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# Dynamics of beams on a periodical foundation: an analytical approach

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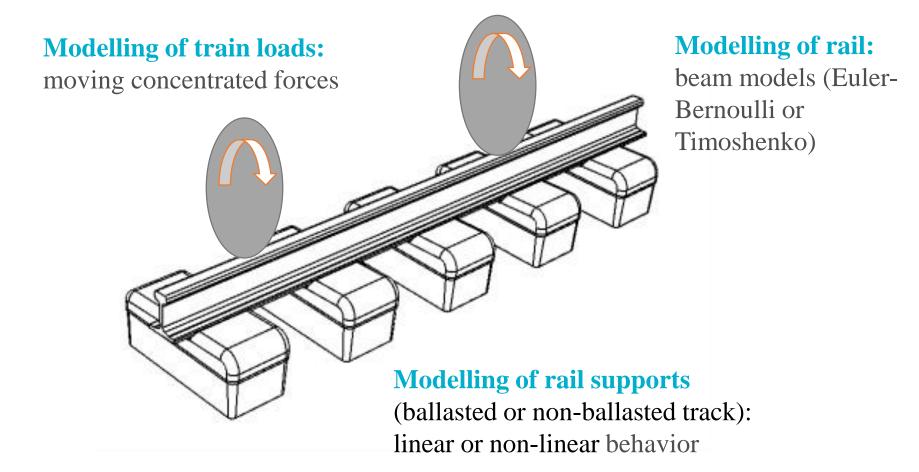


Metz, 27th October 2016



## **Dynamics of railway track**

Models of railway track



Periodic supports

Numerical example

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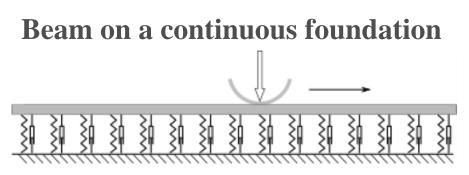
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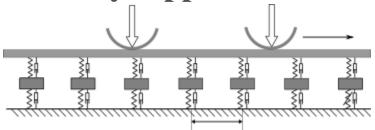
Green function





- The beam is supported by a homogenous foundation<sup>1</sup>
- The reaction force of the foundation applies to all the beam length

### **Periodically supported beam**



- The beam is supported by supports at periodical distances<sup>2</sup>
- The reaction forces of supports are concentrated forces

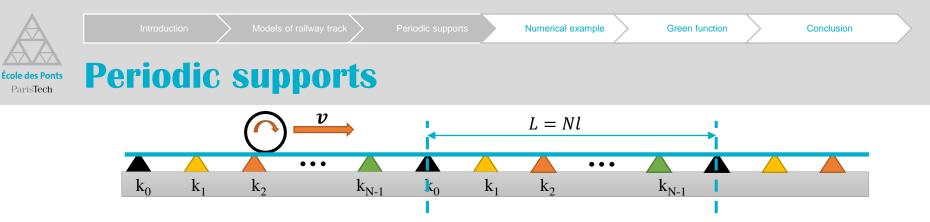
## Influence of non-homogeneous foundation Influence of support width

<sup>1</sup> Fryba, L. Vibration of Solids and Structures Under Moving Loads. Noordhoff International Publishing, Leyden (1972)
 <sup>2</sup> Mead, D. J. Free wave propagation in periodically supported, infite beams, Journal of Sound and Vibration 11 (2) (1970)



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\* *N* different types of supports which are distributed periodically along the beam

 $k_{p+N} = k_p \ (0 \le p < N \ )$ 

Behavior law of the supports

 $\underline{\hat{R}} = -\underline{\mathbf{K}}\,\underline{\widehat{w}} \qquad (1)$ 

• In the steady-state, the responses of supports are repeated when the moving loads cover the length L = Nl

$$R_{p+N}(t) = R_p\left(t - \frac{L}{\nu}\right)$$

where L/v is the time for the moving loads cover the distance *L*.

#### Euler-Bernoulli equation

$$EI\frac{\partial^4 w}{\partial x^4} + \rho S\frac{\partial^2 w}{\partial t^2} + Q\delta(x - vt) = \sum_n R_n(t)\,\delta(x - nl)$$

#### Fourier transforms

• with respect to t, then with respect to x

#### Dirac comb properties

#### Inverse Fourier transform

• with respect to *x* 

#### General relation

• between reaction forces  $R_n$  ( $0 \le n < N$ ) and beam displacement w







Numerical example

#### Matrix form

$$\underline{\underline{\mathbf{D}}}\,\,\underline{\hat{R}} = \eta_e Q_e \underline{I} + \underline{\widehat{w}} \quad (2)$$

where  $\underline{\mathbf{D}}$ ,  $\eta_e$ ,  $Q_e$  depend only on the parameters of the beams and the loads.

Discrete Fourier form

$$\mathcal{F}_q(\underline{\widehat{w}}) = \chi_q \mathcal{F}_q(\underline{\widehat{R}}) - NQ_e \delta_{1q} \quad (3)$$

When the supports are identical, this relation becomes the relation of the equivalent system of a periodically supported beam<sup>3</sup>

$$\hat{w}(x,\omega) = \hat{R}(\omega)\eta(x,\omega) + Q_E(\omega)e^{-i\frac{\omega}{v}x}$$

 Direct method: By combining directly the general relation (2) with the behaviour law (1) of the supports, we can obtain the responses

Green function

$$\underline{\widehat{w}} = \eta_e Q_e \left(\underline{\underline{\mathbf{D}}} \, \underline{\underline{\mathbf{K}}} + \underline{\underline{\mathbf{I}}}\right)^{-1} \underline{I}$$

- Iterative method: by using equation
  (3) to calculate the responses by iteration through 3 steps:
  - Take the responses with the uniform supports as the value of the initial iteration
  - Calculate the variation of forces by using the behavior law
  - Recalculate the displacements by using the relation (3) with FFT.

<sup>3</sup> Hoang et al., Response of a periodically supported beam on a nonlinear foundation subjected to moving loads, Nonlinear Dynamics (2016)



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Conclusion



\* When railway sleepers are damaged, we can do the replacement 1 by 3 in order to reduce the maintenance time. This process creates periodical supports of period N = 3

| Parameter                       | Notation       | Unit             | Value |
|---------------------------------|----------------|------------------|-------|
| Rail mass                       | ρS             | kg/m             | 60    |
| Rail stiffness                  | EI             | MNm <sup>2</sup> | 6,3   |
| Train speed                     | ν              | km/h             | 160   |
| Charge per wheel                | Q              | kN               | 100   |
| Wheels distance                 | Н              | m                | 3     |
| Block mass                      | m              | kg               | 90    |
| Sleeper distance                | l              | m                | 0,6   |
| Stiffness of rail pad           | $k_p$          | MN/m             | 200   |
| Damping coefficient of rail pad | $\eta_p$       | MNs/m            | 1,0   |
| Stiffness of sleeper            | $k_f$          | MN/m             | 20    |
| Damping coefficient of sleeper  | $\eta_f$       | MNs/m            | 0,2   |
| Stiffness of damaged sleeper    | k <sub>d</sub> | MN/m             | 10    |

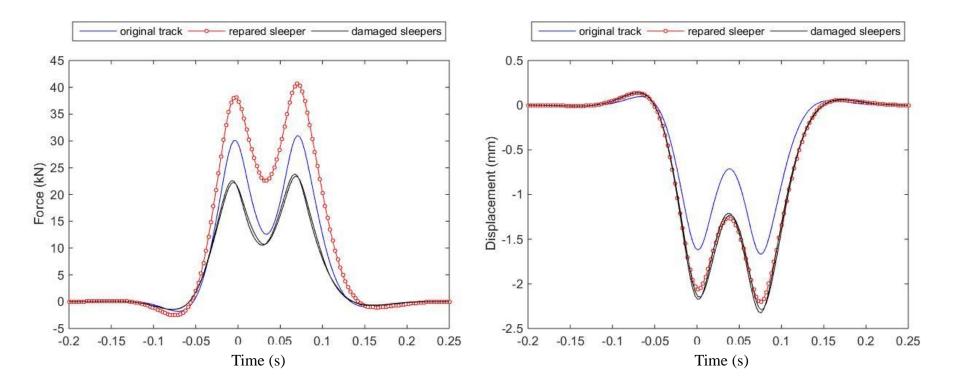


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### **Reaction forces of the supports Displacement of the supports**





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Conclusion



## **Beam on a periodical foundation**

- Consider an Euler-Bernoulli beam on a periodical foundation submitted moving forces
- Constitutive law of the foundation:

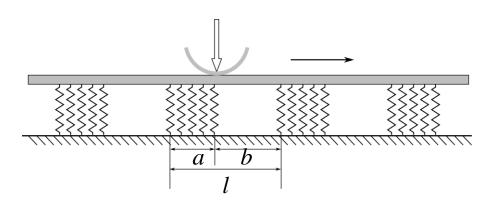
$$R_n(x,t) = \begin{cases} k_a w(x,t) & \text{if } nl < x < nl + a \\ k_b w(x,t) & \text{if } nl + a < x < (n+1)l \end{cases}$$

 Reaction forces of the foundation described by

$$F(x,t) = \sum_{n=-\infty}^{\infty} R_n(x,t)$$

Dynamical equation of the beam

$$EI\frac{\partial^4 w(x,t)}{\partial x^4} + \rho S\frac{\partial^2 w(x,t)}{\partial t^2} + Q\delta(x-vt) = F(x,t)$$



 Condition of periodicity: In the steady state, the reaction forces are repeated when the moving forces cover a distance equal to the period length of the foundation

$$R_n(x,t) = R_0\left(x - nl, t - \frac{nl}{v}\right)$$

where  $R_0(x,t)$  is the reaction force of the foundation in an interval [0,l]





\* Let  $\mathcal{L}$  be the linear operation on the right hand side of the dynamic equation. The Green function of  $\mathcal{L}$  is defined by

$$\mathcal{L}G(x,s) = \sum_{n} \delta(x - nl - s) \tag{4}$$

✤ We can write

$$\int \mathcal{L}G(x,s)\widehat{R}_0(s,\omega)ds = \sum_n \widehat{R}_0(x-nl,\omega) = \mathcal{L}\widehat{w}(x,\omega)$$

Thus, the response of the beam is calculated by

$$\widehat{w}(x,\omega) = \int G(x,s)\widehat{R}_0(s,\omega)ds$$

\* Equation (4) is exactly the dynamic equation of a periodically supported beam with  $R(\omega) \equiv 1$ , which is characterized by the relation<sup>3</sup>

$$\hat{w}_r(x,\omega) = \hat{R}(\omega)\eta(x,\omega) + Q_E(\omega)e^{-i\frac{\omega}{\nu}x}$$

Thus, the Green function is given by

$$G(x,s) = \eta(x-s,\omega) + Q_E(\omega) e^{-i\frac{\omega}{\nu}(x-s)}$$







#### Conclusion

## Conclusion

- The model of beams on a foundation can be extended for modelling the dynamics of railway tracks on a nonhomogeneous foundation
- When the reaction forces of supports are concentrated, the responses of the railway track on a periodical foundation can be calculated by an analytical method
- Moreover, the supports with large width (the reaction forces can not be considered as point forces), the Green function is demonstrated and it can be used to compute the responses by an iterative method.

## Thank you for your attention!

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