

Parameter identification in chloride ingress from accelerated test using Bayesian network

Thanh-Binh Tran, Emilio Bastidas-Arteaga, Stéphanie Bonnet, Franck Schoefs

▶ To cite this version:

Thanh-Binh Tran, Emilio Bastidas-Arteaga, Stéphanie Bonnet, Franck Schoefs. Parameter identification in chloride ingress from accelerated test using Bayesian network. ICASP12 – 12th International Conference on Applications of Statistics and Probability in Civil Engineering, 2015, Vancouver, Canada. 10.14288/1.0076264. hal-01316234

HAL Id: hal-01316234

https://hal.science/hal-01316234

Submitted on 17 May 2016

HAL is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.

Parameter identification in chloride ingress from accelerated test using Bayesian network

Thanh-Binh Tran

PhD Student, University of Nantes, Nantes, France

Emilio Bastidas-Arteaga

Associate Professor, University of Nantes, Nantes, France

Stéphanie Bonnet

Associate Professor, University of Nantes, Nantes, France

Franck Schoefs

Professor, University of Nantes, Nantes, France

ABSTRACT: Chloride ingress into concrete is one of the main causes leading to the degradation of reinforced concrete (RC) structures. Important damages due to chloride-attack are reported after 10-20 years and thus, structures should be inspected periodically to ensure optimal levels of serviceability and safety. Modelling chloride ingress into concrete, therefore, becomes an important task to plan and quantify maintenance operations of structures. Relevant material and environmental parameters required for modelling could be determined from inspection data obtained after each inspection campaign. However, its assessment requires significant experimental time for collecting data that allow considering the time-dependency of the deterioration process. Data from accelerated test could be used as information about long-term performance of concrete or mortar under real exposure conditions if the scale factor reflecting the ratio between exposures times for natural and accelerated tests is determined. The main objective of this paper is to develop a method based on Bayesian updating to identify the 'real' (equivalent) exposure time from accelerated tests that is used to define the scale factor. The proposed methodology is first tested on simulated data and after implemented to real measurements.

1. INTRODUCTION

Chloride penetration into concrete is one of the main factors responsible for generating corrosion in reinforcing bar which in turn cause: reduction of reinforcement cross-section, loss of bond between steel and concrete, and concrete cracking and delamination (Bastidas-Arteaga and Schoefs 2012; Bastidas-Arteaga et al. 2011; Lounis and Amleh 2003). These consequences lead to the reduction of serviceability and safety levels as well as the shortening of the service life of reinforced concrete (RC) structures. Most RC structures are designed for a lifetime of 50-100 years. However, under chloride attack, important damages are observed after 10-20 years (Kumar

Mehta 2004; Poupard et al. 2006). Consequently, RC structures should be inspected periodically to ensure optimal levels of serviceability and safety during their structural life. Data collected after inspection campaigns is often used to determine parameters for chloride ingress or corrosion propagation models under specific environmental exposures. These parameters are very useful for assessment optimisation lifetime and maintenance strategies (Bastidas-Arteaga and Schoefs 2012). However, under real exposure conditions, even for environments with larger chloride content (splash and tidal zones), the rate of chloride ingress into concrete is particularly slow. Consequently, a significant exposure time is required to collect inspection data useful to

characterise the mid- and long-term durability behaviour of concrete and mortar. Non destructive chloride ingress measurements could be very useful to characterise the performance of cement-based materials subject to real conditions (Torres-Luque et al. 2014). However, many of these techniques are still under development.

This study is based on data obtained from an experimental test in which chloride ingress is accelerated by drying the concrete surface after immersion cycles. Data from these tests could provide information about the mid- and long-term performance of concrete or mortar under real exposure conditions and it could be also used for testing new building or repair materials. The main challenge of this approach is to determine a scale factor reflecting the ratio between exposures times for natural and accelerated tests.

Bayesian network (BN) is an effective tool for parameter identification. Some studies (Bastidas-Arteaga et al. 2012; Richard et al. 2012; Tran et al. 2014) proposed methodologies based on BN to identify model/material parameters from real experiment data that shown a good agreement between numerical predictions and experimental measurements. In this study, BN is also used as a tool for parameter identification based on a chloride ingress model. Based on the approach proposed by Tran et al., (2014), data from natural test will be used to identify information about two model parameters: chloride surface concentration (C_s) and the chloride diffusion coefficient (D). This information will be after used as priori information in other BN to identify the equivalent exposure time from data obtained in the accelerated test. A scale factor representing the ratio between experimental time and equivalent time in accelerated test is then proposed.

2. PARAMETER IDENTIFICATION IN CHLORIDE INGRESS USING BN

2.1. Parameter identification using BN

Using BN for identifying parameters in chloride ingress has been investigated in some previous studies (Bastidas-Arteaga et al. 2012; Deby et al. 2008, 2012; Tran et al. 2014). In general, each parameter in the model is represented as a node in the BN. There is an arrow linking from a "parent node" to a "child node" to define the dependency between the two nodes. The Conditional Probability Table (CPT) in BN describes the relationship between each pair of nodes. The CPT for each node could be built from numerical simulations or by real statistic data. The BN allows updating the statistical distributions of parameters in the network with new information from experimental results. In BN modelling of chloride ingress, parameters to identify are modelled as parent nodes and chloride content at different depth points and inspection times are modelled as child nodes. Inspection data is introduced in BN as evidences.

Tran et al. (2014) optimised the use of inspection data to improve the BN configuration for parameter identification in chloride ingress models. They pointed out that there is a specific configuration of BN for each parameter that minimise the identification errors. For the surface chloride concentration (C_s) , a BN with only one child node representing the chloride content at the surface at early inspection time provides a good identification. For the chloride diffusion coefficient (D), to minimize errors, the optimised BN configuration should consider child nodes representing the total inspection with optimal spatial discretisation intervals. Tran et al. (2014) also suggested that different boundaries should be used to define the ranges for child nodes to take advantage of information from deeper points where chloride content is low. These optimised configurations and numerical aspects are applied in this study to minimise the errors in parameter identification.

2.2. Modelling chloride ingress

In this study, chloride ingress is modelled as (Luping and Gulikers 2007):

$$C(x,t) = C_s \operatorname{erfc} \left[\frac{x}{2\sqrt{\frac{D_0}{1-n} \left[\left(1 + \frac{t'_{ex}}{t}\right)^{1-n} - \left(\frac{t'_{ex}}{t}\right)^{1-n} \right] \left(\frac{t'_0}{t}\right)^n t}} \right]$$
(1)

where C(x,t) is the concentration of chloride ions at depth x and time t, C_s is the chloride concentration at the exposure surface, D_θ and t_θ is a pair of known diffusion coefficient and age, t_{ex} is the age of concrete at the start of exposure, t_{ex} is the age factor, and erfc(.) is the complementary error function.

This equation is valid only under these assumptions: 1) concrete is a homogeneous material; 2) chloride binding is time-independent and linearly proportional to the free chloride concentration; 4) the effect of co-existing ions is constant; 5) the diffusion is one dimensional into semi-infinite space. However, this model is considered in this study because it accounts, in a simplified way, for the time-dependency of the chloride ingress process.

3. PARAMETER IDENTIFICATION WITH DATA FROM ACCELERATED TEST

3.1. Description of the experimental setup

The experimental setup of the accelerated tests was carried out within the framework of the project. aimed French MAREO It characterising the durability performance of new materials to repair RC components located in tidal zones by performing both natural and accelerated tests. For natural and accelerated tests, concrete slabs are placed in a tank with salted water. The exposure of the slabs ensures chloride ingress in one dimension. Tidal cycles (high and low) are simulated varying the level of water. For the accelerated tests, ventilators are used to dry the samples during the low tide cycle. The drying accelerates chloride ingress due to an increase of the capillarity sorption of concrete (Hong 1998). Natural tests are subjected to the same tidal cycles without drying. The test is automatically controlled via a Labview® program. The thanks are placed inside a building and its environmental conditions (room temperature, water salinity and relative humidity) are recorded.

For natural tests, the exposure time in laboratory, t_{exp} , could represent the exposure time in real conditions. However, for accelerated tests the chloride contents are larger, and consequently, the exposure time in lab t_{exp} must correspond to an equivalent time, t_{eq} , under natural exposure conditions ($t_{eq} > t_{exp}$).

3.2. Bayesian identification of t_{eq}

Chloride profiles obtained from natural tests could provide information about chloride content at depth x at real exposure times t_{exp} . However, those of accelerated test just can provide information about chloride content at depth x. Consequently, different BN configurations are used for natural and accelerated tests. Tran et al. (2014) modelled chloride ingress into concrete in natural tests by the BN shown in Figure 1 where C_s and D_0 are two parent nodes and there are nchild nodes $C(x_i,t_i)$ representing the discrete chloride concentration measurement in time and space i.e. at depth x_i and inspection time t_i . In accelerated tests, chloride ingress is modelled by the BN described in Figure 2 that has three parent nodes C_s , D_0 and t_{eq} . Child nodes in Figure 2 represent the chloride content at depth x_i .

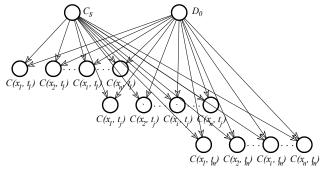


Figure 1: BN configuration for natural test

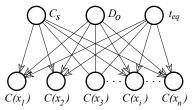


Figure 2: BN configuration for accelerated test

If there is no priori information about the parameters C_s , D, and t_{eq} , we can assume that they follow uniform distributions in given ranges (Bastidas-Arteaga et al. 2012; Cao and Wang 2014; Robinson and Hartemink 2010). However, data from natural tests could be used herein to provide some priori information that reduces the level of uncertainty in the BN by improving the identification process.

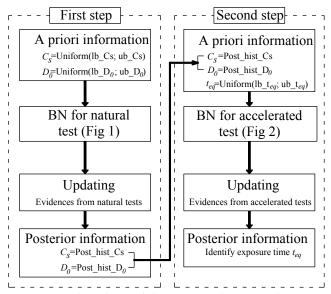


Figure 3: Flowchart of the proposed approach

The proposed approach for identifying t_{eq} is described in Figure 3. The first step of the methodology consists of using the BN and data from natural tests (Figure 1) to identify the parameters C_s and D_0 . C_s and D_0 are respectively related to chloride exposure and concrete diffusivity. Natural and accelerated tests are performed on the same material subject to the same environmental chloride concentration. Then, we assume that these parameters should remain unchanging for natural and accelerated

test. The first step of the methodology aims then at characterising these two parameters under natural exposure conditions. Posterior histograms of the parameters C_s and D_θ are then used in the second step as a priori information for the BN (Figure 2). The equivalent exposure time for accelerated test could be then identified by updating the BN with evidences obtained from accelerated test results.

4. APPLICATION TO NUMERICAL DATA

In practice it is difficult to obtain enough experimental data to validate the proposed procedure and to improve the configuration of the BN that minimises identification errors. Consequently, this section considers numerical data to improve the configurations of BN corresponding to different inspection schemes. Each configuration is evaluated by the error of the identified parameter $Z_{identified}$ with respect to a theoretical value Z_{theory} as:

$$Error(Z) = \frac{\left| Z_{identified} - Z_{theory} \right|}{Z_{theory}} 100\%$$
 (2)

where Z represents the mean or the standard deviation of the parameter to identify and the theoretical values Z_{theory} are used to generate chloride profiles from eq. (1).

4.1. BN modelling for natural tests

Continuous variables are replaced by discrete variables to avoid using approximate BN algorithms. The priori information and the discretisation of the parameters are described in Table 1. We assume that $t_{ex} = t_0 = 30$ days and n = 0.5.

Table 1: A priori information and discretisation of parameters in BN modelling for natural test

Parameter	A priori information	Intervals per node
$C_s[kg/m^3]$	Uniform (10 ⁻³ – 12)	60
$D_0 [10^{-12} \text{m/s}^2]$	<i>Uniform</i> $(0.4 - 3.5)$	60
C(x,t) [kg/m ³]	-	30

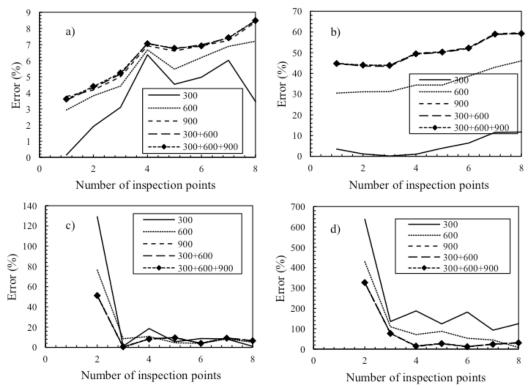


Figure 4: Identification errors: (a) Mean of C_s , (b) Standard deviation of C_s , (c) Mean of D_0 (d) Standard deviation of D_0

Numerical evidences are generated using Monte Carlo simulations with the parameters given in Table 2. Data from different inspection schemes are combined to update the BN (Table 3). These schemes consider various inspection times with the same number of chloride profiles per scheme. The maximum number of days (900 days) corresponds to the time of exposure of natural tests.

With inspection time smaller than 900 days, chloride content at depth x > 20mm is almost null. Consequently, the total inspection depth is fixed at 22mm. Various configurations of BN corresponding to different number of inspection points dividing the inspection depth into equal interval lengths are analysed to evaluate the identification errors. The interval lengths should not be smaller than 3mm due to the accuracy of the semi-destructive tests for determining chloride profiles.

Table 2: Theoretical values of parameters to identify

Parameter	Distribution	Mean	COV
C_s	Lognormal	$2.95 (kg/m^3)$	20%
D_{θ}	Lognormal	$10^{-12} (m/s^2)$	15%

Table 3: Inspection schemes

Inspection scheme	Number of chloride profiles	
300 days	9000	
600 days	9000	
900 days	9000	
300+600 days	4500 + 4500	
300+600+900 days	3000+3000+3000	

The identification errors for C_s presented in Figure 4a and Figure 4b show that evidences obtained near the surface $(x\approx 0)$ with early inspection time ($t_{ins} = 300$ days) could minimize the errors in the estimation of mean and standard deviation of C_s (Tran et al. 2014). However, for D_0 , combining evidences from different inspection times could improve identification. This trend can be observed clearly in Figure 4d where the errors of standard deviation in cases combining data from inspection times (300+600) days or (300+600+900) days are reduced significantly. This is because the deterioration model herein takes into consideration the time-dependency of the chloride diffusion coefficient. This finding could very useful when inspection data is limited because by combining these data, we can take advantage of the information. Figure 4c and Figure 4d also reveal that using more inspection points could reduce the gap between identified and theoretical values for D_0 .

4.2. BN modelling of accelerated test

Both natural and accelerated tests are carried out in the same concrete. Consequently, chloride concentration at surface (C_s) and chloride diffusion coefficient (D_0) , which are determined from natural tests, could be applied for accelerated test (Figure 3). To avoid any assumption about distribution shapes, posterior histograms of C_s and D_θ obtained after updating the BN of natural tests are then used directly as priori distribution in BN modelling accelerated tests. To minimize the identification errors in natural tests, the histogram of C_s and Dwere obtained from improved BN configurations as previously discussed in section 4.1. According to Figure 2, the equivalent exposure time (t_{eq}) is the variable to identify. It is then modelled as a parent node with the priori distribution shown in Table 4.

Table 4: A priori information and discretisation of parameter in BN modelling for natural test

Parameter	Priori information	Intervals
$C_s[kg/m^3]$	Histogram from natural test (10 ⁻³ - 12)	60
$D_0 [m/s^2]$	Histogram from natural test $(4.10^{-13} - 3.5.10^{-12})$	60
t _{eq} [days]	Uniform (0-3600)	100
$C(x)$ [kg/m^3]	-	30

Numerical evidences in the BN for accelerated tests are generated from the theoretical values presented in Table 2 at different inspection times varying from 300 to

3000 days. 9000 chloride profiles are generated for each inspection time. These inspection times correspond to the theoretical t_{eq} to be identified. The total inspection depth is 30mm, being sufficient to describe the penetration of chloride at 3000 days of exposure.

The errors on the identification of the equivalent exposure time from accelerated tests are shown in Figure 5. With a large number of inspection points, the errors for all cases are smaller than 20%. Especially, errors at 300 days, 600 days, and 900 days are smaller than other cases. It is because priori information of C_s and D_0 was obtained from natural tests at these exposure times and then it reduces the identification errors. When dealing with real data, the identified value of equivalent exposure time could be used to calculate a scale factor.

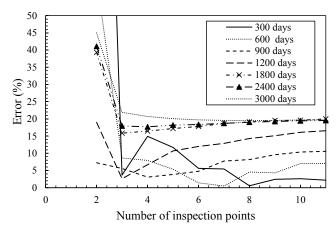


Figure 5: Identification error for the mean t_{eq}

5. APPLICATION TO REAL DATA

5.1. Experimental data

Destructive tests were carried out to determine chloride profiles from both natural and accelerated tests at different exposure times (Table 5 and Table 6). Each chloride profile has 42mm in depth and with a discretisation depth of 3mm. This gives a total of 14 inspection points.

Table 5: Number of chloride profiles for natural tests

I_{j}			
Natural tests	TI	<i>T2</i>	<i>T3</i>
t_{exp} (days)	65	207	320
Number of profiles	3	3	3

Table 6: Number of chloride profiles for accelerated tests

Accelerated tests	TI	<i>T2</i>	<i>T3</i>
t_{exp} (days)	65	212	436
Number of profiles	6	6	6

5.2. Identification of the scale factor k

A scale factor in accelerated tests can be defined as a ratio between the exposure time in the experiment (t_{exp}) and the equivalent exposure time (t_{eq}) :

$$k = \frac{t_{eq}}{t_{exp}} \tag{3}$$

experimental exposure times accelerated tests (t_{exp}) are given in Table 6. The equivalent exposure times (t_{eq}) are determined by the procedure given in Figure 3. Improved BN configurations defined according to the findings of section 4.1 are used to take advantage of limited data and minimise the identification errors in natural tests. Therefore, data at the surface $(x\approx 0)$ from chloride profiles obtained at inspection time T1 are introduced in BN updating to obtain posteriori histogram of C_s . Data combining three inspection times (T1 + T2 + T3) with 14 inspection points in depth are used to obtain posterior histogram of D_0 . These posterior histograms are then used as priori information in BN modeling of accelerated test. Equivalent exposure times are determined by updating the BN with evidences from accelerated tests and scale factors are then computed from Eq. (3).

The results for equivalent exposure times and scale factors are given in Table 7. Identified values of t_{eq} show an expected trend that a longer exposure time (t_{exp}) in the laboratory will be equivalent to a larger value of t_{eq} . By plotting the scale factors with the experimental times (Figure 6), it is noted that the scale factor is not constant. At early inspection time, this factor is high and then it decrease with time. This trend is expected due to the fact that the chloride diffusion coefficient is time-dependent and follow a similar trend (Tang and Nilsson 1996). The

values of scale factors could be more accurate if more experimental data is available.

Table 7: Real exposure and equivalent times

Accelerated test	T1	T2	<i>T3</i>
t_{exp} [days]	65	212	436
t _{eq} [days]	725.2	935.9	1050.3

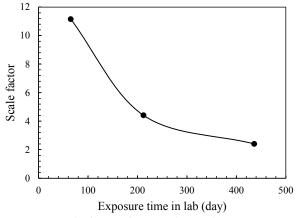


Figure 6: Scale factor for accelerated tests

6. CONCLUSIONS

Chlorides ions are critical agents leading to reinforcement corrosion of RC structures. The characterisation of the performance of cementbased materials under real conditions requires significant experimental times. Consequently, this study proposed a methodology characterising materials these bv using accelerated tests and BN updating. The proposed approach allows determining a scale factor for the accelerated test. Therefore, information about long-term performance of concrete or mortar under real exposure condition could be obtained from accelerated experiments in lab. This information is useful to characterise the performance of new repair techniques or cementbased repair materials.

This paper presented our first findings on this area. Future works will focus on:

- obtaining more experimental data to improve the identification,
- identification of the age factor in the chloride ingress model eq. (1), and
- consideration of measurement and model errors.

7. REFERENCES

- Bastidas-Arteaga, E., Chateauneuf, A., Sánchez-Silva, M., Bressolette, P., and Schoefs, F. (2011). "A comprehensive probabilistic model of chloride ingress in unsaturated concrete." *Engineering Structures*, 33(3), 720–730.
- Bastidas-Arteaga, E., and Schoefs, F. (2012). "Stochastic improvement of inspection and maintenance of corroding reinforced concrete structures placed in unsaturated environments." *Engineering Structures*, 41, 50–62.
- Bastidas-Arteaga, E., Schoefs, F., and Bonnet, S. (2012). "Bayesian identification of uncertainties in chloride ingress modeling into reinforced concrete structures." *Third International Symposium on Life-Cycle Civil Engineering*, Vienna, Austria.
- Cao, Z., and Wang, Y. (2014). "Bayesian model comparison and selection of spatial correlation functions for soil parameters." *Structural Safety*, Elsevier Ltd.
- Deby, F., Carcasses, M., and Sellier, A. (2008). "Toward a probabilistic design of reinforced concrete durability: application to a marine environment." *Materials and Structures*, 42(10), 1379–1391.
- Deby, F., Carcasses, M., and Sellier, A. (2012). "Simplified models for the engineering of concrete formulations in a marine environment through a probabilistic method." *European Journal of Environmental and Civil Engineering*, 16(3-4), 37–41.
- Hong, K. (1998). "Cyclic wetting and drying and its effects on chloride ingress in concrete." MSc Thesis, University of Toronto.
- Kumar Mehta, P. (2004). "High-performance, high-volume fly ash concrete for sustainable development." *International Workshop on Sustainable Development and Concrete Technology*, Beijing, 3–14.
- Lounis, D. Z., and Amleh, L. (2003). "Reliability-Based Prediction of Chloride Ingress and Reinforcement Corrosion of Aging Concrete Bridge Decks." Proceeding of the 3rd International IABMAS Workshop on Life-Cycle Cost Analysis and Design of Civil Infrastructure Systems, Lausanne, Switzerland, 139–147.
- Luping, T., and Gulikers, J. (2007). "On the mathematics of time-dependent apparent

- chloride diffusion coefficient in concrete." *Cement and Concrete Research*, 37, 589–595.
- Poupard, O., L'Hostis, V., Catinaud, S., and Petre-Lazar, I. (2006). "Corrosion damage diagnosis of a reinforced concrete beam after 40 years natural exposure in marine environment." *Cement and Concrete Research*, 36, 504–520.
- Richard, B., Adelaide, L., and Cremona, C. (2012). "A Bayesian approach to estimate material properties from global statistical data." *European Journal of Environmental and Civil Engineering*, (March 2013), 37–41.
- Robinson, J. W., and Hartemink, A. J. (2010). "Learning Non-Stationary Dynamic Bayesian Networks." *The Journal of Machine Learning Research*, 11, 3647–3680.
- Tang, L., and Nilsson, L. (1996). "A numerical method for prediction of chloride penetration into concrete structures." *NATO/RILEM Workshop on the modelling of Microstructure and its Potential for Studying Transport Properties and Durability*, Kluwer, Dordrecht, The Netherlands, 539–552.
- Torres-Luque, M., Bastidas-Arteaga, E., Schoefs, F., Sánchez-Silva, M., and Osma, J. F. (2014). "Non-destructive methods for measuring chloride ingress into concrete: State-of-the-art and future challenges." *Construction and Building Materials*, 68, 68–81.
- Tran, T. B., Bastidas-Arteaga, E., Schoefs, F., and Bonnet, S. (2014). "Bayesian updating for optimization of inspection schedules of chloride ingress into concrete." *Proceeding of the 2nd International Symposium on Uncertainty Quantification and Stochastic Modeling*, Rouen, France.