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► **To cite this version:**

Rita Riedlbeck, Daniele Di Pietro, Alexandre Ern. Equilibrated stress reconstructions for linear elasticity problems with application to a posteriori error analysis. 2017. hal-01433841v1

HAL Id: hal-01433841

<https://hal.science/hal-01433841v1>

Preprint submitted on 13 Jan 2017 (v1), last revised 16 Feb 2017 (v2)

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Equilibrated stress reconstructions for linear elasticity problems with application to a posteriori error analysis

Rita Riedlbeck, Daniele A. Di Pietro, and Alexandre Ern

Abstract We present an a posteriori error estimate for the linear elasticity problem. The estimate is based on an equilibrated reconstruction of the Cauchy stress tensor, which is obtained from mixed finite element solutions of local Neumann problems. We propose two different reconstructions, one using Arnold–Winther mixed finite element spaces providing a symmetric stress tensor and one using Arnold–Falk–Winther mixed finite element spaces with a weak symmetry constraint. The performance of the estimate is illustrated on a numerical test with analytical solution.

Key words: a posteriori error estimate, linear elasticity, equilibrated stress reconstruction, Arnold–Winther finite element spaces, Arnold–Falk–Winther finite element spaces

MSC (2010): 65N15, 74S05

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The work of D. A. Di Pietro was partially supported by ANR grant HHOMM (ANR-15-CE40-0005)

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1 Introduction

We consider the linear elasticity problem

$$-\nabla \cdot \boldsymbol{\sigma}(\mathbf{u}) = \mathbf{f} \quad \text{in } \Omega, \quad (1a)$$

$$\mathbf{u} = \mathbf{0} \quad \text{on } \partial\Omega, \quad (1b)$$

where $\Omega \subset \mathbb{R}^2$ is a simply connected polygon, $\mathbf{u} : \Omega \rightarrow \mathbb{R}^2$ the displacement, and $\mathbf{f} : \Omega \rightarrow \mathbb{R}^2$ denotes the volumetric body force. The Cauchy stress tensor $\boldsymbol{\sigma}$ is given by Hooke's law $\boldsymbol{\sigma}(\mathbf{u}) = \lambda \operatorname{tr}(\boldsymbol{\varepsilon}(\mathbf{u}))\mathbf{I} + 2\mu\boldsymbol{\varepsilon}(\mathbf{u})$, where λ and μ are the Lamé parameters, and the symmetric gradient $\boldsymbol{\varepsilon}(\mathbf{u}) = \frac{1}{2}((\nabla\mathbf{u})^T + \nabla\mathbf{u})$ describes the infinitesimal strain.

In many applications, this problem is numerically solved using H^1 -conforming finite elements. It is well known that, in contrast to the analytical solution, the resulting discrete stress tensor does not have continuous normal components across mesh interfaces, and that its divergence is not locally in equilibrium with the source term \mathbf{f} on mesh cells. In this paper we propose an a posteriori error estimate based on stress tensor functions which are reconstructed from the discrete stress tensor such that they verify both of the above properties. Such equilibrated-flux a posteriori error estimates offer several advantages. First, error upper bounds are obtained with fully computable constants. Second, polynomial-degree robustness can be achieved for the Poisson problem in [3, 8]. Third, they allow to distinguish among various error components, e.g., discretization, linearization, and algebraic solver error components, and to equilibrate adaptively these components in the iterative solution of nonlinear problems [7]. An advantage for more general problems in solid mechanics is that the stress reconstruction is based on the discrete stress (not the displacement) and thus the estimate does not depend on the mechanical behaviour law.

We present two stress reconstructions. Both use mixed finite elements on cell patches around mesh vertices, as proposed for the Poisson problem in [6, 4]. The first one was introduced in [9] and uses the Arnold–Winther (AW) mixed finite element spaces [2] providing a symmetric stress tensor. The second one follows the same approach, but imposing the symmetry only weakly and using the Arnold–Falk–Winther (AFW) mixed finite element spaces [1]. The AFW reconstruction offers some advantages over the AW one: it is cheaper (since by hybridization techniques we can avoid the resolution of saddle point problems), and the extension to three dimensional problems is – from an implementational point of view – easier (the AW element in 3D has 162 degrees of freedom per element). The perspective of this work is to deal with nonlinear elasticity and plasticity problems and to distinguish different error components in the estimate.

2 Setting

We denote by $L^2(\Omega)$ the space of square-integrable functions taking values in \mathbb{R} , and by (\cdot, \cdot) and $\|\cdot\|$ the corresponding inner product and norm. $H^1(\Omega)$ stands for the Sobolev space composed of $L^2(\Omega)$ functions with weak gradients in $[L^2(\Omega)]^2$ and $H_0^1(\Omega)$ for its zero-trace subspace. We assume henceforth that \mathbf{f} lies in $[L^2(\Omega)]^2$.

The weak formulation of (1) reads: find $\mathbf{u} \in [H_0^1(\Omega)]^2$ such that

$$(\boldsymbol{\sigma}(\mathbf{u}), \boldsymbol{\varepsilon}(\mathbf{v})) = (\mathbf{f}, \mathbf{v}) \quad \forall \mathbf{v} \in [H_0^1(\Omega)]^2. \quad (2)$$

The discretization of (2) is based on a conforming triangulation \mathcal{T}_h of Ω , verifying the minimum angle condition. We will use a conforming finite element method of order $p \geq 2$. Let $\mathbb{P}^p(\mathcal{T}_h) := \{v \in L^2(\Omega) \mid \forall T \in \mathcal{T}_h \ v|_T \in \mathbb{P}^p(T)\}$, where $\mathbb{P}^p(T)$ is the space of polynomials on T of degree less than or equal to p . For the sake of simplicity we assume that \mathbf{f} lies in $[\mathbb{P}^{p-1}(\mathcal{T}_h)]^2$. Then the discrete problem reads: find $\mathbf{u}_h \in [H_0^1(\Omega)]^2 \cap [\mathbb{P}^p(\mathcal{T}_h)]^2$ such that

$$(\boldsymbol{\sigma}(\mathbf{u}_h), \boldsymbol{\varepsilon}(\mathbf{v}_h)) = (\mathbf{f}, \mathbf{v}_h) \quad \forall \mathbf{v}_h \in [H_0^1(\Omega)]^2 \cap [\mathbb{P}^p(\mathcal{T}_h)]^2. \quad (3)$$

3 A Posteriori Error Estimate

In this section, we derive an upper bound on the error between the analytical solution of (2) and an arbitrary function $\mathbf{u}_h \in [H_0^1(\Omega)]^2 \cap [\mathbb{P}^p(\mathcal{T}_h)]^2$. We will measure this error in the energy norm

$$\|\mathbf{v}\|_{\text{en}}^2 := (\boldsymbol{\sigma}(\mathbf{v}), \boldsymbol{\varepsilon}(\mathbf{v})) = 2\mu\|\boldsymbol{\varepsilon}(\mathbf{v})\|^2 + \lambda\|\nabla \cdot \mathbf{v}\|^2 \geq 2\mu C_K \|\nabla \mathbf{v}\|^2, \quad (4)$$

where the last bound follows from $\lambda \geq 0$ and Korn's inequality. Owing to (1b), we have $C_K = \frac{1}{2}$. This value would have been different if we had chosen mixed boundary conditions. We start by introducing reconstructed stress tensors that are more "physical" than $\boldsymbol{\sigma}(\mathbf{u}_h)$, which in general does not lie in $\mathbf{H}(\text{div}, \Omega) = \{\boldsymbol{\tau} \in [L^2(\Omega)]^{2 \times 2} \mid \nabla \cdot \boldsymbol{\tau} \in [L^2(\Omega)]^2\}$ and thus cannot verify the equilibrium equation (1b). Notice that, unlike $\boldsymbol{\sigma}(\mathbf{u}_h)$, these reconstructed tensors may not be symmetric.

Definition 1 (Equilibrated stress reconstruction). We call *equilibrated stress reconstruction* any function $\boldsymbol{\sigma}_h \in \mathbf{H}(\text{div}, \Omega)$ constructed from $\boldsymbol{\sigma}(\mathbf{u}_h)$ such that

$$(-\nabla \cdot \boldsymbol{\sigma}_h, \mathbf{z})_T = (\mathbf{f}, \mathbf{z})_T \quad \forall \mathbf{z} \in \mathbf{RM} \ \forall T \in \mathcal{T}_h, \quad (5)$$

where $\mathbf{RM} := \{\mathbf{b} + c(x_2, -x_1)^T \mid \mathbf{b} \in \mathbb{R}^2, c \in \mathbb{R}\}$ is the space of rigid body motions.

Theorem 1 (A posteriori error estimate). Let $\mathbf{u} \in [H_0^1(\Omega)]^2$ be the weak solution of (2) and $\mathbf{u}_h \in [H_0^1(\Omega)]^2$ be arbitrary. Let $\boldsymbol{\sigma}_h$ be a stress reconstruction verifying Definition 1. Then

$$\|\mathbf{u} - \mathbf{u}_h\|_{\text{en}} \leq \mu^{-1/2} \left(\sum_{T \in \mathcal{T}_h} \left(\frac{h_T}{\pi} \|\mathbf{f} + \nabla \cdot \boldsymbol{\sigma}_h\|_T + \|\boldsymbol{\sigma}_h - \boldsymbol{\sigma}(\mathbf{u}_h)\|_T \right)^2 \right)^{1/2}. \quad (6)$$

Proof. From (4) and the symmetry of $\boldsymbol{\sigma}(\mathbf{u} - \mathbf{u}_h)$, we infer that

$$\begin{aligned} \|\mathbf{u} - \mathbf{u}_h\|_{\text{en}} &= \left(\boldsymbol{\sigma}(\mathbf{u} - \mathbf{u}_h), \frac{\boldsymbol{\varepsilon}(\mathbf{u} - \mathbf{u}_h)}{\|\mathbf{u} - \mathbf{u}_h\|_{\text{en}}} \right) \leq \mu^{-1/2} \left(\boldsymbol{\sigma}(\mathbf{u} - \mathbf{u}_h), \frac{\boldsymbol{\varepsilon}(\mathbf{u} - \mathbf{u}_h)}{\|\nabla(\mathbf{u} - \mathbf{u}_h)\|} \right) \\ &\leq \mu^{-1/2} \sup_{\mathbf{v} \in [H_0^1(\Omega)]^2; \|\nabla \mathbf{v}\|=1} (\boldsymbol{\sigma}(\mathbf{u} - \mathbf{u}_h), \nabla \mathbf{v}). \end{aligned} \quad (7)$$

Fix $\mathbf{v} \in [H_0^1(\Omega)]^2$, such that $\|\nabla \mathbf{v}\| = 1$. Using the fact that \mathbf{u} verifies (2), and inserting $(\nabla \cdot \boldsymbol{\sigma}_h, \mathbf{v}) + (\boldsymbol{\sigma}_h, \nabla \mathbf{v}) = 0$ into the term inside the supremum yields

$$(\boldsymbol{\sigma}(\mathbf{u} - \mathbf{u}_h), \nabla \mathbf{v}) = (\mathbf{f}, \mathbf{v}) - (\boldsymbol{\sigma}(\mathbf{u}_h), \nabla \mathbf{v}) = (\mathbf{f} + \nabla \cdot \boldsymbol{\sigma}_h, \mathbf{v}) + (\boldsymbol{\sigma}_h - \boldsymbol{\sigma}(\mathbf{u}_h), \nabla \mathbf{v}). \quad (8)$$

For the first term in the right hand side of (8) we use (5) to insert the mean value $\Pi_T^0 \mathbf{v}$ of \mathbf{v} on T , the Cauchy–Schwarz inequality, and the Poincaré inequality $\|\mathbf{v} - \Pi_T^0 \mathbf{v}\|_T \leq \frac{h_T}{\pi} \|\nabla \mathbf{v}\|_T$ on simplexes $T \in \mathcal{T}_h$, and obtain

$$|(\mathbf{f} + \nabla \cdot \boldsymbol{\sigma}_h, \mathbf{v})| \leq \left| \sum_{T \in \mathcal{T}_h} (\mathbf{f} + \nabla \cdot \boldsymbol{\sigma}_h, \mathbf{v} - \Pi_T^0 \mathbf{v})_T \right| \leq \sum_{T \in \mathcal{T}_h} \frac{h_T}{\pi} \|\mathbf{f} + \nabla \cdot \boldsymbol{\sigma}_h\|_T \|\nabla \mathbf{v}\|_T,$$

whereas the Cauchy–Schwarz inequality applied to the second term directly yields

$$|(\boldsymbol{\sigma}_h - \boldsymbol{\sigma}(\mathbf{u}_h), \nabla \mathbf{v})| \leq \sum_{T \in \mathcal{T}_h} \|\boldsymbol{\sigma}_h - \boldsymbol{\sigma}(\mathbf{u}_h)\|_T \|\nabla \mathbf{v}\|_T.$$

Inserting these results in (7) and again applying the Cauchy–Schwarz inequality yields the result. \square

4 Stress Tensor Reconstructions

For the stress tensor reconstructions we will use mixed finite element formulations on patches around mesh vertices in the spirit of [6, 4] to obtain $\mathbf{H}(\text{div}, \Omega)$ -conforming equilibrated stresses. We propose two reconstructions, one using the Arnold–Winther (AW) mixed finite element spaces [2] providing a symmetric stress tensor, and one using the Arnold–Falk–Winther (AFW) mixed finite element spaces [1] where the symmetry of the stress tensor is imposed weakly.

The set of vertices of the mesh \mathcal{T}_h is denoted by \mathcal{V}_h ; it is decomposed into interior vertices $\mathcal{V}_h^{\text{int}}$ and boundary vertices $\mathcal{V}_h^{\text{ext}}$. For all $a \in \mathcal{V}_h$, \mathcal{T}_a is the patch of elements sharing the vertex a , ω_a the corresponding open subdomain in Ω , \mathbf{n}_{ω_a} its unit outward normal vector, and ψ_a the piecewise affine “hat” function which takes

the value 1 at the vertex a and zero at all the other vertices. For all $T \in \mathcal{T}_h$, \mathcal{V}_T denotes the set of vertices of T and h_T its diameter.

Let from now on \mathbf{u}_h be the solution of (3). The goal is to minimize the error estimate (6), but to avoid a global computation. As a result, both of the proposed reconstructions are based on the following local minimization problems on the patches ω_a :

$$\sigma_h^a := \arg \min_{\tau_h \in \Sigma_h^a, \nabla \cdot \tau_h = \psi_a \mathbf{f}} \|\tau_h - \psi_a \sigma(\mathbf{u}_h)\|_{\omega_a}, \quad (9)$$

where we define Σ_h^a separately for each construction and add a weak symmetry constraint in the second (AFW) construction. The global reconstructed stress tensor σ_h is then obtained summing over all mesh vertices these local solutions σ_h^a .

4.1 Arnold–Winther Stress Reconstruction

For each element $T \in \mathcal{T}_h$, the local AW polynomial mixed finite element spaces [2] of degree $k \geq 1$ are defined by

$$\mathbf{S}_T^{\text{AW}} := \{\tau \in [\mathbb{P}^{k+2}(T)]_{\text{sym}}^{2 \times 2} \mid \nabla \cdot \tau \in [\mathbb{P}^k(T)]^2\}, \quad \mathbf{V}_T^{\text{AW}} := [\mathbb{P}^k(T)]^2,$$

where $[\mathbb{P}^k(T)]_{\text{sym}}^{2 \times 2}$ denotes the subspace of $[\mathbb{P}^k(T)]^{2 \times 2}$ composed of symmetric-valued tensors. Figure 1 shows the corresponding 24 degrees of freedom of the symmetric stress tensor in the lowest-order case $k = 1$: the values of the three components at each vertex of the triangle, the values of the moments of degree zero and 1 of the normal components each edge, and the value of the moment of degree zero of each component on the triangle. On a patch ω_a , the AW mixed finite element spaces are defined as

$$\begin{aligned} \mathbf{S}_h^{\text{AW}}(\omega_a) &:= \{\tau_h \in \mathbf{H}(\text{div}, \omega_a) \cap [\mathbb{P}^{k+2}(T)]_{\text{sym}}^{2 \times 2} \mid \tau_h|_T \in \mathbf{S}_T^{\text{AW}} \forall T \in \mathcal{T}_a\}, \\ \mathbf{V}_h^{\text{AW}}(\omega_a) &:= \{\mathbf{v}_h \in [L^2(\omega_a)]^2 \mid \mathbf{v}_h|_T \in \mathbf{V}_T^{\text{AW}} \forall T \in \mathcal{T}_a\}. \end{aligned}$$

Let now $k := p - 1$. We need to consider subspaces where a zero normal component is enforced on the stress tensor. Since the boundary condition in the exact problem prescribes the displacement and not the normal stress, we distinguish the case whether a is an interior vertex or a boundary vertex. For $a \in \mathcal{V}_h^{\text{int}}$, we set

$$\Sigma_h^a := \{\tau_h \in \mathbf{S}_h^{\text{AW}}(\omega_a) \mid \tau_h \mathbf{n}_{\omega_a} = \mathbf{0} \text{ on } \partial \omega_a, \tau_h(b) = \mathbf{0} \forall b \in \mathcal{V}_{\omega_a}^{\text{ext}}\}, \quad (12a)$$

$$\mathbf{V}_h^a := \{\mathbf{v}_h \in \mathbf{V}_h^{\text{AW}}(\omega_a) \mid (\mathbf{v}_h, \mathbf{z})_{\omega_a} = 0 \forall \mathbf{z} \in \mathbf{RM}\}, \quad (12b)$$

with $\mathcal{V}_{\omega_a}^{\text{ext}} = \mathcal{V}_h \cap \partial \omega_a$, and for $a \in \mathcal{V}_h^{\text{ext}}$, we set

$$\Sigma_h^a := \{\tau_h \in \mathbf{S}_h^{\text{AW}}(\omega_a) \mid \tau_h \mathbf{n}_{\omega_a} = \mathbf{0} \text{ on } \partial \omega_a \setminus \partial \Omega, \tau_h(b) = \mathbf{0} \forall b \in \mathcal{V}_{\omega_a}^{\text{ext}}\},$$

$$\mathbf{V}_h^a := \mathbf{V}_h^{\text{AW}}(\omega_a),$$

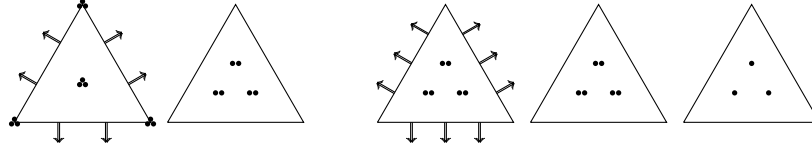


Fig. 1 Element diagrams for $(\mathbf{S}_T^{\text{AW}}, \mathbf{V}_T^{\text{AW}})$ with $k = 1$ (left) and $(\mathbf{S}_T^{\text{AFW}}, \mathbf{V}_T^{\text{AFW}}, \Lambda_T)$ with $k = 2$ (right)

with $\mathcal{V}_{\omega_a}^{\text{ext}} = \mathcal{V}_h \cap (\partial\omega_a \setminus \partial\Omega)$. As argued in [2], the nodal degrees of freedom on $\partial\omega_a$ are set to zero if the vertex separates two edges where the normal stress is zero.

Construction 1 (AW stress reconstruction) Find $\sigma_h^a \in \Sigma_h^a$ and $\mathbf{r}_h^a \in \mathbf{V}_h^a$ such that for all $(\tau_h, \mathbf{v}_h) \in \Sigma_h^a \times \mathbf{V}_h^a$

$$(\sigma_h^a, \tau_h)_{\omega_a} + (\mathbf{r}_h^a, \nabla \cdot \tau_h)_{\omega_a} = (\psi_a \sigma(\mathbf{u}_h), \tau_h)_{\omega_a}, \quad (14a)$$

$$(\nabla \cdot \sigma_h^a, \mathbf{v}_h)_{\omega_a} = (-\psi_a \mathbf{f} + \sigma(\mathbf{u}_h) \nabla \psi_a, \mathbf{v}_h)_{\omega_a}. \quad (14b)$$

Then, extending σ_h^a by zero outside ω_a , set $\sigma_h := \sum_{a \in \mathcal{T}_h} \sigma_h^a$.

For interior vertices, the source term in (14a) has to verify the Neumann compatibility condition

$$(-\psi_a \mathbf{f} + \sigma(\mathbf{u}_h) \nabla \psi_a, \mathbf{z})_{\omega_a} = 0 \quad \forall \mathbf{z} \in \mathbf{RM}. \quad (15)$$

Taking $\psi_a \mathbf{z}$ as a test function in (3), we see that (15) holds.

4.2 Arnold–Falk–Winther Stress Reconstruction

For each element $T \in \mathcal{T}_h$, the local AFW mixed finite element spaces [1] of degree $k \geq 1$ hinge on the Brezzi–Douglas–Marini mixed finite element spaces [5] for each line of the stress tensor and are defined by

$$\mathbf{S}_T^{\text{AFW}} := [\mathbb{P}^k(T)]^{2 \times 2}, \quad \mathbf{V}_T^{\text{AFW}} := [\mathbb{P}^{k-1}(T)]^2, \quad \Lambda_T := \{\mu \in [\mathbb{P}^{k-1}(T)]^{2 \times 2} \mid \mu = \mu^T\},$$

On a patch ω_a the global space $\mathbf{S}_h^{\text{AFW}}(\omega_a)$ is the subspace of $\mathbf{H}(\text{div}, \omega_a)$ composed of functions belonging piecewise to $\mathbf{S}_T^{\text{AFW}}$. The spaces $\mathbf{V}_h^{\text{AFW}}(\omega_a)$ and $\Lambda_h(\omega_a)$ consist of functions lying piecewise in \mathbf{V}_T and Λ_T respectively, with no continuity conditions between two elements.

As for the previous construction, we define subspaces with zero normal components enforced on the stress tensor, and distinguish between interior and boundary vertices. Let $k := p$ and set

$$\Sigma_h^a := \{\tau_h \in \mathbf{S}_h^{\text{AFW}}(\omega_a) \mid \tau_h \mathbf{n}_{\omega_a} = \mathbf{0} \text{ on } \partial\omega_a \quad \text{if } a \in \mathcal{V}_h^{\text{int}}, \quad (17a)$$

$$\tau_h \mathbf{n}_{\omega_a} = \mathbf{0} \text{ on } \partial\omega_a \setminus \partial\Omega \quad \text{if } a \in \mathcal{V}_h^{\text{ext}}\},$$

$$\mathbf{V}_h^a := \{\mathbf{v}_h \in \mathbf{V}_h^{\text{AFW}}(\omega_a) \mid (\mathbf{v}_h, \mathbf{z})_{\omega_a} = 0 \quad \forall \mathbf{z} \in \mathbf{RM} \text{ if } a \in \mathcal{V}_h^{\text{int}}\}, \quad (17b)$$

$$\Lambda_h^a := \Lambda_h(\omega_a). \quad (17c)$$

Construction 2 (AFW stress reconstruction) Find $\sigma_h^a \in \Sigma_h^a$, $\mathbf{r}_h^a \in \mathbf{V}_h^a$ and $\lambda_h^a \in \Lambda_h^a$ such that for all $(\tau_h, \mathbf{v}_h, \mu_h) \in \Sigma_h^a \times \mathbf{V}_h^a \times \Lambda_h^a$,

$$(\sigma_h^a, \tau_h)_{\omega_a} + (\mathbf{r}_h^a, \nabla \cdot \tau_h)_{\omega_a} + (\lambda_h^a, \tau_h)_{\omega_a} = (\psi_a \sigma(\mathbf{u}_h), \tau_h)_{\omega_a}, \quad (18a)$$

$$(\nabla \cdot \sigma_h^a, \mathbf{v}_h)_{\omega_a} = (-\psi_a \mathbf{f} + \sigma(\mathbf{u}_h) \nabla \psi_a, \mathbf{v}_h)_{\omega_a}, \quad (18b)$$

$$(\sigma_h^a, \mu_h)_{\omega_a} = 0. \quad (18c)$$

Then, extending σ_h^a by zero outside ω_a , set $\sigma_h := \sum_{a \in \mathcal{V}_h} \sigma_h^a$.

The condition (15) for all $a \in \mathcal{V}_h^{\text{int}}$ ensures that the constrained minimization problem (18) is well-posed.

4.3 Properties of the Stress Reconstructions

Lemma 1 (Properties of σ_h). Let σ_h be prescribed by Construction 1 or Construction 2. Then $\sigma_h \in \mathbf{H}(\text{div}, \Omega)$, and for all $T \in \mathcal{T}_h$, the following holds:

$$\mathbf{f} + \nabla \cdot \sigma_h = 0. \quad (19)$$

Proof. All the fields σ_h^a are in $\mathbf{H}(\text{div}, \omega_a)$ and satisfy appropriate zero normal conditions so that their zero-extension to Ω is in $\mathbf{H}(\text{div}, \Omega)$. Hence, $\sigma_h \in \mathbf{H}(\text{div}, \Omega)$. Let us prove (19). Since (15) holds for all $a \in \mathcal{V}_h^{\text{int}}$, we infer that (14b) or (18b) is actually true for all $\mathbf{v}_h \in \mathbf{V}_h(\omega_a)$. The same holds if $a \in \mathcal{V}_h^{\text{ext}}$ by definition of \mathbf{V}_h^a . Hence, $(\psi_a \mathbf{f} + \nabla \cdot \sigma_h^a, \mathbf{v}_h)_{\omega_a} = 0$ for all $\mathbf{v}_h \in \mathbf{V}_h(\omega_a)$ and all $a \in \mathcal{V}_h$. Since $\mathbf{V}_h(\omega_a)$ is composed of piecewise polynomials that can be chosen independently in each cell $T \in \mathcal{T}_a$, and using $\sigma_h|_T = \sum_{a \in \mathcal{V}_T} \sigma_h^a|_T$ and the partition of unity $\sum_{a \in \mathcal{V}_T} \psi_a = 1$, we infer that $(\mathbf{f} + \nabla \cdot \sigma_h, \mathbf{v}) = 0$ for all $\mathbf{v} \in \mathbf{V}_T$ and all $T \in \mathcal{T}_h$. The fact that $(\mathbf{f} + \nabla \cdot \sigma_h)|_T \in \mathbf{V}_T$ for any $T \in \mathcal{T}_h$, concludes the proof. \square

5 Numerical Results

We illustrate numerically our theoretical results on a test case with a known analytical solution. We analyze the convergence rates of the error estimates and compare them to those of the analytical error. The computations were performed using the Code_Aster¹ software. The exact solution $\mathbf{u} = (u_x, u_y)$ on the unit square $\Omega = (0, 1)^2$ is given by

$$u_x = \frac{1}{\pi} \sin(\pi x) \cos(\pi y), \quad u_y = -\frac{1}{\pi} \sin(\pi x) \cos(\pi y),$$

¹ <http://web-code-aster.org>

h^{-1}	estimate AFW		estimate AW		$\ \mathbf{u} - \mathbf{u}_h\ _{\text{en}}$	$I_{\text{eff,AFW}}$	$I_{\text{eff,AW}}$	
4	1.707e-2	—	1.707e-2	—	1.704e-2	—	1.00	1.00
8	4.141e-3	2.05	4.124e-3	2.05	4.026e-3	2.08	1.03	1.02
16	1.175e-3	1.82	1.120e-3	1.88	1.116e-3	1.85	1.05	1.00
32	2.835e-4	2.05	2.736e-4	2.03	2.707e-4	2.04	1.05	1.01
64	7.384e-5	1.94	7.244e-5	1.92	7.021e-5	1.95	1.05	1.03

Table 1 Error estimators, analytical error, and effectivity indices under space refinement

with the Lamé parameters $\mu = \lambda = 1$, and the corresponding body force \mathbf{f} . The exact solution is imposed as Dirichlet condition on the whole boundary $\partial\Omega$. The discretization is done on a series of unstructured grids with the polynomial degree $p = 2$ in the conforming finite element method (3).

Table 1 shows the error estimates calculated using the stress reconstruction in the AFW (Const. 2) and in the AW spaces (Const. 1), the analytical error in the energy norm, as well as their convergence rates. The two columns on the right indicate the effectivity indices (overestimation factors) for both reconstruction methods, calculated as the ratio of the estimate to the analytical error.

Since we chose $p = 2$, the convergence rates are close to 2, with the rates of the estimates reproducing very closely the ones of the actual error. Furthermore, the effectivity indices close to 1 indicate the reliability of the estimates.

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