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E. El Haber, C. Cornou, D. Jongmans, D. Youssef Abdelmassih, Fernando Lopez-caballero, et al.. Influence of 2D heterogeneous elastic soil properties on surface ground motion spatial variability. Soil Dynamics and Earthquake Engineering, 2019, 123, pp.75-90. 10.1016/j.soildyn.2019.04.014 . hal-02126090

# HAL Id: hal-02126090 https://hal.science/hal-02126090v1

Submitted on 22 Oct 2021

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1	Influence of 2D heterogeneous elastic soil properties on surface ground motion
2	spatial variability
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11	
12	Abstract
13	Spatial variability of earthquake ground motion (SVEGM) refers to the differences in amplitude and phase
14	between recordings of the same earthquake at different locations. In the near-surface, geological processes
15	(sedimentation, erosion) and anthropogenic activities can lead to small scale spatial heterogeneities of soil
16	mechanical properties, which may affect SVEGM. In this paper, the effect of shallow 2D spatial variability of
17	the shear wave velocity (Vs) on the surface ground motion is assessed through a set of numerical experiments,

1 using a simple 2D velocity structure (a sedimentary layer over a half-space). Non-linearity or damping are not 18 19 considered in the wave propagation calculation in order to solely focus on the effects of soil elastic property 20 variability. Vs is modeled as a random field using the EOLE method (Expansion Optimal Linear Estimation) and 21 considering three statistical parameters: the coefficient of variation, and the horizontal and vertical 22 autocorrelation distances. Seismic ground motions are numerically simulated for a plane wave excitation with 23 SV polarization. Modeling results clearly highlight the scattering of surface waves by ground heterogeneities, 24 leading to large spatial variations in surface ground motion. We computed surface ground motion indicators 25 (resonance frequency, spectral amplification, Arias intensity and duration) and we showed that their spatial

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variability is mainly controlled by the *Vs* coefficient of variation. A comparison between 2D and 1D ground motion probabilistic modeling shows that the 1D probabilistic approach may correctly reproduce average fundamental resonance frequencies and corresponding amplification. However, the 1D approach significantly under-predicts both ground motion amplification at higher frequencies and related variabilities, as well as Arias intensities and inferred durations, which are all controlled by the generation of locally diffracted surface waves.

31 Key words: Seismic response, spatial variability, probabilistic modeling.

#### 32 1. Introduction

33 Spatial variation of earthquake ground motion (SVEGM) refers to the differences in amplitude and phase 34 between recordings of the same earthquake at different locations. This variation is mainly caused by seismic 35 source rupturing heterogeneities, regional wave propagation scattering and modification of ground motion by 36 surface geology, most often referred as site effects (Harichandran, 1999). One of the key factor controlling site 37 effects is the presence of soft deposits overlying a more rigid geological formation. In sedimentary structures 38 like valleys, soft surficial deposits may exhibit lateral variation in thickness, which lead to significant changes in 39 surface ground motion even at close distance (e.g. Moczo and Bard, 1993; Field et al., 1996; Graves et al., 1998; 40 Pagliaroli et al., 2014b). At local scale (tens to hundreds of meters), the spatial heterogeneity of near-surface 41 material results from the surface natural processes of erosion and sedimentation (Einsele, 2000) and human 42 activities (construction, mining, ...) generating anthropogenic deposits that can reach more than 10 m (e.g. 43 Jongmans and Campillo, 1990; Pagliaroli et al., 2014a). Additionally, intrinsic variation in properties within the 44 soil layer contributes to the spatial variability (Jenny, 1941; Burrough, 1993). The soil heterogeneity at various 45 spatial scales may then have a significant effect on SVGEM, as frequently pointed out by the damage studies 46 after earthquakes, especially on long-span structures such as dams, bridges and lifeline facilities (Ariman et al., 47 1981; Anagnostopoulos, 1988; Trifunac et al., 1997; Trifunac, 2009; Bradley et al., 2011; for a summary of 48 historic observations, refer to Kozák, 2009). In particular, SVGEM has been put forward to explain the damage 49 to the high rise buildings during the San Fernando earthquake in 1971 (Hart et al., 1975) and to the bridges 50 during the earthquake of Loma Prieta in 1989 (Kiureghian et al. 1992), of Northridge in 1994 (Hall et al., 1994) 51 and Christchurch in 2011 (Chouw and Hao, 2012).

52 There is therefore a need to quantify the soil parameter spatial variability and uncertainty for predicting 53 SVGEM. To address this issue, probabilistic modeling approaches using random fields (Popescu, 1995) have 54 been widely used in geotechnical engineering. In particular, such studies have been made on soil liquefaction

(Lopez-Caballero and Modaressi, 2010), on the stability of soil dams and slopes (Fenton and Griffiths, 1996; 55 56 Abdellah et al., 2000; Griffiths and Fenton, 2004), on the analysis of shallow foundations (Youssef Abdel 57 Massih and Soubra, 2008; Soubra and Youssef Abdel Massih, 2010; Al-bittar and Soubra, 2013 and Al-bittar 58 and Soubra 2017) and on the analysis of equipartition time and diffusion regime in random heterogeneous media 59 (Khazaie et al., 2016 and 2017). In contrast, these techniques have aroused only limited interest among the 60 seismological community, probably because of the difficulty of measuring small spatial scale variation in soil 61 mechanical properties (Pagliaroli, 2014a; Salloum et al., 2014). The few 2D probabilistic approaches performed 62 so far in earthquake engineering (Assimaki et al., 2003; Nour et al., 2003; Thompson et al., 2009; Pagliaroli et al., 2014a) have highlighted the influence of the spatial variability of the soil properties on the surface ground 63 64 motion. However, these papers mix effect of variable input ground motion with variability of linear and/or nonlinear soil properties on different surface ground motion indicators (spectral response, Peak Ground 65 Acceleration, amplification, Housner intensity), making thus difficult the understanding of the respective impact 66 67 of source of variability (input ground motion, linear and non-linear soil spatial variability) on SVGEM.

68 This paper aims at numerically studying the effect of the 2D soil spatial variability on SVGEM in the linear 69 domain for a model consisting of one variable soil layer overlying an elastic bedrock, derived from a well-70 known site (alluvial plain of Beirut, Lebanon). The chosen random soil parameter is the shear-wave velocity 71 (Vs), the spatial variability being discretized by random fields using the Expansion Optimal Linear Estimation 72 method (EOLE) (Li et Der Kiureghian, 1993). Synthetic seismograms at surface receivers are then computed 73 using the FLAC2D Finite difference code. First, the effect of the 2D spatial variability of Vs at a single surface 74 receiver is evaluated on the following scalar indicators: resonance frequency, corresponding amplification, as 75 well as Arias based intensity and duration. Second, impact of spatially variable ground structure on the spatial 76 correlation of surface ground motion is studied. Since some authors (Rahtje et al., 2010; Rodriguez-Marek et al., 77 2014; Haji-Soltani et al., 2017) chose to replace the 2D spatially variable seismic velocity profiles by 1D 78 spatially variable ones for site-specific hazard assessment, we also compare surface ground motion indicators 79 and related variability inferred from 2D and 1D modelling by using 1D Vs profiles extracted from the 2D Vs 80 model.

#### 82 2. Introducing ground structure spatial variability in wave propagation modeling

# 83 2.1. <u>Strategy</u>

84 Spatial variability of soil properties can be modeled by using the random field theory (Vanmarcke, 2010), 85 defined by two functions (1) a Probability Density Function (pdf), which is usually positive-skewed and non-86 Gaussian in soils (Popescu, 1995), and (2) an autocorrelation function  $\rho(x,z)$ . In this paper, the 2D shear-wave 87 velocity structure,  $V_s(x,z)$ , is modeled as a random field with  $\mu_{Vs}$  and  $\sigma_{Vs}$  being the mean and the standard deviation of the *PDF*, respectively. The coefficient of variation,  $COV = \mu_{Vs}/\sigma_{Vs}$ , quantifies the range of dispersion 88 89 of Vs around the mean. The autocorrelation function is defined by its spatial fluctuation scale described by the 90 correlation distances  $\theta_x$  and  $\theta_z$  along the horizontal and vertical directions, respectively, which are the distances 91 over which the soil property values are not correlated any more.

92 A number n of discretized 2D Vs models (called realizations) are generated using the probability density 93 function and then used to simulate seismic wave propagation. Like any probabilistic modeling, n should be large 94 enough to ensure the statistical convergence of the surface ground motion indicators (resonance frequency, 95 spectral amplification, Arias intensity, duration ...) in terms of average value and standard deviation. Among the 96 numerous methods of random field discretization (Sudret et al., 2000), the Expansion Optimal Linear Estimation 97 method (EOLE) belonging to the family of the series expansion methods (Li and Der Kiureghian, 1993) is used 98 in this study. The main reason is that the convergence of ground motion indicators is reached after a lower 99 number of seismic wave propagation simulations, compared to traditional Monte Carlo approaches (Sudret et 100 al., 2000).

In order to understand near-surface spatial *Vs* variation effects on surface ground motion, we focus on a parametric sensitivity study for a simple case of a spatially variable soil layer overlying a bedrock with constant elastic properties. The *Vs* structure of the alluvial plain in Beirut (Lebanon) is chosen as a test site since detailed geotechnical and geophysical campaigns carried out at this site have allowed to image spatial variation of the underground geological structure (Saloum et al., 2014; Salloum, 2015).

### 106 2.2

#### 2.2. Soil statistical parameters

In the approach adopted in this paper, the three statistical parameters describing the soil spatial variability are the coefficient of variation *COV* and the two autocorrelation distances  $\theta_x$  and  $\theta_z$ . Numerous studies attempted to determine these parameters from geotechnical laboratory or in situ tests (for a recent synthesis, see Salloum, 2015). In contrast, very few studies were made on the quantification of the *Vs* variability. This probably results
from the difficulty to obtain reliable *Vs* values through in situ tests.

112 In geotechnics, penetration tests (SPT – Standard penetration Tests or CPT – Cone penetration Tests) are widely 113 used for soil investigation (Cornforth, 2005). They provide resistance parameters (N: number of blows for SPT; 114  $q_c$ : cone resistance for CPT) that, like Vs, mainly increase with soil density or compactness if the effect of the 115 effective stress due to overburden pressure is removed (Karray et al., 2011). For these two parameters, the 116 coefficient of variation was found to cover a wide range between 5 and 43% (e.g Phoon and Kulhawy, 1999; 117 Srivastava and Babu, 2009; Zhao et al., 2018). For getting the coefficient of variation (COV) of Vs, Thomson et al. (2007) compiled results of Vs measurements using different techniques (down-hole tests, seismic cone tests) 118 119 in various geologic units (Holzer et al, 2005; Wills and Clahan, 2006; Moss, 2008). They found that COV 120 values are generally contained within the interval from14 to 46% for Vs measurements in the same geologic 121 unit. COV values in a similar range are also reported by Haji-Soltani and Penscheck (2017) for a very soft 122 sediment (5 to 15%) and by Rodriguez-Marek et al. (2014) for very hard rock site (10 to 25%). Making use of 123 the Electric Power Research Institute (EPRI, 1993) Vs profile database, Toro (1995) found COV ranging 124 between about 30% and 45% for various NEHRP site classes.

126 Table 1: Properties in the two layers (soil and bedrock) for the probabilistic modeling. The bedrock properties

127 are fixed, as well as the density  $\rho$  and the P-wave velocity Vp in the soil layer. The soil S-wave velocity is

- 128 considered as a random field and characterized by four statistical parameters ( $\mu_{Vs}$ , COV,  $\theta_x$ ,  $\theta_z$ ). Nine models
- 129 are defined with various COV,  $\theta x$  and  $\theta z$  (see text for details). The so-called deterministic model is defined by
- 130 *the values shown in italics in the Table.*

	Properties						
Layers	ρ (Kg/m3)	Vp (m/s)	$\mu_{Vs}$ (m/s)	<i>COV</i> (%)	θx	θy	Model #
				007 (70)	(m)	(m)	
				5	5	2	1
				5	10	2	2
5.5 m					5	2	3
ss of 1				20	10	1	4
ickne	1600 1500	1500	220		-	2	5
th a th					20	2	6
Soil with a thickness of 15.5 m				40	5	2	7
					10	1	8
					2	9	
Bedrock	2500	3000	1000				

131

132 The autocorrelation parameters in a soil layer  $\theta_x$  and  $\theta_z$  can be estimated by using several methods (e.g. Popescu, 133 1995; Assimaki et al., 2003; Thomson et al., 2009), depending on the available data and the chosen 134 autocorrelation function. As most sedimentary layers are characterized by a strong anisotropy resulting from 135 depositional processes, the horizontal autocorrelation distances are expected to be greater than vertical ones. In-136 situ geotechnical and Vs tests are usually performed along vertical profiles and most of the available information is on the vertical autocorrelation distance  $\theta_r$ . Thus,  $\theta_r$  is usually poorly constrained. Penetration test analyses 137 138 provide a relatively wide range for the vertical autocorrelation distances, from a few cm to a few m, while the 139 horizontal distances were estimated from around 1 m to more than 100 m (e.g., Alonso et al., 1975; Phoon and 140 Kulhawy, 1996; Fenton, 1999; Jaksa et al., 1997; Assimaki et al., 2003; see Salloum, 2015 for a synthesis).

141 Using suspension log slowness data, Thomson et al. (2009) determined a maximum  $\theta_z$  value of about 5 m;  $\theta_x$ 142 was estimated to 50 m to 100 m, assuming an anisotropy factor of 10 to 20. These results illustrate the wide 143 scattering of soil statistical parameter values, resulting from the wide variety in geological depositional contexts 144 (marine, lake, glacial, alluvial sediments), and from the large uncertainty on the horizontal autocorrelation 145 distance quantification.

146 As our parametric study is built on the alluvial plain of the river Nahr Beirut (Beirut, Lebanon), the statistical 147 parameters determined from the extensive near-surface geotechnical (borehole measurements, SPT, laboratory 148 measurements) and geophysical (seismic, resistivity) campaigns (Salloum et al., 2014; Salloum, 2015) are used 149 hereafter. These experiments showed the presence of interbedded Quaternary layers of pebble, gravel, sand and 150 clay overlying marly limestone of Tertiary age, with strong vertical and horizontal variability. The typical soil 151 column is, from the surface to depth: (1) a 7.5 m thick gravel layer with Vs=350 m/s, (2) a softer clay layer of 8 152 m in thickness with Vs=150 m/s and (3) weathered limestone (Vs>400 m/s). Fitting a theoretical lognormal 153 probability density function on down-hole tests data led to a Vs coefficient of variation (COV) of 13% and 44% 154 in the clay and gravel layers, respectively (Salloum, 2015). The vertical autocorrelation distance  $\theta_r$  was found to 155 vary between 0.5 m and 2 m in the soil layers, using the same data. ERT (Electrical Resistivity Tomography) profiles were used to evaluate the horizontal autocorrelation distance  $\theta_x$ , which was bracketed between 3.8 and 156 10.6 m (Salloum, 2015), assuming that the variability is the same for the two geophysical parameters (shear 157 158 wave velocity and electrical resistivity).

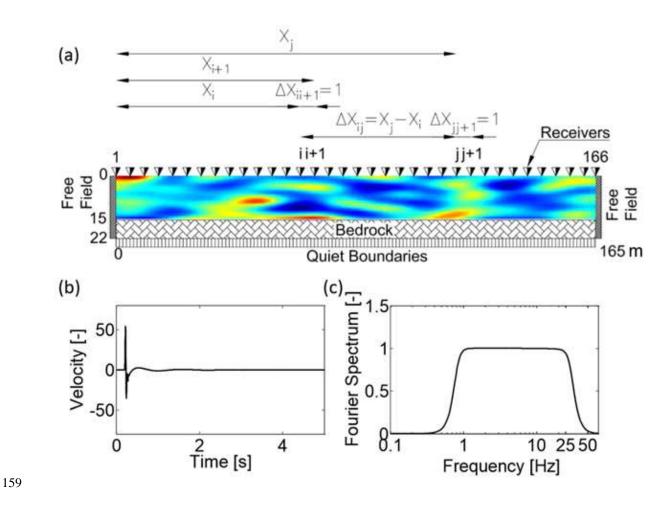


Figure 1: (a) Two-layered ground model composed of a 15,5m thick soil layer overlying bedrock (Vs=1000 m/s). The Vs distribution in the soil layer was computed using the EOLE method with the following statistical parameters:  $\mu$ Vs = 220 m/s; COV= 40%;  $\theta$ x= 10 m;  $\theta$ z =2 m (model # 9 in Table 1). X<sub>i</sub> refers to the distance of the receiver i from the left side of the model.  $\Delta$ X<sub>ij</sub> refers to the distance between receiver i and j. Synthetic velocities are computed at 166 surface receivers 1 m apart (some locations are shown with black triangles). (b) Horizontal input signal (particle velocity) applied at the model base. (c) Fourier amplitude Spectrum of the input signal.

For the purpose of simplifying the 2D modeling, the two soil layers were merged into a unique 15.5 m thick bed with a mean value  $\mu_{Vs} = 220$  m/s overlying a homogeneous sound bedrock (Vs = 1000 m/s) (Figure 1a). The statistical parameters in the sedimentary layer are chosen in a range covering the values found during the survey and in the literature (5% < COV < 40%; 5 <  $\theta_x$  < 20 m; 1 <  $\theta_z$  <2 m). The 9 probabilistic models used for the sensitivity analysis are given in Table 1. No attenuation or damping is considered in the analysis in order to focus only on the effects of elastic properties variability. The fundamental resonance frequency of the soil layer for the mean *Vs* is  $f_{0D}$ = 3.54 Hz, with a theoretical amplification  $AF_D$ = 7.1 at the resonance frequency. This case will be referred to as the deterministic model in the following.

#### 176 2.3. <u>Random field discretization method</u>

The Expansion Optimal Linear Estimation method (EOLE), originally proposed by Li and Der Kiureghian (1993) is used herein to discretize the random field of Vs(x,z). It is described by a lognormal *probability density function* f(Vs) (Eq. 1):

$$f(V_s) = \frac{1}{V_s \sigma_{ln} \sqrt{2\pi}} \exp(-\frac{(\ln(V_s) - \mu_{ln})^2}{2\sigma_{ln}^2})$$
(1)

180 where  $\mu_{ln}$  and  $\sigma_{ln}$  are the mean and the standard deviation of *Vs* natural logarithm, respectively, and by an 181 anisotropic square exponential autocorrelation function  $\rho[(x, z), (x', z')]$  (Eq. 2), which gives the values of the 182 correlation function between two arbitrary points (x, z) and (x', z'):

$$\rho[(x,z),(x',z')] = \exp\left(-\frac{|x-x'|^2}{\theta_x} - \frac{|z-z'|^2}{\theta_z}\right)$$
(2)

183 where  $\theta_x$  and  $\theta_z$  are the autocorrelation distances along x and z, respectively.

In this discretization method, one should first define a stochastic mesh composed of Nq grid points (or nodes) and determine the autocorrelation matrix  $\sum_{\chi;\chi}$  that gives the correlation between each grid point of the stochastic mesh and the other grid points using Eq. 2. The stochastic mesh dimensions  $(D_X, D_Y)$  are taken equal to half of the autocorrelation distance of the model in each direction  $(D_X = \theta_X/2, D_Z = \theta_Z/2)$ . The random field Vs(x,z) at any point is estimated by the following equation, using the autocorrelation matrix :

$$\widetilde{V}_{s}(\mathbf{x},\mathbf{z}) = \mu + \sigma \sum_{i=1}^{s} \frac{\xi_{i}}{\sqrt{\lambda_{i}}} (\phi_{j})^{T} \sum_{V_{s}(\mathbf{x},\mathbf{z});\chi} \mathbf{i} = 1 \dots \mathbf{s}$$
(3)

where  $\tilde{V}_{s}(x, z)$  is the approximate of the random field,  $\mu$  and  $\sigma$  are the mean and standard deviation values of *Vs*,  $\xi_{j}$  is the vector of independent standard gaussian random variables,  $\lambda_{j}$  and  $\phi_{j}$  are the eigenvalues and eigenvectors of the autocorrelation matrice  $\sum_{\chi;\chi}$ ,  $\sum_{V_{s}(x,z);\chi}$  is the correlation vector between the values of the random field at the different nodes of the stochastic grid and its value at the arbitrary point (*x*, *z*) as obtained using Eq. 2. The series of Eq. 3 is truncated to a number of terms *s* (expansion order), which is smaller than the number of grid points, after sorting the eigenvalues  $\lambda_j$  (j=1... S) in a descending order. This number should ensure that the variance of the error is smaller than a prescribed tolerance (10% in our study). The variance of the error on the random field in the EOLE method is given by (Li and Derkiureghian, 1993):

$$Var[V_{s}(x,z) - \widetilde{V}_{s}(x,z)] = \sigma^{2}_{V_{s}} - \sum_{i=1}^{N} \frac{1}{\lambda_{i}} \left( (\phi_{j})^{T} \sum_{V_{s}(X);\chi} \right)^{2}$$

$$\tag{4}$$

where  $V_s(x, z)$  and  $\tilde{V}_s(x, z)$  are the exact and the approximate values of the random fields at a given point (x, z), respectively.

199 Each vector of standard Gaussian random variables  $\xi_j$  (j=1... S) provides (when substituted into Eqs. 3.3 and 200 3.4) spatial variations in Vs that fulfill the correlation structure of this field and are called Vs realizations. This is performed by computing, for this vector, the values of Vs at the centroids and the nodes of the different elements 201 of the stochastic mesh. For each Since the finite difference mesh of FLAC<sup>2D</sup> used to simulate wave propagation 202 203 is much smaller than the stochastic mesh, the kriging method (Sacks et al. 1989) is used for determining  $V_s$  at 204 the centre of each finite difference mesh. In this study, the ordinary kriging and an anisotropic square 205 exponential function for the correlation function are used. For each model (Table 1), the autocorrelation 206 distances used for the kriging method are similar to the ones defined in the EOLE method. The lower and upper 207 boundaries of the autocorrelation distances were taken equal to 0.5 m and 100 m, respectively. Thus, in both directions, 5 Vs are determined within one autocorrelation distance using the EOLE method and the others are 208extrapolated using the ordinary kriging method. By the combination of these two discretization methods, 209 210 Figures 2d and 2h show example of one Vs realization for the 2 models #5 and #9 (Table 1), which have the 211 same correlation distances ( $\theta_r$ =10 m and  $\theta_r$ =2 m) and differ by the COV value (20% and 40%, respectively). In 212 order to check the accuracy of the generated 2D Vs realizations, the average shear wave velocity ( $\mu_{Vs}$ ) and the coefficient of variation (COV) are calculated for the two examples. The autocorrelation distances  $\theta_x$  and  $\theta_z$  are 213 214 then evaluated respectively for the horizontal and vertical 1D samples extracted from the 2D realizations (refer 215 to the black dashed rectangles in Figures 2d and 2h). The re-estimated statistical parameters are summarized in Table 2. The results show that the generated 2D realizations have about the same statistical parameters initially 216 217 introduced during the random field discretization phase.

219 Table 2: Statistical parameters: the average shear wave velocity ( $\mu_{Vs}$ ), the coefficient of variation (COV) and

the autocorrelation distances  $(\theta_x, \theta_z)$  computed for the two V<sub>s</sub> realizations shown in Figures 2d and 2h. For each realization  $\theta_s$  and  $\theta_s$  are estimated respectively for the horizontal and vertical restances of each figure

221	each realization, $\theta_x$ and $\theta_y$	<sub>z</sub> are estimated,	respectively, for	the horizontal a	and vertical rectai	igle of each figure.	

Statistical parameters	Model #5	Model #9
$\mu_{Vs}$ (m/s)	222	217
COV (%)	20.93	38.81
$\theta_{x}$ (m)	11.09	10.85
$\theta_{z}$ (m)	1.91	1.98

# 223 2.4. <u>Wave propagation simulation</u>

The finite difference code FLAC<sup>2D</sup> is used to propagate seismic waves in the 2D Vs structures (Bouckovalas et 224 225 al., 2005; Pagliaroli, 2006; Stamatopoulos et al., 2007 & 2009). After discretizing  $V_s$  for all the models listed in 226 Table 1, the minimum value of Vs is found to be 50 m/s and the maximum value is 750 m/s. An example of one 227 realization of the Vs structure obtained for model #9 (Table 1) is shown in Figure 1a. The source time function is a pseudo-Dirac having a flat Fourier amplitude spectrum between 1 and 25 Hz (Figures 1b and 1c). According 228 229 to the frequency and  $V_s$  ranges, the minimum and maximum wavelengths in the sediment layer are 2 m and 750 230 m, respectively. The mesh size  $\Delta l$  is chosen less than one tenth of the minimum wavelength to avoid numerical 231 dispersion phenomena (Eq. 5) (Kuhlemeyer, R. L. and Lysmer, 1973):

$$\Delta l_{\max} \le \frac{\lambda_{\min}}{10} = \frac{V_{S_{\min}}}{10.f_{\max}}$$
<sup>(5)</sup>

where  $\Delta l_{max}$  is the maximum size of the finite difference mesh,  $\lambda_{min}$  is the minimum wave length,  $Vs_{min}$  is the minimum shear wave velocity, and  $f_{max}$  is the maximum wave frequency.

234 With the parameters of this study, the maximum mesh size,  $\Delta l_{max}$ , is 0.2 m ( $\Delta l_{max} \le \frac{50}{10.25}$ ).

Zero horizontal displacements are applied along lateral boundaries of the model, while the horizontal and vertical movements are fixed at its base. Free field boundaries are applied to the side edges of the model. A quiet boundary (absorbing effect defined in FLAC<sup>2D</sup> (Itasca, 2011)), is applied to the model base in order to model a flexible base that absorbs the energy emitted by the waves reflected on the surface and arriving to the model base. After initializing the stresses, a seismic shear stress excitation consisting of a vertically incident plane SV-type wave is applied to the base of the model:

$$\sigma s = 2. (\rho. Vs). \upsilon s \tag{6}$$

241 Where  $\sigma_s$  is the applied shear stress,  $\rho$  is the soil density,  $V_s$  is the medium shear wave velocity and  $v_s$  is the 242 horizontal component of the velocity signal.

The 2D soil structure used in the simulation is a 22 m x 165 m model. The width of the model is relatively small compared to the maximum wavelength of 750 m. Therefore, in order to check that the boundary conditions are efficient and that such a model size does not affect the wave propagation, we compared seismograms computed for two different model sizes (750 m and 165 m wide) keeping fixed all the other parameters. The simulations performed over a duration of 2 seconds show similar results (Appendix 1). Finally, the computation time for one simulation and 10-second seismograms is around 5 hours on a single processor and a PC having an i7-core and 2.4 Hz CPU frequency, leading to a total computation time of about 4500 hours for all models.

### 250 3. Characterization of the ground motion spatial variability

In this section, we present an analysis of the synthetic seismograms computed for two probabilistic models (one realization), discuss the probalistic simulation stability and provide the definition of the ground motion indicators extracted from the seismograms.

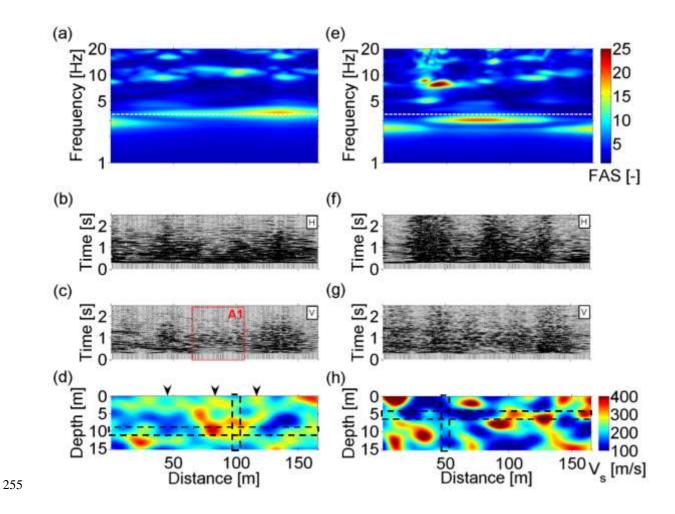
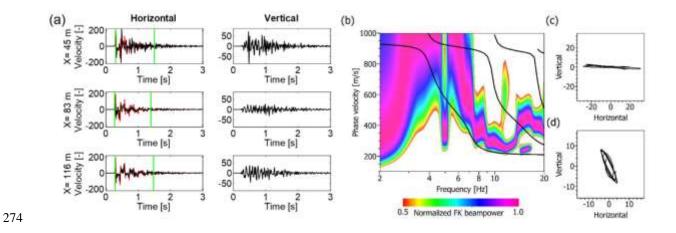


Figure 2: Seismic responses for the two Vs realizations of models #5 (left column) and # 9 (right column). (a) and (e) Fourier amplitude spectra (FAS) for the horizontal velocity seismograms shown in (b) and (f); (b) and (f) 3-second horizontal seismograms (H) computed for the Vs realizations shown in (d) and (h), respectively. (c) and (g) 3-second vertical seismograms (V) ; (d) and (h) Vs realizations for models #5 (COV=20%  $\theta x=10m$ and  $\theta z=2m$ ) and #9 (COV=40%  $\theta x=10m$  and  $\theta z=2m$ ) (see Table 1). The white dashed lines in (a) and (e) correspond to the fundamental frequency of the deterministic model,  $f_{oD}$ .

# 3.1.1. <u>Ground motion synthetics</u>

3.1. Ground motion synthetics and probabilistic simulation stability

Figure 2 shows the simulation results of one *Vs* realization for the 2 models #5 and #9 (Table 1). For each case, the *Vs* model, the vertical and horizontal surface velocity signals, and the Fourier amplitude spectra of the horizontal seismograms are shown. The increase in COV (40%) is clearly visible on the *Vs* model #9 (figure 2h), which exhibits larger velocity contrasts than model #5 (Figure 2d; COV=20%). The *Vs* spatial variation turned out to have a strong influence on the surface ground motion characteristics: seismograms exhibit larger amplitude and longer duration at receivers located over near-surface low velocity zones (see Figures 2b, c, f and 270 g), especially for the horizontal seismograms and for COV=40%. The Fourier amplitude spectra (Figures 2a and 271 2e) show that the amplification at the fundamental resonance frequency occurs at or a little lower the 272 deterministic 1D resonance frequency ( $f_0=3.54$  Hz), with lateral variations resulting from the surface wave 273 propagation. Amplification at higher frequencies are also spatially variable (Figure 2e).



275 Figure 3: (a) 3-second seismograms (horizontal and vertical velocities) simulated for the Vs model shown in Figure 2d at 3 different locations spotted by black arrows in Figure 2d (curves in black). Seismograms 276 277 computed for the deterministic model are indicated in red. The vertical green lines represent the duration 278 corresponding to 5 and 95% of the total energy calculated for the seismograms computed for the probabilistic 279 model (see text for details). (b) Dispersion curves for the vertical component of the synthetic ground motion 280 obtained for distance between 65 and 115 m between t=0 s and 8 s (refer to rectangle A1 in Figure 2c); the theoretical dispersion curves for the first three modes of the deterministic model are shown in black. (c) Particle 281 282 motion of the 1-5 Hz band-pass filtered velocities recorded at X = 83 m between t=0.1 s and t=0.8 s (top) and 7-283 11 Hz band-pass filtered velocities recorded at X = 83 m between t=1s and 1.8s (bottom).

Figure 3a shows the first three seconds of the ground surface seismograms (horizontal and vertical components) calculated for the model #5 at three distances indicated in Figure 2d. The horizontal signals are compared with the ones computed for the deterministic model considering no variability in the soil properties ( $\mu_{Vs} = 220$  m/s;  $\sigma_{Vs}=0$ ). Figure 3a illustrates the motion spatial variability generated by the 2D probabilistic models in terms of amplitude of the first arrivals (e.g. see the signals at 45 m and 116m) and generation of strong later seismic waves (e.g. at 45 m) both on the horizontal and vertical components.

291 In order to determine the wave types, the particle motions are plotted in Figures 3c and 3d within two time 292 windows (0.1-0.8s) and (1-1.8s), respectively. As expected, the early seismic waves are horizontally polarized 293 shear waves, while elliptical motion of the late waves indicate presence of Rayleigh waves. By using the f-k 294 method (Lacoss et al., 1969), the dispersion characteristics of the surface waves were calculated by using 295 vertical seismograms recorded at receivers located between 65 m and 115 m. The dispersion diagram shown in 296 Figure 3b indicates two branches of dispersion curves between 4Hz and 20 Hz, while the image is blurred below 297 4 Hz. This results from the high-pass filtering of the soil layer (Scherbaum et al. 2003), which leads the energy 298 vanishing below the soil fundamental frequency (see also the Fourier amplitude spectrum in Figure 2a). Although the theoretical curves from the deterministic model (first three modes, black curves) do not perfectly 299 300 match the dispersion maxima caused by Vs differences between the 2D and the deterministic cases, the 301 comparison suggests that the first two Rayleigh wave modes have been excited in the 2D probabilistic model.

302

### 3.1.2. <u>Convergence of probabilistic ground motion simulations</u>

303 Probabilistic modeling approaches require simulating ground motion for a large enough number of realizations 304 of discretized *Vs* models to ensure the statistical convergence of the average and standard deviation estimators 305 of any surface ground motion parameter. We define the convergence of a ground motion parameter  $\alpha$  as a 306 fraction of the relative variation of  $\alpha$  between realization *i* and realization *i*+1:

$$CONV(\alpha) = \frac{|\alpha_i - \alpha_{i+1}|}{|\alpha_i|}$$
(7)

The convergence is considered to be reached when  $CONV(\alpha) < 5\%$  (Haldar et al., 2008). For both the average 307 308 and standard deviation estimators, the convergence was tested on both surface ground motion parameters in time 309 (Arias Intensity and duration) and frequency (Fourier amplitude spectra, site fundamental frequency and 310 corresponding amplification) (see section 3.2 for definition). As an example, the convergence for the Fourier 311 amplitude spectra (horizontal component) at different frequencies and the duration are shown in Appendix 2 for 312 model #9 that exhibits the largest Vs range. According to the defined criterion, the convergence is reached after 313 86 realizations for all frequencies for Fourier amplitude spectra and after 82 realizations for the duration. All the tests performed on other indicators showed that 100 realizations ensure the convergence. Finally, due to the 314 315 presence of absorbent boundaries, receivers located less than 30 m from the borders were omitted in the 316 analysis.

#### 317 3.2. <u>Surface ground motion indicators</u>

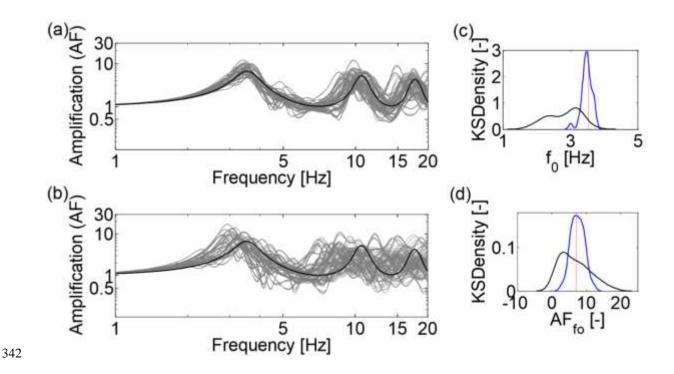
In this paper, we consider surface ground motion indicators in time and frequency domain, as described 318 319 hereafter. In the frequency domain, the spectral amplification AF(f) is defined as the ratio between the Fourier 320 amplitude spectra of the signals recorded at the soil surface and at the outcropping bedrock. For vertically 321 incident waves, this latter is 2 times the spectrum shown in Figure 1c. Two parameters are extracted from the 322 spectral amplification: the fundamental frequency  $f_0$  and the corresponding amplification,  $AF_{f0}$ . In the time 323 domain, we consider the Arias based intensity parameter  $(A_bI)$  that provides a measure of the shaking intensity 324 (Arias, 1970), here defined as the integral of the square surface horizontal velocity as function of time. The 325 second time parameter is the effective duration  $(D_{Abl})$  defined as the difference between the time where 5% and 326 95% of the total  $A_b I$  is reached. The durations inferred for models #5 are indicated in Figure 3a.

#### 327 4. Influence of ground structure variability on surface ground motion indicators at a single

#### 328 <u>station</u>

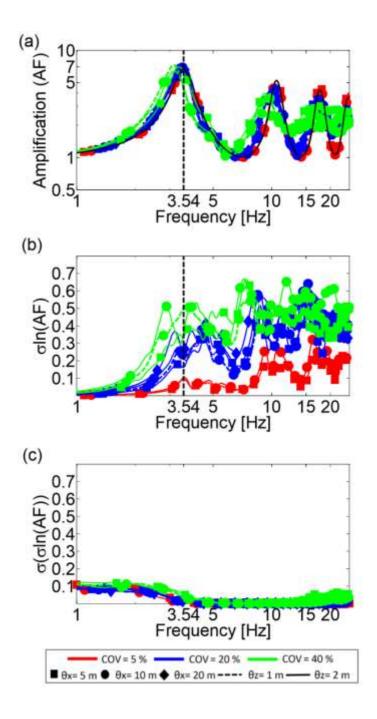
#### 329 4.1. <u>Resonance frequency and related amplification</u>

330 The amplification AF was calculated at the central receiver (located at x=83 m) for all models (Table 1) from the 331 10-second seismograms after smoothing the Fourier amplitude spectra by using the Konno and Ohmachi 332 algorithm with b=50 (Konno and Ohmachi, 1998). Spectral amplifications for the 100 simulations are shown in Figures 4a and 4b for models #5 (COV=20%) and #9 (COV=40%), respectively. The variability of the 333 334 amplification curves increases with COV, both in terms of maximum amplification and corresponding 335 frequency. We computed the probability density functions of  $f_0$  and  $AF_{f_0}$  (Figures 4c and 4d) using the nonparametric kernel density estimation technique (Rosenblatt, 1956; Parzen, 1962). Compared to the deterministic 336 337 fundamental frequency  $f_{0D}$  (dashed red line), the probabilistic  $f_0$  values tend to be lower than  $f_{0D}$  by about 2% and 20% for COV=20% and 40%, respectively (Figure 4c). The distribution of amplifications at the fundamental 338 339 resonance frequency peaks ( $AF_{fo}$ ) (Figure 4d) is slightly higher by about 6.7% than the deterministic one ( $AF_{foD}$ ) 340 for COV = 20%, with  $AF_{fo}$  values ranging between 3.15 and 12.0, while it is significantly shifted to a lower amplification values for COV=40% with  $AF_{fo}$  values ranging between 1.6 and 17.2. 341



343 Figure 4 : Amplification functions at the central receiver. (a) and (b) Amplification curves (100 realizations in 344 grey) for parameter set #5 (COV=20%  $\theta x=10m$  and  $\theta z=2m$ ) and for the parameter set #9 (COV=40%  $\theta x=10m$ 345 and  $\theta z=2m$ ), respectively. The black curve is the theoretical transfer function for the deterministic model 346 (Vs=220 m/s). (c) KSdensity of the fundamental resonance frequency f0 computed by considering all 347 realizations for parameter set #5 (in blue) and #9 (in black). The red dashed line indicates the fundamental 348 resonance frequency fOD of the deterministic model. (d) KSdensity of the amplification AFf0 at the fundamental 349 resonance frequency for parameter set #5 (in blue) and #9 (in black). The red dashed line indicates the 350 amplification AFD for the deterministic model.

The average amplification and the standard deviation computed on the natural logarithm of amplifications at the central receiver (x= 83 m) are shown for all models in Figures 5a and 5b, respectively. The average *AF* curves provide fundamental resonance frequencies close to the deterministic one, except for the higher *COV* (40%). For that *COV*, the fundamental frequency  $f_0$  is shifted down to values around 3 Hz, as already highlighted in Figure 4c. At frequencies equal or over  $f_0$ , standard deviations increase with frequency, the higher standard deviation being found for larger *COV* values as a consequence of stronger scattering of the *AF* curves (Figure 5b), which in turns leads to a decrease in average amplification values at frequencies larger than  $f_0$  (Figure 5a).



360

Figure 5 : (a) Average amplification at the central receiver (X=83 m) for different values of COV,  $\theta x$  and  $\theta z$ obtained by computing the geometric mean of the transfer functions for the 100 realizations. The amplification and the fundamental frequency for the deterministic model are shown by the black curve and black dashed line, respectively. (b) Standard deviation of the natural logarithm of the Amplification ( $\sigma ln(AF)$ ) at the central receiver. (c) Standard deviation of  $\sigma ln(AF)$  obtained by considering all receivers location. The values of COV,  $\theta x$  and  $\theta z$  are shown with different colors, symbols and lines, respectively.

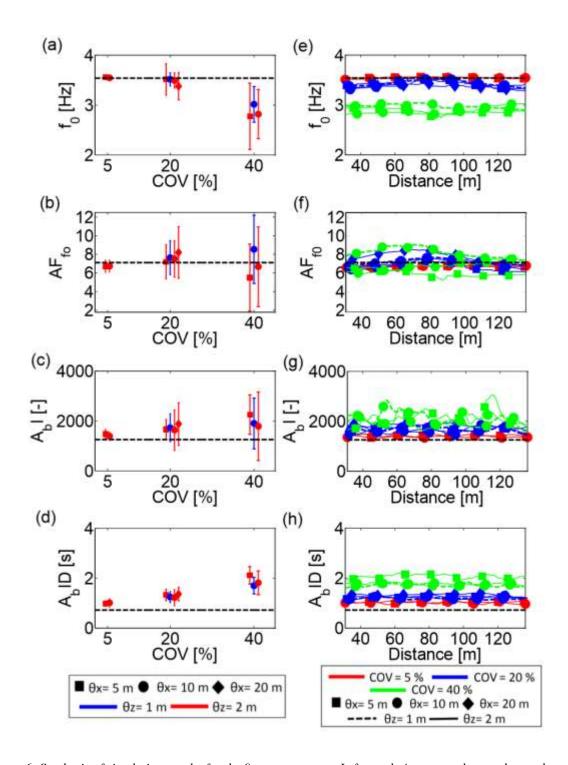


Figure 6: Synthesis of simulation results for the 9 parameter sets. Left panel: Average values and error bars (+one standard deviation) for the four spectral and temporal parameters at the central receiver: (a) average
fundamental frequency (fo), (b) average amplification at the fundamental frequency (AFfo), (c) average of the
Arias based intensity (AIb) and (d) average of the Arias based duration (AbID). The black dashed lines indicate
values of, f0, AFfo, AIb and the related duration of the deterministic model. Right panel: (a) average f0, (b)
average AFfo, (c) average AbI and (d) duration at different locations on the surface for the 100 simulations. The
values of COV, θx and θz are shown with different colors, symbols and lines, respectively.

376 In order to evaluate the effect of the receiver position on amplification, we calculated the standard deviation 377  $\sigma(ln(AF))$  for all receiver locations. The curves are presented in Figure 5c at the same scale as the ones in Figure 378 5b. The standard deviation is slightly affected by the receiver location at frequencies higher than 15 Hz for the 379 greatest COV (40%). The main impact of considering all receivers is however observed for frequencies lower 380 than  $f_0$ , a range for which the standard deviation is significantly greater, whatever the COV value. Such increase 381 results from the filtering effect of the sedimentary layer, which yields little energy below the fundamental 382 frequency (Scherbaum et al., 2003) and greater variability in amplification. Moreover, Appendix 3 compares the 383 standard deviation of  $\ln(AF)$  computed by considering, firstly, all the receivers of all the realizations (in dashed 384 lines) and secondly, the central receiver of each realization (in continuous lines) for the model #6 having COV=20%,  $\theta_z=2m$  and  $\theta_x=10m$  and model #9 having COV=40%,  $\theta_z=2m$  and  $\theta_x=10m$ . The standard 385 386 deviations calculated by the two methods are very similar. This observation emphasis the stationarity of  $\ln(AF)$ 387 due to the stationary random field generated by the EOLE discretization method.

The simulation results are synthesized for the 9 probabilistic models in Figure 6. First, the average  $f_0$  values along with error bars for all parameter sets are shown as a function of *COV* at the center receiver in Figure 6a. As already evidenced in Figure 4 for two models, the average  $f_0$  at the central receiver is close to the deterministic value  $f_{0D}$ = 3.54 Hz for low *COV* but sharply decreases to around 2.8-3 Hz for *COV*=40%. The two other probabilistic parameters ( $\theta_x$  and  $\theta_z$ ) do not seem to significantly influence neither the average nor the dispersion of  $f_0$ . This significant influence of *COV* on average  $f_0$  values is observed at all receivers (Figure 6e),  $\theta_x$  and  $\theta_z$  having little effect. The variations in  $f_0$  over distance are small et remain below 10%.

395 The same kind of analysis are presented for the average amplification  $AF_{fo}$  in Figures 6b and f. At the central 396 receiver (Figure 6b), the average amplification is close to the deterministic value  $AF_D$  and, again,  $\theta x$  and  $\theta z$  seem 397 to have little influence. However, the average amplification variability increases largely for COV=40% with 398 values being higher or lower than  $AF_D$ , depending on the combination of  $\theta x$  and  $\theta z$ . Figure 6f shows that, in 399 contrast to  $f_0$ ,  $AF_{f_0}$  is spatially variable, especially for COV=40%. Such spatial variation is due to large lateral 400 Vs variation and/or to the presence of a double amplification peak close to the fundamental resonance frequency 401 on the AF curve at some receiver location. The second peak generated by low Vs superficial zones may exhibit an amplitude higher than the first one identified as the amplification at the fundamental resonance frequency 402 403 (see Figure 2e for receivers located between X=30 m and X=120 m).

#### 404 4.2. <u>Arias based intensity and duration</u>

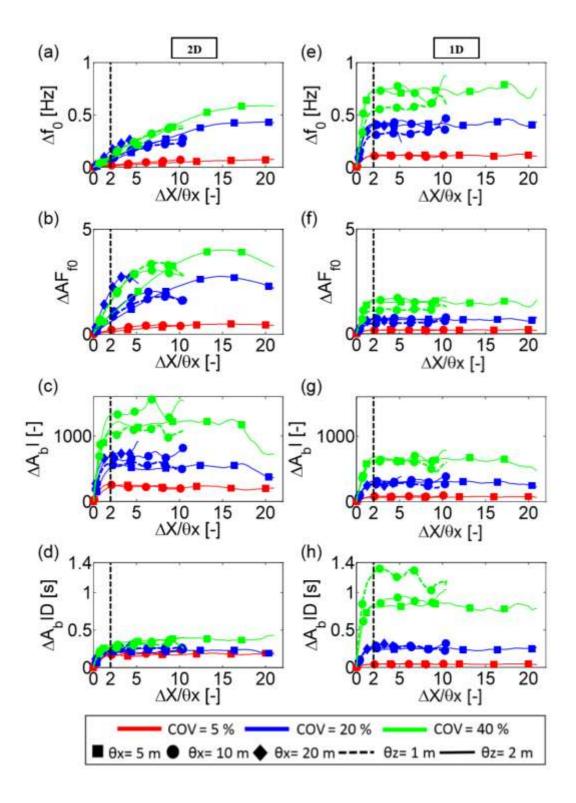
405 Similarly to the spectral parameters, the results for temporal parameters (Arias intensity  $A_bI$  and duration  $A_bID$ ) 406 are synthesized in Figure 6c, d, g and h. The average Arias intensity and duration (Figures 6c and 6d, respectively) are systematically larger than those corresponding to the deterministic Vs structure, and increase 407 408 with COV, as well as the standard deviations. This effect is particularly clear for  $A_b ID$  (Figure 6d), which may 409 reach values two to three times higher than the deterministic one for COV=20% and 40\%, respectively. Also, 410 these two temporal parameters seem to be slightly influenced by the receiver location, as shown in Figures 6g 411 and 6h, except  $A_b I$  for COV=40%. In this case, large lateral variations of 50% can be observed over short 412 distances.

In summary, this study outlines the predominant influence of *COV* on the surface ground motion indicators in terms of average and standard deviation compared to the correlation distances. However, for the largest *COV* (40%), ground motion average indicators related to the amplitude of seismic signals (amplification  $AF_{fo}$ , Arias intensity  $A_bI$ ) exhibit larger variation over distance for  $\theta x=5$  m and  $\theta z=2$  m suggesting then an influence of the size of the heterogeneity, while the average fundamental frequency  $f_0$  and duration  $A_bID$  turned out to be spatially independent, whatever the *COV* value.

419

#### 420 **5.** Effects of spatial ground variability on spatial correlation of indicators

421 In previous sections, the variability of indicators was analyzed at a single station. However, studying the impact 422 of spatial ground variations on the spatial correlation of ground motion is also of interest for understanding or predicting damage to long-span civil engineering structures (e.g. Abrahamson et al., 1990; Schneider et al., 423 424 1992; Ancheta et al., 2011; Goda and Atkinson, 2008; Liu and Hong, 2015; Koufoudi et al., 2018). Here, we 425 focus on investigating the spatial correlation of the four ground motion scalar indicators defined in section 4 (resonance frequency and corresponding amplification, Arias based intensity and duration) and one frequency-426 427 dependent indicator (Fourier amplitude spectrum). We estimated the spatial correlation by computing the 428 absolute differences of indicators between two receivers separated by a distance  $\Delta X$ ,  $\Delta X$  spanning all possible 429 inter-receiver distances and being normalized by the horizontal correlation distance  $\theta_{\rm x}$ .



430

431 Figure 7: Spatial variability of the four scalar indicators as a function of the receiver spacing normalized by the 432 horizontal autocorrelation distance  $\Delta X/\theta x$ . (a), (b), (c) and (d), Variation of the differences in fundamental 433 resonance frequency f0, related amplification AF, Arias based Intensity A<sub>b</sub>I and Arias based Intensity duration 434 A<sub>b</sub>ID using 2D ground motion synthetics. (e), (f), (g) and (h), the same for 1D ground motion synthetics. The 435 values of COV,  $\theta x$  and  $\theta z$  are shown with different colors, symbols and lines, respectively.

#### 437 5.1. <u>Scalar indicators</u>

We first computed the mean spatial correlation of the 4 scalar indicators for the 100 Vs realizations. Figures 7a, 7b, 7c and 7d display the spatial correlation of the resonance frequency ( $\Delta f_o$ ), the corresponding amplification ( $\Delta AFf_o$ ), the Arias based intensity ( $\Delta A_b I$ ) and the Arias based intensity duration ( $\Delta A_b ID$ ), respectively, as a function of  $\Delta X/\theta_x$  for all the models (Table 1).

442 The variability of  $\Delta A_b I$  and  $\Delta A_b ID$  increases linearly until reaching a constant value for  $\Delta X$  larger than  $2\theta_x$ , a larger variability being systematically observed for larger COV values. This general trend is explained by the 443 444 random field discretization in Vs (see section 2). For a given realization, the Vs vertical profiles below receivers 445 located at  $X + -\theta_x$  are correlated, leading to close seismic responses at the surface. In contrast, the seismic responses at two receivers with a spacing larger than  $2\theta_x$  are not expected to be similar. Thus, the seismic 446 447 indicators ( $A_bI$  and  $A_bID$ ) that are strongly controlled by the most energetic seismic phase (the incident SV wave 448 (Figure 3a), mostly itself influenced by the locally 1D Vs structure) will lead to indicator differences not 449 correlated anymore for  $\Delta X$  larger than  $2\theta_x$  and thus constant whatever the receiver spacing.

450 The spatial correlation of the fundamental resonance frequency  $(\Delta f_0)$  and the corresponding amplification 451  $(\Delta AFf_o)$  (Figures 7a, 7b and Appendix 4) indicates an increase in variability with distance, reaching constant or maximum values for  $\Delta X/\theta_x$  ranging between 2.5 and 4 for  $\theta_x=20$  m, between 6 and 8 for  $\theta_x=10$  m and between 452 453 13 and 16 for  $\theta_x=5$  m. These ratios correspond to an absolute value  $\Delta X$  ranging between 50 and 80 m, i.e. close 454 to the wavelength at the fundamental resonance frequency for the deterministic Vs structure ( $\lambda$ =62 m). As for 455 other indicators, the  $f_0$  and  $AFf_0$  variability increases with COV and their convergence seems to be independent 456 of  $\theta_x$  like the time domain parameters (see also Appendix 4 where results are plotted in terms of non-normalized 457  $\Delta X$ ).

#### 458 5.2. <u>Frequency-dependent indicator</u>

Following Abrahamson et al. (1990) and Schneider et al. (1992), we computed the standard deviation of the absolute difference of the natural logarithms of the Fourier amplitude spectra ( $\sigma(\Delta|\ln(AF)|)$ ). Figures 8a, 8b, 8c and 8d display  $\sigma(\Delta|\ln(AF)|)$  versus the receiver spacing  $\Delta X$  for four frequencies (3.54 Hz, 7 Hz, 10 Hz and 24 Hz, respectively), while Figures 8e and 8f show  $\sigma(\Delta|\ln(AF)|)$  as a function of frequency for two receiver spacing values (5 and 50 m, respectively). The lowest frequency (3.54 Hz) corresponds to the fundamental resonance frequency  $f_{0D}$  previously defined. For all frequencies,  $\sigma(\Delta|\ln(AF)|)$  increases with  $\Delta X$  until reaching a

constant value that significantly increases with COV (Figures 8a to 8d). For small frequencies ( $< f_{0D}$ ), the 465 466 wavelengths are much larger than  $\theta x$  (5 and 10 m) and  $\theta z$  (1 and 2 m) and the analyzed inter-receivers distances  $(\Delta X)$ . Then,  $\sigma(\Delta \ln(AF))$  increases with the increase of  $\Delta X$  and no convergences is observed with  $\Delta X$  in the range 467 468 between 0 and 100m. The correlation distances  $\theta x$  and  $\theta_z$  appear to have little influence on the variability, except 469 for one frequency (7 Hz) for COV=40%. The spatial amplitude variability with frequency shows a different 470 picture depending on the receiver spacing  $\Delta X$  (Figures 8e and 8f). For  $\Delta X = 5$  m, the variability is weak at low 471 frequency and progressively increases with frequency (see for instance the curve for COV=40%). This shows 472 the effect of the wavelength. Indeed, for COV=40%,  $V_s$  varies from 50 to 750 m/s, corresponding to minimum wavelengths of 2, 5, 7 and 14 m for  $V_s = 50$  m/s and at 24, 10, 7 and 3.54 Hz, respectively. For wavelengths 473 474 much larger than the size of the heterogeneity (i.e. at low frequency) the amplitude variability is low, while 475 maximum when wavelength is about the size of the heterogeneity. It could explains that maximum variability is observed for  $\theta x = 5m$  at high frequency (Figure 8e). For large spacing ( $\Delta X = 50m > \theta_x$ .), the Vs structure below 476 477 the receivers is not correlated anymore and  $\sigma(\Delta |\ln(AF)|)$  is maximum at the resonance frequencies ( $f_{0D}$ ,  $3 f_{0D}$  and 478 5  $f_{0D}$ ), with no clear dependency on  $\theta_x$  (Figure 8f).

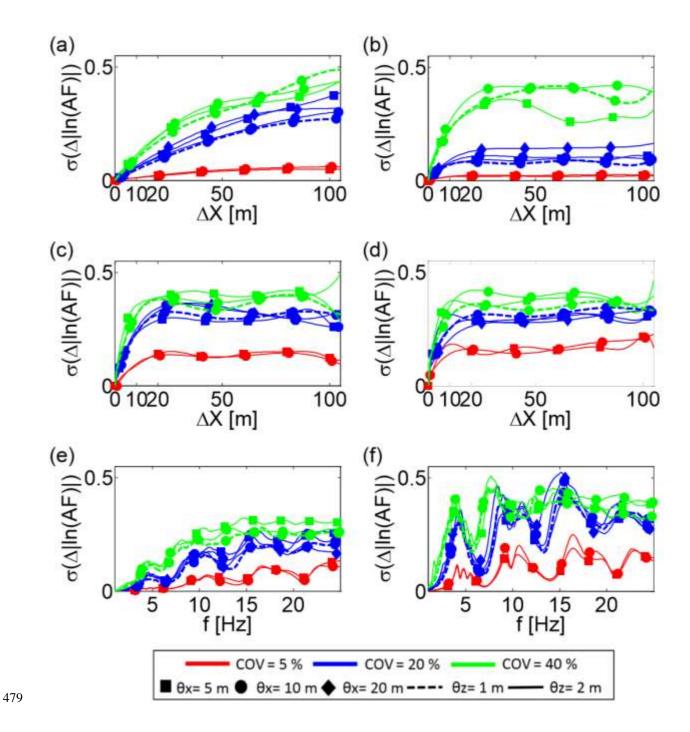


Figure 8: Frequency-dependent indicator variability computed from 2D ground motion synthetics. The standard deviation of the absolute difference of natural logarithm of Fourier amplitude spectra ( $\sigma(\Delta ln(AF))$ ) as a function of receiver spacing at frequencies equal to (a) 3.54 Hz, (b) 7 Hz, (c) 10 Hz and (d) 24 Hz.  $\sigma(\Delta ln(AF))$  as a function of frequency for two receiver spacing values (d)  $\Delta X=5$  m and (f)  $\Delta X=50$  m. The values of COV,  $\theta x$  and  $\theta z$  are shown with different colors, symbols and lines, respectively.

#### 486 6. Can 2D modeling be replaced by 1D modeling?

487 In order to evaluate the importance of accounting for 2D wave propagation on surface ground motion for site specific hazard assessment compared to alternative modelling approaches that simplify the 2D spatial variation 488 of ground structure into a suite of 1D variable soil profiles (e.g. Rahtje et al., 2010; Rodriguez-Marek et al., 489 490 2014; Haji-Soltani et al., 2017), we extracted 100 1D soil columns at each receiver from the Vs 2D 491 discretization. 1D wave propagation simulation was performed and spectral amplification, Arias based intensity 492 and duration are computed at each receiver and compared to the values obtained from 2D wave propagation 493 modeling. The 1D and 2D modeling results are compared from various perspectives in Figures 9, 10 and 11, 494 showing the parameter ratios and standard deviation differences for all realizations at the central receiver, the 495 spatial evolution of the four parameters along the profile for one given realization, and the average and standard 496 deviation of 2D/1D ratio for all realizations, respectively

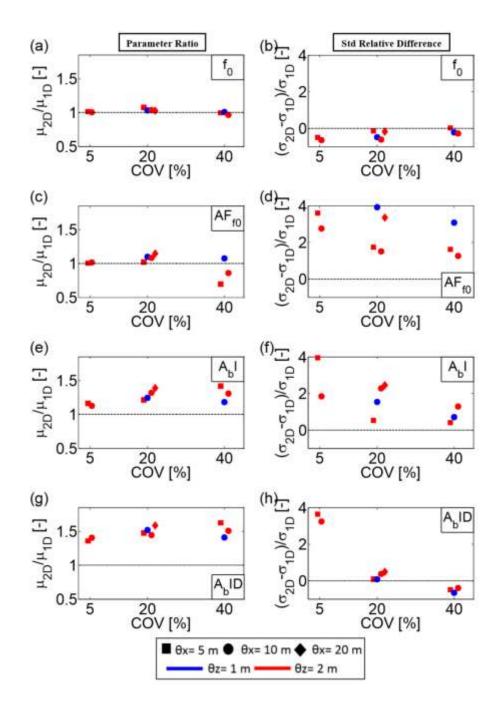
#### 497 6.1. <u>Fundamental frequency, corresponding amplification and spectral amplification</u>

498 Figure 6.1a and b compare in terms of ratio the average fundamental resonance frequency and amplification 499 together with related standard deviation derived from 1D and 2D modeling approaches. Average resonance 500 frequencies are very similar for both modeling types (Figure 9a), while standard deviation is larger for 1D 501 modelling (Figure 9b). Such higher standard deviation comes from 1D resonance frequencies being only 502 controlled by SH body waves propagating through locally 1D soil profiles, while 2D wave propagation 503 modeling also incorporates locally diffracted surface waves that sample larger underground volume and lead to 504 homogenize seismic response. Figure 10a illustrates, for the ground model realization shown in Figure 2d, 505 differences between spatial distribution of resonance frequency derived from 1D and 2D wave propagation 506 modeling. The resonance frequency computed from the 2D ground motion synthetics is very weakly sensitive to 507 the spatial variation of ground velocity structure compared to that computed from 1D synthetics, except when 508 strong lateral discontinuities of Vs occur (e.g. at X=65 m in Figure 10e). Although average amplification 509 computed from 1D and 2D ground motion synthetics are very similar, especially for COV of 5% and 20%, 510 standard deviation of amplification computed from 2D synthetics are significantly larger (by 15% to 40%) than 511 the one inferred from 1D synthetics, whatever the COV value. Such a large increase in standard deviation is easily explained when looking at the spatial variation of 2D amplification for a single Vs realization (Figure 512 513 10b). Amplification variation is either due to large lateral Vs variation prone to efficiently generate (or not) 514 surface waves (Figures 2b and 10b) or the presence at some receivers of a double amplification peak close to the

resonance frequency, leading in most cases to an amplitude at the fundamental resonance frequency much lower than the one at the secondary peak (see Figure 2e for receivers located between X=30 m and X=120 m).

In order to analyze the effect of 2D wave propagation modeling approach compared to the 1D one over the whole frequency range from 1 Hz to 25 Hz, we computed the ratio between Fourier amplitude spectra obtained from 2D and 1D ground motion synthetics. Accounting for 2D wave propagation modeling in spatially variable Vs structure obviously leads to larger amplification at frequencies higher than the fundamental resonance frequency of the probabilistic model, except for the frequency range in-between the fundamental and first higher mode (Figure 11a). This increase in amplification, especially significant for *COV*=20% and *COV*=40% (Figure 11a), is also associated with a higher variability (Figure 11b), which increases with *COV*.

These observations clearly outline that, although the 1D probabilistic approach may correctly reproduce average fundamental resonance frequencies and corresponding amplification, it under-predicts ground motion amplification at high frequencies and related variabilities whose origin relates to the presence of locally diffracted surface waves.



528

529 Figure 9: Ratio between the parameter extracted from 2D wave propagation modeling and 1D ones at the 530 central receiver (left column), and the relative difference between standard deviation of this parameter inferred 531 from 1D and 2D wave propagation modelling (right column): (a) and (b), average fundamental resonance 532 frequency (c) and (d) average amplification at the fundamental resonance frequency, (e) and (f) average of the 533 Arias based intensity, (g) and (h) average of Arias based duration.: (e) fundamental resonance frequency, (f) 534 average amplification at the fundamental resonance frequency, (d) average of the Arias based intensity, e) 535 average of Arias based duration. The values of COV,  $\theta x$  and  $\theta z$  are shown with different colors, symbols and 536 lines, respectively.

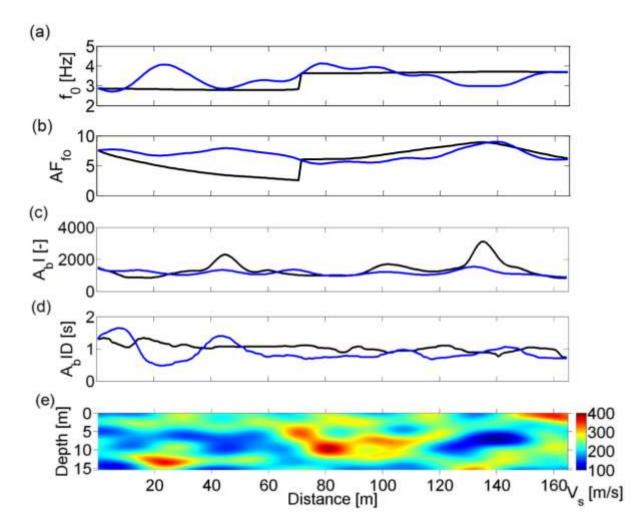


Figure 10: Comparison of 2D (black line) and 1D modeling (blue line) results for the Vs realization shown in (e). (a) Fundamental resonance frequency ( $f_0$ ), (b) amplification at  $f_0$  (AF<sub>f0</sub>), (c) Arias based Intensity ( $A_bI$ ) and (d) Arias based Duration. The test parameter is #5 in Table 1 (COV=20%  $\theta x$ =10m and  $\theta z$ =2m).

541

#### 6.2. Arias based intensity and duration

542 Average Arias based intensity and duration inferred from 2D ground motion synthetics are systematically higher 543 than the ones inferred from 1D synthetics, by a factor of 1.2 to 1.6, whatever the considered COV and 544 autocorrelation distances (Figures 9c and 9d). The increase in intensity and duration for 2D modeling is simply 545 explained by the presence of the locally generated surface waves that contaminates the strongest S-wave phase 546 used in the computation (e.g. see Figure 2b for receivers close to X = 50 m, X = 100 m and X = 130 m and the 547 corresponding Arias intensity increase intensity in Figure 10c). As a consequence, standard deviation of Arias based intensity is larger for 2D synthetics compared to 1D ones. Interestingly however, the relative difference 548 549 between standard deviation of duration is decreasing with COV, reaching values below one for COV=40%. This 550 feature is explained by the increase of the duration standard deviation in 1D modelling due to the presence of overall very stiff or very soft 1D *Vs* profiles leading to significant change in the duration of S-wave main pulse at the surface. This is illustrated in Figure 10d with duration derived from 1D synthetics being short above overall stiff areas while large above overall soft areas (e.g. at receivers located at X = 10 m or X = 45 m).

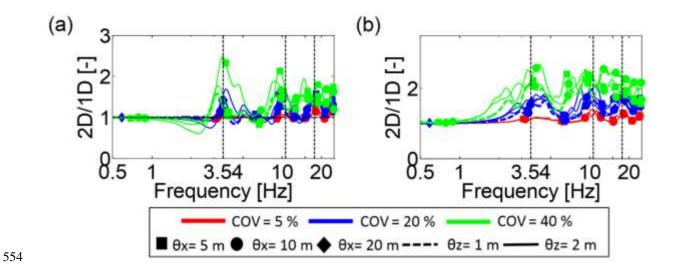


Figure 11: Ratio between the Fourier amplitude spectra computed on 2D and 1D probabilistic ground motion synthetics: (a) average ratio and (b) standard deviation. Colors and symbols refer to the different test parameters (Table 1).

558

#### 6.3. Spatial correlation of indicators

Spatial correlation of scalar indicators for the 100 Vs realizations using 1D motion synthetics have been 559 560 computed. Figures 7e, 7f, 7g and 7h display the spatial correlation of the resonance frequency ( $\Delta f_o$ ), the 561 corresponding amplification ( $\Delta A F f_o$ ), the Arias based intensity ( $\Delta A_b I$ ) and the Arias based intensity duration  $(\Delta A_b ID)$ , respectively, as a function of  $\Delta X/\theta_r$  for all models (Table 1). The variability of all indicators increases 562 linearly until reaching a constant value for  $\Delta X$  larger than  $2\theta_x$ , over which correlation between signals does not 563 564 exist anymore. The larger variability is systematically observed for larger COV values. This is consistent with 565 the control of 1D ground motion synthetics by the locally 1D Vs structures. The 1D  $f_0$  variability for all COV values is larger than that computed for 2D synthetics (compare Figures 7a and 7e) in the range  $\Delta X/\theta_x < 10$ . This 566 567 is due to the presence of surface waves in the 2D seismograms, which are little sensitive to short scale variations of locally 1D velocity structure but rather to the homogenized ground structure elastic properties. In contrast, the 568 variability in  $AFf_o$  is smaller than that observed for 2D synthetics (compare Figures 7b and 7f) for  $\Delta X/\theta_x > 2$ , 569 570 whatever the value of COV. This could be related to energetic diffracted surface waves and the double 571 amplification peak close to the resonance frequency, as already mentioned in section 4.1. For  $\Delta X > 2\theta_x$ , the

spatial variability in  $A_bI$  computed from 2D synthetics is greater than the one derived from 1D synthetics, while the opposite is observed for  $A_bID$ . The larger variability in  $A_bI$  (Figure 7c) results from the presence of body and surface waves diffracted at near-surface heterogeneities in the early seismic phase (see Figure 3a). The lower variability in *AbID* (Figure 7d) most probably results from the presence of diffracted surface waves in the analysis time window leading to large duration as shown in Figure 7d.

577

## 578 7. Discussion and conclusion

In this paper, the effect of the *Vs* spatial variability on the surface ground motion is assessed through a set of numerical computations. A simple elastic 2D velocity structure is used for surface ground motion simulations in order to solely focus on the effects of *Vs* variability. *Vs* is modeled as a random field, considering the coefficient of variation (*COV*) and the horizontal and vertical autocorrelation distances ( $\theta x$  and  $\theta z$ , respectively) as parameters.

We show that spatial variation of shear-wave velocity structure efficiently generate locally diffracted surface waves. Hence, the fundamental resonance frequency is only very weakly sensitive to the local *Vs* heterogeneities while the Arias Intensity and derived duration are clearly increased due to the locally diffracted body and surface waves that contaminate the most energetic phase. Analysis of synthetic seismograms at a single station clearly outlines that the coefficient of variation on the shear-wave velocity (*COV*) mostly controls the variability of the four ground motion indicators (resonance frequency and corresponding amplification, Arias intensity and duration), as compared to the horizontal and vertical correlation distances.

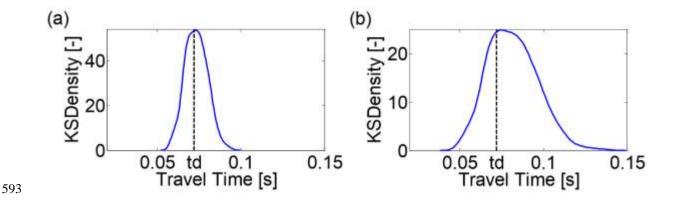


Figure 12. Travel-time distribution (blue thick line) in the soil column below the central receiver for the 100 realizations for (a) model #5 (COV=20%) and (b) model #9 (COV=40%) listed in Table 1. The travel time computed from the deterministic model (td=0.0705 s) is shown a black dashed line.

597 As regards spectral amplifications, considering shear-wave spatial variability in the modeling leads to reduce 598 mean amplification, and conversely to increase variability, at frequencies higher than the site fundamental 599 resonance frequency, as also observed in Assimaki et al. (2003). A striking result is the 20% decrease in mean 600 fundamental resonance frequency ( $f_o$ ) for COV = 40% (Figures 5a and 6a) compared to the resonance frequency 601 of the deterministic model. This reduction of  $f_{0}$ , similarly observed on 1D and 2D ground motion probabilistic 602 modeling, is related to the Vs random field discretization. Indeed, we calculated the SV waves travel time for the 1D soil columns extracted from 2D Vs realizations for two parameter sets with COV = 20% and COV = 40%. 603 604 respectively (probabilistic models #5 and #9, Table 1). The SV wave travel time for the deterministic sedimentary layer is  $t_d$ = 0.0705 s. The travel time for the parameter set having a COV= 20 % (Figure 12a) is 605 606 found to be almost normally distributed around  $t_d$ , while log-normally distributed for the parameter set with 607 COV = 40 % (Figure 12b). In this latter case, a larger number of realizations has travel times exceeding the  $t_d$ , 608 leading to a reduction of the mean fundamental resonance frequency for the 1D ground motion synthetics. 609 Although the calculation of travel times for heterogeneous 2D soil models from 1D soil columns is approximate, 610 these results suggest that long travel times in 2D realizations with COV=40% may be the cause of the resonance 611 frequency drop. Like other authors for 2D probabilistic modeling (Assimaki et al., 2003; Nour et al.; 2003; 612 Thompson et al., 2009) or 1D modeling (Li and Assimaki, 2010; Rathje et al., 2010; Rodriguez-Marek et al., 613 2014; Haji-Soltani et al., 2017), we discretized Vs because it is the physical parameter that can be measured in 614 the field. Considering however that seismic wave propagation is primarily controlled by travel time, random field discretization on travel time would be the most natural and relevant choice. Although such discretization is easily achievable for 1D body or surface wave propagation modeling (e.g. Pilz and Parolai, 2014; Teague and Cox, 2016; Pilz and Fäh, 2017; Teague et al., 2018), it remains more complex for body and surface wave propagation in 2D heterogeneous medium.

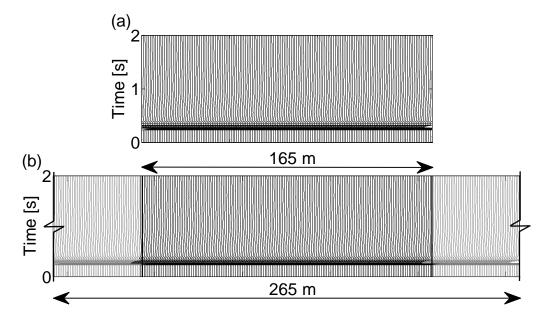
619 Comparison between spectral amplification obtained from 2D and 1D probabilistic modeling synthetics clearly 620 outlines that, although both modeling approaches predict similar mean fundamental resonance frequency and 621 corresponding amplification, 2D wave propagation modeling leads to larger amplification and related variability 622 at frequencies higher than the fundamental resonance frequencies, especially for large COV, compared to 1D wave propagation probabilistic modeling approach (Figures 9 and 11). This increase in amplification and 623 624 standard deviation is definitely caused by the presence of locally diffracted surface waves in 2D wave propagation modeling and, hence, questions the reliability of using a population of 1D Vs profiles to account for 625 626 spatial variability of the seismic properties as proposed for site-specific hazard assessment (Rathje et al., 2010; 627 Rodriguez-Marek et al., 2014; Haji-Soltani et al., 2017).

628 Our results also indicate that the indicator spatial variability increases linearly until reaching a constant value, 629 with greater variability being systematically observed for larger COV. Effect of horizontal correlation distance,  $\theta x$ , is only observed for frequency dependent (and thus wavelength dependent) indicators (Fourier amplitude 630 631 spectra) when receiver spacing, wavelength and lateral size of heterogeneity ( $\theta x$ ) are of the same order. Interestingly, ground motion indicators (Arias Intensity and derived duration) computed on the strongest seismic 632 633 phase dominated by S waves are not anymore spatially correlated for receiver spacing larger than two times the 634 horizontal correlation distance. This opens up interesting perspectives in measuring horizontal autocorrelation 635 distances from available seismological recordings at dense array data. Maximum of the spatial correlation of the 636 natural logarithms of the Fourier amplitude spectra obtained on real data (Abrahamson et al., 1990; Schneider et 637 al., 1992; Ancheta et al., 2011; Goda and Atkinson, 2008; Liu and Hong, 2015; Koufoudi et al., 2018) is close to 638 1 whatever the site condition, i.e. larger than the constant value obtained in our numerical simulations. This may 639 be due to the fact that our modeling is not accounting for 3D wave propagation, attenuation and broadband 640 seismograms.

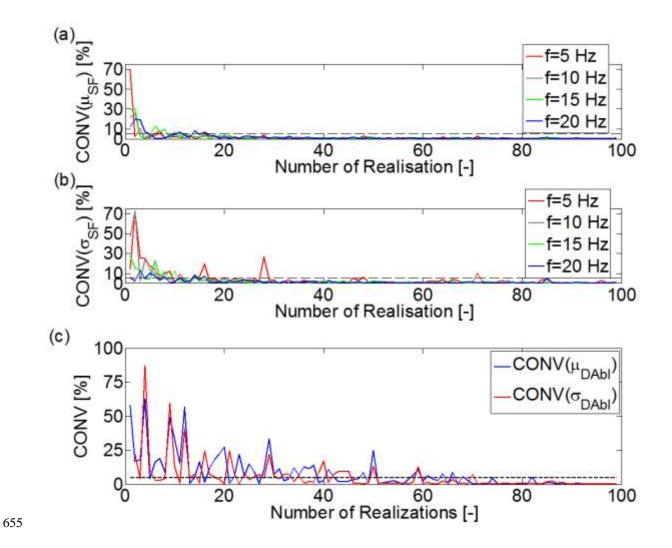
To sum up, our study clearly outlines the main control of *COV* on the spatial variability of surface ground motion indicators and that stochastic ground motion modeling by using 1D variable soil profiles to account for spatial variation of ground structure properties will underestimate surface ground motion spatial variability as 644 compared to full 2D/3D wave propagation modeling. However, the simple case of a sedimentary layer over a
 645 bedrock does not represent the real site conditions. To confirm our results, further numerical simulations should

be carried out for a set of typical ground structure, considering also attenuation and non-linear soil behavior.

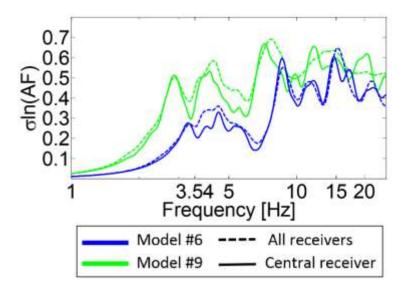
# 647 8. <u>Appendices</u>



Appendix 1 : Synthetic horizontal surface obtained for a homogeneous layer over a half-space for a vertically incident SV plane wave excitation at the base of the homogeneous layer. Source time function is a delta-like with flat Fourier amplitude spectrum between 1 and 25 Hz. Shear-wave velocity in the homogeneous layer is 750 m/s and 1000 m/s in the halfspace. The computational model is 15.5 m along depth axis and (a) 165 m and (b) 750 m width along the horizontal direction. In (b), the computed seismograms are only shown for the 265 central receivers out of 750 receivers.

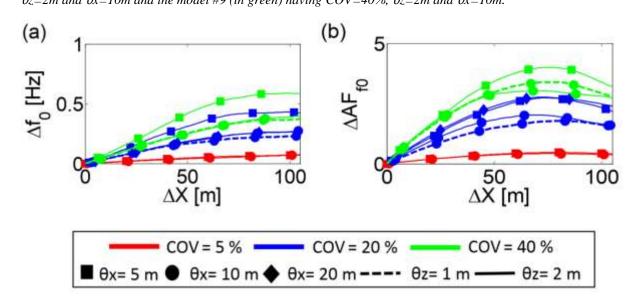


Appendix 2 : Convergence of (a) the average, (b) standard deviation of the Fourier amplitude spectra at various frequencies and (c) the average (in blue) and standard deviation (in red) of the duration as a function of the number of realizations for the horizontal velocity for model #9 with a COV=40%  $\theta x=10$  m and  $\theta z=2$  m. The dashed black line shows the 5% convergence criteria.



661 Appendix 3 : The standard deviation of ln(AF) obtained by considering all the receivers locations (dashed line)

and the central receiver of each realization (continuous line) for the model #6 (in blue) having COV=20%,  $\theta_z=2m$  and  $\theta_x=10m$  and the model #9 (in green) having COV=40%,  $\theta_z=2m$  and  $\theta_x=10m$ .



664

665 Appendix 4 : Spatial variation of the two scalar indicators as a function of the receiver spacing  $\Delta X$ . (a) and (b), 666 Variation of the differences in fundamental resonance frequency  $f_0$  and the related amplification AF using 2D 667 ground motion synthetics. The values of COV,  $\theta x$  and  $\theta z$  are shown with different colors, symbols and lines, 668 respectively.

#### 669 9. <u>Acknowledgments</u>

- 670 This work was supported mainly by Institut de Recherche pour le Développement (IRD), especially the ARTS
- doctoral thesis program and the IRD Young Research Unit JEAI SAMMOVA and by the Lebanese University,
- 672 especially the research project "The effect of soil spatial variability on soils responses".

# 673 10. <u>References</u>

- 674 [1] Abdallah, I., Malkawi, H., Hassan, W., Abdulla, F. (2000). Uncertainty and reliability analysis applied
- 675 to slope stability. Structural Safety 22, 161-187.

- 676 [2] Ancheta, T., Stewart, J., & Abrahamson, N., 2011. Engineering characterization of earthquake ground
  677 motion coherency and amplitude variability, in Proc. 4th IASPEI / IAEE Int. Sym. on Effects of
  678 Surface Geology on Seismic Motion, pp. 23–26.
- [3] Abrahamson, N., Schneider, J., & Stepp, J., 1990. Spatial variation of strong ground motion for use in
  soil structure interaction analyses, in Proceedings of the Fourth U.S. National Conference on
  Earthquake Engineering, Palm Springs, CA.
- [4] Al-Bittar, T., & Soubra, A. H. (2013). Bearing capacity of strip footings on spatially random soils using
  sparse polynomial chaos expansion. International Journal for Numerical and Analytical Methods in
  Geomechanics, 37(13), 2039-2060.
- [5] Al-Bittar, T., & Soubra, A. H. (2017). Bearing capacity of spatially random rock masses obeying
  Hoek–Brown failure criterion. Georisk: Assessment and Management of Risk for Engineered Systems
  and Geohazards, 11(2), 215-229.
- 688 [6] Alonso, E. E., & Krizek, R. J. (1975). Stochastic formulation of soil properties. [No source information
  689 available], 9-32.
- 690 [7] Anagnostopoulos, S. A. (1988). Pounding of buildings in series during earthquakes. Earthquake
  691 engineering & structural dynamics, 16(3), 443-456.
- 692 [8] Arias, A. (1970): Measure of earthquake intensity. Massachusetts Inst. of Tech., Cambridge. Univ. of
  693 Chile, Santiago de Chile.
- 694 [9] Ariman, T. and G. E. Muleski (1981) : A review of the response of buried pipe-lines under seismic
  695 excitations. Earthquake Engineering & Structural Dynamics, 9(2) :133–152.
- [10] Assimaki, A., Pecker, A., Popescu, R., & Prevost, J. (2003): Effects of spatial variability of soil
   properties on surface ground motion. Journal of earthquake engineering, 1-44.
- [11] Bouckovalas, G. D., & Papadimitriou, A. G. (2005). Numerical evaluation of slope topography effects
   on seismic ground motion. Soil Dynamics and Earthquake Engineering, 25(7), 547-558.
- [12] Bradley, B. A., & Cubrinovski, M. (2011). Near-source strong ground motions observed in the 22
   February 2011 Christchurch earthquake. Seismological Research Letters, 82(6), 853-865.
- 702 [13] Burrough, P. A. (1993). Soil Variability:: A Late 20th Century View. CAB International.
- [14] Chouw, N., & Hao, H. (2012). Pounding damage to buildings and bridges in the 22 February 2011
- 704 Christchurch earthquake. International Journal of Protective Structures, 3(2), 123-139.

- 705 [15] Cornforth, D. H. (2005), Landslides in Practice: Investigation, Analysis and Remedial/Preventative
  706 Options in Soils, 624 pp., Wiley, Hoboken, N. J.
- 707 [16] Einsele, G. (2000). Sedimentary basins: evolution, facies, and sediment budget. Springer.
- [17] Electric Power Research Institute (EPRI) (1993). Guidelines for Determining Design Basis Ground
   Motions, Vol. 1–5, Palo Alto, California, Electric Power Research Institute, EPRI TR–102293.
- [18] Fenton, G.A., Griffiths, D.V., (1996). Statistics of free surface flow through stochastic earth dam.
  Journal of Geotechnical Engineering, ASCE 122 (6), 427–436.
- [19] Fenton, G. A. (1999). Estimation for stochastic soil models. Journal of Geotechnical and
  Geoenvironmental Engineering, 125(6), 470-485.
- [20] Field, E. H. (1996). Spectral amplification in a sediment-filled valley exhibiting clear basin-edgeinduced waves. Bulletin of the Seismological Society of America, 86(4), 991-1005.
- [21]Goda, K. and Hong, H.P. (2008) Estimation of Seismic Loss for Spatially Distributed Buildings,
  Earthquake Spectra, 24, pp. 889-910.
- [22] Graves, R. W., Pitarka, A., & Somerville, P. G. (1998). Ground-motion amplification in the Santa
  Monica area: Effects of shallow basin-edge structure. Bulletin of the Seismological Society of
  America, 88(5), 1224-1242.
- [23] Griffiths, D. V., & Fenton, G. A. (2004). Probabilistic slope stability analysis by finite elements.
  Journal of Geotechnical and Geoenvironmental Engineering, 130(5), 507-518.
- [24] Haji Soltani, A., & Pezeshk, S. (2017). A Comparison of Different Approaches to Incorporate Site
   Effects into PSHA: A Case Study for an LNG Tank. Bulletin of the Seismological Society of America,
   107(6), 2927-2947.
- [25] Haldar, S., & Babu, G. S. (2008). Effect of soil spatial variability on the response of laterally loaded
   pile in undrained clay. Computers and Geotechnics, 35(4), 537-547.
- [26] Hall, J. F., Holmes, W. T., & Somers, P. (1994). Northridge earthquake, January 17, 1994. Preliminary
   reconnaissance report.
- [27] Harichandran, R. S. (1999). Spatial variation of earthquake ground motion, what is it, how do we
  model it, and what are its engineering implications. Dept. of Civil and Environmental Engineering,
  Michigan State Univ., East Lansing, Mich.
- [28] Hart, G. C., Lew, M., & DiJulio, R. M. (1975). Torsional response of high-rise buildings. Journal of the
  Structural Division, 101(2), 397-416.

- [29] Holzer, T. L., Padovani, A. C., Bennett, M. J., Noce, T. E., & Tinsley, J. C. (2005). Mapping NEHRP
  VS30 site classes. Earthquake Spectra, 21(2), 353-370.
- 737 [30] ITASCA. (2011). FLAC Fast Lagrangian Analysis of Continua (Vol. Version 7.0). Minneapolis,
  738 USA: Itasca Consulting Group.
- [31] Jaksa, M. B., Brooker, P. I., & Kaggwa, W. S. (1997). Modelling the spatial variability of the
  undrained shear strength of clay soils using geostatistics. In Proc. of 5th Int. Geostatistics Congress (pp.
  1284-1295).
- [32] Jenny, H. (1941). Factors of soil formation: A system of quantitative pedology, 281 pp.
- [33] Jongmans, D., & Campillo, M. (1990). The 1983 Liege earthquake: damage distribution and site
  effects. Earthquake spectra, 6 (4), 713-737.
- [34] Karray, M., Lefebvre, G., Ethier, Y., & Bigras, A. (2011). Influence of particle size on the correlation
  between shear wave velocity and cone tip resistance. Canadian Geotechnical Journal, 48(4), 599-615.
- [35] Khazaie, S., Cottereau, R., & Clouteau, D. (2016). Influence of the spatial correlation structure of an
  elastic random medium on its scattering properties. Journal of Sound and Vibration, 370, 132-148.
- [36] Khazaie, S., Cottereau, R., & Clouteau, D. (2017). Numerical observation of the equipartition regime in
  a 3D random elastic medium, and discussion of the limiting parameters. Computers & Geosciences,
  102, 56-67.
- [37] Kiureghian, Armen Der and Ansgar Neuenhofer (1992) : Response spectrum me- thod for multi support seismic excitations. Earthquake Engineering & Structural Dynamics, 21(8) :713–740.
- [38] Koufoudi, E., Chaljub, E., Douste Bacqué, I., Roussel, S., Bard, P. Y., Larose, E., ... & Baillet, L.
- 755 (2018). A High Resolution Seismological Experiment to Evaluate and Monitor the Seismic Response
  756 of the Saint Guérin Arch Dam, French Alps. Seismological Research Letters.
- [39] Konno, K., & Ohmachi, T. (1998). Ground-motion characteristics estimated from spectral ratio
  between horizontal and vertical components of microtremor. Bulletin of the Seismological Society of
  America, 88(1), 228-241.
- [40] Kozák, J. T. (2009). Tutorial on earthquake rotational effects: historical examples. Bulletin of the
  Seismological Society of America, 99(2B), 998-1010.
- 762 [41] Kramer, S. L. (1996). Geotechnical earthquake engineering. Pearson Education India.
- [42] Kuhlemeyer, R. L., & Lysmer, J. (1973). Finite element method accuracy for wave propagation
   problems. Journal of Soil Mechanics & Foundations Div, 99(Tech Rpt).

- [43] Lacoss, R. T., Kelly, E. J., & Toksöz, M. N. (1969). Estimation of seismic noise structure using arrays.
  Geophysics, 34(1), 21-38.
- 767 [44] Li, W., & Assimaki, D. (2010). Site-and motion-dependent parametric uncertainty of site-response
  768 analyses in earthquake simulations. Bulletin of the Seismological Society of America, 100(3), 954-968.
- [45] Li, C.-C., & Der Kiureghian, A. (1993). Optimal discretization of random fields. Journal of
   Engineering Mechanics, 119 (6), 1136-1154.
- [46] Liu T. J. and H. P. Hong. (2015) Application of Spatially Correlated and Coherent Records of Scenario
   Event to Estimate Seismic Loss of a Portfolio of Buildings. Earthquake Spectra 31:4, 2047-2068.
- [47] Lopez-Caballero, Fernando, & Modaressi-Farahmand-Razavi, A. (2010). Assessment of variability and
  uncertainties effects on the seismic response of a liquefiable soil profile. Soil Dynamics and
  Earthquake Engineering , 30 (7), 600-613.
- [48] Moczo, P., & Bard, P. Y. (1993). Wave diffraction, amplification and differential motion near strong
  lateral discontinuities. Bulletin of the Seismological Society of America, 83(1), 85-106.
- [49] Moss, R. E. S. (2008). Quantifying measurement uncertainty of thirty-meter shear-wave velocity.
  Bulletin of the Seismological Society of America, 98(3), 1399-1411.
- [50] Nour, A., Slimani, A., Laouami, N., & Afra, H. (2003). Finite element model for the probabilistic
  seismic response of heterogeneous soil profile. Soil dynamics and earthquake engineering, 23 (5), 331348.
- [51] Pagliaroli, A. (2006). Studio numerico e sperimentale dei fenomeni di amplificazione sismica locale di
  rilievi isolati. Roma: Ph.D. Thesis, Università di Roma "La Sapienza".
- [52] Pagliaroli, A., Lanzo, G., Tommasi, P., & Di Fiore, V. (2014a). Dynamic characterization of soils and
  soft rocks of the Central Archeological Area of Rome. Bulletin of earthquake engineering , 12 (3),
  1365-1381.
- [53] Pagliaroli, A., Moscatelli, M., Raspa, G., & Naso, G. (2014b). Seismic microzonation of the central
  archaeological area of Rome: results and uncertainties. Bull. Earthq. Eng., 12, 1405–1428.
- [54] Parzen, E. (1962). On estimation of a probability density function and mode. The annals of
  mathematical statistics, 33(3), 1065-1076.
- 792 [55] Phoon, K. K., & Kulhawy, F. H. (1996). On quantifying inherent soil variability.
- [56] Phoon, K. K., & Kulhawy, F. H. (1999). Characterization of geotechnical variability. Canadian
  Geotechnical Journal, 36(4), 612-624.

- [57] Pilz, M., & Fäh, D. (2017). The contribution of scattering to near-surface attenuation. Journal of
  Seismology, 21(4), 837-855.
- [58] Pilz, M., & Parolai, S. (2014). Statistical properties of the seismic noise field: influence of soil
  heterogeneities. Geophysical Journal International, 199(1), 430-440.
- [59] Popescu, R. (1995). Stochastic variability of soil properties: data analysis, digital simulation, effects on
  system behavior. Princeton University.
- [60] Rathje, Ellen M., Albert R. Kottke, and Whitney L. Trent (2010). Influence of input motion and site
   property variabilities on seismic site response analysis, Journal of geotechnical and geoenvironmental
   engineering 136.4: 607-619.
- 804 [61] Rodriguez Marek, A., Rathje, E. M., Bommer, J. J., Scherbaum, F., & Stafford, P. J. (2014).
- 805 Application of single station sigma and site response characterization in a probabilistic seismic -
- hazard analysis for a new nuclear site. Bulletin of the Seismological Society of America, 104(4), 16011619.
- [62] Rosenblatt, M. (1956). Remarks on some nonparametric estimates of a density function. The Annals of
   Mathematical Statistics, 27(3), 832-837.
- [63] Sacks, J., Welch, W. J., Mitchell, T. J., & Wynn, H. P. (1989). Design and analysis of computer
  experiments. Statistical science, 409-423.
- [64] Salloum, N, D. Jongmans, C. Cornou, D. Youssef Abdel-Massih, F. Hage Chehade, C. Voisin, & A.
  Mariscal. (2014). The shear wave velocity structure of the heterogeneous alluvial plain of Beirut
  (Lebanon): combined analysis of geophysical and geotechnical data, soumis à Geophysical Journal
  International.
- [65] Salloum, N. (2015). Evaluation de la variabilité spatiale des paramètres géotechniques du sol à partir de
  mesures géophysiques: application à la plaine alluviale de Nahr-Beyrouth (Liban). Phd thesis,
  Universite Joseph Fourier, Grenoble.
- [66] Scherbaum, F., Hinzen, K. G., & Ohrnberger, M. (2003). Determination of shallow shear wave velocity
  profiles in the Cologne, Germany area using ambient vibrations. Geophysical Journal International,
  152(3), 597-612.
- [67] Schneider, J., Stepp, J., & Abrahamson, N., 1992. The spatial variation of earthquake ground motion
  and effects of local site onditions, in Proceedings of 10th World Conference on Earthquake
  Engineering, Madrid, Spain.

- [68] Soubra, A. H., & Massih, D. Y. A. (2010). Probabilistic analysis and design at the ultimate limit state
  of obliquely loaded strip footings. Géotechnique, 60(4), 275-285.
- 827 [69] Srivastava, A., & Babu, G. S. (2009). Effect of soil variability on the bearing capacity of clay and in
  828 slope stability problems. Engineering Geology, 108(1-2), 142-152.
- [70] Stamatopoulos, C. A., Bassanou, M., Brennan, A. J., & Madabhushi, G. (2007). Mitigation of the
  seismic motion near the edge of cliff-type topographies. Soil Dynamics and Earthquake Engineering,
  27(12), 1082-1100.
- [71] Stamatopoulos, C. A., & Bassanou, M. (2009). Mitigation of the seismic motion near the edge of clifftype topographies using anchors and piles. Bulletin of Earthquake Engineering, 7(1), 221-253.
- [72] Sudret, B., & Der Kiureghian, A. (2000). Stochastic finite element methods and reliability: a state-ofthe-art report. Department of Civil and Environmental Engineering, University of California.
- [73] Teague, D. P., & Cox, B. R. (2016). Site response implications associated with using non-unique vs
  profiles from surface wave inversion in comparison with other commonly used methods of accounting
  for vs uncertainty. Soil Dynamics and Earthquake Engineering, 91, 87-103.
- [74] Teague, D. P., Cox, B. R., & Rathje, E. M. (2018). Measured vs. predicted site response at the garner
  valley downhole array considering shear wave velocity uncertainty from borehole and surface wave
  methods. Soil Dynamics and Earthquake Engineering, 113, 339-355.
- [75] Thompson, E. M., Baise, L. G., & Kayen, R. E. (2007). Spatial correlation of shear-wave velocity in
  the San Francisco Bay Area sediments. Soil Dynamics and Earthquake Engineering, 27(2), 144-152.
- [76] Thompson, E., Baise, L., Kayen, R., & Guzina, B. (2009). Impediments to Predicting Site Response:
  Seismic Property Estimation and Modeling Simplifications. Bulletin of the Seismological Society of
  America, BSSA, 99, 2927-2949
- [77] Toro, G. R. 1995. Probabilistic models of site velocity profiles for generic and site-specific ground motion amplification studies, Technical Rep. No. 779574, Brookhaven National Laboratory, Upton,
- N.Y.Trifunac, M. D., & Todorovska, M. I. (1997). Northridge, California, earthquake of 1994: density
  of pipe breaks and surface strains. Soil Dynamics and Earthquake Engineering, 16(3), 193-207.
- [78] Trifunac, M. D., & Todorovska, M. I. (1997). Northridge, California, earthquake of 1994: density of
  pipe breaks and surface strains. Soil Dynamics and Earthquake Engineering, 16(3), 193-207.
- 853 [79] Trifunac, M. D. (2009). Review: rotations in structural response. Bulletin of the Seismological Society
- 854 of America, 99(2B), 968-979.

- 855 [80] Vanmarcke, E. (2010). Random fields: analysis and synthesis. World Scientific.
- [81] Wills, C. J., & Clahan, K. B. (2006). Developing a map of geologically defined site-condition
  categories for California. Bulletin of the Seismological Society of America, 96(4A), 1483-1501.
- [82] Youssef Abdel Massih, D. S., Soubra, A. H., & Low, B. K. (2008). Reliability-based analysis and
  design of strip footings against bearing capacity failure. Journal of geotechnical and geoenvironmental
  engineering, 134(7), 917-928.
- [83] Zhao, T., Montoya-Noguera, S., Phoon, K. K., & Wang, Y. (2017). Interpolating spatially varying soil
  property values from sparse data for facilitating characteristic value selection. Canadian Geotechnical
  Journal, 55(2), 171-181.