15 [Figure 2]

17 **Fig. 2** Schematic illustration of the DEM (discrete element model) grid overlying the fluid pressure  
 18 nodes. The DEM grid is triangular whereas the fluid grid is square and larger than the DEM. Both  
 19 grids are mapped onto each other using tent functions to minimize grid effects. The fluid grid is  
 20 stationary whereas the DEM grid can move

23 In the model the triangular network is constructed of disk-shaped particles of constant radius  
 24 interconnected by springs (discrete element model or DEM). Such model configuration in 2D  
 25 mimics the isotropic elastic behavior of solid materials and can be used to model deformation  
 26 problems in systems described by linear elastic theory (Flekkøy et al., 2002). The intrinsic  
 27 stiffness coefficient  $k$  is governed by the macro-scale parameters  $E$  and  $\nu$  (Young's modulus  
 28 and Poisson ratio) through the consistency measures of strain energy between the 2D elastic  
 29 lattice of the triangular network and solid continua (Flekkøy et al., 2002)

$$30 \quad k = \frac{\sqrt{3}}{2} El \quad (2)$$

32 where  $l$  stands for the thickness of the two-dimensional model. The model runs as a step  
 33 function and therefore plain strain deformation is produced for each time step  $\Delta t$ . At the end  
 34 of every time step all springs are checked and if the predetermined stress threshold for a  
 35 spring is reached, it breaks releasing elastic energy, the spring is removed from the lattice and  
 36 a fracture forms. The elastic energy contained in the removed spring is further redistributed  
 37 among the neighboring springs via the relaxation algorithm. The breakage occurs once either  
 38 the critical tensile normal stress  $\sigma_0$  or the critical shear stress  $\tau_0$  is reached. In order to include  
 39 combinations of tensile and shear failure it is assumed that the critical strain value  $E_c$  is seen

1 as a sum of tensile ( $U_t$ ) and shear ( $U_c$ ) energies (Sachau & Koehn, 2014). There are separate  
 2 critical values for both tensile ( $E_c, \sigma$ ) and shear ( $E_c, \tau$ ) criteria and their relationship can be  
 3 expressed as

$$\frac{U_s}{E_c} + \frac{U_t}{E_c} = \left(\frac{\sigma_n}{\sigma_0}\right)^2 + \left(\frac{\tau}{\tau_0}\right)^2 = 1 \quad (3)$$

4 which describes an ellipse in  $\sigma_n$ - $\tau$  space (Sun & Jin, 2012). Therefore, the failure can occur as  
 5 a combination of both shear and tensile factors, or if one of the two components is absent, it  
 6 will be equivalent to pure tensile or pure shear failure.

7 The 2D DEM lattice is overlain by a square grid of fluid pressure nodes. The fluid pressure  
 8 diffusion is derived from mass conservation of the fluid and solid using Darcy's law to  
 9 express the seepage velocity through the porous medium (Ghani et al., 2013, 2015)

$$\phi\beta(\partial_t P + u_s \nabla \cdot P) = \nabla \cdot \left[ (1 + \beta P) \frac{K}{\mu} \nabla P \right] - (1 + \beta P) \nabla \cdot u_s \quad (4)$$

10 where  $\phi$  is the porosity,  $\beta$  is the fluid compressibility,  $P$  is the fluid pressure deviation from  
 11 hydrostatic, and  $u_s$  is the solid velocity field.  $K$  and  $\mu$  stand for permeability and fluid  
 12 viscosity respectively. The left hand side of the equation is the Lagrangian derivative of pore  
 13 pressure following the solid matrix, the first term on the right expresses the Darcy fluid  
 14 pressure diffusion relative to the particles, the last term is a source term. The source term  
 15 expresses pressure change as a function of a change in the solid if particles move apart in the  
 16 local reference of the Darcy flow (Ghani et al., 2013). The Kozeny-Carman relation is used to  
 17 express  $K$  as a function of local porosity  $\phi$

$$K(\phi) = \frac{d^2(\phi)^3}{180(1 - \phi)^2} \quad (5)$$

18 where  $d$  is the particle diameter and  $1/180$  is an empirical constant for packing of spheres  
 19 (Carman, 1937; Ghani et al., 2013, 2015).

20 Deformation mechanics are driven by the momentum exchange between the two phases, solid  
 21 and fluid. The total force applied on the particles is compiled from three main constituents

$$F_n^i = f_e + f_p + f_g \quad (6)$$

22 where  $f_e$  is the interaction force between the particles (either due to a connection with the  
 23 spring or repulsive), fluid pressure force  $f_p$  and external force  $f_g$  applied due to the gravity

1 and large scale tectonic strain on the external boundaries. In this instance  $f_e$  is expressed as a  
 2 function of the spring elasticity constant  $k$  and an equilibrium distance between particles  $a$ ,  
 3 which equals the sum of the initial radii of the two connected particles

$$f_e = \sum_j k_{ij} (|\vec{a}_{ij}| - |\vec{x}_i - \vec{x}_j|) \cdot \hat{n}_{ij} \quad (7)$$

4 where  $x_i$  and  $x_j$  are the positions of the connected particles,  $k_{ij}$  is the elastic (spring) constant  
 5 of their contact,  $\hat{n}_{ij}$  is unit vector pointing from the centroid of particle  $i$  to particle  $j$  and the  
 6 sum runs over all the connected neighbours  $j$ . Once the change in relative particle positions  
 7 occurs, the resulting force can be decomposed into tensile and shear components. Further  
 8 details on this methodology can be found in work by Sachau & Koehn (2014).  
 9

10 The fluid force  $f_p$  that acts on the surface normal  $dA$  of the unit cell is a function of the fluid  
 11 flow due to the pressure gradient and is described as

$$f_p = - \int P_0 \hat{n} dA \quad (8)$$

12 where  $P_0 = P + \rho_f g z$  is the local total fluid pressure,  $z$  depth,  $\rho_f$  density of the fluid and  $g$   
 13 gravity constant. It is the sum of a term due to viscous forces arising in the case of fluid flow  
 14 through the solid,  $-\int P \hat{n} dA$ , and a buoyancy term,  $-\int \rho_f g z \hat{n} dA$ .  
 15

16 Lastly the gravity force that is acting on every particle is calculated according to

$$f_i^g = \rho_s \pi R_i^2 g C \quad (9)$$

17 where  $\rho_s$  denotes the solid mass density,  $R_i^2 = r_i^2 S$ , where  $S$  is the dimension of the real  
 18 system (1000m),  $g$  is the gravitational acceleration vector and  $C = 2/3$  is a scale factor (after  
 19 Ghani et al., 2015) used to acquire a compatible one dimensional lithostatic stress that can be  
 20 applied to an isotropic 2D linear elastic solid.  
 21

22 The buoyancy term itself can be written according to Ostrogradsky's theorem as  
 23  $-\int \rho_f g z \hat{n} dA = -\rho_f \pi R_i^2 g C$  (Mory, 2013), so that the total effect of gravity, the direct one  
 24 plus the buoyancy effect, is  $(\rho_s - \rho_f) \pi R_i^2 g C$ , which incorporates the effects that gravity has  
 25 on both solid and fluid: this drives the effective stress field  $\sigma' = (\rho_s - \rho_f) g z$  in the system in a  
 26 hydrostatic situation (Niebling et al., 2010a).  
 27

28 The solid porosity that is used for the fluid pressure evolution in the fluid lattice is described  
 29 by the solid mass fraction of solid particles within a fluid cell and changes due to  
 30 compression, solid movement or fracturing. The solid movement itself between two  
 31 deformation steps affects the source term in equation 4.  
 32  
 33  
 34  
 35  
 36  
 37  
 38  
 39  
 40  
 41  
 42  
 43  
 44  
 45  
 46  
 47  
 48  
 49  
 50  
 51  
 52  
 53  
 54  
 55  
 56  
 57  
 58  
 59  
 60  
 61  
 62  
 63  
 64  
 65

1 Finally, the model is set up in a way that the fluid continuum grid overlies the DEM so that  
 2 their boundaries coincide. Since the fluid grid is set to be twice as large as the DEM matrix,  
 3 the model uses the “cloud in the cell” method to facilitate the two-way interaction between  
 4 the porous matrix and the hydrodynamic phase (Ghani et al., 2013, Johnsen et al., 2006,  
 5 Vinningland et al., 2007, 2012). The interaction between the porous solid and hydrodynamic  
 6 phases is accomplished through the projection operator from the discrete space to the fluid  
 7 grid space by the help of the smoothing function  $s(r_i-r_0)$ , which distributes the weight of the  
 8 particle over the four nearest fluid grid nodes

$$9 \quad s(r - r_0) = \begin{cases} \left(1 - \frac{w_1}{\Delta x}\right) \left(1 - \frac{w_2}{\Delta z}\right) & \text{if } w_1 < \Delta x, w_2 < \Delta z \\ 0 & \text{otherwise} \end{cases} \quad (10)$$

11 where  $r(x,z)$  and  $r_o(x_o,z_o)$  are the positions of the particle and the continuum node  
 12 respectively,  $w_1 = |x - x_o|$  and  $w_2 = |z - z_o|$  are the relative distances.

13 Once a fracture occurs, it is possible to calculate the seismic moment of the event. To do this  
 14 we assume that the fracture plane lies perpendicular to the midpoint of the spring connecting  
 15 the two fracturing particles. The total amount of movement can be calculated and then  
 16 subdivided into  $u_t$  and  $u_s$  which represent pure tensile and pure shear movement respectively.  
 17 For simplicity we assume that the fracture area is equal to the sum of radii of particles broken  
 18 across the plane times  $l$ , the thickness of the two-dimensional model. These values allow us  
 19 to calculate the seismic moment ( $M_0$ ) according to

$$20 \quad M_0 = A * \mu * u \quad (11)$$

21 where  $A$  represents the area of the fracture,  $\mu$  is a shear modulus and  $u$  the total movement  
 22 (Aki & Richards, 2002). Further we can convert this value to fit the moment magnitude  $M_w$   
 23 (Hanks & Kanamori, 1979) scale using the equation

$$24 \quad M_w = \frac{2}{3} \log_{10}(M_0/1 J) - 10.7. \quad (12)$$

25 [Figure 3]

26 **Fig. 3** Calculation of the Moment Tensor after failure takes place (spring between particles breaks).  
 27 The figure shows the decomposition of total particle displacement ( $U_{tot}$ ) into shear ( $U_s$ ) and tensile  
 28 components ( $U_t$ ), which are respectively parallel and perpendicular to the imagined failure surface.  
 29 The failure surface lies perpendicularly to the broken elastic spring. Lastly, as we have both pure  
 30 tensile and pure shear movements, we introduce the concept of movement mode ratio which is  
 31 defined as  $U_s/U_t$ . In this case ratios smaller than 1 are dominated by extensional fracturing whereas  
 32 ratios larger than 1 become shear fracture dominated



The numerical models have been extensively used in the literature and have been shown to reproduce experimental and analytical solutions for fracturing (Niebling, 2012), roughness growth (Koehn et al., 2003, 2012) and solid fluid interactions in granular (Niebling et al., 2010a, 2010b) and solid media (Flekkøy et al., 2002, Ghani et al., 2013, 2015) as well as in landslides (Parez et al., 2016) or soil liquefaction (Clément et al., 2018; Zeev et al., 2017).

Since time steps in the model are discrete they had to be calibrated in order to avoid overstepping the breaking threshold by unreasonably large amounts, which would lead to the model stepping controlling the behavior. The time step was decreased until changes in stress and magnitudes as well as fracturing behavior became independent of the model step size. Therefore, we assume that at these small time steps the model behaves quasi-continuously.

### 3 Results

#### 3.1 Parameters of interest

The model has the following parameters: it is assumed to simulate a system that is 1 km<sup>2</sup> in area represented by 400x400 particles. The sides and the bottom of the model box are represented by fixed walls that are frictionless. The upper boundary is open, and force is applied controlled as a function of gravity of the overlying rock units. Fluid is injected in one fluid cell at the centre of the lower part of the model (Fig. 4). Time steps in the model are 30 seconds each and the models typically run 10'000 time steps. The fluid has a density of 1000 kg/m<sup>3</sup> and a viscosity of  $8.90 \times 10^{-4}$  Pa·s, which are the properties of water at temperature of 25°C. Variables that were changed include depth (overload varying from 1 to 3 km with a density of overlying sediments of  $2.5 \times 10^3$  kg/m<sup>3</sup>), initial porosity, fluid injection rates, Young's modulus and brittleness (breaking strength and its distribution) of the rock. The aim was to study how these changes affect the tensile/shear movement ratio distribution, the magnitude and the fracture pattern. A setting with 3 km depth, 0.33 Poisson's ratio, 6 GPa Young's modulus, ~1% porosity, 34 MPa tensile strength and 80000Pa/30s injection rate was our default setting. This setting was chosen because the mechanical properties of the rock are close to those of a typical shale, yet the increased depth guaranteed an increased principal stress and thus a larger amount of fracturing, which provides more data. Variations of these settings were performed in order to study the effects of the different variables on the hydrofracturing (Table 1).

	Depth (m)	Young's Moduli (GPa)	Poisson's Ratio	Porosity	Injection rate	Tensile Strength (MPa)	No. of simulations conducted
Default	3000	6	0.33	~1%	80000Pa/30s	34	10x10000 time steps (each setting)
Young's Modulus		4.5; 6; 7; 8; 9; 10; 12.5; 15; 20; 30 ;40			50000Pa/30s		
Lower injection rate							
Shallower depth	1000	27.2; 34; 44.2; 54.4					
Variable tensile strength	3000						

**Table 1** The default setting for the hydrofracturing experiments was set to 3 km depth, 0.3 Poisson's ratio, 6 GPa Young's modulus, ~1% porosity, 34 MPa tensile strength and 8000Pa/30s injection. During the experiments the Young's modulus was varied in addition to the depth, injection rate and tensile strength of the rock

[Figure 4]

**Fig. 4** The setup of the numerical model is 2D, bounded by fixed walls on the right and left hand sides and the bottom while a stress corresponding to the weight of the overburden is applied from the top. Fluid is injected at the centre of the lower part of the model

### 3.2 Hydrofracture evolution in single events

The model follows a similar behaviour and fracturing pattern in all presented cases that show hydrofracturing. In cases where the injection rate is too low or the porosity too high the fluid just seeps away and no fractures develop. Hydrofracturing typically takes place in four distinct stages: 1) build up of stress, 2) fracturing, 3) residual fracturing, 4) seeping (Fig. 5). In the first stage the fluid is injected at the centre of the model in several intervals. The fluid pressure diffuses into the pore space slower than the injection rate so that sufficient fluid pressure gradients build up that lead to fracturing. Stage two resembles the active hydrofracturing where most of the seismic events can be observed. This stage typically lasts for about 2500-3000 model steps. During the third stage residual fracturing takes place. In this stage the overall porosity and permeability of the system have increased enough due to fracturing and fracture opening for the fluid to seep away. At this point it takes quite long to build up enough pressure to overcome the tensile breaking strength of the rock so that only minor fracturing events take place. In the fourth and final stage the system reaches a steady state where porosity and permeability have increased enough for the fluid to seep away and no additional fractures develop.

[Figure 5]

**Fig. 5** a) The model shows four stages of fracture evolution: 1) build up of stress, 2) fracturing, 3) residual fracturing, 4) seeping. Timing and intensity of the fracturing depends upon the parameters, for the default setting time step intervals for these stages are 1) 0-1000, 2) 1000-4000, 3) 4000-7500, 4) >7500. Changing the parameters of the model moderates the timing and intensity of the fracturing, however the overall pattern is preserved in all cases. b) Figure shows changes in the vertical fluid pressure gradient averaged over the entire system. The model starting parameter is hydrostatic pressure. 10 KPa value is calculated as a difference between top and bottom boundary of the simulation box, which is 3 and 4 km. In the default setting the fluid pressure gradient is less susceptible to changes due to a low initial porosity and relatively high fluid injection rate. Once the

1 injection rate is lowered or seepage is increased via increase in porosity, it becomes more sensitive to  
 2 further overall porosity increase through fracturing

3 Looking at the vertical component of the fluid pressure gradients averaged over the entire  
 4 system, one can observe (Fig. 5b) that in the default case the fluid pressure gradient changes  
 5 coincide with fracturing. In this case once fracturing occurs the overall porosity increases and  
 6 seepage intensifies, thus decreasing the gradient. In the two other cases shown in figure 5 the  
 7 gradient is lower from the beginning of the experiment on as the model was run with a) a  
 8 lower injection rate in the experiment and b) a shallower depth, where there is lower  
 9 compaction, which allows for greater porosity and thus greater seepage. In the latter two  
 10 cases the gradient is more susceptible to change as its slope decreases just as the fracturing is  
 11 initiated. All three cases shown in figure 5b illustrate a decrease in fluid pressure gradient  
 12 growth from the initial gradient produced by the injection rate to the final gradient in the  
 13 system once steady state is reached, the rock has fractured and the fluid is seeping.

14  
 15 [Figure 6]

16  
 17  
 18 **Fig. 6 a)** Figure shows the moment magnitude of the fracturing events in a model versus the mode of  
 19 fracturing in a default setup. All results produce an observable trend of an initial moment magnitude  
 20 increase as fracturing becomes more shear-like followed by a decrease of magnitude and increase of  
 21 shear component. This shows that there is no simple linear relationship between the amount of shear  
 22 in the fracture and the moment magnitude of the event. b) Figure illustrates the evolution of moment  
 23 magnitude values as a function of time for a model run in a default setup. The greatest moment  
 24 magnitude values can be observed during the phase of the most active fracturing (time-step 1000-  
 25 4000) and tend to decrease towards the later phases of the model evolution

26  
 27 Figure 6a shows the moment Magnitude of fracturing events in a simulation versus the mode  
 28 of fracturing ratio (shear divided by tensile). Most of the fracturing events in the simulations  
 29 were mode I dominated (mode of fracturing ratio < 1) with the mode II fracture proportion  
 30 increasing with the magnitude of the event (Fig 6a). However, this trend is not linear and it  
 31 can be seen that there are some mixed mode I – mode II events that have a very low  
 32 magnitude. Figure 6 further illustrates the bulk of the events record on the negative side of  
 33 the magnitude scale. The few events that do record on the positive side do not overstep a  
 34 magnitude of 0.5 and thus are very low. Figure 6b illustrates the evolution of moment  
 35 magnitude values as a function of time for a model run. Fracturing events with higher  
 36 magnitudes occur early at stages of most active fracturing and magnitudes decrease towards  
 37 the later stages of the fracture development.

### 38 39 3.3 Aseismic component

40

1 The given model is limited in the possibility to observe aseismic events due to logging only  
 2 the position of the particles at the end of each time step. Therefore, the dynamics of the  
 3 particle movement during the relaxation phase of the lattice are not observed or recorded.  
 4 There is however significant potential for aseismic activity. To illustrate this we recorded the  
 5 average amount of movement of particles located along the fracture walls during the time  
 6 steps when no active fracturing takes place (Fig. 7). Figure 7 shows results from the default  
 7 model setup but identical trends are observable in all control parameter configurations. The  
 8 amount of movement is an order of magnitude lower than that during time steps when bond  
 9 breakage occurs, however it follows the same pattern where there is greater increase in values  
 10 in the initial stages of fracturing and subsequent decline in the later stages. This would be  
 11 indicative of the hypothesis that large amounts of energy are being released aseismically  
 12 alongside the energy released during active fracturing and may not be directly observed.

13  
 14 [Figure 7]

15  
 16 **Fig. 7** Average displacement of particles during the time steps when active fracturing (seismic) takes  
 17 place versus average displacement of all the particles across the fracture walls during time steps when  
 18 no active fracturing is occurring (aseismic)

### 19 20 21 22 3.4 Variation of parameters

23  
24 Each of the following results is represented by stacked data points obtained from up to ten  
 25 consecutive runs of the model with the same parameters. This is done in order to better  
 26 illustrate the outcome and to minimize the uncertainties of the results. The following results  
 27 are shown using the logistic distribution which is similar to the normal distribution with a  
 28 more convenient choice of parameters where mode and mean is described by the same  
 29 parameter. The logistic distribution is also more efficient in dealing with datasets that have  
 30 greater spread of values. Histograms in figure 8 show the percentage of the total data points  
 31 falling into every binning interval. Logistic probability density curve (PDF) is then calculated  
 32 for the given result distribution. Complete data in figure 9 is represented using only logistic  
 33 PDFs in order to avoid clutter. In the first set of experiments the Young's modulus (E) was  
 34 varied from 4.5 GPa to 6, 7, 8, 9, 10, 12.5, 15, 20, 30 and 40 GPa. In order to illustrate the  
 35 model behavior we studied 4 parameters, the moment magnitude release during model runs  
 36 (Fig. 9a), the distribution of fracture wall displacement or aperture (Fig. 9b), the distribution  
 37 of the amount of broken bonds for single fracture events (Fig. 9c) and the distribution of  
 38 movement mode ratios in model runs (mode I versus mode II, Fig. 9d). The plots on the right  
 39 hand side of the figure show the evolution distributions' modes/means when changing  
 40 Young's modulus from soft rock setting through to hard rock setting.

41 [Figure 8]

1 **Fig. 8** Displacement data for experiments with  $E=4.5$  GPa (left) and  $E=30.0$  GPa is binned according  
 2 to the percentage of the total data falling within each of the intervals. Logistic distribution probability  
 3 density functions are then calculated for data point distributions

4  
 5  
 6 [Figure 9]

7  
 8 **Fig. 9** Logistic distribution curves on the left hand side and means on the right hand side as a function  
 9 of changes in Young's Modulus for the simulations after 10000 time steps. Soft, intermediate and  
 10 hard rocks are indicated. A) Moment magnitude variation shows a decrease from soft to intermediate  
 11 rock followed by an increase towards the hardest rocks. B) Displacement or aperture of fracture  
 12 shows a clear trend from a uniform very thin crack for hard rocks followed by a non-linear increase of  
 13 the displacement or aperture towards soft rocks. C) The area of the fracture or amount of broken  
 14 bonds shows a relatively stable value for soft to intermediate rocks and then an increase in area  
 15 towards hard rocks. D) The movement mode ratio or the amount of extensional versus shear fracture  
 16 shows an increase in shear fracturing towards harder rock whereas intermediate and soft rocks show  
 17 dominant extensional fracturing

18  
 19  
 20  
 21  
 22  
 23 The moment magnitude (Fig. 9a) changes in a complicated, non-linear manner as a function  
 24 of changes in the Young's Modulus. From 4.5 to 10 GPa the moment magnitude decreases  
 25 followed by an increase towards higher Young's modulus values. The average displacement  
 26 of the fractures shows clearly that the displacement median/mode is becoming smaller with  
 27 an increase of the Young's Modulus until 12.5 GPa from where it stays constant (Fig. 9b).  
 28 The overall distribution becomes smaller with increasing Young's modulus leading to a  
 29 constant displacement at high values. The fracture area shows a minor initial decrease from  
 30 4.5 GPa to 10 GPa and then an increase in area (Fig. 9c). The increase in fracture area is most  
 31 pronounced from 20 to 30 and 40 GPa. Simulations with the highest elastic constants seem to  
 32 have events where a larger amount of bonds break accounting for longer fractures and thus a  
 33 larger fracture area. A change of the Young's modulus has a strong influence on the fracture  
 34 mode ratio during the simulations (Fig. 9d). At low Young's modulus (from 4.5 to 10 GPa)  
 35 the fracture mode is mainly extensional with a relatively narrow distribution whereas higher  
 36 Young's moduli (12.5 to 40 GPa) show a much higher amount of shear fractures in addition  
 37 to mixed extensional/shear failure. The transition between these two regimes is very rapid  
 38 between the values of 10 and 12.5 GPa followed by unchanging behaviour at greater values.  
 39 The mean value of the logistic distribution is almost constant, around 1 to 3 (0.3-0.5) ratio of  
 40 shear versus tensile movement, for the lower Young's moduli, and becomes constant again,  
 41 around 1 to 1 (1.0) ratio, once the fracture mode changes at higher Young's moduli. In  
 42 summary an increase in Young's modulus has a strong influence on the mode of fracturing  
 43 changing from extensional mode dominated at Young's moduli below 10 GPa to mixed mode  
 44 with a higher amount of shear fractures at higher values. The behaviour of the moment  
 45 magnitude as a function of Young's modulus is complex. Fracture wall displacement or  
 46 aperture of the fractures decreases with an increase of Young's modulus and becomes  
 47 stationary at high values ( $>20$  GPa). The amount of broken bonds or the fracture area is not  
 48 changing much for small Young's modulus values but increases from values of 20 GPa and  
 49 higher.

1  
2 To see how the change in overburden pressure would influence the fracturing process, the  
3 depth of the simulation was changed from 3 km to 1 km and the Young's modulus was varied  
4 between the simulations in a similar fashion to the default case. Reducing the overburden  
5 produced a significant decrease in fracturing in general, so that sometimes the results yielded  
6 as much as ten times less fractures than at 3 km. The lower overburden stress led to an  
7 increased porosity, which also meant that the fluid pressure gradients were lower and less  
8 fractures developed (Fig. 5b). This resulted in a smaller amount of data points and thus more  
9 noise in the data. However, when analysing the fracture displacement or aperture data, a very  
10 similar trend to the 3 km cases can be observed where there is a rapid decrease of wall  
11 displacement with an increase in Young's modulus followed by a steady aperture with the  
12 turning point at around 15 GPa (Fig 10).

13 The other parameters (moment magnitude, mode ratio, fracture area), when analysed,  
14 produced a high amount of noise in the results and thus showed very high uncertainty and no  
15 clear relation with changes in Young's modulus.

16  
17 In a third set of simulations we changed the Young's modulus in simulations with a lower  
18 injection rate of 50000Pa/30s.

19  
20  
21  
22  
23  
24  
25  
26  
27 [Figure 10]

28  
29 **Fig. 10** Logistic distribution curves showing average fracture wall displacements for a series of  
30 Young's moduli at depth of 1 km. The behaviour is very similar to the 3 km deep reference case  
31  
32

33  
34 Changing the injection rate from 80000Pa/30s to 50000Pa/30s did not change the behaviour  
35 of the simulations much. Both displacement and area change trends showed the same pattern  
36 that was observed at higher injection rate with a change in behaviour at 10 GPa. The moment  
37 magnitude pattern was almost identical in both cases with differing injection rates. The only  
38 significant change was the movement mode pattern where the behaviour changes at 9 GPa  
39 rather than 10 GPa.  
40  
41

42  
43  
44  
45 [Figure 11]

46  
47 **Fig. 11** The figure shows the amount of bonds broken during an experiment for a series of Young's  
48 moduli normalized by the tensile breaking strength of the rock. Increasing Young's modulus and  
49 decreasing the breaking strength both lead to more fractures  
50  
51

52  
53  
54 In a final set of experiments we varied the breaking strength of the springs and compared the  
55 effect of breaking strength and Young's modulus variation on the overall amount of broken  
56 bonds in single simulations. Figure 11 shows the number of broken bonds as a function of  
57 Young's modulus divided by breaking strength. The different colours indicate runs with  
58 constant breaking strength but a variation in Young's modulus. Breaking strength and  
59 Young's modulus do not have the same effect on the amount of broken bonds. An increase in  
60  
61  
62  
63  
64  
65

1 Young's modulus shows a roughly linear increase in the amount of broken bonds. This  
 2 behaviour is irrespective of the breaking strength. A decrease in breaking strength increases  
 3 the amount of broken bonds, however in contrast to the Young's modulus this trend is not  
 4 linear. At a breaking strength of 44.2 and 54.4 MPa the amount of fractures is equally low,  
 5 whereas at a low breaking strength of 27.2 MPa the amount of fracturing has increased 5  
 6 times.

#### 8 4 Discussion

9 Our simulations indicate that a change in the Young's modulus has a pronounced effect on  
 10 the fracture behaviour. At a lower Young's modulus the fracture mode is dominated by  
 11 extension, the amounts of broken bonds is low and the fracture wall movement or aperture is  
 12 large. At higher Young's moduli the fracture mode becomes mixed with a larger amount of  
 13 shear fractures, the fracture aperture becomes constant while the amount of broken bonds and  
 14 thus the fracture area increase. The fracture behaviour changes quite abruptly at around 10  
 15 GPa. Below 10 GPa the rock is quite soft and forms elliptical shaped fractures which almost  
 16 "inflate" and do not really propagate. Above 10 GPa the rock is tougher, thus it is much more  
 17 prone to tensile movement and stress is relieved through propagation of fractures and thus  
 18 more shear like movement.

19 To explain this behaviour we look at the effective stress increase at the tip of a mode I  
 20 fracture. The effective stress at the crack tip  $\sigma'$ , is a function of  $\sigma_0$ , the external stress (as a  
 21 function of the fluid pressure gradient), length  $L$  of the crack and the radius of curvature  $r$  at  
 22 the crack tip (Irwin, 1957)

$$\sigma' = 2\sigma_0 \sqrt{\frac{L}{2r}}$$

(13)

39 [Figure 12]

27 **Fig. 12** Figure shows the calculated crack tip stress from the crack aperture and length as a function of  
 28 Young's Modulus (E). Both crack aperture and length values are derived as means/modes using the  
 29 logistic distribution function for all the data. The plot shows two regimes, below 10 to 11 MPa  
 30 Young's Modulus the crack tip stress increases rapidly, whereas at Young's Moduli higher than 11  
 31 MPa the crack tip stress increase is only minor. These two regime represent two failure regimes,  
 32 below the critical Young's Modulus the fractures are soft and open whereas at higher values the  
 33 aperture is constant the crack propagates by an increase in length

35 Figure 12 shows a plot of the solid effective stress at crack tips in the simulations that was  
 36 calculated using the average opening and amount of broken bonds for different Young's  
 37 moduli. Two distinctly different regimes appear, a steep slope below an elastic constant of 10  
 38 GPa to 12 GPa and a shallower slope above. A smaller elastic constant produces larger  $r$   
 39 values (aperture) and smaller  $l$  values (fracture length). It can be seen that  $r$  changes much

1 more than  $l$ , therefore large elastic constants have a smaller  $r$  and a slightly larger  $l$ . This  
 2 means that the ratio below the square root in equation 13 is smaller for smaller values of the  
 3 elastic constant but it changes a lot with a significant change of  $r$  towards an elastic constant  
 4 of about 10 to 12 GPa. Then the  $r$  value seems to saturate and stay more or less constant and  
 5 only the length changes increase. This effect is overprinted by the square root function.

6 Two different geometrical factors affect the effective stress at the crack tip, the aperture and  
 7 the fracture length. Soft rocks have larger elasticity and tend to be more easily compressed  
 8 leading to wide open fractures. The change in fracture aperture being strongly sensitive to  
 9 changes in elasticity. In this regime, fracture length is fairly stable and in fact slowly decreases  
 10 with increasing Young's Modulus. At larger Young's Moduli the change in fracture aperture  
 11 halts, whereas the change in fracture length becomes important.

12 To better understand the dynamics at the fracture tip we have reconstructed the  $\sigma_{xx}$  and  $\sigma_{yy}$   
 13 stress net change between the initial and final time steps in a profile line orientated vertically  
 14 and horizontally through the simulated fractured area (Fig. 13, 14).

15 [Figure 13]

16  
 17 **Fig. 13** Figure shows schematic placement of the profiles used to calculate the difference in  $\sigma_{xx}$   
 18 (horizontal) and  $\sigma_{yy}$  (vertical) stress between the first and last time steps in the experiments. The  
 19 profiles cover fracture networks rather than single fractures and thus show fluctuations in stress

20 [Figure 14]

21  
 22  
 23  
 24 **Fig. 14** Profiles of change in  $\sigma_{xx}$  (horizontal) and  $\sigma_{yy}$  (vertical) stress throughout the experiment along the  
 25 horizontal (left) and vertical (right) lines of profile. The horizontal profile shows an increase in  $\sigma_{xx}$  stress on each  
 26 side of the fractured area representing a compaction zone, which is much more pronounced in the soft rock. The  
 27 vertical profile shows a significant difference of  $\sigma_{xx}$  stress for soft versus hard rocks with hard rocks showing  
 28 only compression whereas soft rocks have a tensile region at the top of the fracture. Both vertical profiles are  
 29 anisotropic reflecting gravity

30  
 31  
 32  
 33  
 34  
 35  
 36  
 37  
 38  
 39  
 40  
 41  
 42  
 43  
 44  
 45  
 46  
 47  
 48  
 49  
 50  
 51  
 52  
 53  
 54  
 55  
 56  
 57  
 58  
 59  
 60  
 61  
 62  
 63  
 64  
 65

22 The data line is uneven as rather than going through a single fracture, it crosses a fracture  
 23 network and this causes fluctuations. After performing this procedure multiple times for  
 24 several cases we can observe certain patterns emerge, which are schematically depicted in  
 25 Figure 15.

26 [Figure 15]

27  
 28  
 29  
 30  
 31  
 32  
 33  
 34  
 35  
 36  
 37  
 38  
 39  
 40  
 41  
 42  
 43  
 44  
 45  
 46  
 47  
 48  
 49  
 50  
 51  
 52  
 53  
 54  
 55  
 56  
 57  
 58  
 59  
 60  
 61  
 62  
 63  
 64  
 65

29 **Fig. 15** Figure shows schematic illustration of stress in soft and hard rocks. Arrows pointing towards  
 30 the centre of fracture indicate compressive stress while arrows pointing outwards – extensional

31 Both cases (soft and hard rock fractures) have isotropic (explosive) behavior at the injection  
 32 point and tend to have stress changes on left and right hand side. The  $\sigma_{xx}$  (horizontal) stress in  
 33 both cases shows an increase on both sides of the crack representing a compaction zone of  
 34 elevated local stress field. This compaction zone is much more pronounced in the soft rock.



1 The  $\sigma_{yy}$  (vertical) stress behavior is different for soft and hard rocks. In the case of a soft rock  
 2 it tends to be slightly extensional at the boundary of the fracture and then changes to  
 3 compressive further away. When looking at the hard rock, the stress is mainly compressive  
 4 with a small tensile area at the very center and the switch from tensile to compressive is more  
 5 abrupt. At the crack tip soft and hard rocks behave quite differently, especially at the top of  
 6 the fractured area. Both soft and hard rock scenarios show a bimodal behavior where both  $\sigma_{xx}$   
 7 and  $\sigma_{yy}$  stresses change from extensional to compressive closer to the end of the fractured  
 8 area. However, in the case of soft rocks, the  $\sigma_{xx}$  stress becomes sharply extensional just  
 9 outside the fractured area, while in the case of the hard rock scenario, no sudden change in  
 10 stress is observed. This backs up the theory that fracture development in the case of soft rocks  
 11 is mostly governed through lateral extension and increase of the fracture aperture as the stress  
 12 regimes readily allow for lateral expansion at the crack tip. The relatively higher  $\sigma_{xx}$  stress  
 13 values at the sides of the fracture suggest extensive expansion in that direction. In the case of  
 14 the hard rock, the length is the key value behind the crack growth. In the case of the soft rock  
 15 the fracture tip is already in extension before the crack propagates and the fluid pushes  
 16 outwards to pull the solid apart for further fracturing. This behavior is very similar to  
 17 fracturing in granular media where grains are being pushed apart (Eriksen et al. 2017, 2018,  
 18 Niebling et al. 2012) and would represent mode I extensional failure. In contrast the hard  
 19 rock fracture tip is actually under compression. The stress concentration in the hard rock case  
 20 is very high at the crack tip, so that almost any tensile stress at the tip exceeds the breaking  
 21 strength and leads to fracture propagation. The effect of these different mechanisms results  
 22 in different slopes for hard rocks in Figure 12.

23 Figure 15 illustrates that in soft rocks the cracks are shorter and have rounder shape. In this  
 24 case the fluid is potentially filling the whole crack and pushing at the walls (creating  
 25 gradients depending on seepage) for further fracturing. In the case of hard rocks the upper  
 26 fracture tip is quite far from the injection point. In this case the fluid overpressure may not  
 27 reach the tip for fracturing to occur. This leads to a low fluid pressure crack tip in the case of  
 28 saturated rocks (as modeled here), and would lead to a dry crack tip if the solid is not water  
 29 saturated.

30 The resolution of the model (amount of particles in the simulation) has an influence on the  
 31 amount of fractures that can be produced in the model but the crack tip stress is not directly  
 32 dependent on the particle size. The stress at the crack tip is given by the extension or shearing  
 33 of springs which is only determined by the relaxation threshold that is used to solve the stress  
 34 field, which is in the order of 4 magnitudes smaller than the particle size. Figure 16 shows  
 35 that the fracture network itself does not change in geometry at different resolutions, except  
 36 for the fact that a higher resolution simulation produces a more detailed fracture network with  
 37 more branches.

38 [Figure 16]

39  
 40 **Fig. 16** Figure shows the fracture patterns (blue particles) in detail for a hard (50 GPa) versus a soft  
 41 (5GPa) rock. a) Simulation of a soft rock with four distinct shear fractures and a central mode I  
 42 region. b) Simulation of a hard rock with a much more pronounced central fractured area in  
 43 comparison to the soft rock. c) shows details of the central parts where distinct openings can be seen  
 44 in the soft rock in the central part representing an opening elliptical crack when the rock is soft. d)  
 45 shows a high resolution simulation of the hard rock. In this case the ratio between the fluid input area  
 46 and the particle size is changed to twice the size in the horizontal and vertical direction, meaning that  
 47 fluid is injected in four cells and not only in one cell as is the case with a) and b). The fracture pattern

1 in the high resolution simulation is the same as in b) but twice the size illustrating that the scale and  
 2 pattern of the fracture network is not dependent on the resolution of the model.

## 3 4 5 **5 Conclusions**

6 In this contribution we show numerically how hydrofracturing during fluid injection develops  
 7 in 4 distinct stages: 1) build up of stress, 2) fracturing, 3) residual fracturing, 4) seeping. The  
 8 Young's modulus of rocks and their breaking strength change the fracturing behavior  
 9 resulting in two predominant fracturing mechanisms. In the cases when the fractured rock is  
 10 soft, fracturing occurs due to a critical tensile strength or cohesion being exceeded, change in  
 11 the stress at the crack tip as a function of the Young's Modulus is then governed by the  
 12 change in the fracture aperture and the fracture tip is constantly under extension. When the  
 13 rock is hard, the fracture aperture is small and constant and the fracture length becomes the  
 14 key driver of the stress change. The fracture tip is under compression and fracturing is driven  
 15 strongly by stress intensification at the tip, causing fractures to propagate in more shear like  
 16 failure.

16 We show that the relationship between Young's modulus and the failure mechanism works  
 17 independently of the tensile strength of the rock as changes in Young's modulus for rocks  
 18 with different tensile strengths produce the same relative increase in the amount of fracturing.  
 19 The number of fractures that develop is a non-linear function of the breaking strength.

20 Changes in secondary parameters such as the amount of overburden or fluid injection rate  
 21 have an impact on the amount of fractures that develop but have little to no effect on the  
 22 described failure mechanisms. The transition between the failure due to a critical tensile  
 23 strength for soft rocks versus a strong dependence on stress intensification for hard rock lies  
 24 at a Young's modulus of about 10 GPa in the simulations. Since Young's moduli for natural  
 25 shales vary from 8 to 50 GPa one has to be careful to use the right failure criterion depending  
 26 on the shale type.

## 27 28 **References**

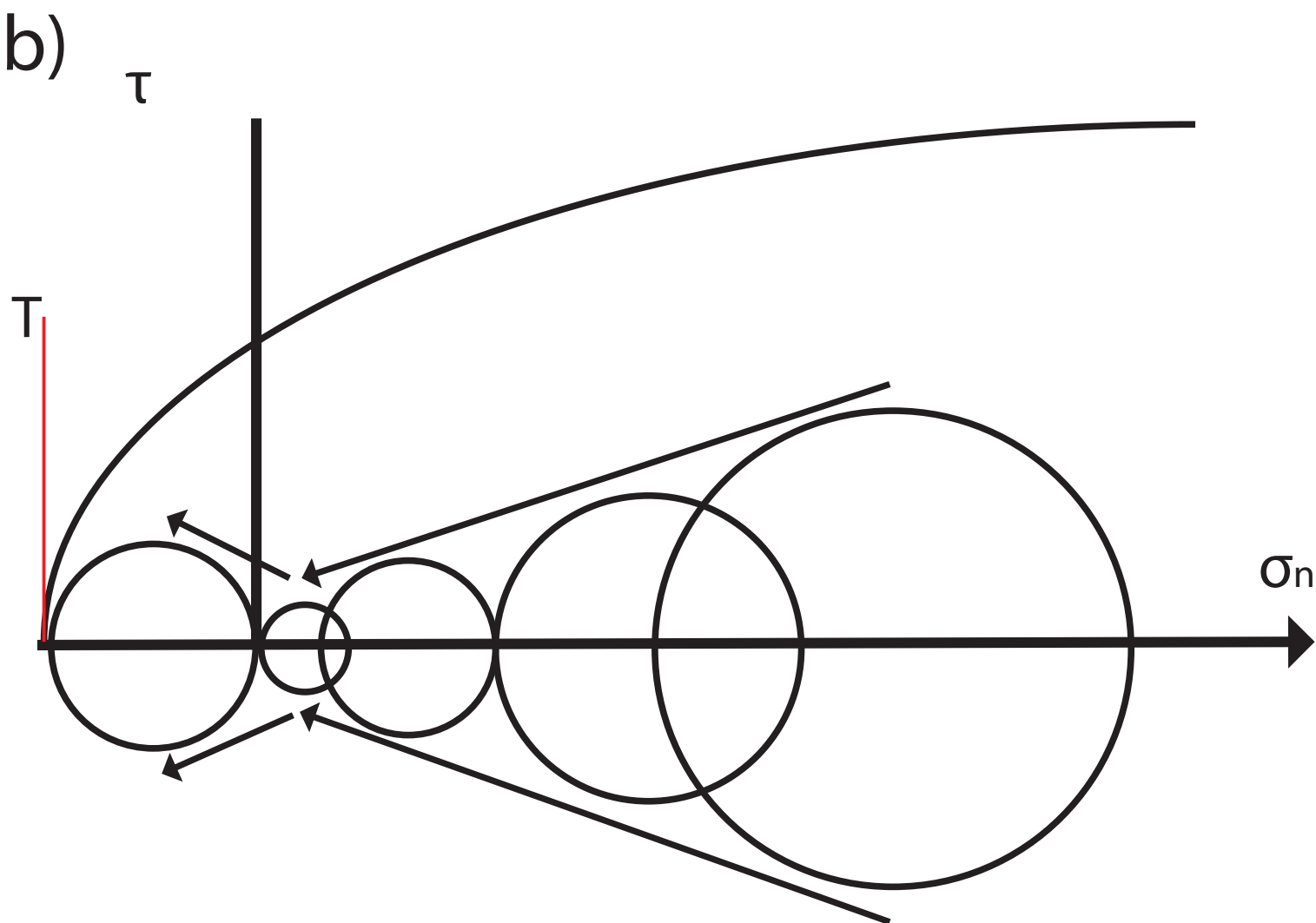
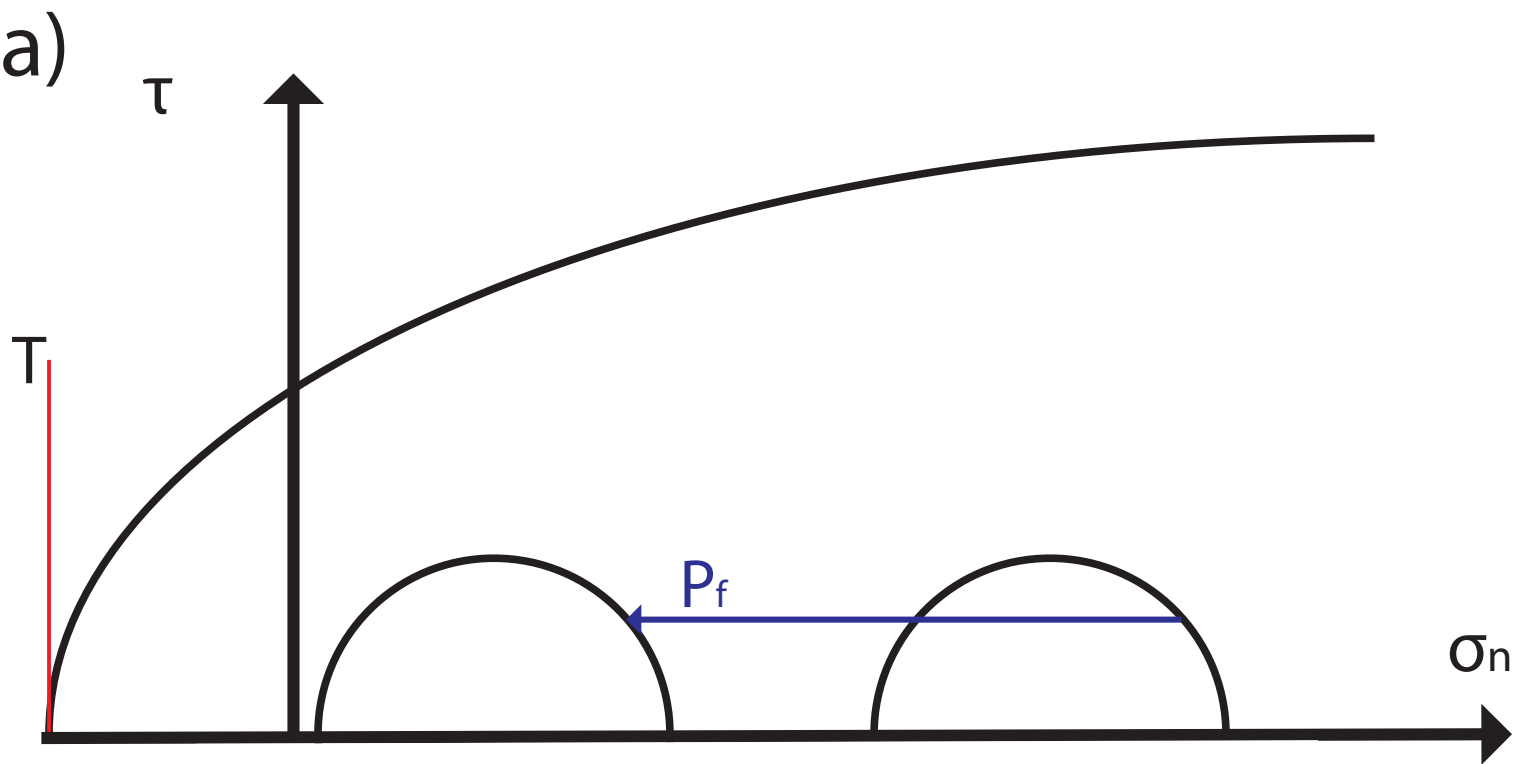
- 29 Aki, K., and Richards P. G. (2002), *Quantitative Seismology*, University Science Books.
- 30 Anderson, T. L. (2005), *Fracture Mechanics: Fundamentals and Applications*, CRC Press LLC.
- 31 Baria, R., Baumgärtner J., Rummel F., Pine R. J., and Sato Y. (1999), HDR/HWR reservoirs:  
 32 concepts, understanding and creation, *Geothermics*, 28(4), 533-552.
- 33 Bons, P., Koehn D., and Jessell M. W. (2007), *Microdynamics simulation*, Springer Science &  
 34 Business Media.
- 35 Carman, P. C. (1937). Fluid flow through granular beds. *Trans. Inst. Chem. Eng.*, 15, 150-166.
- 36 Clément, C., Toussaint, R., Stojanova, M., & Aharonov, E. (2018). Sinking during earthquakes:  
 37 Critical acceleration criteria control drained soil liquefaction. *Physical Review E*, 97(2), 022905.
- 38 Cobbold, P. R., and Rodrigues N. (2007), Seepage forces, important factors in the formation of  
 39 horizontal hydraulic fractures and bedding- parallel fibrous veins ('beef' and 'cone- in- cone'),  
 40 *Geofluids*, 7(3), 313-322.

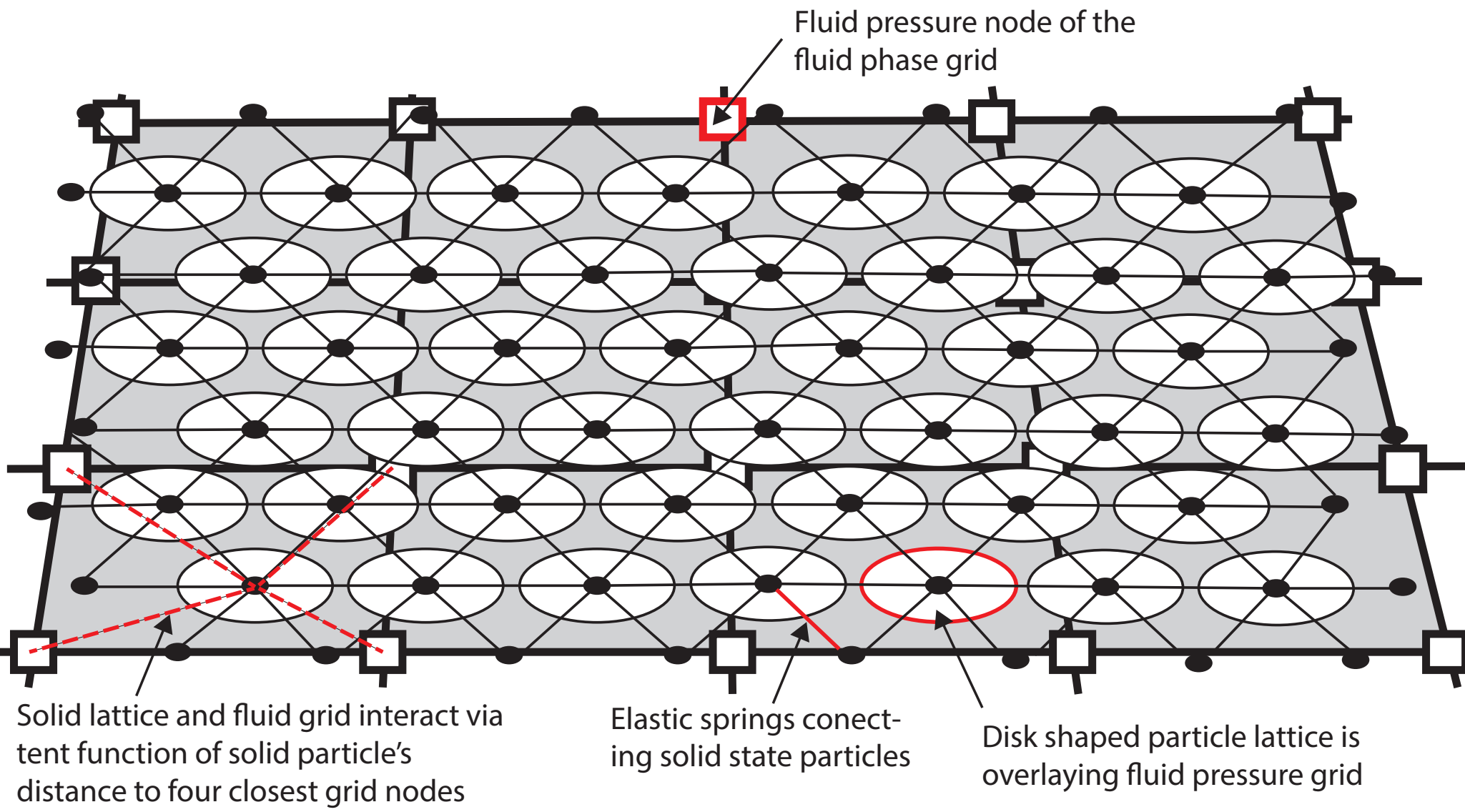
- 1 Detournay, E., and Cheng A.H.D. (1993), Fundamentals of poroelasticity1, Chapter 5 in  
2 Comprehensive Rock Engineering: Principles, Practice and Projects, II, 113-171.
- 3 Eriksen, F. K., Toussaint, R., Turquet, A. L., Måløy, K. J., & Flekkøy, E. G. (2017). Pneumatic  
4 fractures in confined granular media. *Physical Review E*, 95(6), 062901.
- 5  
6 Eriksen, F. K., Toussaint, R., Turquet, A. L., Måløy, K. J., & Flekkøy, E. G. (2018). Pressure  
7 evolution and deformation of confined granular media during pneumatic fracturing. *Physical Review*  
8 *E*, 97(1), 012908.
- 9  
10 Flekkøy, E. G., Malthe-Sørenssen A., and Jamtveit B. (2002), Modeling hydrofracture, *Journal of*  
11 *Geophysical Research: Solid Earth*, 107(B8).
- 12  
13 Fyfe, W. S. (2012), *Fluids In The Earth's Crust: Their Significance In Metamorphic, Tectonic And*  
14 *Chemical Transport Process*, Elsevier.
- 15  
16 Ghani, I., Koehn D., and Toussaint R. (2015), Dynamics of hydrofracturing and permeability  
17 evolution in layered reservoirs, *Frontiers in Physics*, 3, 67.
- 18  
19 Ghani, I., Koehn D., Toussaint R., and Passchier C. W. (2013), Dynamic Development of  
20 Hydrofracture, *Pure and Applied Geophysics*, 170(11), 1685-1703, [http://dx.doi.org/10.1007/s00024-](http://dx.doi.org/10.1007/s00024-012-0637-7)  
21 [012-0637-7](http://dx.doi.org/10.1007/s00024-012-0637-7).
- 22  
23 Griffith, A. A. (1921), The phenomena of rupture and flow in solids, *Philosophical transactions of the*  
24 *royal society of london. Series A, containing papers of a mathematical or physical character*, 221,  
25 163-198.
- 26  
27 Groenenboom, J., and van Dam, D. B. (2000). Monitoring hydraulic fracture growth: Laboratory  
28 experiments. *Geophysics*, 65(2), 603-611.
- 29  
30 Guest, A., and Settari A. (2010), Relationship Between the Hydraulic Fracture and Observed  
31 Microseismicity in the Bossier Sands, Texas, paper presented at Canadian Unconventional Resources  
32 and International Petroleum Conference, Society of Petroleum Engineers.
- 33  
34 Hanks, T. C. and Kanamori, H. (1979), A moment magnitude scale, *Journal of Geophysical Research:*  
35 *Solid Earth*, 84(B5), 2348--2350, <http://dx.doi.org/10.1029/JB084iB05p02348>.
- 36  
37 Hazzard, J. F., and Young R. P. (2002), Moment tensors and micromechanical models,  
38 *Tectonophysics*, 356(1), 181-197.
- 39  
40 Hubbert, M. K., and Rubey W. W. (1959), Role of fluid pressure in mechanics of overthrust faulting I.  
41 *Mechanics of fluid-filled porous solids and its application to overthrust faulting*, *Geological Society*  
42 *of America Bulletin*, 70(2), 115-166.
- 43  
44 Inglis, C. (1913), Stress in a plate due to the presence of sharp corners and cracks, *Trans. Roy. Inst.*  
45 *Naval Architects*, 60, 219-241.
- 46  
47 Irwin, G. R. (1953). The effect of size upon fracturing. *ASTM STP*, 158, 176-194.
- 48  
49 Irwin, G. R. (1957), Analysis of Stresses and Strains Near the End of a Crack Traversing a Plate, *J.*  
50 *Appl. Mech.*
- 51  
52 Johnsen, Ø., Toussaint, R., Måløy, K. J., & Flekkøy, E. G. (2006). Pattern formation during air  
53 injection into granular materials confined in a circular Hele-Shaw cell. *Physical Review E*, 74(1),  
54 011301.
- 55  
56 Mory, M. (2013), *Fluid Mechanics for Chemical Engineering*, Wiley.
- 57  
58  
59  
60  
61  
62  
63  
64  
65

- 1 Murphy, S., O'Brien G., McCloskey J., Bean C. J., and Nalbant S. (2013), Modelling fluid induced  
2 seismicity on a nearby active fault, *Geophysical Journal International*, 194(3), 1613-1624.
- 3 Niebling, M. J., Flekkøy, E. G., Måløy, K. J., & Toussaint, R. (2010a). Sedimentation instabilities:  
4 impact of the fluid compressibility and viscosity. *Physical Review E*, 82(5), 051302.
- 5  
6 Niebling, M. J., Flekkøy, E. G., Måløy, K. J., & Toussaint, R. (2010b). Mixing of a granular layer  
7 falling through a fluid. *Physical Review E*, 82(1), 011301.
- 8  
9 Niebling, M. J., Toussaint, R., Flekkøy, E. G., & Måløy, K. J. (2012). Dynamic aerofracture of dense  
10 granular packings. *Physical Review E*, 86(6), 061315.
- 11  
12 Nordgren, R. (1972), Propagation of a vertical hydraulic fracture, *Society of Petroleum Engineers*  
13 *Journal*, 12(04), 306-314.
- 14  
15 Ohta, A., Suzuki N., and Mawari T. (1992), Effect of Young's modulus on basic crack propagation  
16 properties near the fatigue threshold, *International journal of fatigue*, 14(4), 224-226.
- 17  
18 Parez, S., Aharonov, E. and Toussaint, R. (2016), Unsteady granular flows down an inclined plane.  
19 *Physical Review E*, 93(4), .042902.
- 20  
21 Pearson, C. (1981), The relationship between microseismicity and high pore pressures during  
22 hydraulic stimulation experiments in low permeability granitic rocks, *Journal of Geophysical*  
23 *Research: Solid Earth*, 86(B9), 7855-7864.
- 24  
25 Prasad, M., Kopycinska M., Rabe U., and Arnold W. (2002), Measurement of Young's modulus of  
26 clay minerals using atomic force acoustic microscopy, *Geophysical Research Letters*, 29(8).
- 27  
28 Rutqvist, J., Rinaldi A. P., Cappa F., and Moridis G. J. (2013), Modeling of fault reactivation and  
29 induced seismicity during hydraulic fracturing of shale-gas reservoirs, *Journal of Petroleum Science*  
30 *and Engineering*, 107, 31-44.
- 31  
32 Sachau, T., Bons P. D., and Gomez-Rivas E. (2015), Transport efficiency and dynamics of hydraulic  
33 fracture networks, *Frontiers in Physics*, 3, 63.
- 34  
35 Sachau, T., and Koehn D. (2014), A new mixed-mode fracture criterion for large-scale lattice models,  
36 *Geoscientific Model Development*, 7(1), 243-247.
- 37  
38 Sachpazis, C. (1990), Correlating Schmidt hardness with compressive strength and Young's modulus  
39 of carbonate rocks, *Bulletin of the International Association of Engineering Geology-Bulletin de*  
40 *l'Association Internationale de Géologie de l'Ingénieur*, 42(1), 75-83.
- 41  
42 Sayers, C. M. (2013), The effect of anisotropy on the Young's moduli and Poisson's ratios of shales,  
43 *Geophysical Prospecting*, 61(2), 416-426.
- 44  
45 Scott Jr, T., Zeng Z.W., and Roegiers J.C. (2000), Acoustic emission imaging of induced  
46 asymmetrical hydraulic fractures, paper presented at 4th North American Rock Mechanics  
47 Symposium, American Rock Mechanics Association.
- 48  
49 Sun, C. T., & Jin, Z.H. (2012), *Fracture Mechanics*, Academic Press, Boston,  
50 <http://dx.doi.org/10.1016/B978-0-12-385001-0.00012-2>.
- 51  
52 Tandaiya, P., Ramamurty U., Ravichandran G., and Narasimhan R. (2008), Effect of Poisson's ratio  
53 on crack tip fields and fracture behavior of metallic glasses, *Acta Materialia*, 56(20), 6077-6086.
- 54  
55 Urbancic, T., Shumila V., Rutledge J., and Zinno R. (1999), Determining hydraulic fracture behavior  
56 using microseismicity, paper presented at Vail Rocks 1999, The 37th US Symposium on Rock  
57 Mechanics (USRMS), American Rock Mechanics Association.
- 58  
59  
60  
61  
62  
63  
64  
65

- 1 Valko, P., and Economides M. (1995), *Hydraulic Fracture Mechanics*, (Vol. 28). New York: Wiley.
- 2 Vavryčuk, V. (2011), Tensile earthquakes: Theory, modeling, and inversion, *Journal of Geophysical*  
3 *Research: Solid Earth*, 116(B12), <http://dx.doi.org/10.1029/2011JB008770>.
- 4 von Terzaghi, K. (1925), *Principles of soil mechanics*, Eng News-Record, 95.
- 5 Vinningland, J. L., Johnsen, Ø., Flekkøy, E. G., Toussaint, R., & Måløy, K. J. (2007). Granular  
6 rayleigh-taylor instability: Experiments and simulations. *Physical review letters*, 99(4), 048001.
- 7 Vinningland, J. L., Toussaint, R., Niebling, M., Flekkøy, E. G., & Måløy, K. J. (2012). Family-Vicsek  
8 scaling of detachment fronts in granular Rayleigh-Taylor instabilities during sedimentating  
9 granular/fluid flows. *The European Physical Journal Special Topics*, 204(1), 27-40.
- 10 Warpinski, N. R., Steinfort T. D., Branagan P. T., and Wilmer R. H. (1999), Apparatus and method  
11 for monitoring underground fracturing, U.S. Patent No. 5,934,373. Washington, DC: U.S. Patent and  
12 Trademark Office.
- 13 Zeev, S. B., Goren, L., Perez, S., Toussaint, R., Clement, C., & Aharonov, E. (2017). The Combined  
14 Effect of Buoyancy and Excess Pore Pressure in Facilitating Soil Liquefaction. In *Poromechanics VI*  
15 (pp. 107-116).
- 16 Zoback, M. D., and Harjes H. P. (1997), Injection- induced earthquakes and crustal stress at 9 km  
17 depth at the KTB deep drilling site, Germany, *Journal of Geophysical Research: Solid Earth*, 102(B8),  
18 18477-18491.

1  
2  
3  
4  
5  
6  
7  
8  
9  
10  
11  
12  
13  
14  
15  
16  
17  
18  
19  
20  
21  
22  
23  
24  
25  
26  
27  
28  
29  
30  
31  
32  
33  
34  
35  
36  
37  
38  
39  
40  
41  
42  
43  
44  
45  
46  
47  
48  
49  
50  
51  
52  
53  
54  
55  
56  
57  
58  
59  
60  
61  
62  
63  
64  
65



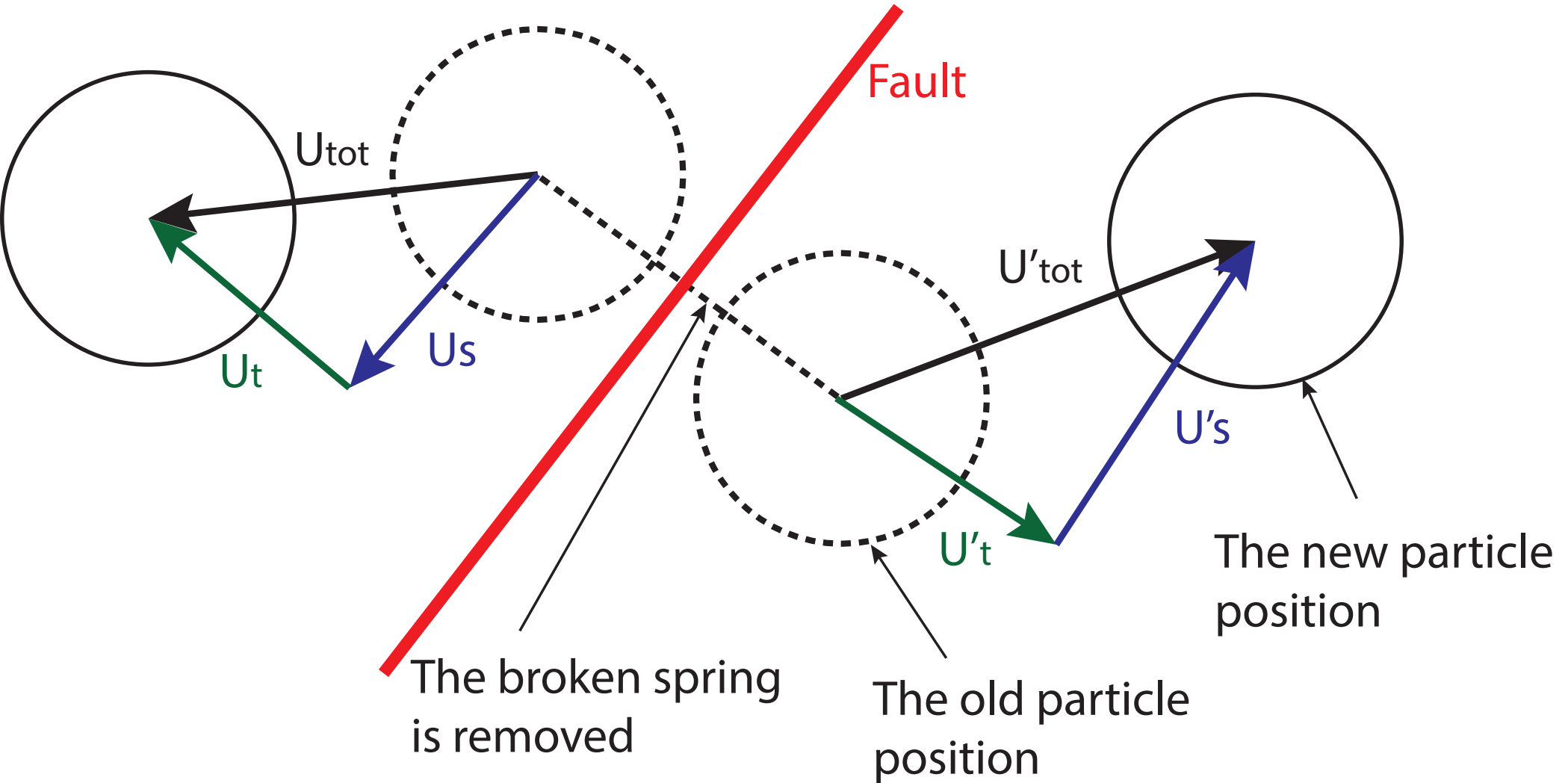


Fluid pressure node of the fluid phase grid

Solid lattice and fluid grid interact via tent function of solid particle's distance to four closest grid nodes

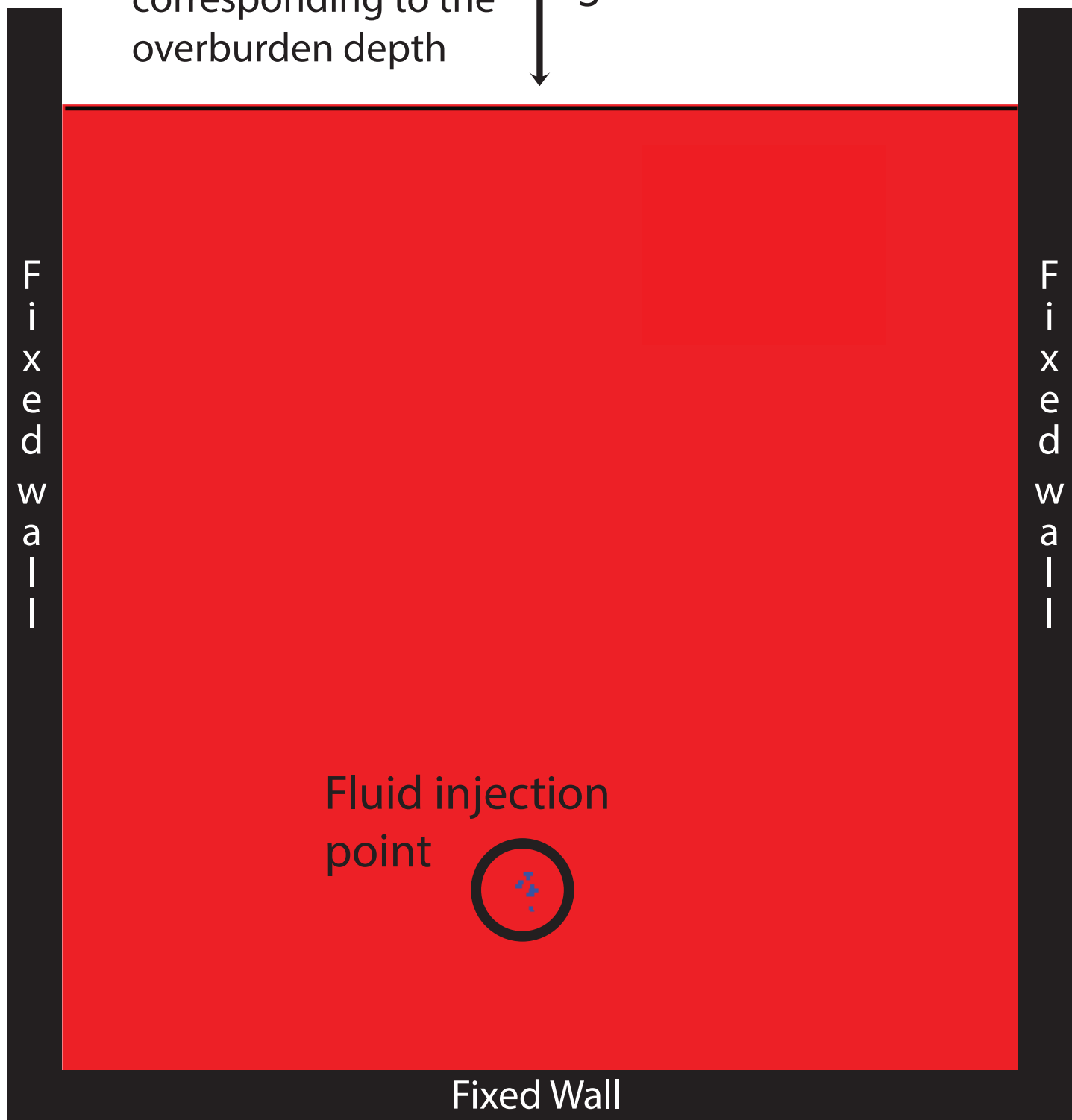

Elastic springs connecting solid state particles

Disk shaped particle lattice is overlaying fluid pressure grid





Pressure is applied  
corresponding to the  
overburden depth



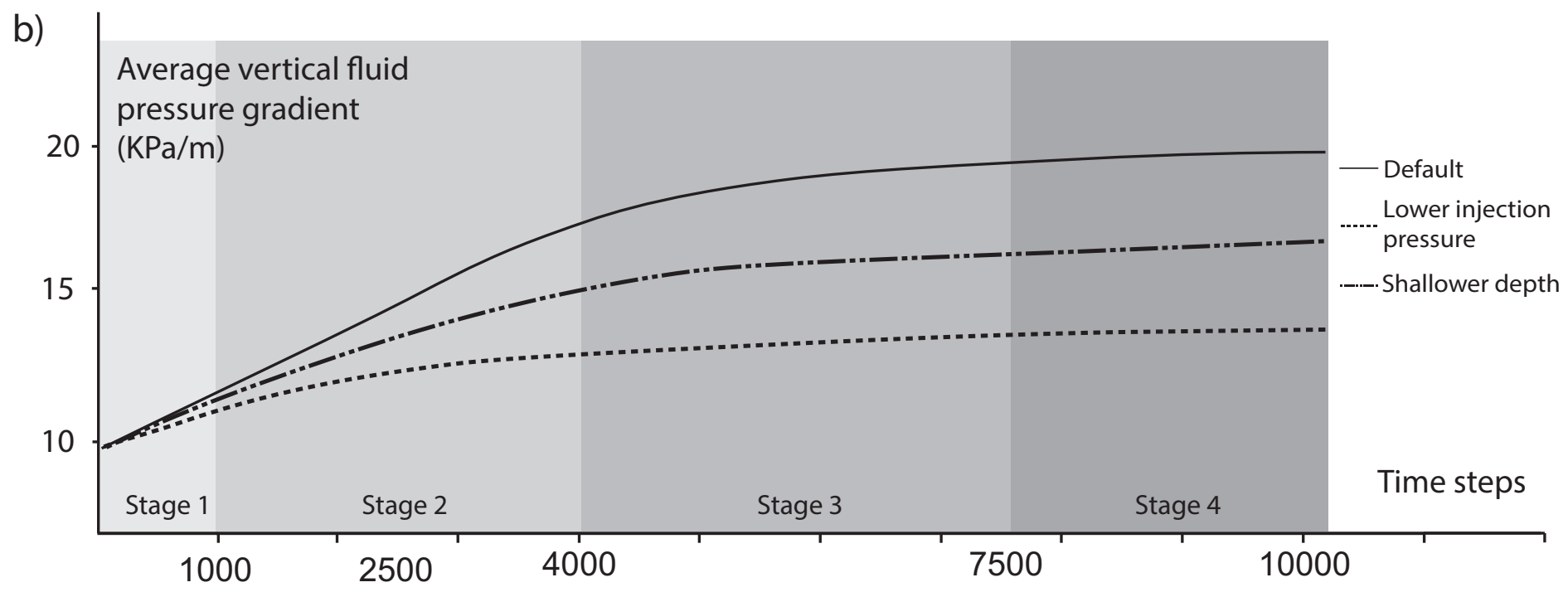
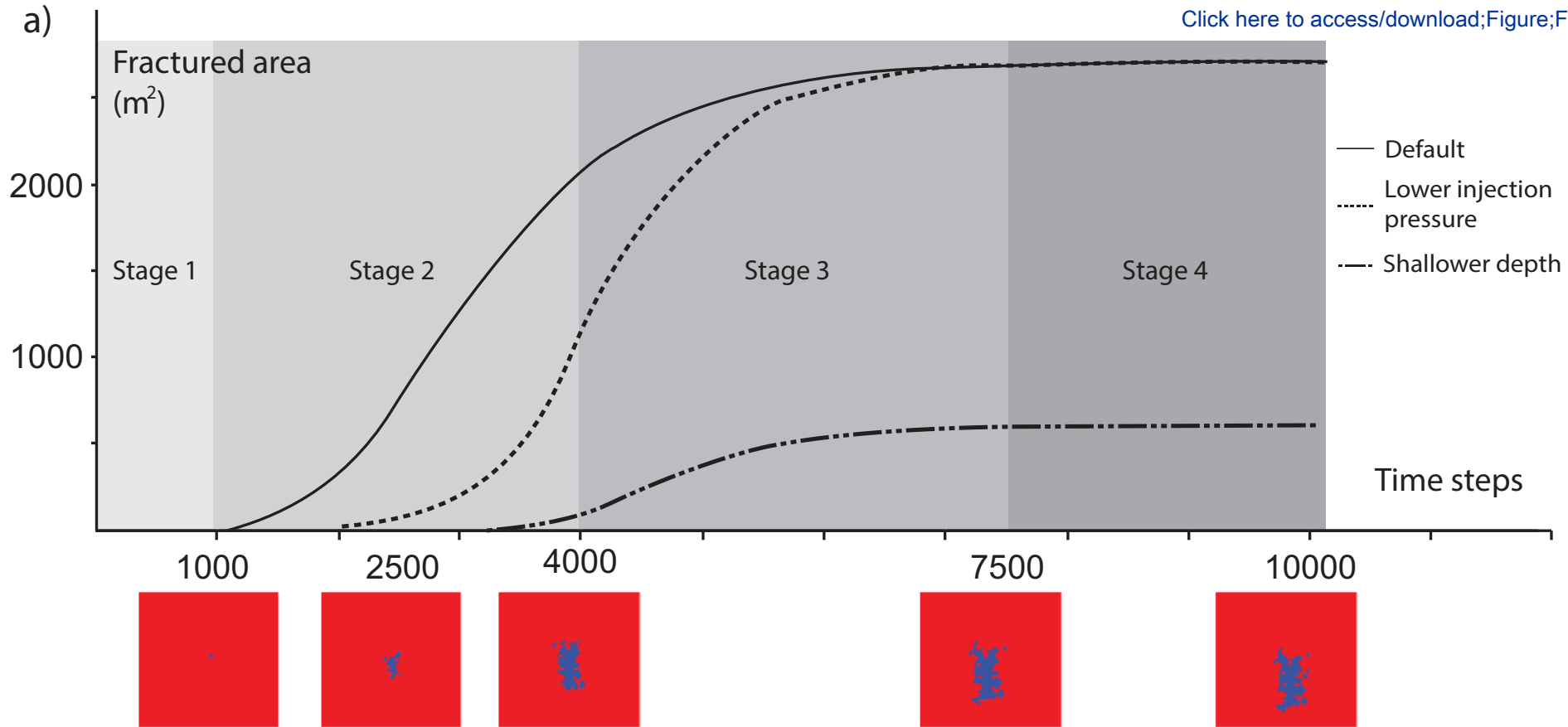
F  
i  
x  
e  
d  
w  
a  
l  
l

F  
i  
x  
e  
d  
w  
a  
l  
l

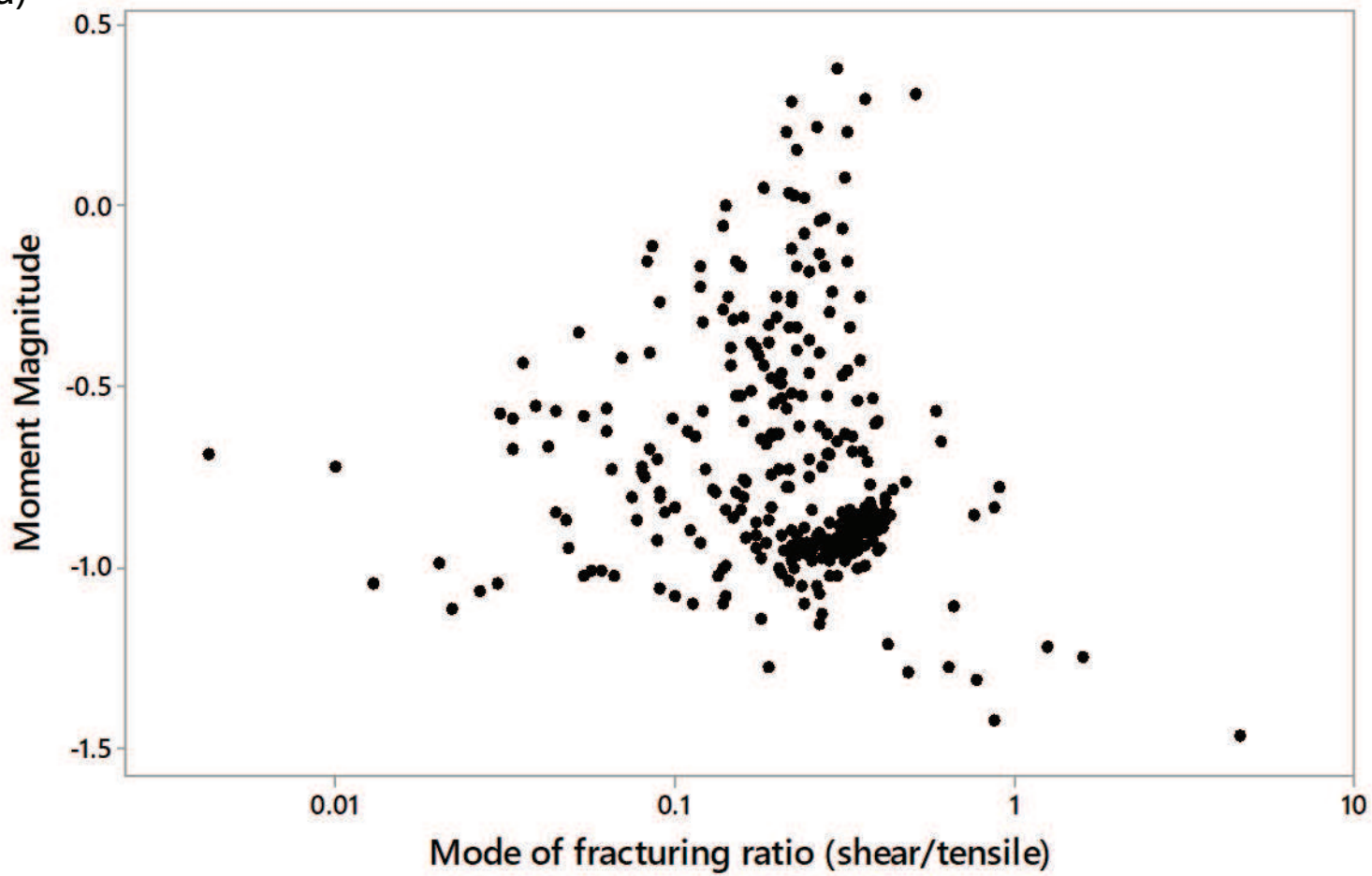
Fluid injection  
point



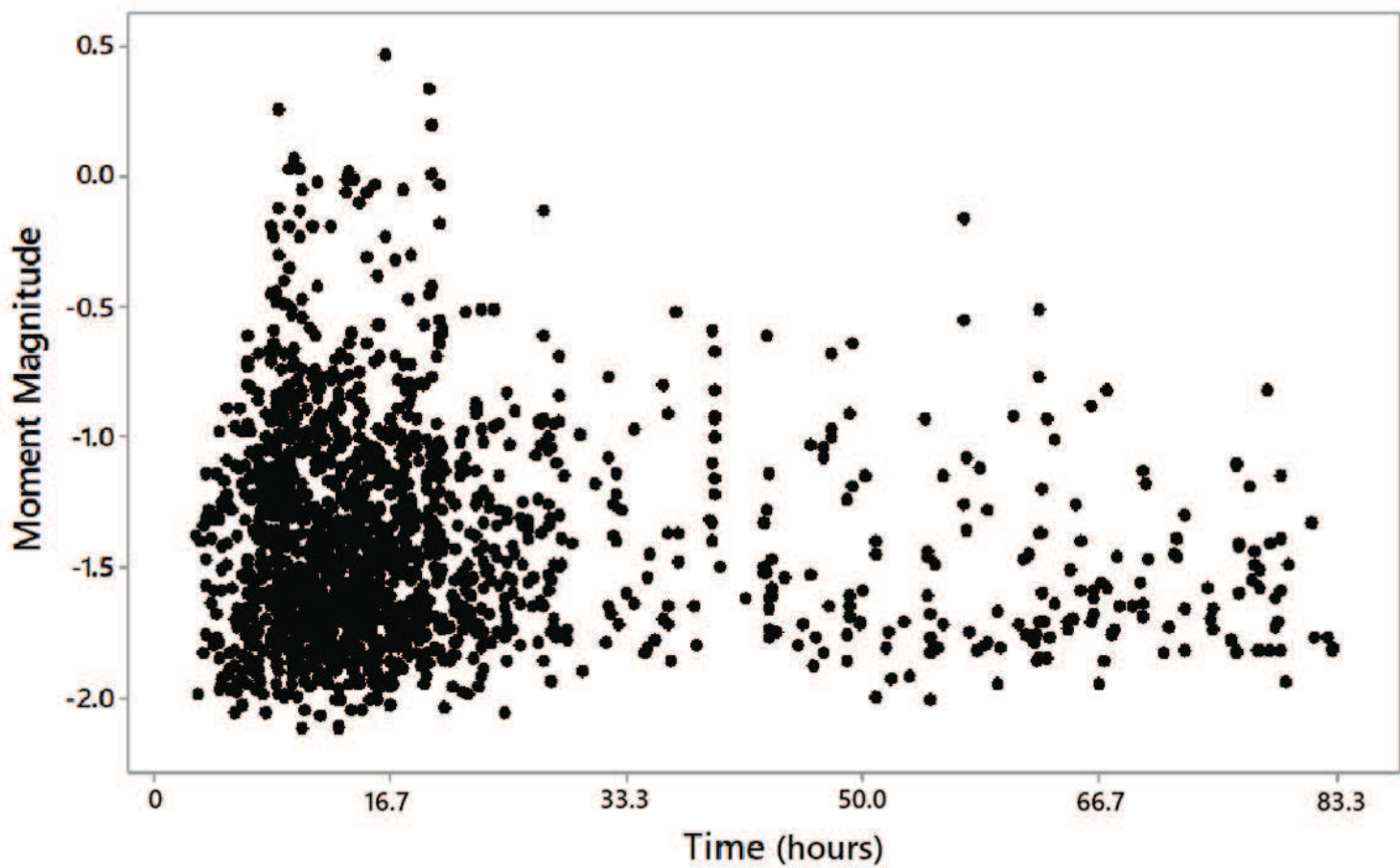
Fixed Wall

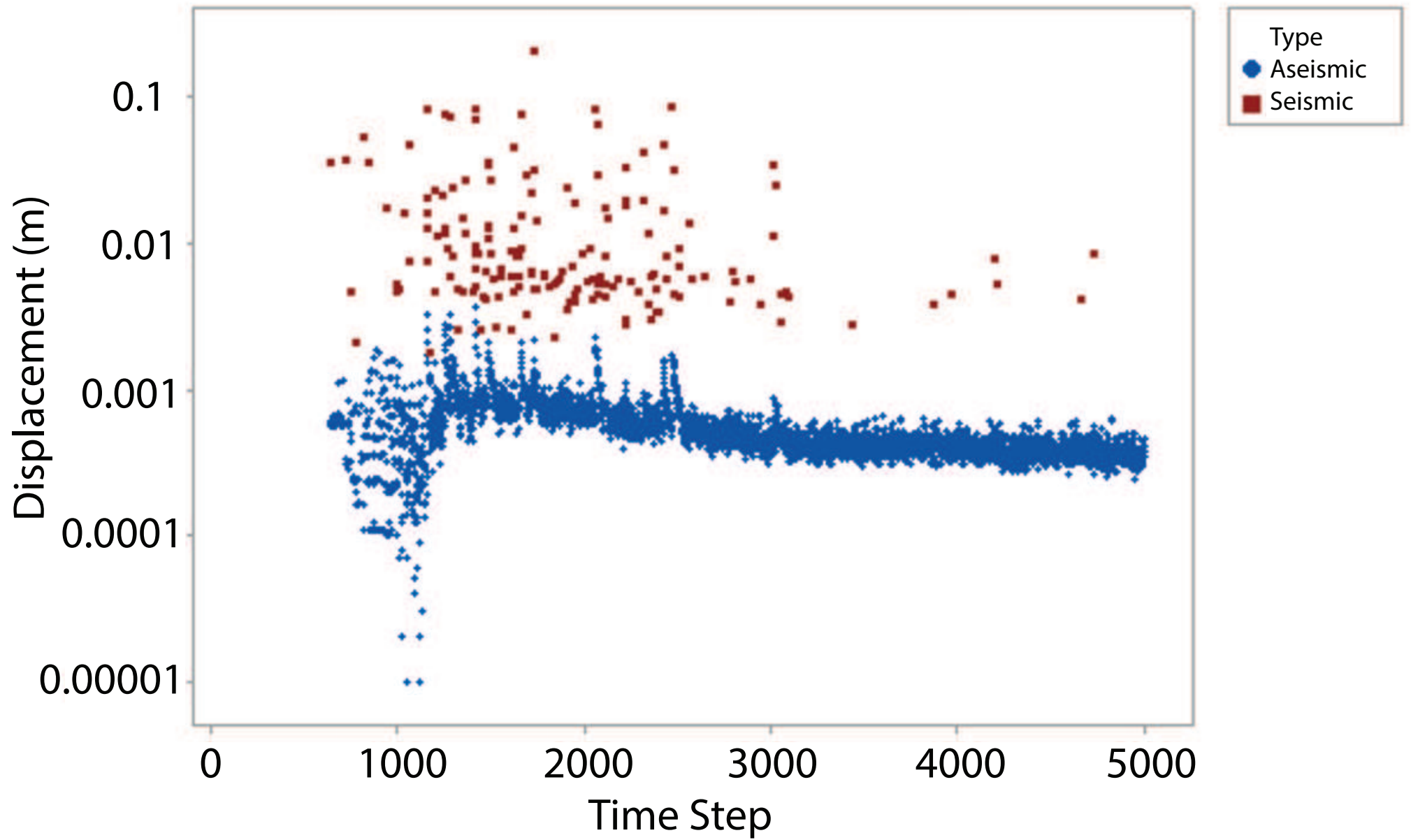


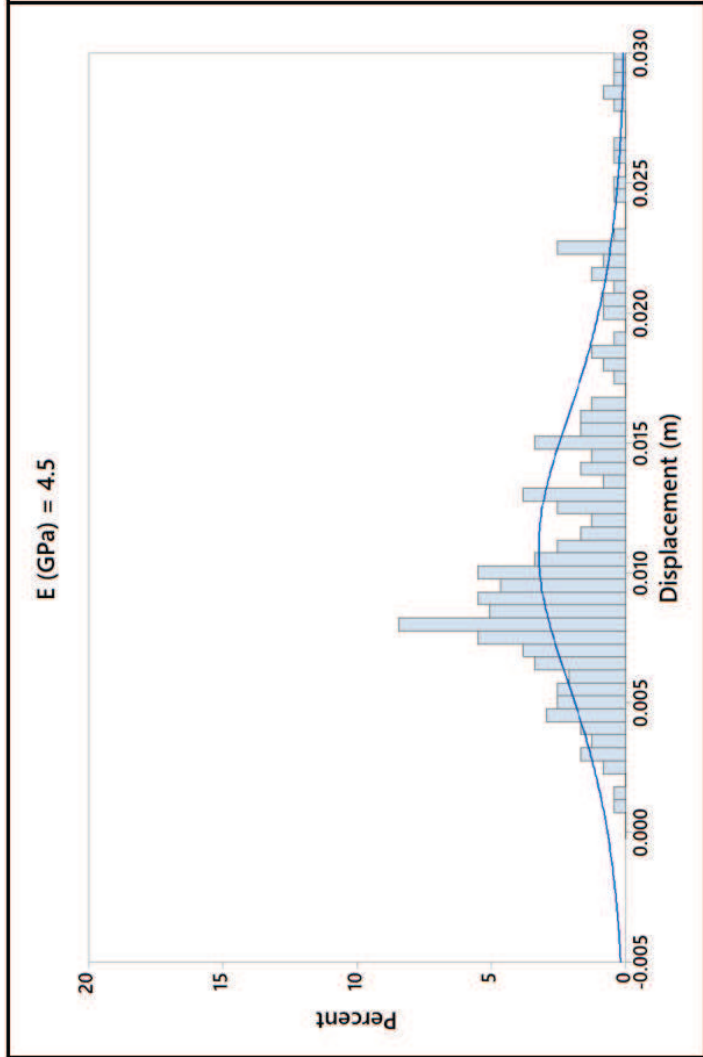
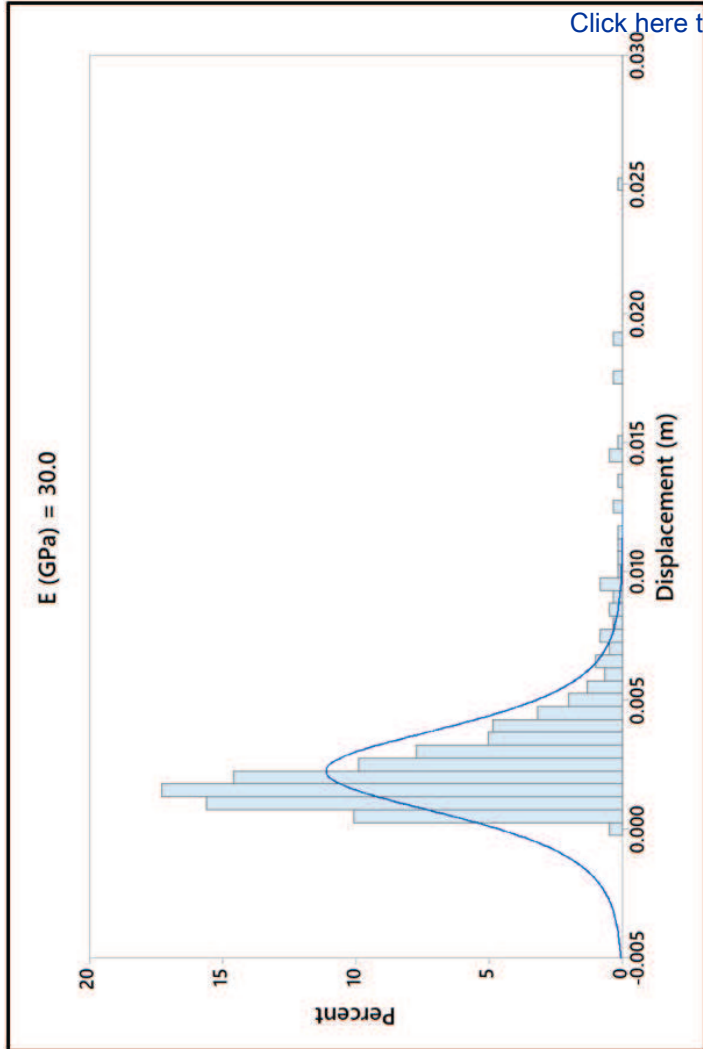
a)

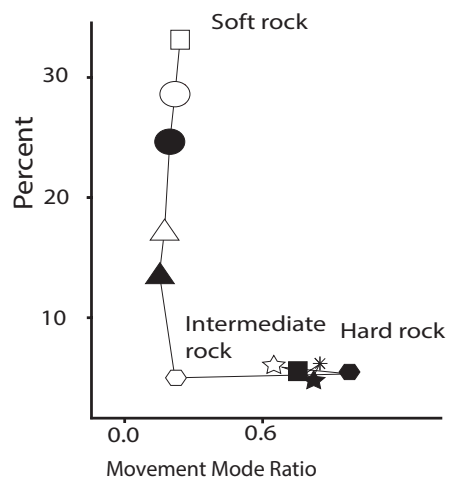
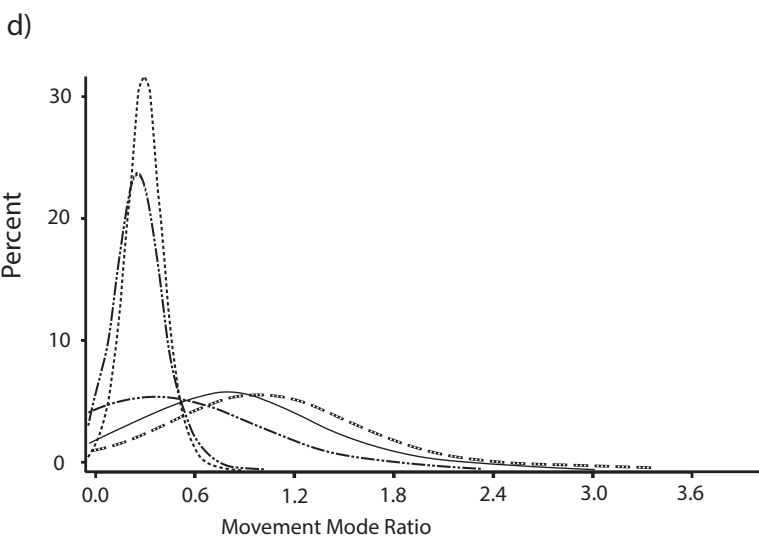
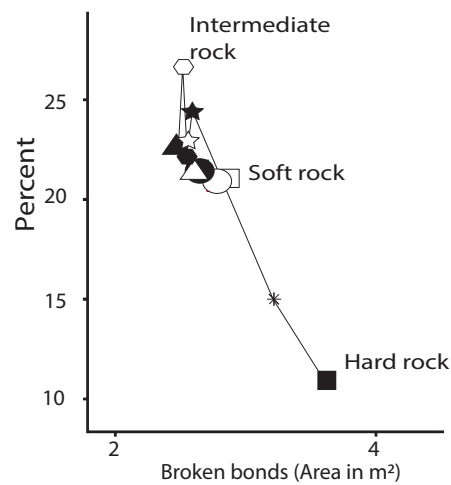
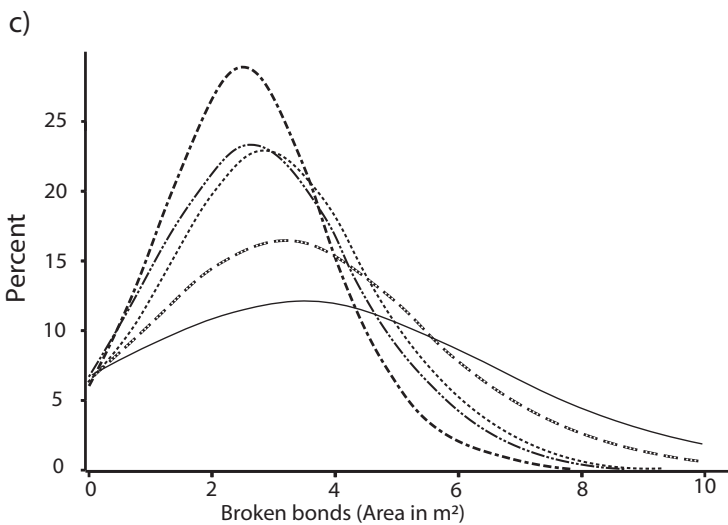
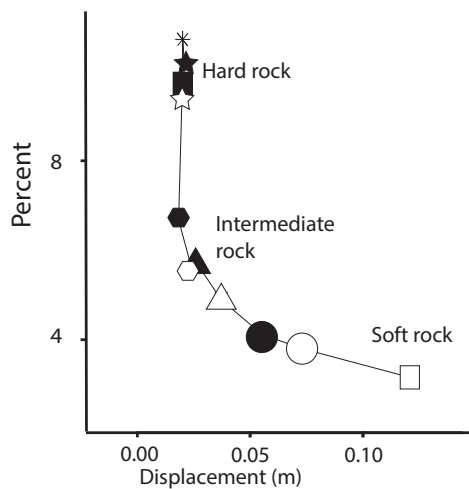
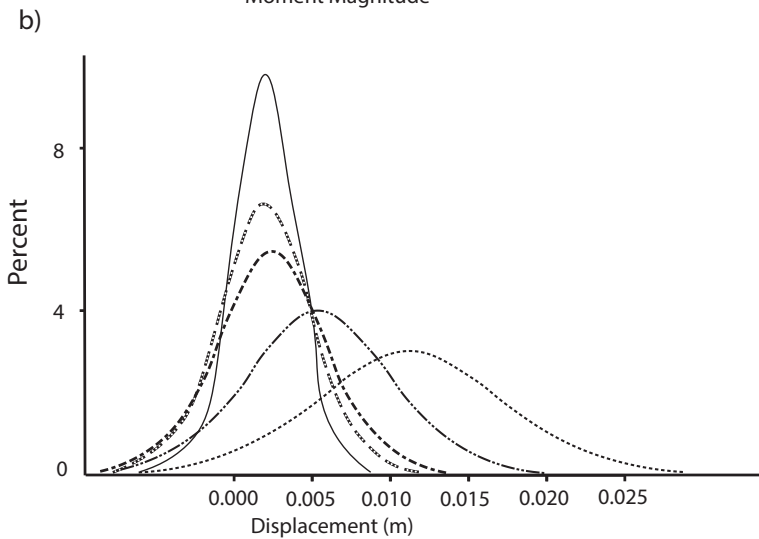
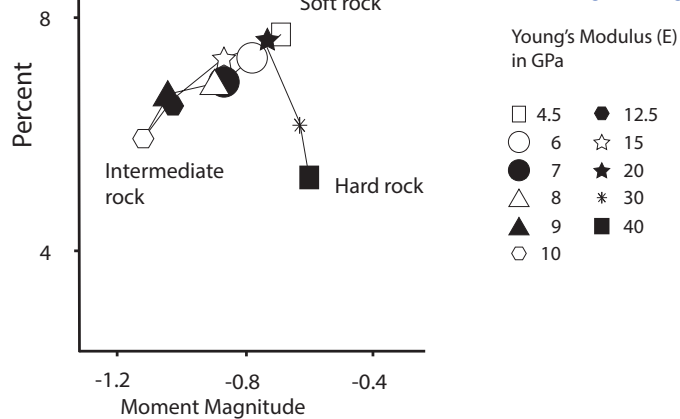
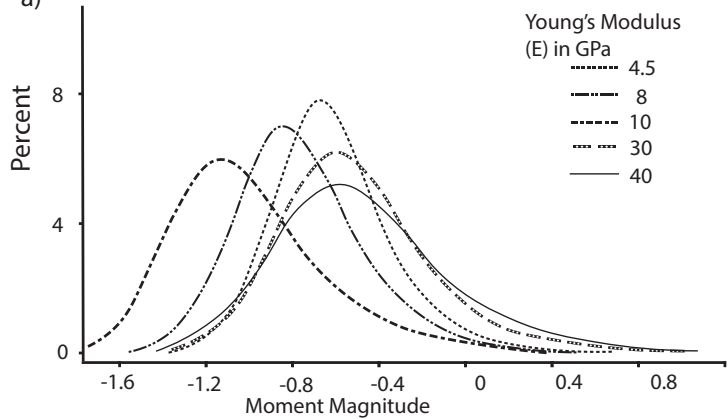


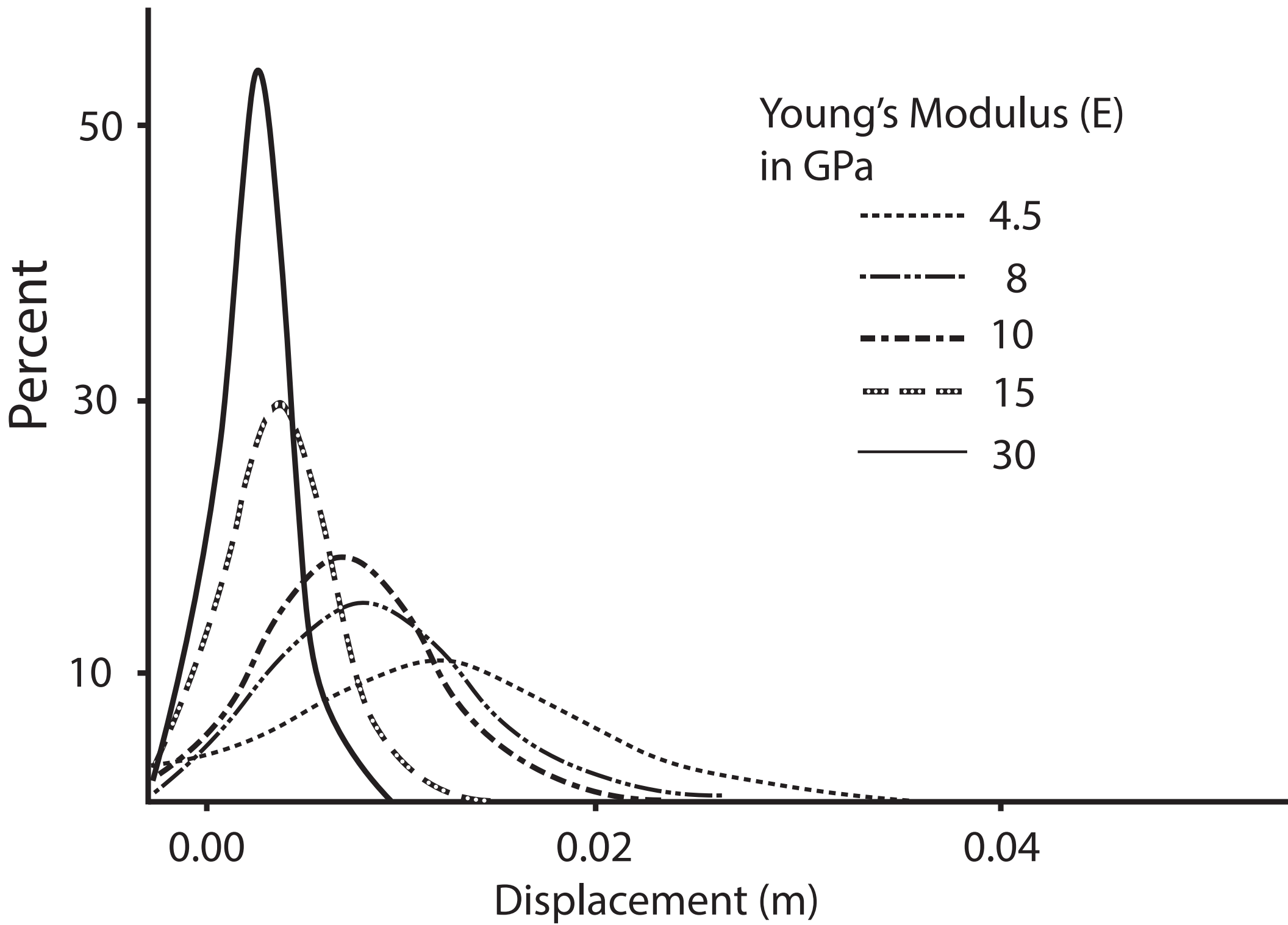
b)

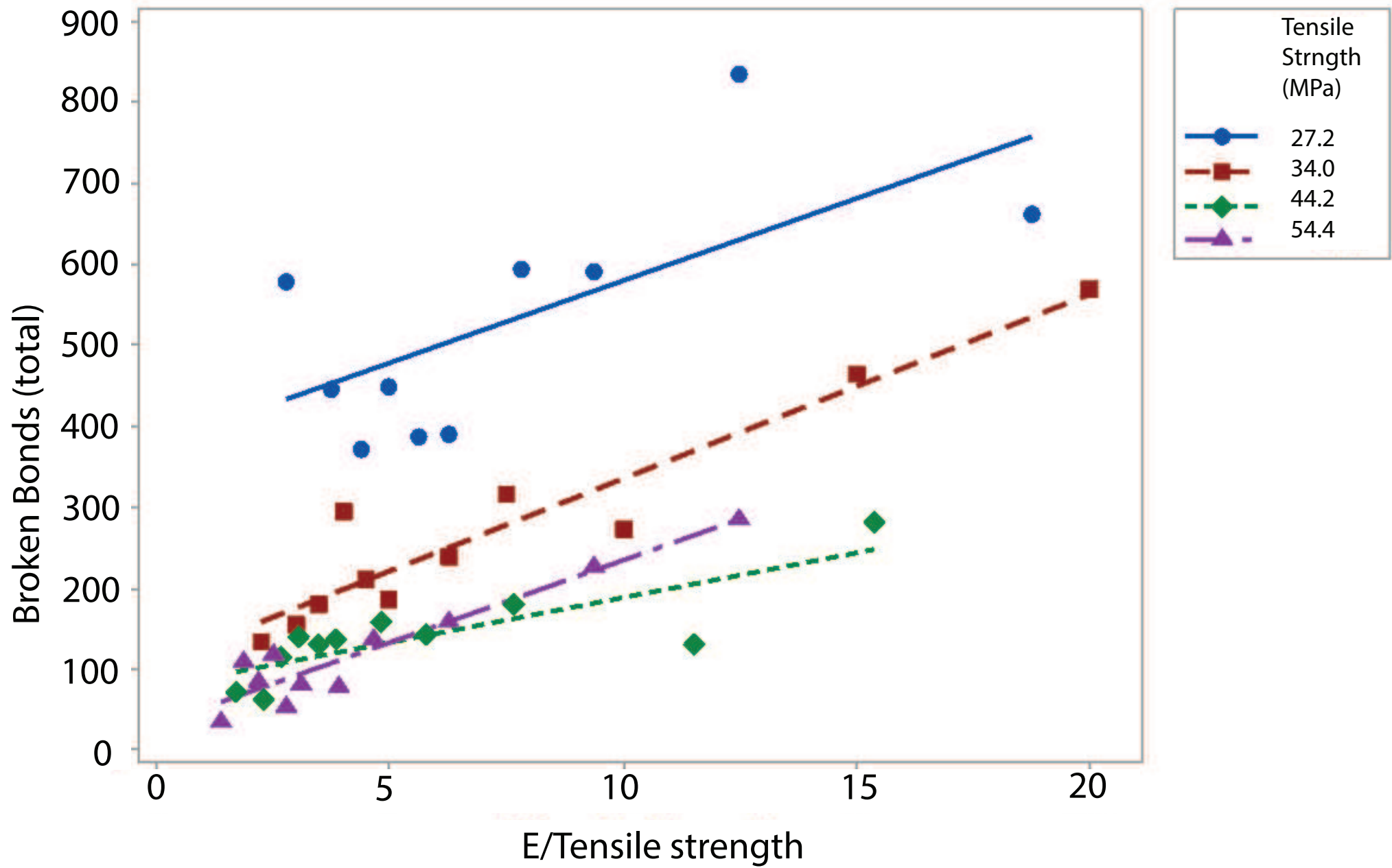




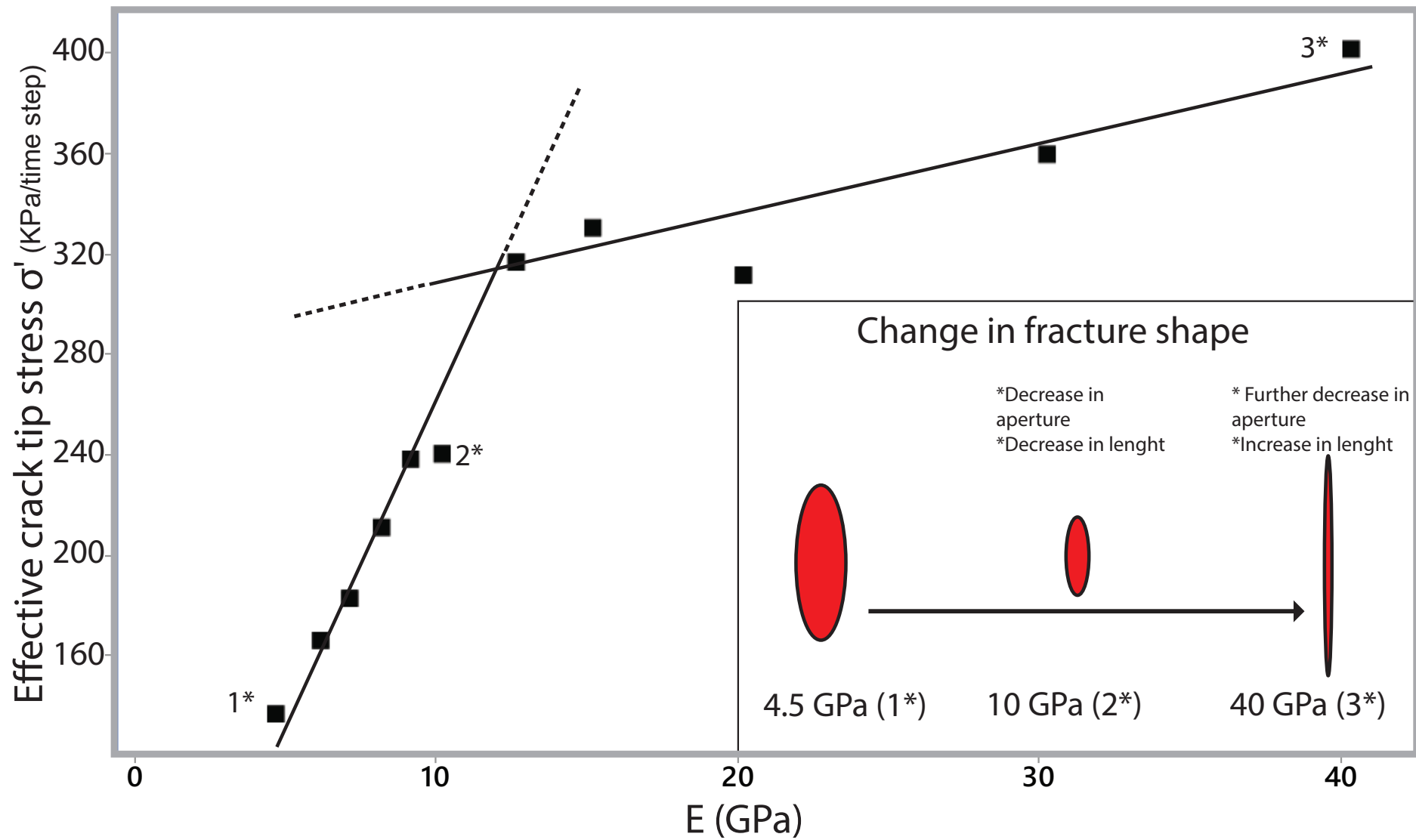


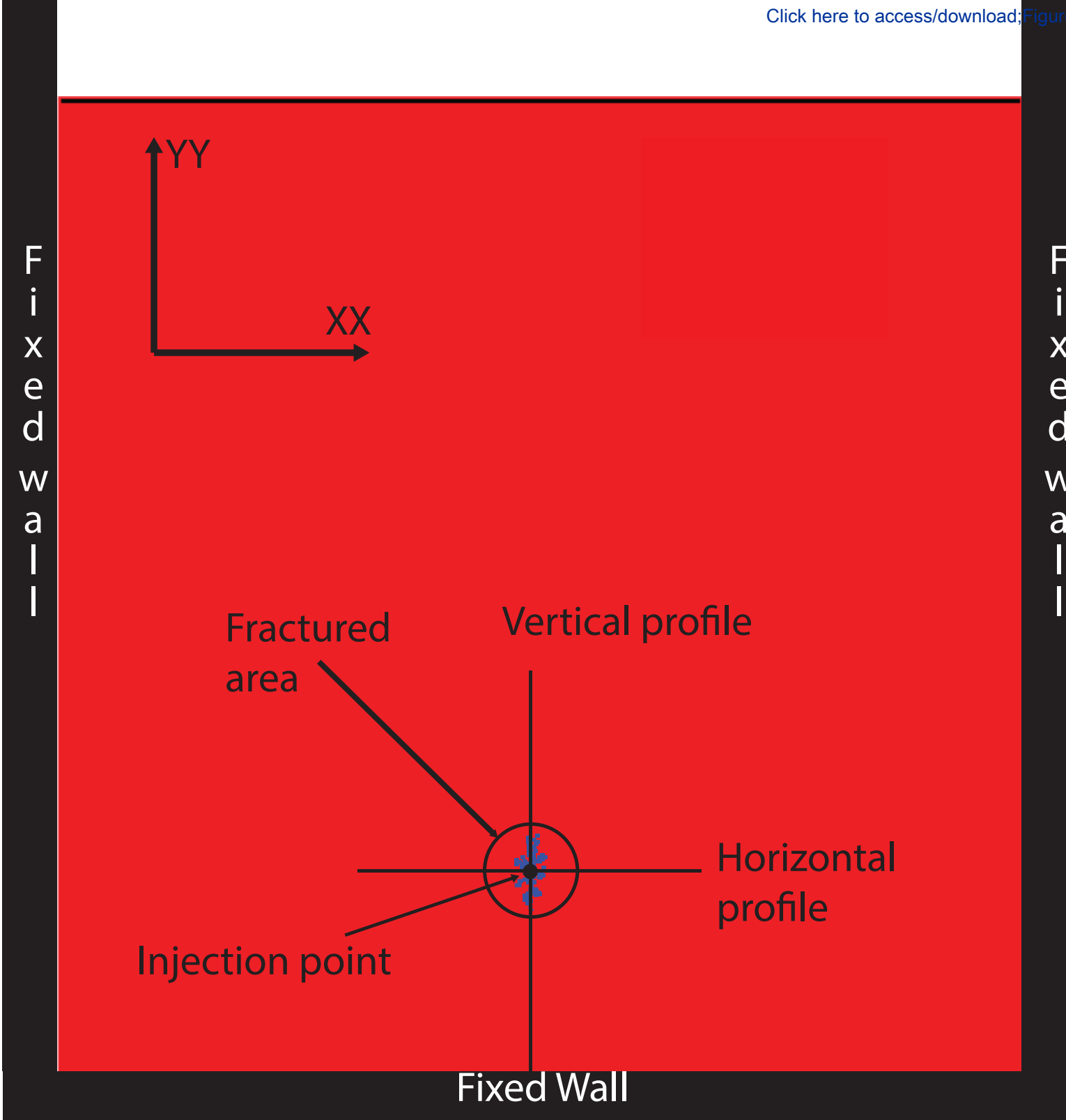


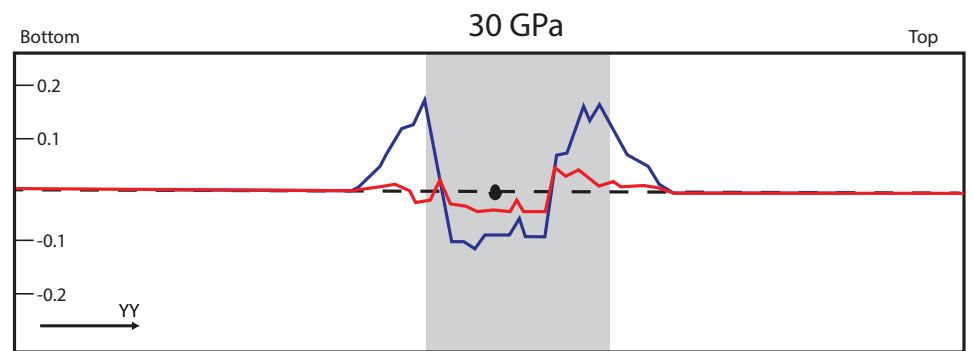
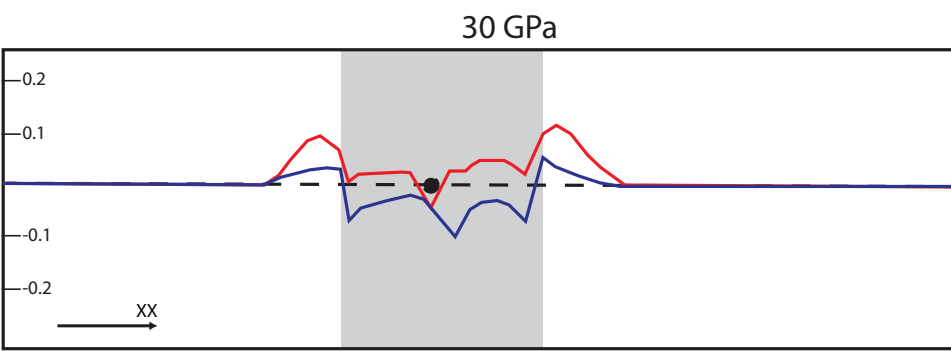
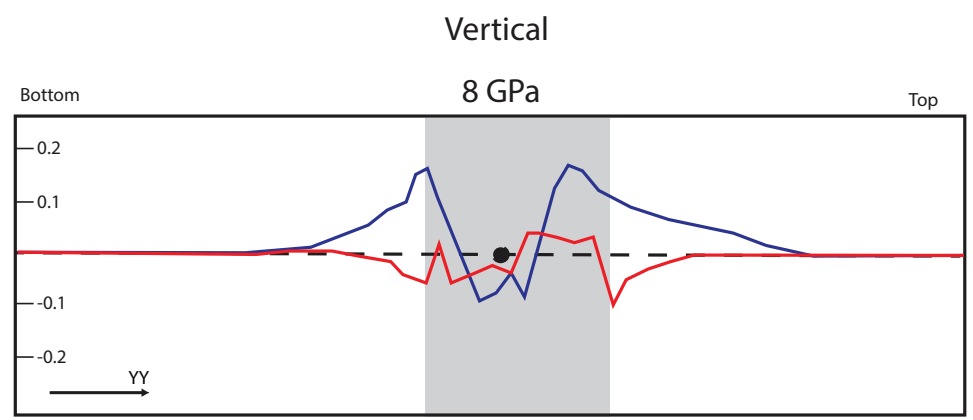
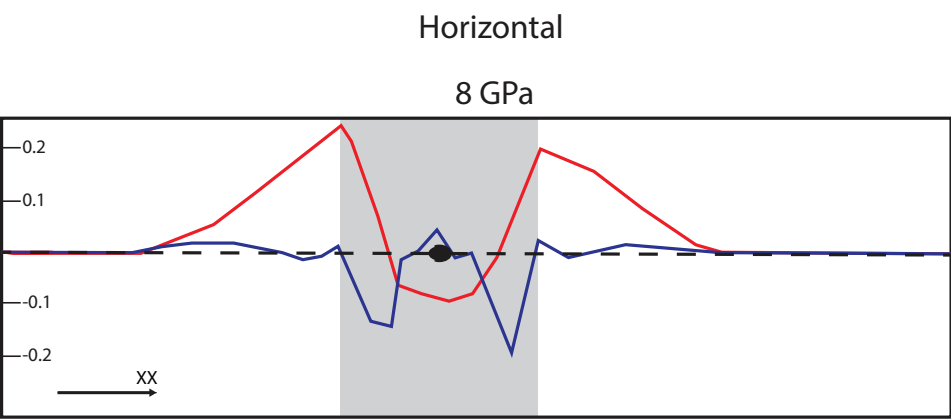








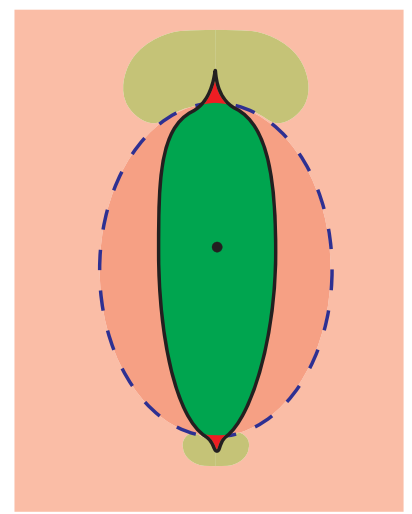
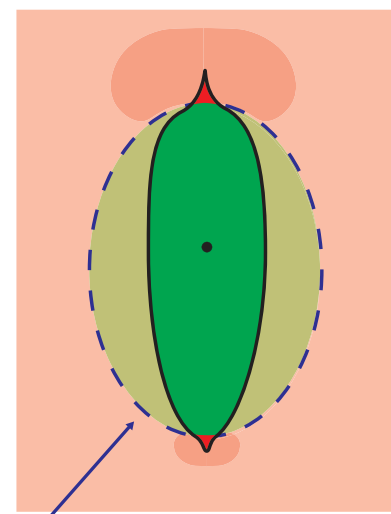




— YY stress — XX stress — Fractured Area

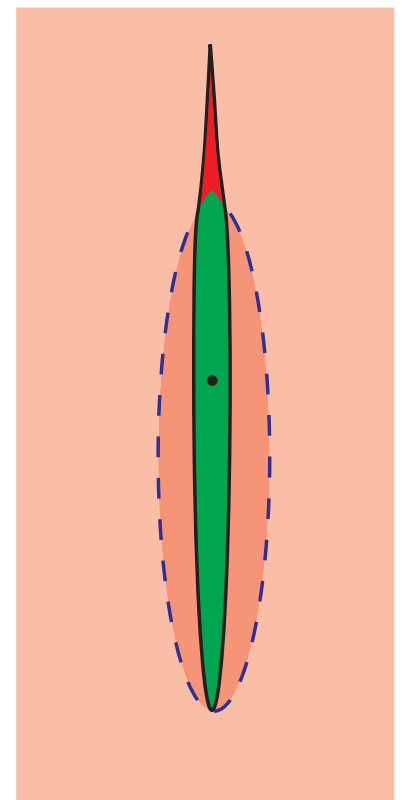
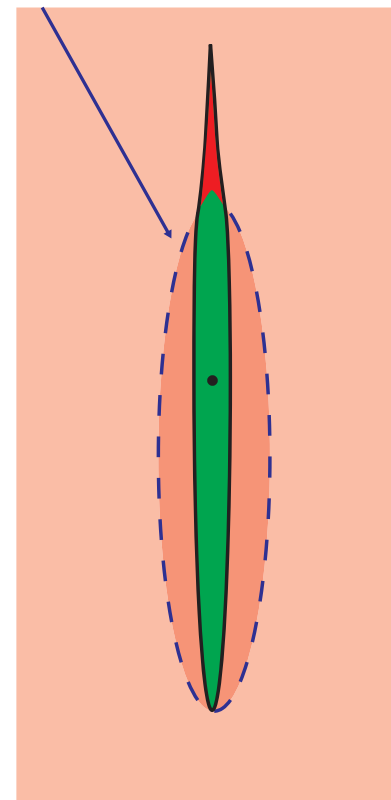
— YY stress — XX stress — Fractured Area


8 GPa




Compaction zone

30 GPa



 Compactional regime

 Extensional regime

