



HAL
open science

How granular materials deform in quasistatic conditions

Jean-Noël Roux, Gaël Combe

► **To cite this version:**

Jean-Noël Roux, Gaël Combe. How granular materials deform in quasistatic conditions. IUTAM-ISIMM Symposium on Mathematical Modeling and Physical Instances of Granular Flow, Sep 2009, Reggio Calabria, Italy. pp.260. hal-00532833

HAL Id: hal-00532833

<https://hal.science/hal-00532833>

Submitted on 4 Nov 2010

HAL is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.

How granular materials deform in quasistatic conditions

J.-N. Roux ^{*} and G. Combe [†]

^{}Université Paris-Est, Laboratoire Navier, 2 Allée Kepler, Cité Descartes, 77420 Champs-sur-Marne, France*

[†]Laboratoire 3SR, Université Joseph Fourier, 38041 Grenoble, France

Abstract. Based on numerical simulations of quasistatic deformation of model granular materials, two rheological regimes are distinguished, according to whether macroscopic strains merely reflect microscopic material strains within the grains in their contact regions (type I strains), or result from instabilities and contact network rearrangements at the microscopic level (type II strains). We discuss the occurrence of regimes I and II in simulations of model materials made of disks (2D) or spheres (3D). The transition from regime I to regime II in monotonic tests such as triaxial compression is different from both the elastic limit and from the yield threshold. The distinction between both types of response is shown to be crucial for the sensitivity to contact-level mechanics, the relevant variables and scales to be considered in micromechanical approaches, the energy balance and the possible occurrence of macroscopic instabilities

Keywords: granular materials, elastoplasticity, stress-strain behavior

PACS: 81.05.Rm, 83.80.Fg

INTRODUCTION

The quasistatic limit, the rigid limit and the macroscopic limit

Although they are modeled, at the macroscopic level, with constitutive laws in which physical time and inertia play no part [1, 2], granular materials are most often investigated at the microscopic level by “discrete element” numerical methods (DEM) in which the motion of the solid bodies is determined through integration of dynamical equations involving masses and accelerations. Fully quasistatic approaches, in which the system evolution in configuration space, as some loading parameter is varied, is regarded as a continuous set of mechanical equilibrium states, are quite rare in the numerical literature [3, 4, 5]. It is regarded as a natural starting point, on the other hand, to perform suitable averages of the mechanical response of the elements of a contact network to derive the macroscopic material response [6]. Whether and in which cases it is possible to dispense with dynamical ingredients of the model at the granular level and how the quasistatic limit is approached are fundamental issues that still need clarification.

Another set of open questions are related to the role of particle deformability. Most DEM studies include contact elasticity in the numerical model. Experimentally, elastic behavior is routinely measured in quasistatic tests [7] and sound propagation. Yet, most often, contact deflections are quite negligible in comparison with grain diameters. In the “contact dynamics” method [8, 9], which is used to simulate quasistatic granular rheology [10, 11, 12], grains are modeled as rigid, undeformable solid bodies. The influence of contact deformability on the macroscopic behavior, the existence of a well-defined rigid limit are thus other basic issues calling for further investigations.

Small granular samples, as the ones used in DEM studies, often exhibit quite noisy mechanical properties. The approach to a macroscopic behavior expressed with smooth stress-strain curve might seem problematic, especially in the presence of rearrangement events, associated with instabilities at the microscopic level [13, 14].

The origins of strain

The present communication shows how one may shed light on the interplay between the quasistatic, rigid and macroscopic limits on distinguishing two different rheological regimes and delineating their conditions of occurrence, in simple model materials. Macroscopic strain in solidlike granular materials has two obvious physical origins: first, grains deform near their contacts, where stresses concentrate (so that one models the grain interaction with a point force); then, grain packs rearrange as contact networks break, and then repair in a different stable configuration. We refer here respectively to the two different kinds of strains as type I and II. The present paper, based on numerical

simulations of simple materials, identifies the regimes, denoted as I and II accordingly, within which one mechanism or the other dominates, and discusses the consequences on the quasistatic rheology of granular materials.

NUMERICAL MODEL MATERIALS AND SIMULATION PROCEDURES

Two sets of numerical simulation results are exploited below. Two-dimensional (2D) assemblies of polydisperse disks, as in Refs. [15, 3, 16], are subjected to fully stress-controlled biaxial tests, for which a quasistatic computation method [15, 3, 17] is exploited, in addition to standard DEM simulations. The behavior of three-dimensional (3D) packs of monosized spherical particles, as in Refs. [18, 19, 20] is studied in simulated triaxial compression tests, with special attention to strains in the quasistatic limit. Part of the results are presented in the references (mostly in some conference proceedings) cited just above, pending the publication of a more comprehensive study.

Two-dimensional material and stress-controlled tests

2D systems are simulated in order to investigate basic rheophysical mechanisms with good accuracy, in the simplest conceivable, yet representative, model material. Samples made of polydisperse disks in 2D, with a uniform diameter distribution between $0.5a$ and a , are first assembled on isotropically compressing frictionless particles, thus producing dense packs with solid fraction $\Phi = 0.8434 \pm 3 \times 10^{-4}$ and coordination number z close to 4 in the large system limit. Those values are extrapolated from data averaged on sets of samples with $N = 1024, 3025$ and 4900 disks. The samples are enclosed in a rectangular cell framed by solid walls, 2 of which are mobile orthogonally to their direction, which enables us to carry out biaxial compression tests (Fig. 1). Finite system effects on Φ and z are mainly due to the surrounding walls and can be eliminated (they are proportional to perimeter to area ratio).

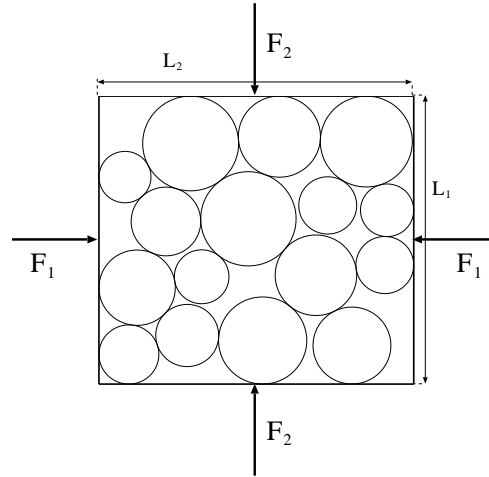


FIGURE 1. Schematic representation of the biaxial tests simulated on 2D disk samples. $\sigma_2 = F_2/L_2$ is kept constant, equal to the initial isotropic pressure P , while $\sigma_1 = F_1/L_1$ is stepwise increased.

Stress-increment controlled DEM simulations

Once prepared in mechanical equilibrium under an isotropic pressure P , disk samples, in which contacts are now regarded as frictional, with friction coefficient $\mu = 0.25$, are subjected to biaxial tests as sketched in Fig. 1. Strains $\epsilon_1 = -\Delta L_2/L_2$, $\epsilon_2 = -\Delta L_1/L_1$, “volumetric” strain $\epsilon_v = \epsilon_1 + \epsilon_2$ are measured in equilibrium configurations, while the stress deviator $q = \sigma_2 - \sigma_1$ is the control parameter. We use soil mechanics conventions, for which compressive stresses and shrinking strains are positive. q is stepwise increased by small intervals $\Delta q = 10^{-3}P$. The contact model is the standard (Cundall-Strack [21]) one with normal (K_N) and tangential (K_T) stiffness constants such that $K_N = 2K_T = 10^5P$. A normal viscous force is also introduced in the contact, in order to reach equilibrium

configurations faster. After each deviator step, one waits for the next equilibrium configuration, in which forces and moments are balanced with good accuracy (with a tolerance below $10^{-5}Pa$ for forces on grains, below $10^{-5}PL$ for forces on walls). We refer to this procedure as *stress-increment controlled* (SIC) DEM.

The strictly quasistatic approach: SEM calculations



FIGURE 2. Normal (left) and tangential (right) contact behavior in 2D disk samples, as schematized with rheological elements: springs with stiffness constants K_N , K_T , dashpot with damping constant η_N , plastic slider with threshold related to normal force by coefficient μ .

The *static elastoplastic method* (hereafter referred to as SEM), amounts to dealing with the initial sample configuration as a network of springs and plastic sliders corresponding to contact behavior, as in Fig. 2 – with the dashpots ignored, as they play no role in statics. The evolution of the system under varying load is determined as a continuous trajectory in configuration space, each point of which is an equilibrium state. It has been implemented in [15, 3], and a similar approach was used in [4]. The algorithm will not be described here, as it is presented in [17]. It relies on resolution of linear system of equations, with the form of the matrix (the elastoplastic stiffness matrix) depending on contact status (nonsliding, sliding, open). The bases of the approach are also discussed in [5].

SEM calculations are possible as long as only type I strains are obtained, and the results reported here [15] correspond to the deviator interval $0 \leq q \leq q_1$ in biaxial compressions from the chosen initial state (in which all tangential forces are equal to zero), in which a type I response is obtained.

Triaxial compression of 3D bead assemblies

Triaxial compression tests of assemblies of $N = 4000$ single-sized spherical beads of diameter a are simulated by DEM, with the more standard procedure in which the *axial strain rate* $\dot{\epsilon}_a$ is kept constant (hereafter *strain-rate controlled* or SRC DEM). The deviator stress, q , is measured, as a function of axial strain $\epsilon_a = \epsilon_1$, as $q = \sigma_1 - \sigma_3$, where σ_1 is the major (“axial”) principal stress conjugate to ϵ_a , while the other two (lateral) principal stresses $\sigma_2 = \sigma_3$ are kept equal to the initial isotropic pressure P . To allow for comparisons with laboratory experiments, the beads are attributed the elastic properties of glass (Young modulus $E = 70$ GPa, Poisson ratio $\nu = 0.3$) and friction coefficient $\mu = 0.3$. The contact law is a somewhat simplified version of the Hertz-Mindlin ones [22], as in Ref. [19], which might be consulted for more details. It leads to favorable comparisons of elastic moduli [20] obtained in simulations and measured in experiments on glass beads. Preparation of cuboidal samples with periodic boundaries in all three directions (and thus statistically homogeneous and devoid of wall effects) under prescribed pressures in the range $10 \text{ kPa} \leq P \leq 1 \text{ MPa}$ is detailed in Refs. [19, 23]. It is shown [19] that one may obtain, depending on the assembling procedure, for densities close to the random close packing limit $\Phi \simeq 0.64$ under low pressure, coordination numbers ranging from $z \simeq 4$ (or $z^* \simeq 4.5$ if the “rattlers”, i.e., grains carrying no force, are excluded from the count) to $z = 6$ (more exactly $z^* = 6$, with 1 or 2% of rattlers) in the limit of $P \rightarrow 0$. As in [19], the low-coordination systems ($z^* \simeq 4.5$) are referred to as “C samples” in the sequel, while those with $z^* \simeq 6$ are called “A samples”. We can thus assess the influence of initial coordination number on the small strain (pre-peak) behavior of a dense material.

Dimensionless control parameters

The contact law and the simulated mechanical test lead to the definition of useful dimensionless numbers. The *inertia parameter* $I = \dot{\epsilon}_a \sqrt{m/aP}$ (in 3D) or $I = \dot{\epsilon}_a \sqrt{m/P}$ (in 2D) characterizes the importance of inertial effects in strain-rate controlled tests under pressure P (m is the grain mass). The parameter I has been used repeatedly to describe the state of granular materials in steady flow, both in experiments [24] and in simulations [25, 26, 27], or the departure from equilibrium in a slow compression [23]. It also plays a central role in the recent formulation of a successful constitutive law for dense granular flows [28].

The importance of contact deflections, relative to grain diameter a , is expressed by the stiffness number, κ , which is defined as $\kappa = K_N/P$ in 2D models with linear contact elasticity, and as $\kappa = \left(\frac{E}{(1-\nu^2)P} \right)^{2/3}$ with 3D beads and Hertzian contacts. In both cases, typical contact deflections h satisfy $h/a \propto \kappa^{-1}$ [19].

The three limits mentioned in the introduction can be defined as $I \rightarrow 0$ (quasistatic limit), $\kappa \rightarrow \infty$ (rigid limit), $N \rightarrow \infty$ (macroscopic limit).

SIMULATION RESULTS

Biaxial tests in 2D

Type I response interval, quasistatic approach

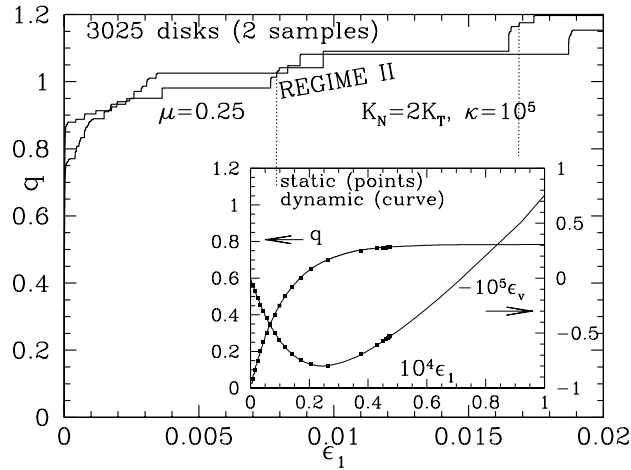


FIGURE 3. q (normalized by P) versus ϵ_a in SIC tests on 2 samples of 3025 disks, showing a very stiff increase (confused with vertical axis), and then a staircase regime. Inset: detail of very small strains, with comparison of SIC DEM and SEM calculations.

In Fig. 3, $q(\epsilon_a)$ curves as obtained by SIC DEM are shown for two samples of 3025 disks. The curves first exhibit a very sharp increase of deviator q , which, as revealed once the strain scale is blown up by a factor of 10^4 in the insert, is in fact an interval of type I response: direct SEM calculation are possible, and coincide with DEM results. The smoothness of the stress variations versus strain in that range is characteristic of a continuous trajectory of equilibrium states in configuration space. Beyond the transition to type II strain regimes, a staircase-shaped deviator curve (Fig. 3) is observed, exhibiting intervals of stability (nearly vertical parts of curve in Fig. 3), separated by rearrangement events (horizontal parts of curve in Fig. 3) in which the system gains kinetic energy before a new stable contact network is formed. We could check that the SEM procedure is able to reproduce the stability intervals obtained with SIC DEM. On reversing the load (stepwise decreasing q), a considerably larger deviator range is accessible to SEM calculations, and thus in regime I, as illustrated by the two (quasi-vertical) dotted lines on the main plot of Fig. 3.

TABLE 1. Average and standard deviation of q_1 as obtained over 26 samples with $N=1024$, 10 samples with $N=3025$ and 6 samples with $N=4900$.

SEM	N=1024	N=3025	N=4900
$\langle q_1 \rangle$	0.750 ± 0.050	0.774 ± 0.033	0.786 ± 0.024

Role of contact stiffness

As the system, in regime I, is equivalent to a network of springs and plastic sliders (Fig. 2), type I strains are all inversely proportional to stiffness level κ , provided the compression that decreases κ does not significantly affect the sample geometry. The curves pertaining to different κ values coincide if expressed with stress ratios and variables $\kappa\epsilon$, as shown in Fig 4.

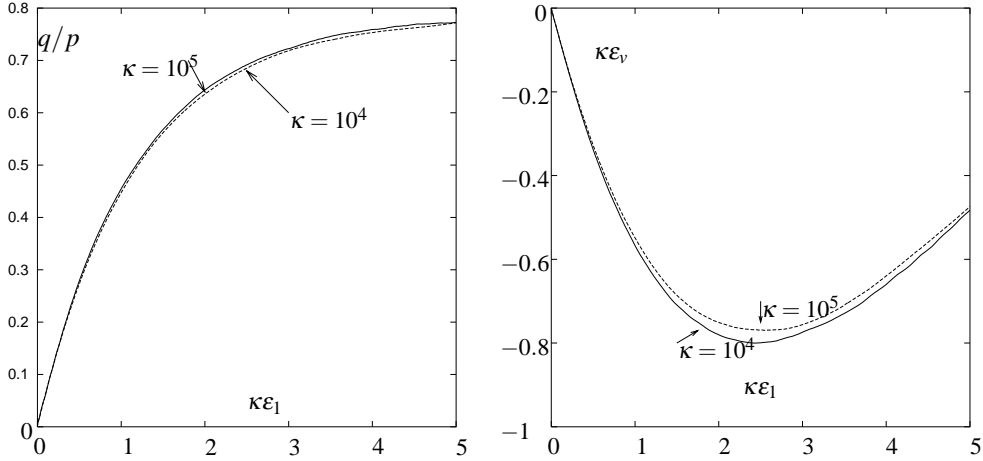


FIGURE 4. Stress ratio q/p (left) and rescaled volumetric strain $\kappa\epsilon_v$ (right) vs. rescaled axial strain $\kappa\epsilon_1$ for $\kappa = 10^5$ and $\kappa = 10^4$.

Approach to the macroscopic limit

The staircase-shaped loading curves in regime II should approach in the large sample limit a smooth stress-strain curve, as observed in very slow laboratory tests. To check for the approach of such a macroscopic limit, the average $\langle q(\epsilon_1) \rangle$ and the standard deviation $\sigma(q)(\epsilon_1)$ are computed as functions of axial strain for sets of samples of three different sizes, and the region of the $\epsilon_1 - q$ plane corresponding to $\langle q \rangle(\epsilon_1) - \sigma(q) \leq q \leq \langle q \rangle(\epsilon_1) + \sigma(q)$ is shaded on Fig. 5, darker zones corresponding to larger N . Fluctuations about the average curve decrease as the system size increases, and the insert shows that the standard deviation, as averaged over the interval $0 \leq \epsilon_1 \leq 0.02$, regresses as $N^{-1/2}$. Thus staircase curves get smoothed in the large system limit, which implies that the “stairs” become increasingly small and numerous: as N increases rearrangement events (microscopic instabilities) become smaller and smaller, but more and more frequent. A similar regression is observed for the volumetric strain curve.

Unlike the small type I response intervals observed within regime II, the stability range $q \leq q_1$ of the initial, isotropic structure does not dwindle as the system size increases. As shown in Table 1, the initial regime I deviator interval even increases a little, approaching a finite limit as $N \rightarrow \infty$.

Our implementation of SEM involves no creation of new contacts (although this could be taken into account in a more refined version). This approximation becomes exact in the rigid limit of $\kappa \rightarrow \infty$, because a finite strain increment is necessary for additional contacts to close, while type I strains scale as κ^{-1} . The near coincidence of SEM and DEM approaches, the latter involving contact creations, shows that new contacts are indeed negligible for $\kappa = 10^5$. q_1 thus represents the maximum deviator stress supported by the initial contact network, beyond which [5], due to contact sliding and opening, an instability or a “floppy mode” appears. The hallmark of such instabilities is the negativity of

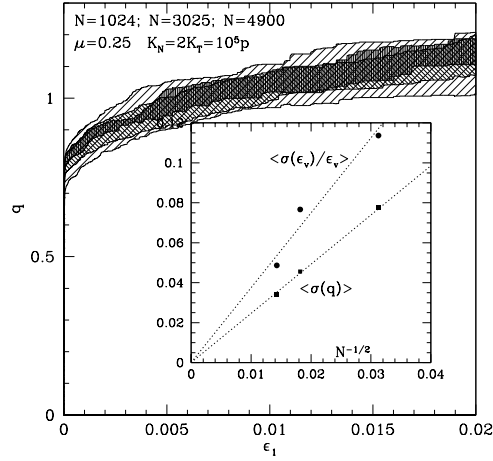


FIGURE 5. Main plot: sample to sample average of q versus ϵ_1 . Shaded regions extend to one standard deviation about the average, with, in this order, darker and darker shades of gray for $N=1024, 3025, 4900$. Inset: regression of fluctuations, proportional to $N^{-1/2}$. Average standard deviations $\sigma(q)$ and $\sigma(\epsilon_v)$ over interval $0 \leq \epsilon_1 \leq 0.02$.

the second-order work [5, 17], *viz.*

$$\Delta^2 W(\Delta \mathbf{U}) = \Delta \mathbf{U} \cdot \underline{\underline{\mathbf{K}}} \cdot \Delta \mathbf{U}$$

for some direction of displacement increment vector $\Delta \mathbf{U}$. Vector $\Delta \mathbf{U}$ comprises all increments of grain displacements and rotations, and $\underline{\underline{\mathbf{K}}}$ is the stiffness matrix, which depends, *via* the status of contacts, on the direction of $\Delta \mathbf{U}$. $\Delta^2 W(\Delta \mathbf{U}) < 0$ implies that the increment of contact forces resulting from a small perturbation $\Delta \mathbf{U}$ will accelerate the resulting motion, whence a spontaneous increase of kinetic energy.

Transition stress q_1 and the “critical yield analysis” approach

One may wonder whether q_1 marks the upper bound q_u of the deviator interval for which contact forces balancing the external load (i. e., statically admissible) and satisfying Coulomb’s inequality (i. e., plastically admissible) can be found in the network. This is the “critical yield analysis” approach to failure in structural mechanics. It is known that q_1 and q_u would coincide if the sliding in contacts where friction is fully mobilized implied dilatancy, with an angle equal to the friction angle (the discrete analog of an “associated” flow rule). Fig. 6 shows that q_1 is well below q_u . With a dilatant sliding rule, the material response in biaxial compression would be stiffer, and initially (rather paradoxically) more contractant, and the deviator would reach $1.3P$ (instead of about $0.8P$) before failure of the initial contact network.

Evolution of microscopic state variables

As mentioned above, contact creation is negligible in regime I and the fabric evolution is essentially due to contacts opening, mostly in the direction of extension (direction 2). As the initial coordination number is maximal, because of the absence of friction in the assembling process, very few contacts are gained in the direction of compression. In the initial state, all contacts only bear normal force components. Friction mobilization is gradual, but the proportion of sliding contacts, as shown in Fig. 7 steadily increases from zero in regime I, and reaches an apparent plateau in regime II. This means that the interval of elastic response is, strictly speaking, reduced to naught, even though the stress-strain curve can *approximately* be described as elastic in a very small range. The appearance of sliding contacts can effectively reduce the degree of static indeterminacy in the system. If the status is assumed to be fixed for all contacts, the Coulomb condition, satisfied as an equality, reduces the number of independent contact components from $2dN_c$ (in 2D) to $2N_c - \chi_s$, with χ_s the number of sliding contacts among a total of N_c . It has been speculated [29, 30] that failing

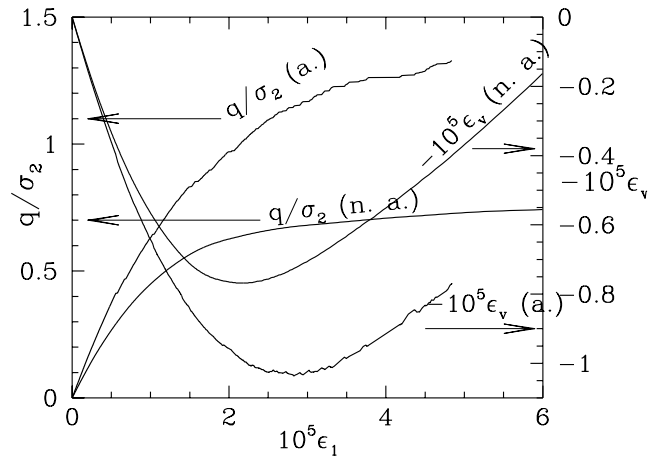


FIGURE 6. Comparison of SEM calculation with the normal (parallel, curves marked “n. a.” for non associated) and the “associated” (dilatant, curves marked “a.”) sliding rule in contacts in sample with 1024 disks.

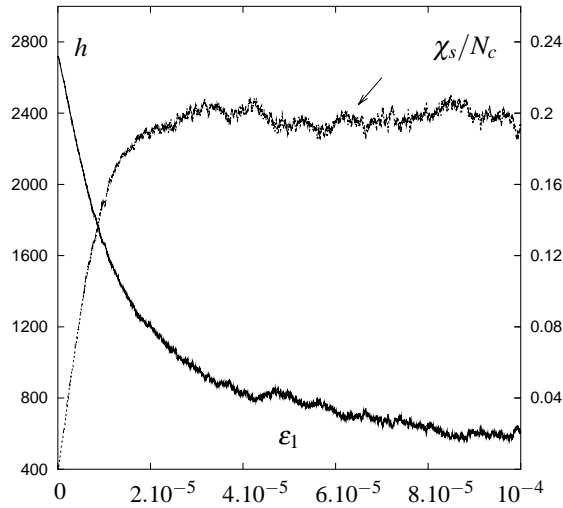


FIGURE 7. In a sample with $N=3025$, degree of force indeterminacy h (solid line) and proportion of sliding intergranular contacts χ_s/N_c (dotted line), versus axial strain ϵ_1 .

contact networks (regime II) should correspond to vanishing force indeterminacy. The data of Fig. 7 provide evidence against such a prediction, as h stabilizes to about 600, a moderate (10% of the total number of degrees of freedom in a sample of 3025 disks), yet finite value.

3D triaxial tests

The simulations reported here compare dense states A (high coordination number) and C (low coordination number). State A is similar to the dense disk sample studied in the previous section, as both were initially assembled with frictionless grains. (A samples, once packed under low pressure, are nevertheless compressed to the desired confining pressure with the value $\mu = 0.3$ of the friction coefficient used in the triaxial tests [19]). Pressure values correspond to glass beads, and vary between 10 kPa ($\kappa = 39000$) and 1 MPa ($\kappa = 1800$). We first check for the approach of the quasistatic and the macroscopic limit in 3D, strain-rate controlled DEM simulations, then discuss the influence of coordination number, and regimes I and II, in the light of the previous 2D study.

Reproducibility, quasistatic limit

As the system size increases, sample to sample fluctuations should regress, as checked in 2D (Fig. 5). Our 3D results are based on 5 samples of 4000 beads of each type, and Fig. 8 checks for stress-strain curve reproducibility in both A and C cases, for small axial strains. Thanks to the fully periodic boundary conditions [19], the macroscopic mechanical behavior is quite well defined with $N = 4000$. The approach to the quasistatic limit, in SRC tests can be assessed on checking for the innocuousness of the dynamical parameters, i.e., inertial number I , and reduced damping parameter ζ . ζ is defined as the ratio of the viscous damping constant in a contact to its critical level, given the instantaneous value of the stiffness constant. We found it convenient to use a constant value of ζ in our simulations, as in [19]. Fig. 8 also shows that provided inertial number I , characterizing dynamical effects, is small enough, both I and ζ become irrelevant. Fig. 8 shows that the quasistatic limit is correctly approached for $I \leq 10^{-3}$, quite a satisfactory result, given

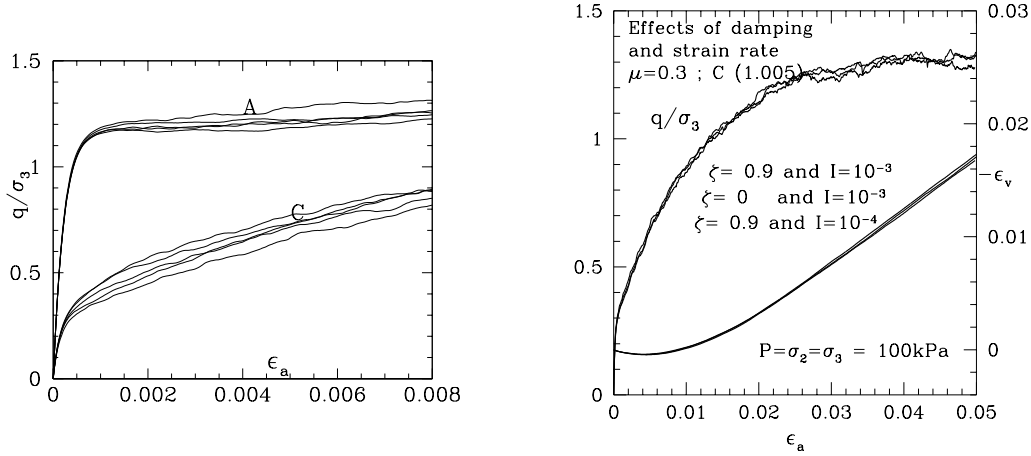


FIGURE 8. Left: small strain part of $q(\epsilon_a)$ curves for 5 different samples of each type, A (top curves) and C (bottom ones) with $N = 4000$ beads. Right: $q(\epsilon_a)$ and $\epsilon_v(\epsilon_a)$ curves in one type C sample for the different values of ζ and I indicated.

that usual laboratory tests with $\dot{\epsilon}_a \sim 10^{-5} \text{ s}^{-1}$ correspond to $I \leq 10^{-8}$.

Influence of initial coordination number

Fig. 9 compares the behavior of initial states A and C, in triaxial compression with $P = 100 \text{ kPa}$ ($\kappa \simeq 6000$). Although, conforming to the traditional view that the peak deviator stress is determined by the initial sample density,

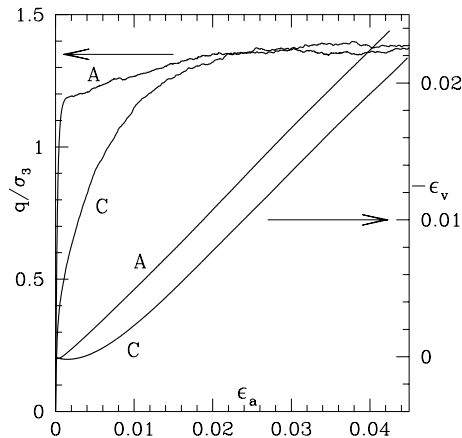


FIGURE 9. $q(\epsilon_a)$ (left scale) and $\epsilon_v(\epsilon_a)$ (right scale) curves for A and C states under $P = 100 \text{ kPa}$. Averages over 5 samples of 4000 spherical grains.

maximum q values are very nearly identical in systems A and C, the mobilization of internal friction is much more

gradual for C. For A, the initial rise of deviator q for small axial strain is quite steep, and the volumetric strain variation becomes dilatant almost immediately, for $\epsilon_a \sim 10^{-3}$. In [20] it was shown that measurements of elastic moduli provide information on coordination numbers. It is thus conceivable to infer the rate of deviator increase as a function of axial strain from very small strain ($\sim 10^{-5}$ or below [7, 20]) elasticity. Most experimental curves obtained on sands, which do not exhibit q maxima or dilatancy before $\epsilon_a \sim 0.01$, are closer to C ones. However, some measurements on glass bead samples [31] do show fast rises of q at small strains, somewhat intermediate between numerical results of types A and C.

Influence of contact stiffness

The small strain (say $\epsilon_a \leq 5 \cdot 10^{-4}$) interval for A samples, with its fast q increase, is in regime I, as one might expect from 2D results on disks. This is readily checked on changing the confining pressure. Fig. 10 shows the curves for triaxial compressions at different P values (separated by a factor $\sqrt{10}$) from 10 kPa to 1 MPa, with a rescaling of the strains by the stiffness parameter κ , in one A sample. Their coincidence for $q/P \leq 1$ evidences a wide deviator range in regime I. For larger strains, curves separate on this scale, and tend to collapse together if

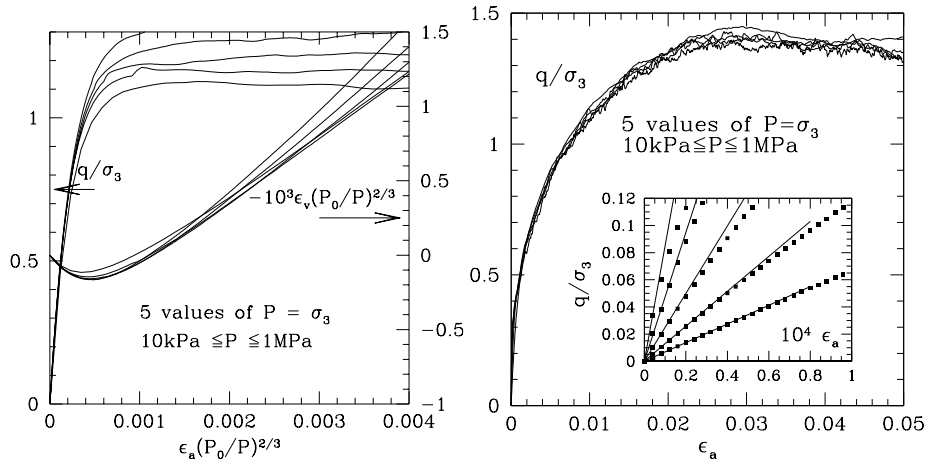


FIGURE 10. Left: $q(\epsilon_a)/P$ and $\epsilon_v(\epsilon_a)$ curves for one A sample and different P values. Strains on scale $(P/P_0)^{2/3} \propto \kappa^{-1}$, $P_0 = 100$ kPa. Right: $q(\epsilon_a)/P$ for the same P values in one C sample. Inset: detail with blown-up ϵ scale, straight lines corresponding to Young moduli in isotropic state.

q/P , ϵ_v are simply plotted versus ϵ_a . The strain dependence on stress ratio is independent from contact stiffness. This different sensitivity to pressure is characteristic of regime II. Fig 10 also shows that it applies to C samples almost throughout the investigated range, down to small deviators (a behavior closer to usual experimental results than type A configurations). At the origin (close to the initial isotropic state, see inset on fig. 10, right plot), the tangent to the curve is given by the elastic (Young) modulus of the granular material, E_m , and therefore q/P scales with κ , but curves quickly depart from this behaviour (around $q = 0.2P$). The approximately elastic range [20] is quite small, as observed in experiments [7, 32, 33].

Calculations with a fixed contact list

Within regime I, the mechanical properties of the material can be successfully predicted on studying the response of one given set of contacts. Those might slide or open, but the very few new contacts that are created can be neglected. To check this in simulations, one may restrict at each time step the search for interacting grains to the list of initially contacting pairs. Fig. 11 compares such a procedure to the complete calculation. The curve marked “NCC” for *no contact creation* is indistinguishable from the other one for $q \geq 0.8$. We thus check that, in regime I, the macroscopic behavior is essentially determined by the response of a fixed contact network.

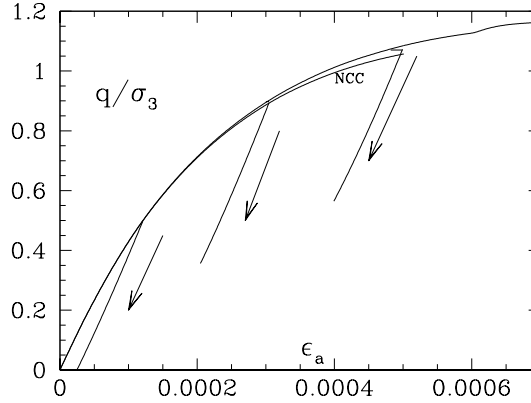


FIGURE 11. Very small strain part of $q(\epsilon_a)$ curve in one A sample, showing beginning of unloading curves (arrows). Curve marked NCC was obtained on calculating the evolution of the same sample without any contact creation.

Type I strains and elastic response

Fig. 11 also shows that the small strain response of A samples, within regime I, close to the initial state, is already irreversible: type I strains are not elastic. An approximately elastic behavior is only observed for very small strains, as depicted in the inset of Fig. 10 (right part). In this small interval near the initial equilibrium configuration, the stress-strain curve is close to its initial tangent, defined by the elastic modulus. Moduli [20] can be calculated from the stiffness matrix of contact networks. One may also check that the unloading curves shown on Fig. 11 (and the ones of Fig. 3 in 2D as well) comprise a small, approximately elastic part, with the relevant elastic modulus (the Young modulus for a triaxial test at constant lateral stress) defining the initial slope. At the microscopic level, a small elastic response is retrieved upon reversing the loading direction because contacts stop sliding. The elastic range is strictly included in the larger range of type I behavior.

Fluctuations and length scale

Finally, let us note that regimes I and II also differ by the importance of sample to sample fluctuations: curves in Fig. 8 (left plot) pertaining to the different samples of type A or C are confused as long as $q \leq 1.1P$ (case A) or $q \leq 0.3P$ (case C), which roughly corresponds to the transition from regime I to regime II. Larger fluctuations imply that the characteristic length scale associated with the displacement field (correlation length) is larger in regime II. Whether and in what sense rearrangements triggered by instabilities in regime II, in a material close to the rigid limit (large κ), can be regarded as local events is still an open issue.

CONCLUSION

Numerical studies thus reveal that the two regimes, in which the origins of strain differ, exhibit contrasting properties. Although the reported studies in 2D and 3D differ in many respects (linear versus Hertzian contacts, wall versus periodic boundaries, SIC versus SRC DEM), the same phenomena were observed in both cases. Regime I corresponds to the stability range of a given contact structure. It is larger in highly coordinated systems. It is observed in the beginning of monotonic loading tests, in which the deviator stress increases from an initial isotropic configuration, and also after changes in the direction of load increments (hence a loss in friction mobilization). Strains, for a given stress level, are then inversely proportional to contact stiffnesses. The deviator range in regime I, $q \leq q_1$, in usual monotonic tests, is strictly larger than the small elastic range, but strictly smaller than the maximum deviator. It does not vanish in the limit of large systems, unlike in the singular case of rigid, frictionless particle assemblies [13, 34]. Regime I is limited by the occurrence of elastoplastic instabilities in the contact network and does not coincide with the prediction of the critical yield approach. In regime I, the work of the externally applied load is constantly balanced by the one of contact forces, so that the kinetic energy approaches zero in the limit of slow loading rates. A remarkable consequence

is that the instability condition based on the negativity of the macroscopic second-order work [35] is never fulfilled, as macroscopic and microscopic works coincide, and the latter is positive. In regime II, network rearrangements are triggered by instabilities and some bursts of kinetic energy are observed [14]. Larger fluctuations witness longer-ranged correlations in the displacements. The microscopic origin of macroscopic strains, which are independent on contact elasticity for usual stiffness levels κ , lies in the geometry of grain packings.

On attempting to predict a macroscopic mechanical response from packing geometry and contact laws, the information about which kind of strain should dominate is crucial.

A promising perspective is the study of correlated motions associated with rearrangement events.

REFERENCES

1. G. Gudehus, F. Darve, and I. Vardoulakis, editors, *Constitutive Relations for Soils*, Balkema, Rotterdam, 1984.
2. D. M. Wood, *Soil Behaviour and Critical State Soil Mechanics*, Cambridge University Press, 1990.
3. J.-N. Roux, and G. Combe, *Comptes Rendus Physique* **3**, 131–140 (2002).
4. K. Kaneko, K. Terada, T. Kyoya, and Y. Kishino, *Int. J. Solid Struct.* **40**, 4043–4069 (2003).
5. S. McNamara, and H. J. Herrmann, *Phys. Rev. E* **74**, 061303 (2006).
6. L. La Ragione, V. Magnanimo, and J. T. Jenkins, “Constant Pressure Axisymmetric Compression of an Aggregate of Identical Elastic, Frictional Spheres,” in [36], pp. 1100–1103.
7. S. Shibuya, F. Tatsuoka, S. Teachavorasinskun, X.-J. Kong, F. Abe, Y.-S. Kim, and C.-S. Park, *Soils Found.* **32**, 26–46 (1992).
8. M. Jean, *Computational Methods in Applied Mechanics and Engineering* **177**, 235–257 (1999).
9. F. Radjaï, and V. Richefeu, *Mechanics of Materials* **41**, 715–728 (2009).
10. F. Radjaï, and S. Roux, “Contact dynamics study of 2D granular media : critical states and relevant internal variables,” in *The Physics of Granular Media*, edited by H. Hinrichsen, and D. E. Wolf, Wiley-VCH, Berlin, 2004, pp. 165–187.
11. F. Radjaï, H. Troadec, and S. Roux, “Basic features of granular plasticity,” in *Granular Materials: Fundamentals and Applications*, edited by S. J. Antony, W. Hoyle, and Y. Ding, Royal Society of Chemistry, Cambridge, 2004, pp. 157–183.
12. F. Radjaï, *ArXiv e-prints* (2008), 0801.4722.
13. G. Combe, and J.-N. Roux, *Phys. Rev. Lett.* **85**, 3628–3631 (2000).
14. L. Staron, J.-P. Vilotte, and F. Radjaï, *Phys. Rev. Lett.* **89**, 204302 (2002).
15. G. Combe, *Mécanique des matériaux granulaires et origines microscopiques de la déformation*, Presses du Laboratoire Central des Ponts et Chaussées, Paris, 2002.
16. G. Combe, and J.-N. Roux, “Discrete numerical simulations, quasistatic deformation and the origins of strain in granular materials,” in *Deformation characteristics of geomaterials*, edited by H. di Benedetto, T. Doanh, H. Geoffroy, and C. Sauzéat, Swets and Zeitlinger, Lisse, 2003, pp. 1071–1078.
17. J.-N. Roux, and G. Combe, “Méthodes quasi-statiques,” in *Modélisation numérique discrète des matériaux granulaires*, edited by F. Radjaï, and F. Dubois, Hermes, 2009.
18. J.-N. Roux, “The nature of quasistatic deformation in granular materials,” in *Powders and Grains 2005*, edited by R. García Rojo, H. J. Herrmann, and S. McNamara, Balkema, Leiden, 2005, pp. 261–265.
19. I. Agnolin, and J.-N. Roux, *Phys. Rev. E* **76**, 061302 (2007).
20. I. Agnolin, and J.-N. Roux, *Phys. Rev. E* **76**, 061304 (2007).
21. P. A. Cundall, and O. D. L. Strack, *Géotechnique* **29**, 47–65 (1979).
22. K. L. Johnson, *Contact Mechanics*, Cambridge University Press, 1985.
23. I. Agnolin, and J.-N. Roux, *Phys. Rev. E* **76**, 061303 (2007).
24. GDR MiDi, *European Physical Journal E* **14**, 341–365 (2004).
25. F. da Cruz, S. Emam, M. Prochnow, J.-N. Roux, and F. Chevoir, *Phys. Rev. E* **72**, 021309 (2005).
26. T. Hatano, *Phys. Rev. E* **75**, 060301(R) (2007).
27. P.-E. Peyneau, and J.-N. Roux, *Phys. Rev. E* **78**, 011307 (2008).
28. P. Jop, Y. Forterre, and O. Pouliquen, *Nature* **441**, 727–730 (2006).
29. J. Lanier, and M. Jean, *Powder Technology* **109**, 206–221 (2000).
30. N. P. Kruyt, and L. Rothenburg, “Plasticity of Granular Materials: a structural mechanics view,” in [36], pp. 1073–1079.
31. S. Emam, J. Canou, A. Corfdir, J.-C. Dupla, and J.-N. Roux, “Élaboration et comportement mécanique de matériaux granulaires solides modèles : expériences et simulations numériques,” in *Rhéologie des pâtes et des matériaux granulaires*, edited by B. Cazacliu, and J.-N. Roux, Presses du Laboratoire Central des Ponts et Chaussées, Paris, 2006, vol. SI12 of *Etudes et Recherches des Laboratoires des Ponts et Chaussées*, pp. 105–145.
32. H. di Benedetto, H. Geoffroy, C. Sauzéat, and B. Cazacliu, “Sand behaviour in very small to medium strain domains,” in *Pre-failure Deformation Characteristics of Geomaterials*, edited by M. Jamiolkowski, R. Lancellotta, and D. Lo Presti, Balkema, Rotterdam, 1999, pp. 89–96.
33. R. Kuwano, and R. J. Jardine, *Géotechnique* **52**, 727–749 (2002).
34. P.-E. Peyneau, and J.-N. Roux, *Phys. Rev. E* **78**, 041307 (2008).
35. F. Prunier, F. Laoufa, and F. Darve, *European Journal of Environmental and Civil Engineering* **13**, 135–147 (2009).
36. M. Nakagawa, and S. Luding, editors, *Powders and Grains '09*, vol. 1145 of *AIP Conference Proceedings*, 2009.