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Response of periodic structures due to moving loads

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Abstract

A method is proposed to calculate the response of periodic structures subjected to moving loads. It is based on the Floquet decomposition which allows the restriction of the analysis for the overall system to a generic cell. The main contribution of the approach presented hereafter is that the response is directly deduced from transfer functions in the space-wavenumber domain calculated in an unbounded generic cell. Moreover, the equivalence of this new solution with the response of invariant structures obtained using Fourier transforms is established.

Résumé

Réponse de structures périodiques à des charges mobiles. Une méthode est proposée afin de calculer la réponse de structures périodiques soumises à des charges mobiles. Celle-ci est basée sur la décomposition de Floquet qui permet de restreindre l'analyse du système entier à une cellule de référence. La principale contribution de l'approche présentée ci-après est que la réponse est directement déduite à partir de la fonction de transfert dans le domaine espace-nombre d'onde calculée dans une cellule de référence non bornée. De plus, l'équivalence entre la solution obtenue et la réponse de structures invariante calculée avec la transformée de Fourier est établie.

Keywords: Solids and structures; Periodic structure; Floquet transform; Moving load

Mots-clés : Solides et structures ; Structure périodique ; Transformée de Floquet ; Charge mobile

1. Introduction

Vibrations induced by railway traffic are of a major concern [1] since they have an important impact on the human comfort and on the built environment. The study of these vibrations requires three-dimensional (3D) models of unbounded domains, that is the soil-railway track.

In this Note, these domains are supposed to be periodic along one given direction. Then, the analysis for the overall system is substituted by one for a generic cell using the Floquet decomposition [2]. This paradigm has been already

applied, but only to 1D or 2D periodically supported structures [3,4] for which the generic cell is then bounded. In order to take into account a structure interacting with a soil half-space, a new method [5] has been proposed which allows for 3D unbounded generic cells. This approach has been initially developed for the study of the dynamic responses of structures subjected to 3D seismic loading. This methodology is here extended to the case of moving loads and a new formulation is presented in which the response is directly deduced from transfer functions calculated in the unbounded generic cell.

In a first part, the Floquet transform is defined. Its relation with the Fourier transform is introduced: this formula is used in the last paragraph to get the relationship between the responses due to moving loads of periodic and invariant structures. In Section 3, the generic problem is built. It corresponds to the dynamic problem posed in the reference cell. Section 4 is devoted to the expression of the response of periodic structures due to moving loads. The proposed method is then applied for a real railway structure. Finally, the connection with the case of invariant structures is presented in Section 5.

2. Floquet transforms

In this section, we recall the important results related to Floquet transforms. For a deeper review, one can refer to [2]. Moreover, relations between Floquet and Fourier transforms are introduced.

2.1. Definitions

Let Ω be an unbounded open set. The position vector of any point of this domain is given by:

$$\mathbf{x} = x_1 \mathbf{e}_1 + x_2 \mathbf{e}_2 + x_3 \mathbf{e}_3 \quad (1)$$

with $(\mathbf{e}_1, \mathbf{e}_2, \mathbf{e}_3)$ a Cartesian reference system. The domain Ω is assumed to be periodic in the \mathbf{e}_2 direction with a period L , i.e. it is invariant through any translation of vector $nL\mathbf{e}_2$, where n is a signed integer. The Floquet transform of any function f defined in Ω is a function \tilde{f} defined in $\tilde{\Omega} \times [-\pi/L, \pi/L]$, with $\tilde{\Omega} = \{\mathbf{x} \in \Omega \mid 0 < \mathbf{x} \cdot \mathbf{e}_2 < L\}$, and such that:

$$\tilde{f}(\tilde{\mathbf{x}}, \kappa) = \sum_{n=-\infty}^{+\infty} f(\tilde{\mathbf{x}} + nL\mathbf{e}_2) e^{in\kappa L} \quad (2)$$

in which the wavenumber $\kappa \in [-\pi/L, \pi/L]$ and where $\tilde{\mathbf{x}}$ is the position vector in the reference cell $\tilde{\Omega}$ defined by $\tilde{\mathbf{x}} = \tilde{x}_1 \mathbf{e}_1 + \tilde{x}_2 \mathbf{e}_2 + \tilde{x}_3 \mathbf{e}_3$ with $\tilde{x}_1 = x_1$, $\tilde{x}_2 = x_2 - nL$, $\tilde{x}_3 = x_3$. It should be noted that the Floquet transform \tilde{f} is periodic of the second kind, that is:

$$\tilde{f}(\tilde{\mathbf{x}} + L\mathbf{e}_2, \kappa) = e^{-i\kappa L} \tilde{f}(\tilde{\mathbf{x}}, \kappa) \quad (3)$$

Finally, for any $\mathbf{x} = \tilde{\mathbf{x}} + nL\mathbf{e}_2$ (\mathbf{x} in Ω and $\tilde{\mathbf{x}}$ in $\tilde{\Omega}$), f may be recovered from its Floquet transform by:

$$f(\mathbf{x}) = \frac{L}{2\pi} \int_0^{2\pi/L} \tilde{f}(\tilde{\mathbf{x}}, \kappa) e^{-in\kappa L} d\kappa \quad (4)$$

2.2. Relations between Floquet and Fourier transforms

Let $F(x)$ be a function (defined on \mathbb{R} for the sake of simplicity). It is recalled that its Fourier transform \hat{F} can be defined by:

$$\hat{F}(k) = \int_{-\infty}^{+\infty} e^{ikx} F(x) dx, \quad \forall k \in \mathbb{R} \quad (5)$$

The function F can be recovered from its Fourier transform as follows:

$$F(x) = \frac{1}{2\pi} \int_{-\infty}^{+\infty} e^{-ikx} \hat{F}(k) dk \quad (6)$$

Using these conventional formulae, the Fourier transform of the function f_{x_1, x_3} defined by:

$$f_{x_1, x_3}(x_2) = f(\mathbf{x}) \quad (7)$$

can be calculated from the Floquet transform of f as follows [5]:

$$\hat{f}_{x_1, x_3}(k_{x_2}) = \int_0^L e^{ik_{x_2}\tilde{x}_2} \tilde{f}(x_1\mathbf{e}_1 + \tilde{x}_2\mathbf{e}_2 + x_3\mathbf{e}_3, \kappa) d\tilde{x}_2 \quad (8)$$

where κ is in $[-\pi/L, \pi/L]$ and $k_{x_2} = \kappa + 2n\pi/L$. Moreover, the Floquet transform of f is recovered from \hat{f}_{x_1, x_3} by:

$$\tilde{f}(\tilde{\mathbf{x}}, \kappa) = \frac{1}{2\pi} \sum_{n=-\infty}^{+\infty} e^{-i(\kappa + \frac{2n\pi}{L})\tilde{x}_2} \hat{f}_{x_1, x_3}\left(\kappa + \frac{2n\pi}{L}\right) \quad (9)$$

3. Generic problem associated to the periodic domain

This section deals with the dynamic response of the soil-track system subjected to any external forces. This domain is assumed to have an elastic behaviour. Using Floquet transform defined previously, the dynamic problem for the overall system can be substituted by one posed in a reference cell. The generic problem is then introduced.

Let $\mathbf{u}(\mathbf{x}, t)$ be the elastodynamic displacement field in the unbounded domain Ω . Its Fourier transform, $\hat{\mathbf{u}}(\mathbf{x}, \omega)$, satisfies:

$$\text{div } \sigma(\hat{\mathbf{u}}) = -\rho\omega^2\hat{\mathbf{u}}, \quad \mathbf{x} \in \Omega \quad (10)$$

$$\mathbf{t}(\hat{\mathbf{u}}) = \mathbf{f}_0, \quad \mathbf{x} \in \Gamma \quad (11)$$

where ρ is the mass density, $\sigma(\hat{\mathbf{u}})$ is the elastic stress tensor associated to the displacement field $\hat{\mathbf{u}}$ and $\mathbf{t}(\hat{\mathbf{u}}) = \sigma(\hat{\mathbf{u}})\mathbf{n}$ corresponds to the traction vector on the considered boundary using the outer normal convention for \mathbf{n} . Finally, Γ is a part of the boundary of the domain Ω on which the external forces \mathbf{f}_0 are applied.

Let $\check{\mathbf{u}}(\tilde{\mathbf{x}}, \kappa, \omega)$ be a displacement field that satisfies:

$$\text{div } \sigma(\check{\mathbf{u}}) = -\rho\omega^2\check{\mathbf{u}}, \quad \tilde{\mathbf{x}} \in \tilde{\Omega} \quad (12)$$

$$\mathbf{t}(\check{\mathbf{u}}) = \tilde{\mathbf{f}}_0, \quad \tilde{\mathbf{x}} \in \tilde{\Gamma} \quad (13)$$

$$\check{\mathbf{u}}(\tilde{\mathbf{x}}) = e^{-i\kappa L}\check{\mathbf{u}}(\tilde{\mathbf{x}} - L\mathbf{e}_2), \quad \tilde{\mathbf{x}} \in \tilde{\Gamma}_L \quad (14)$$

where $\tilde{\Gamma}$ is the restriction of Γ on the generic cell $\tilde{\Omega}$ and $\tilde{\Gamma}_L = \{\mathbf{x} \in \Omega \mid \mathbf{x} \cdot \mathbf{e}_2 = L\}$. Therefore, it is proved [2] that:

$$\check{\mathbf{u}} = \hat{\mathbf{u}} \quad (15)$$

where $\hat{\mathbf{u}}$ is the Floquet transform of $\hat{\mathbf{u}}$. Hence, instead of solving the problem for the whole domain (Eqs. (10) and (11)), one can solve the generic problem defined by Eqs. (12)–(14). The response in the overall domain is recovered from the response in the generic cell by using the inverse Floquet transform defined by Eq. (4). It should be noted here that a method [5] has been proposed to solve the generic problem in the case of an unbounded cell.

4. Response of periodic structures due to moving loads

In this section, the dynamic behaviour of the railway track structure is focused on. The external forces applying on it are supposed to be moving loads (which can model a train for instance).

Consider a general pinpoint load moving along the \mathbf{e}_2 axis with a constant velocity V :

$$\begin{aligned} \mathbf{G}(\mathbf{y}, t) &= g(y_2 - X_2)\delta(y_1 - X_1)\delta(y_2 - X_2 - Vt)\delta(y_3 - X_3)\mathbf{e}_3 \\ &= G(y_2, t)\delta(y_1 - X_1)\delta(y_3 - X_3)\mathbf{e}_3 \end{aligned} \quad (16)$$

with $\mathbf{y} = y_1\mathbf{e}_1 + y_2\mathbf{e}_2 + y_3\mathbf{e}_3$ and where $(X_1, X_2, X_3) = \mathbf{X}$ is the position of the moving force at the time $t = 0$; $G(y_2, t) = g(y_2 - X_2)\delta(y_2 - X_2 - Vt)$ in which $g(y_2 - X_2)$ is the variation of the moving force amplitude.

Without any lose of generality, we can assume that the point \mathbf{x} , where the response is calculated, is in the track generic cell. So $\mathbf{x} = \tilde{\mathbf{x}}$. Let $\hat{\mathbf{u}}(\mathbf{x}, \mathbf{X}, \omega)$ be the displacement in \mathbf{x} due to a moving load acting on \mathbf{X} at $t = 0$. This field is given by:

$$\hat{\mathbf{u}}(\mathbf{x}, \mathbf{X}, \omega) = \int_{-\infty}^{+\infty} \hat{G}(y_2, \omega) \hat{\mathbf{h}}(\mathbf{x}, \mathbf{y} = X_1 \mathbf{e}_1 + y_2 \mathbf{e}_2 + X_3 \mathbf{e}_3, \omega) dy_2 \quad (17)$$

where $\hat{\mathbf{h}}(\mathbf{x}, \mathbf{y}, \omega)$ is the displacement in the frequency domain of a point \mathbf{x} due to an impulse force at a point \mathbf{y} . Moreover, \hat{G} is the Fourier transform of G and is given by:

$$\hat{G}(y_2, \omega) = e^{i\frac{\omega}{V}(y_2 - X_2)} g(y_2 - X_2) \quad (18)$$

For the sake of clarity, arguments \mathbf{x} , \mathbf{X} and ω will be often omitted in the following. Then, replacing Eq. (18) in Eq. (17) and expressing $g(y_2 - X_2)$ with its Fourier transform, one gets:

$$\hat{\mathbf{u}} = \frac{1}{2\pi} \int_{-\infty}^{+\infty} \int_{-\infty}^{+\infty} e^{-ik^*(y_2 - X_2)} \hat{g}(k_{y_2}) \hat{\mathbf{h}}(y_2) dy_2 dk_{y_2} \quad (19)$$

with $k^* = (k_{y_2} - \omega/V)$. Eq. (19) is also equivalent to:

$$\hat{\mathbf{u}} = \frac{1}{2\pi} \int_{-\infty}^{+\infty} \hat{g}(k_{y_2}) \sum_{n=-\infty}^{+\infty} \int_{nL}^{(n+1)L} e^{-ik^*(y_2 - X_2)} \hat{\mathbf{h}}(y_2) dy_2 dk_{y_2} \quad (20)$$

Taking $\tilde{y}_2 = y_2 - nL$ leads to:

$$\hat{\mathbf{u}} = \frac{1}{2\pi} \int_{-\infty}^{+\infty} \hat{g}(k_{y_2}) \int_0^L \sum_{n=-\infty}^{+\infty} e^{-ik^*(\tilde{y}_2 + nL - X_2)} \hat{\mathbf{h}}(\tilde{y}_2 + nL) d\tilde{y}_2 dk_{y_2} \quad (21)$$

Due to the geometric periodicity, we have:

$$\hat{\mathbf{h}}(\mathbf{x}, \tilde{y}_2 + nL) = \hat{\mathbf{h}}(\mathbf{x} - nL\mathbf{e}_2, \tilde{y}_2) \quad (22)$$

Moreover, the Floquet transform of the function $\hat{\mathbf{h}}(\mathbf{x}, \tilde{y}_2)$, with respect to the variable \mathbf{x} , is the function $\tilde{\hat{\mathbf{h}}}_{\mathbf{x}}(\mathbf{x}, \tilde{y}_2, \kappa)$ defined by:

$$\tilde{\hat{\mathbf{h}}}_{\mathbf{x}}(\mathbf{x}, \tilde{y}_2, \kappa) = \sum_{n=-\infty}^{+\infty} \hat{\mathbf{h}}(\mathbf{x} + nL\mathbf{e}_2, \tilde{y}_2) e^{in\kappa L} \quad (23)$$

From Eqs. (21)–(23) one gets:

$$\hat{\mathbf{u}}(\mathbf{x}) = \frac{1}{2\pi} \int_{-\infty}^{+\infty} \hat{g}(k_{y_2}) e^{ik^* X_2} \int_0^L e^{-ik^* \tilde{y}_2} \tilde{\hat{\mathbf{h}}}_{\mathbf{x}}(\mathbf{x}, \tilde{y}_2, \kappa_0) d\tilde{y}_2 dk_{y_2} \quad (24)$$

where $\kappa_0 = k^* - \frac{2m\pi}{L}$, m being the signed integer such as $\kappa_0 \in]-\pi/L, \pi/L[$. It is worth to notice that $\tilde{\hat{\mathbf{h}}}_{\mathbf{x}}$ is the solution of the problem (12)–(14) with $\tilde{\mathbf{f}}_0(\tilde{\mathbf{x}}) = \delta(\tilde{x}_1 - X_1) \delta(\tilde{x}_2 - \tilde{y}_2) \delta(\tilde{x}_3 - X_3)$.

Eq. (24) has two advantages. At first, only the response of the generic cell is necessary instead of the response of the whole domain as needed in Eq. (17). Secondly, the displacement due to a moving load in the frequency domain is directly derived from the transfer function $\tilde{\hat{\mathbf{h}}}_{\mathbf{x}}$ in the $\omega - \kappa$ domain. Then, no inverse Floquet transform (see Eq. (4)) is needed.

In the case of a constant moving load, $G(y_2, t)$ defined by Eq. (16) is given by:

$$G(y_2, t) = G_0 \delta(y_2 - X_2 - Vt) \quad (25)$$

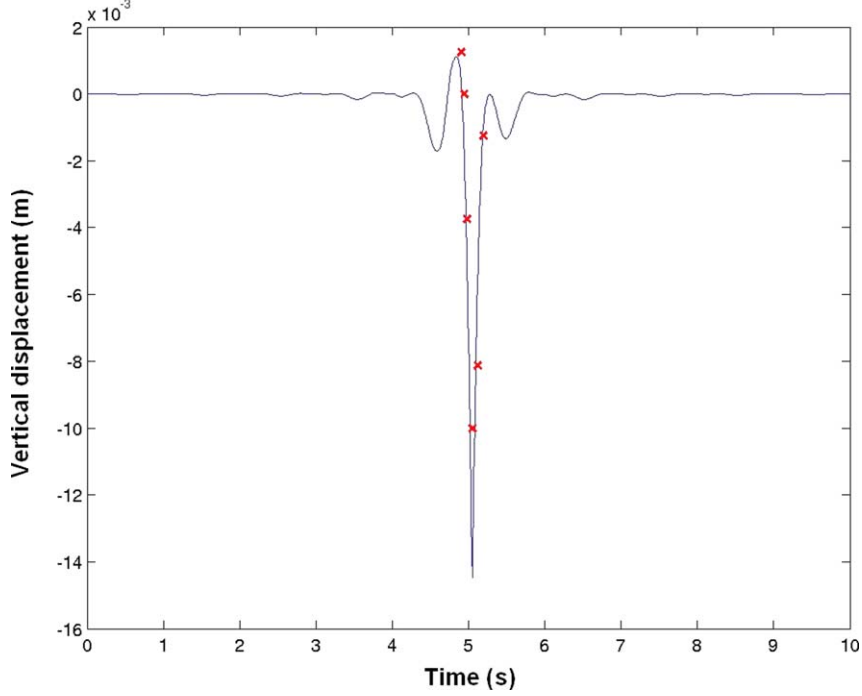


Fig. 1. Vertical displacement response with respect to time of a point in the railway track, due to the passage of one axle for a train travelling at 200 km/h. Comparison between measurement (crosses) and simulation (continuous line).

with G_0 a real constant. Eq. (24) is then equivalent to:

$$\hat{\mathbf{u}} = \frac{G_0}{2\pi} e^{-i\frac{\omega}{V}X_2} \int_0^L e^{i\frac{\omega}{V}\tilde{y}_2} \tilde{\mathbf{h}}_{\mathbf{x}}(\tilde{y}_2, \kappa_0) d\tilde{y}_2 \quad (26)$$

where $\kappa_0 = -\frac{\omega}{V} - \frac{2m\pi}{L}$. In this case, Fig. 1 compares simulations (continuous line) with measurements from [6] (cross point) for the vertical displacement of one point in the railway track, due to the passage of one axle for a train travelling at 200 km/h (cf. [7] for more details). A good agreement between the calculated and the measured displacement is achieved and the dynamic effects described in [6] have been correctly modelled.

5. Correspondence with the case of invariant structures

At first, it should be noted that using Eqs. (22) and (23), we can deduce the Floquet transform of $\hat{\mathbf{h}}$ with respect to the variable \mathbf{y} from its Floquet transform with respect to the variable \mathbf{x} :

$$\tilde{\mathbf{h}}_{\mathbf{y}}(\mathbf{x}, \tilde{\mathbf{y}}, \kappa) = \tilde{\mathbf{h}}_{\mathbf{x}}(\mathbf{x}, \tilde{\mathbf{y}}, -\kappa) \quad \text{with } \mathbf{x} = \tilde{\mathbf{x}} \quad (27)$$

The structure is now assumed to be invariant in the \mathbf{e}_2 -direction. Consider Eq. (19) which is deduced from Eq. (17) without any assumption about the periodicity. Using notations introduced in Eq. (7), it can be rearranged as follows:

$$\hat{\mathbf{u}} = \frac{1}{2\pi} \int_{-\infty}^{+\infty} \hat{g}(k_{y_2}) e^{ik^*X_2} \hat{\mathbf{h}}_{X_1, X_3}(-k^*) dk_{y_2} \quad (28)$$

with

$$\hat{\mathbf{h}}_{X_1, X_3}(-k^*) = \int_{-\infty}^{+\infty} e^{-ik^*y_2} \hat{\mathbf{h}}(y_2) dy_2 \quad \text{and} \quad k^* = k_{y_2} - \frac{\omega}{V} \quad (29)$$

Using Eqs. (8) and (27), we deduce that:

$$\hat{\mathbf{h}}_{X_1, X_3}(-k^*) = \int_0^L e^{-ik^* \tilde{y}_2} \tilde{\mathbf{h}}_{\mathbf{y}}(\tilde{y}_2, -\kappa_0) d\tilde{y}_2 \quad (30)$$

with $\kappa_0 = k^* - \frac{2m\pi}{L}$. Replacing $\hat{\mathbf{h}}_{X_1, X_3}$ in Eq. (28) by its expression given in Eq. (30), it is shown that the response of periodic structures to moving loads using Floquet transforms (expressed by Eq. (24)) is equivalent to the response due to moving loads of invariant structures using Fourier transforms (expressed by Eq. (28)).

6. Conclusions

In this Note, Floquet transforms are used to obtain the response of periodic structures due to moving loads from the transfer function in the frequency-wavenumber domain calculated in an unbounded generic cell. The proposed method has been applied for a real railway structure. Finally, we established the equivalence of this new solution with the response of invariant structures using Fourier transforms.

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