



Impact of grading on steady-state strength

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► To cite this version:

Sandra Linero Molina, Emilien Azéma, Nicolas Estrada, Stephen Fityus, J. Simmons, et al.. Impact of grading on steady-state strength. *Géotechnique Letters*, 2019, 9 (3), pp.1-19. 10.1680/jgele.18.00216 . hal-02314994

HAL Id: hal-02314994

<https://hal.science/hal-02314994>

Submitted on 14 Oct 2019

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Submitted: 03 December 2018

Published online in ‘accepted manuscript’ format: 10 June 2019

Manuscript title: Impact of grading on steady-state strength

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Abstract

In the mining industry, waste-dumps are earthen structures typically built by loose waste tipping. They may reach heights of hundreds of meters and undergo large deformations. For this reason, their stability design is based on the steady-state shear strength of the waste material. Waste materials are widely graded and may contain particles of up to metric order. Particle shape depends on the pattern of dissecting discontinuities at the source rock mass, and the relation between the size of the fragments and discontinuity spacing. The shear strength of this material is determined in the laboratory using scaled samples with altered Particle Size Distribution (PSD). However, altering the PSD is known to impact shear strength, and this impact is poorly studied. The representativeness of laboratory parameters obtained from scaled samples is thus arguable. Discrete element simulations are used here to investigate steady-state shear strength changes with alteration of the PSD when particle size and shape are correlated. It is observed, that shear strength changes result from the variation of the particle shapes induced by the alteration of the PSD. Consequently, identifying size-shape correlations and their potential impact on shear-strength is paramount when scaling materials for laboratory testing.

Keywords: Discrete-element modelling; Particle-scale behaviour; Shear strength

Introduction

Open-pit mining involves partitioning waste material to areas prepared for storage. An operational challenge is designing the waste-dumps, especially in steep terrains or restricted space. Some waste-dumps in Chile are designed to contain over 1000 million tonnes of material and will reach frontal heights of up to 900 m. (Linero *et al.*, 2007). Improperly designed dumps may represent source of pollution or stability hazards.

Waste-dumps are typically built by loose tipping and undergo large deformations. For stability design, the steady-state shear strength of the waste material is required. However, because blasted waste contains very large particles, the material must be scaled in the laboratory for testing, to suit available size-constrained equipment (Lowe, 1964; Duncan *et al.*, 2014; Fumagalli, 1969). Scaled samples have a smaller maximum particle-size and may have a different PSD shape compared with the original material.

Experimentally, it has been observed that manipulating the PSD affects shear strength, but the results are contradictory (Marachi *et al.*, 1972; Marsal, 1972; Varadarajan *et al.*, 2003; Linero *et al.*, 2007; Ovalle, 2013).

Numerical research 2D and 3D suggest that steady-state shear strength is independent of PSD for materials composed of unbreakable particles with similar shape across sizes (mono-shaped) (Azema *et al.*, 2017; Azéma *et al.*, 2017; Estrada, 2016; Voivret *et al.*, 2009; Nguyen *et al.*, 2015; Cantor *et al.*, 2018). This non-intuitive result motivated the investigation of particle size-shape correlation effects in greater detail in this research.

Correlation of particle size and shape can occur due to geological settings (CIRIA *et al.*, 2007) but it is rarely investigated. Size-shape correlation was observed for the particles between 4.75 mm and 75 mm of a mine-waste material, eroded from a sedimentary Precambrian-aged banded Iron formation in the Pilbara, Australia. This material was investigated at large-scale at the University of Newcastle. See Linero *et al.* (2017) for the size-shape statistics. In this correlation, larger pieces are more platy or discoidal than smaller particles. Size-shape correlation was also observed in particles between 2 mm and 100 mm quarried from a quartzitic schist in Trois Vellées, l'Orne, France. The material was investigated at large-scale at the University of Nantes in Ovalle (2013). In this correlation, smaller particles were thinner and more elongated than larger particles, in accordance with its anisotropic fabric that was prone to delamination under impacts and loads. Variability of shape by size can also be observed in Zeller & Wullimann (1957).

When particle size and shape are correlated, scaling simultaneously alters the distributions of both particle size and particles shape. This manuscript presents the results of 18 biaxial simulations prepared to evaluate systematically the effect of material gradation and size-shape correlation on steady-state shear strength.

Model description and numerical approach

The simulations were conducted with the Contact Dynamics (CD) method (Jean & Moreau, 1987, 1992; Jean, 1999). CD is a discrete element approach for simulation of non-smooth granular dynamics with contact laws expressing inter-particle mutual exclusion and dry friction. CD is unconditionally stable and well suited for simulating rigid particles and inter-particle frictional contacts; see Dubois *et al.* (2018), Radjaï & Dubois (2011), and Azéma *et al.* (2012). The multi-purpose software LMGC90 was used for the simulations. It was developed in Montpellier and is capable of modelling collections of particles of arbitrary shape (https://git-xen.lmgc.univ-montp2.fr/lmgc90/lmgc90_user/wikis/home).

To isolate the effects of gradation and size-shape correlation of particles on steady-state shear strength, two different materials were simulated: One was composed of mono-shaped particles (same shape across all sizes), and other was composed of size-shape correlated particles, with larger particles more elongated than the smaller ones. The idealised size-shape correlation imposed was inspired by the characteristics of the material analysed in Newcastle (Linero *et al.*, 2017).

To make the analysis systematic in terms of PSD variation, 9 different gradations were analysed, and the gradation was controlled using the size-span parameter s (Voivret *et al.*, 2007).

$$s = \frac{d_{max} - d_{min}}{d_{max} + d_{min}} \quad (1)$$

The maximum particle size d_{max} was fixed to 15 mm, and s was varied in the range $[0.1, 0.2 \dots 0.9]$ to cover a wide spectrum of gradations: from very uniform to very well-graded samples. See Figure 1.

Two simulations series were conducted. Series 1 evaluated the 9 gradations but using the material composed of mono-shaped disc particles. Series 2 evaluated the 9 gradations but using the material containing particles that are size to shape correlated.

The shape chosen to model the particles is rounded cap rectangles (RCRs) (Azéma & Radjaï, 2010). RCRs are characterised by diameter d (diameter of the inscribed circle) and aspect ratio $\lambda = L/d$ (Figure 2). A disc is a special case of RCR with $\lambda = 1$, corresponding to the shape selected for the simulations in Series 1.

To achieve a systematic correlation between particle size and shape in Series 2, a continuous function was found to define the elongation of each particle as a function of its diameter and of the gradation of its sample. With this, the broad spectrum was covered from samples of uniform size and shape, to samples of very polydisperse size and shape.

Two parameters that vary in the range $[0, 1]$ were introduced for convenience to define the correlation function: These are the reduced diameter d_r and the reduced aspect ratio. λ_r . d_r represents the relative size of a particle between d_{min} and d_{max} (Nguyen *et al.*, 2015). λ_r

represents the relative aspect ratio, with $\lambda_{min} = 1$, and λ_{max} corresponding to the maximum aspect ratio present in a given sample.

$$d_r = \frac{d - d_{min}}{d_{max} - d_{min}} \quad (2)$$

$$\lambda_r = \frac{\lambda - \lambda_{min}}{\lambda_{max} - \lambda_{min}} \quad (3)$$

To establish the correlation, and for simplicity, two hypotheses were made: (i) particle size and shape are linearly correlated (i.e., $d_r = \lambda_r$) and (ii) the maximum aspect ratio λ_{max} is also linearly correlated with s in the following way:

$$\lambda_{max} = \lambda_{min} + s(\lambda_{max}^* - \lambda_{min}) \quad (4)$$

λ_{max}^* , corresponds to the absolute maximum particle aspect ratio in Series 2. It was arbitrarily set to 5 with the most elongated particles only present in the most widely graded sample (i.e., for $s \rightarrow 1$).

With the previous conditions, the aspect ratio of a particle, as a function of its size and the size-span of its sample, is given by Equation 5. In it, large particles in a sample are also more elongated, and more widely graded samples (more polydisperse in particle sizes) are also more polydisperse in particle shapes. The latter aligns with the general situation observed in the industry; mine waste is well-graded and polydisperse in shapes right after production, but sampling and scaling reduce its polydispersities.

$$\lambda(d) = \lambda_{min} + s \frac{d - d_{min}}{d_{max} - d_{min}} (\lambda_{max}^* - \lambda_{min}) \quad (5)$$

Particle generation to compose each sample involved the following steps:

- The minimum sample diameter d_{min} was determined based on desired s .
- The size range was divided into ten sub-ranges, with limits that corresponded to reduced diameters $d_r = 0, 0.1, \dots, 1$.
- A population of particles was generated with diameters uniformly distributed in each sub-range of size.
- The aspect ratio of each particle was calculated using Equation 5 and applied.
- Particles were sampled from each sub-range, targeting an equal total area of particles by size sub-range, and 10,000 particles in the sample.

To obtain homogeneous systems for testing, the particles of each sample were mixed and oriented randomly before placing them into a rectangular testing box. Gravity and inter-particle friction were set to zero to promote homogeneous packings, and the systems were isotropically compressed at $\sigma_0 = 10 \text{ kN/m}^2$ (Figure 3). The compression was maintained until

coordination number (average number of contacts per particle) and packing fraction (see Equation 9) stabilised (see close-up views at the end of compression in Figure 4).

For biaxial shearing, friction between the box wall and particles was maintained to zero and inter-particle friction was set to 0.4. The samples were biaxially sheared, by imposing a vertical velocity v_y to the top wall of the box and maintaining the confining stress imposed for compression.

v_y was set for an inertial number $I = 5 \cdot 10^{-4}$. This condition ensured quasi-static stress state during shearing, as discussed in GRD-MiDi (2004). I is defined as $(v_y / H) \langle d \rangle \sqrt{\rho / \sigma_0}$, where H is the sample height, $\langle d \rangle$ is the mean particle diameter, and ρ is particle density. The test was finalised at 50% of vertical strain (Figure 5).

Mechanical behaviour

The shear strength was described by the internal friction angle φ , obtained as:

$$\varphi = \arcsin \frac{q}{p} \quad (6)$$

where p and q are the mean and deviatoric stresses respectively. This required the stress tensor σ to be calculated from the contact network and inter-particle force data (Rothenburg & Selvadurai, 1981; Christoffersen *et al.*, 1981; Bagi, 1999).

To calculate σ , the internal moment tensor M of each grain g must first be computed by:

$$M_{i,j}^g = \sum_{c \in g} f_i^c \ell_j^c \quad (7)$$

where i and j represent the components in an orthonormal reference frame, f is the force exerted on the grain at contact c , ℓ is the branch vector (joining the centres of the particles in contact), and the sum runs over all contacts of the grain. The stress tensor of a granular assemblage of area A is given by

$$\sigma_{i,j}^A = \frac{1}{A} \sum_{g \in A} M_{i,j}^g \quad (8)$$

The sample's density was quantified by the packing fraction ν :

$$\nu = \frac{A_p}{A} \quad (9)$$

where A_p is the area of all particles and the sample's porosity is $n = 1 - \nu$.

Figure 6 presents the evolution of q/p and ν as functions of the vertical strain ε_y .

As classically observed during a biaxial test of well-packed collections of particles, the shear stress increased rapidly at the beginning of the test, reaching a maximum value before decreasing towards a constant value at large strain (steady state). The jump observed at $\varepsilon_y = 0$ is typical in this kind of numerical test. It reflects both rigidity of the particles and good packing at the initial state. The packing fractions reduced across the test and tended to a constant value at large strain.

Figure 7 shows the steady-state internal friction angle φ_{ss} and packing fraction ν_{ss} (i.e., the average values for $\varepsilon_y > 0.35$), as functions of material gradation characterised by the size-span s .

In the case of discs (mono-shaped), φ_{ss} remains almost invariant, in the range of 16 ± 1 degree, for any of the 9 gradations analysed (from very uniform to very well graded samples). It is established that the strength properties of granular media result from the build-up of anisotropic structures of geometrical and mechanical origins (Rothenburg & Bathurst, 1989). The independence of strength with PSD in the case of mono-shaped particles has been discussed previously in the literature in Azema *et al.* (2017), Azéma *et al.* (2017), Estrada (2016), Voivret *et al.* (2009), Nguyen *et al.* (2015), and Cantor *et al.* (2018). It has been attributed to a compensation mechanism which involves both the contact orientation anisotropy which decreases with the size span, and the branch length anisotropy which behaves inversely.

In contrast, in the case of RCR particles (size and shape correlated), φ_{ss} increases from 18 to 23 degrees when the gradation improves from $s = 0.1$ to $s = 0.5$; Then remains about constant between $s = 0.5$ and $s = 0.8$, and finally reduces for the most graded sample $s = 0.9$. In this case, the mechanism explaining the behaviour is more complex as geometrical effects induced by particle size are enhanced by orientational effects introduced by elongated particles (Azéma & Radjaï, 2010) as well as mechanical effects in presence of faceted particles (Azéma & Radjaï, 2010; Nguyen *et al.*, 2015). This analysis will be presented by the authors in a separate paper.

Packing fraction, as expected, increases with gradation (size-span s) for both series.

Conclusions and perspectives

Altering the PSD of materials composed of particles with the same shape across all different sizes has a marginal effect on steady-state shear strength. But in contrast, if the size and shape of the particles are correlated, the alteration of the PSD affects the steady state shear strength.

This finding suggests that shape distribution has a higher influence than size distribution on the steady-state shear strength of granular materials.

This implies, that the steady-state shear strength changes observed in physical testing when the PSD is modified may be more related to the alteration in the distribution of particles' shapes than to the distribution of particles' sizes.

The results are important because correlations between the size and shape of natural and blasted materials due to geological and structural processes happens as has already been described in the literature.

Hence, sampling and scaling of coarse granular materials like mine wastes, for laboratory testing, may inadvertently cause alteration of particle shape polydispersity, making it difficult to capture the mechanical behaviour of the original material in the laboratory.

Alteration of the particle shape polydispersity in sample preparation for physical testing should be recognised and its potential implications discussed in standard geotechnical practice.

Acknowledgements

Thanks to Fortescue Metals Group Ltd. for funding this research along with SRK Consulting, the University of Newcastle and the Australian Government Department of Education and Training. This work has been also supported by the project Ecos Nord No. C19P01/63672. Thanks to Aaron Scott and Remy Mozul for IT support at the University of Newcastle and Montpellier respectively.

Notations

d_{max}	maximum particle size
s	size-span parameter
d	diameter
d_r	reduced diameter
λ_{max}^*	corresponds to the absolute maximum particle aspect ratio in Series 2
p and q	are the mean and deviatoric stresses respectively
M	moment tensor
i and j	represent the components in an orthonormal reference frame

References

- Azéma, E., Estrada, N. & Radjai, F. (2012). Nonlinear effects of particle shape angularity in sheared granular media. *Physical Review E* **86**, No. 4, 041301.
- Azéma, E., Linero, S., Estrada, N. & Lizcano, A. (2017). Does modifying the particle size distribution of a granular material (ie, material scalping) alters its shear strength? *European Physical Journal Conferences*. **140**, No. 06001, doi:DOI: 10.1051/epjconf/201714006001.
- Azema, E., Linero, S., Estrada, N. & Lizcano, A. (2017). Shear strength and microstructure of polydisperse packings: The effect of size span and shape of particle size distribution. *Physical Review E* **96**, No. 022902, doi:10.1103/PhysRevE.96.022902.
- Azéma, E. & Radjaï, F. (2010). Stress-strain behavior and geometrical properties of packings of elongated particles. *Physical Review E* **81**, No. 5, 051304.
- Bagi, K. (1999). Microstructural stress tensor of granular assemblies with volume forces. *Journal of Applied Mechanics* **66**, No. 4, 934–936.
- Cantor, D., Azema, E., Sornay, P. & Radjai, F. (2018). Rheology and structure of polydisperse three-dimensional packings of spheres. *Physical Review E* **98**, 052910.
- Christoffersen, J., Mehrabadi, M. & Nemat-Nasser, S. (1981). A micromechanical description of granular material behavior. *Journal of Applied Mechanics* **48**, No. 2, 339–344.
- CIRIA, CUR & CETMEF (2007). *The rock manual - the use of rock in hydraulic engineering*. 2 edn., London: C683, CIRIA.
- Dubois, F., Acary, V. & Jean, M. (2018). The contact dynamics method: A nonsmooth story. *Comptes Rendus Mécanique* **346**, No. 3, 247–262.
- Duncan, J. M., Wright, S. & Brandon, T. L. (2014). *Soil strength and slope stability*. 2 edn., Wiley.
- Estrada, N. (2016). Effects of the grain size distribution on the packing fraction and shear strength of frictionless disk packings. *Physical Review E*.
- Fumagalli, E. (1969). Tests on cohesionless materials for rockfill dams. *Soil Mechanics and Foundation Division ASCE* **6353**, No. SM1.
- GRD-MiDi, G. (2004). On dense granular flows. *The European Physical Journal E* **14**, No. 4, 341–365.
- Jean, M. (1999). The non-smooth contact dynamics method. *Computer Methods in Applied Mechanics and Engineering* **177**, No. 3-4, 235–257.
- Jean, M. & Moreau, J. J. (1987). Dynamics in the presence of unilateral contacts and dry friction: a numerical approach. In *Unilateral Problems in Structural Analysis—2*, Springer, pp. 151–196.
- Jean, M. & Moreau, J. J. (1992). Unilaterality and dry friction in the dynamics of rigid body collections. In *Proceedings of Contact Mechanics International Symposium*, vol. 1, pp. 31–48.
- Linero, S., Fityus, S., Simmons, J. & Cassidy, J. (2017). Trends in the evolution of particle morphology with size in colluvial deposits overlying channel iron deposits. *European Physical Journal Conferences*. **140**, No. 14005, doi:DOI: 10.1051/epjconf/201714014005.
- Linero, S., Palma, C. & Apablaza, R. (2007). Geotechnical characterization of waste material in very high dumps with large scale triaxial testing. In *Slope Stability 2007* (ACG, ed.), Australian Centre for Geomechanics.

- Lowe, J. (1964). Shear strength of coarse embankment dam materials. In *Congress on Large Dams ICOLD*, vol. 3, pp. 745–761.
- Marachi, N. D., Chan, C. K. & Seed, B. (1972). Evaluation of properties of rockfill materials. *Journal of Soil Mechanics and Foundation Division (JSMFD)*, American Society of Civil Engineers (ASCE) **98**, No. SM1, 95–115.
- Marsal, R. J. (1972). *Resistencia y compresibilidad de enrocamientos y gravas*, vol. 306. México: UNAM, Instituto de Ingeniería.
- Nguyen, D.-H., Azema, E., Sornay, P. & Radjai, F. (2015). Effects of shape and size polydispersity on strength properties of granular materials. *Physical Review E* **91**, No. 032203.
- Ovalle, C. (2013). *Contribution a l'étude de la rupture des grains dans les matériaux granulaires*. Theses, Ecole Centrale de Nantes (ECN), URL <https://tel.archives-ouvertes.fr/tel-00979827>.
- Radjaï, F. & Dubois, F. (2011). *Discrete-element modeling of granular materials*. isbn: 978-1-84821-260-2. Wiley-Iste.
- Rothenburg, L. & Bathurst, R. (1989). Analytical study of induced anisotropy in idealized granular materials. *Geotechnique* **39**, No. 4, 601–614.
- Rothenburg, L. & Selvadurai, A. (1981). A micromechanical definition of the cauchy stress tensor for particulate media. *Mechanics of Structured Media*, 469–486.
- Varadarajan, A., Sharma, K. G. & Venkatachalam, K. (2003). Testing and modeling two rockfill materials. *Journal of Geotechnical and Geoenvironmental Engineering ASCE* **129**, No. 3, 206–218.
- Voivret, C., Radjai, F., Delenne, J.-Y. & El Youssoufi, M. S. (2007). Space-filling properties of polydisperse granular media. *Physical Review E*. **76**, No. 021301, 12.
- Voivret, C., Radjai, F., Delenne, J.-Y. & El Youssoufi, M. S. (2009). Multiscale force networks in highly polydisperse granular media. *Physical Review Letters* **102**, No. 17, 178001.
- Zeller, J. & Wullimann, R. (1957). The shear strength of the shell materials for the goeschenenalp dam, switzerland. In *Proceedings of the Fourth International Conference on Soil Mechanics and Foundation Engineering*, vol. 2, London: ISSMGE, pp. 399–404.

Figure captions

- Figure 1. Particle size distribution of tested samples, with uniform solids volume by particle size. The colours illustrate the different values of size-span used s . The limits $s \rightarrow 0$ and $s \rightarrow 1$ represent uniform and very well graded samples, respectively.
- Figure 2. The particles were rounded cap rectangles (RCR). (a) RCR with aspect ratio $\lambda = L/d > 1$, (b) Disc (i.e., RCR with $\lambda = 1$).
- Figure 3. Frontal view. Sample of RCR particles and size-span $s=0.3$ (a) Close-up view of the particles at Initial state, after deposition (b) Close-up view of the particles at the end of the isotropic compression. The colour scale represents total compression displacements.
- Figure 4. Closeup views of some of the samples at the end of isotropic compression. (a) Discs series. (b) RCRs series. Grey particles represent floating particles (i.e. with zero or one contact).
- Figure 5. Frontal view. Illustration of the biaxial shear test. Sample RCR $s=0.50$. The gradient of colour indicates vertical displacements. (a) Beginning of shearing. (b) Halfway of shearing.
- Figure 6. Evolution of shear stress ratio q/p and packing fraction ν as functions of the vertical strain ε_y , for (a) the series composed of mono-shaped particles (discs), and (b) the series composed of particles with size and shape correlated (RCRs). Colours represent different values of the sample size-span s , which vary between $s=0.1$ (close to uniform) and $s=0.9$ (very well graded or polydisperse).
- Figure 7. Comparison of steady-state angle of internal friction ϕ_{ss} and packing fraction ν_{ss} as functions of the PSD-size-span s of the two different series: Series 1 composed of mono-shaped (discs) particles, and Series 2 composed of particles with size and shape correlated (RCRs). Values shown are averages of data for strains over 35% ($\sim$ steady state, see Figure 6). The error bars capture the scatter of the readings (see Figure 6) and indicate the standard deviation.

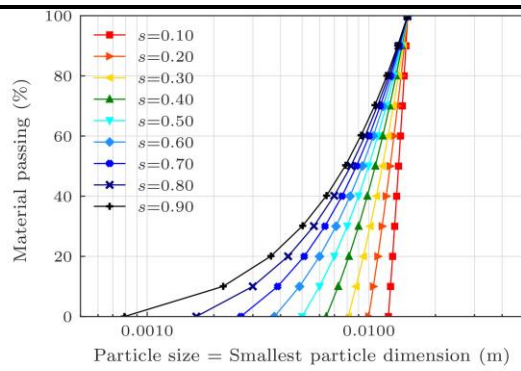


Fig. 1.

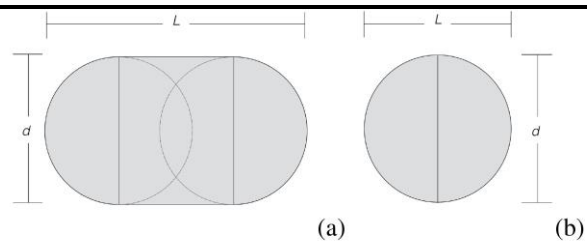


Fig. 2.

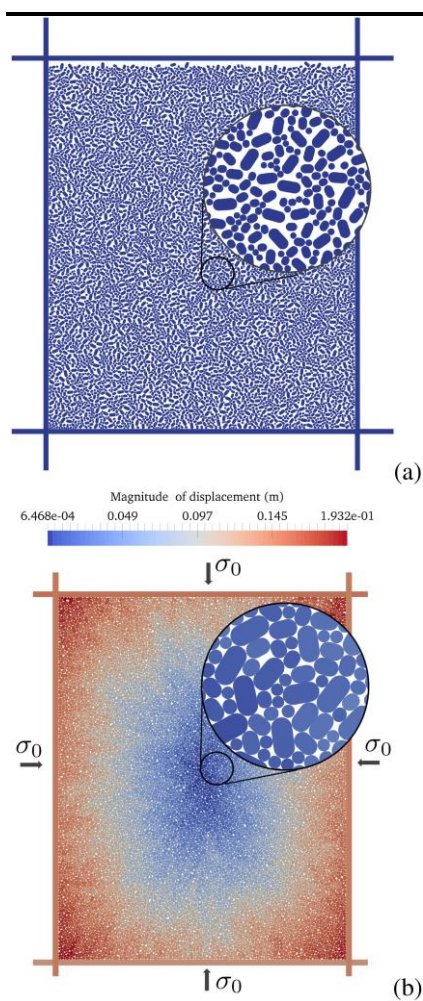


Fig. 3.

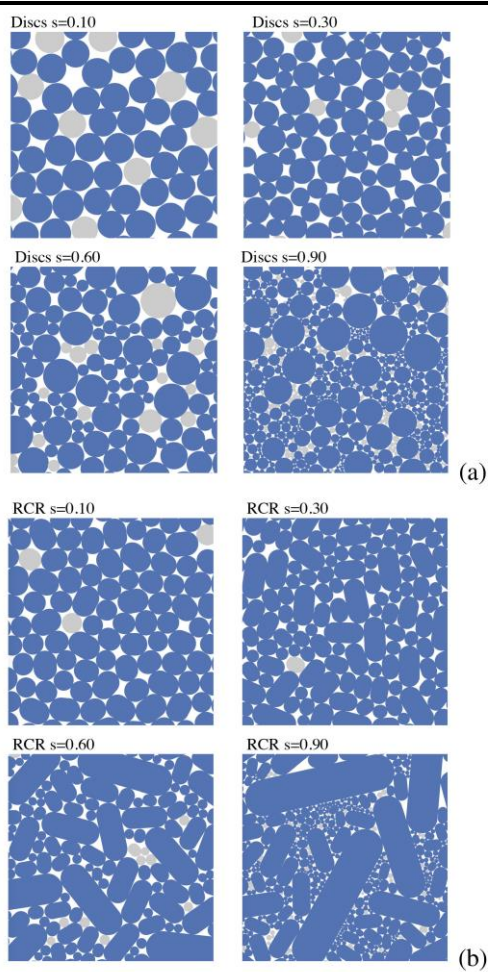


Fig. 4.

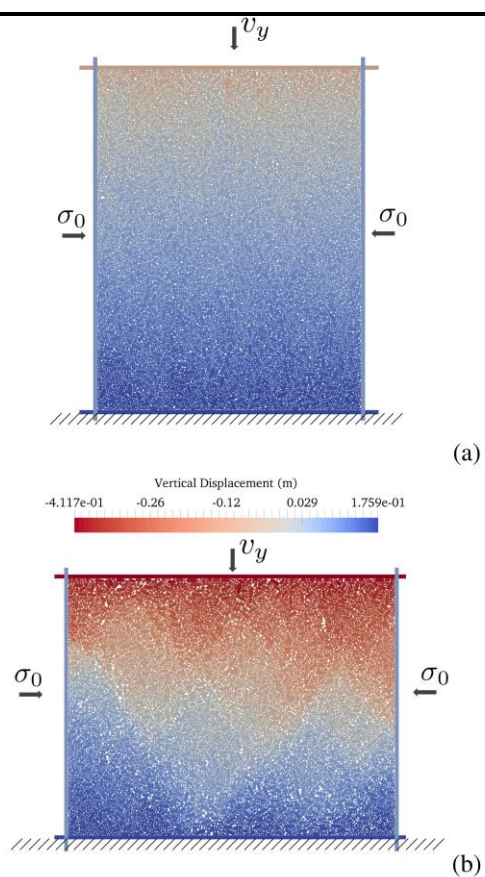
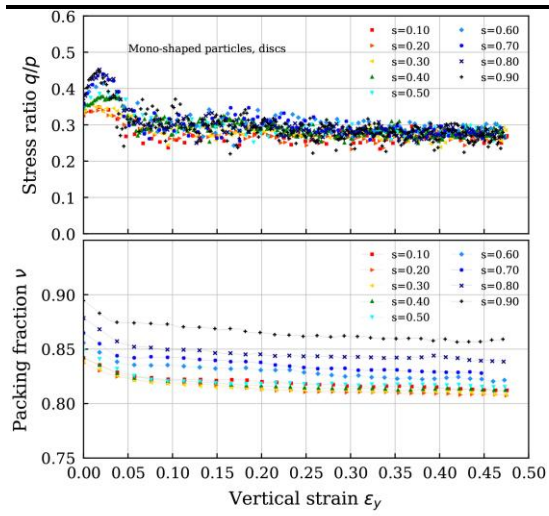
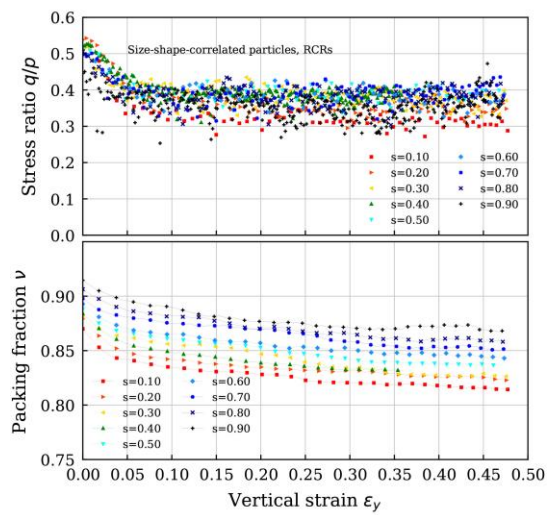


Fig. 5.



(a)



(b)

Fig. 6.

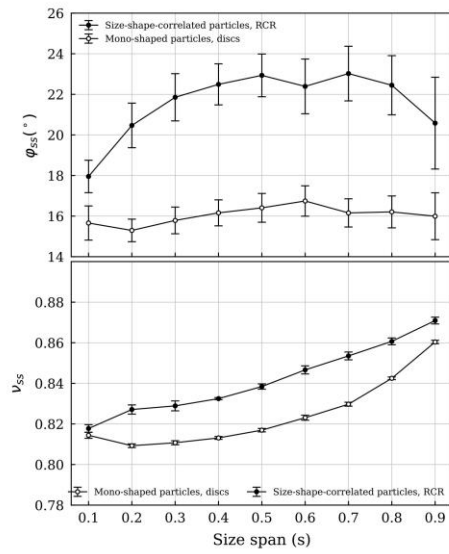


Fig. 7.