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## HORIZONTAL DISPLACEMENTS CONTRIBUTION TO TSUNAMI WAVE ENERGY BALANCE

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ABSTRACT. The main reason for the generation of tsunamis is the deformation of the bottom of the ocean caused by an underwater earthquake. Usually, only the vertical bottom motion is taken into accound while the horizontal displacements are neglected. In the present paper we study both the vertical and the horizontal bottom motion while we propose a novel methodology for reconstructing the bottom coseismic displacements field which is transmitted to the free surface using a new three-dimensional Weakly Nonlinear (WN) approach. We pay a special attention to the evolution of kinetic and potential energies of the resulting wave while the contribution of horizontal displacements into wave energy balance is also quantified. Approaches proposed in this study are illustrated on the July 17, 2006 Java tsunami.

#### 1. Introduction

During the years 2004 to 2006 several interesting tsunami events took place in the Indian Ocean. In December 26, 2004, the Great Sumatra-Andaman earthquake ( $M_w = 9.1$ , cf. C. Ammon et al. (2005), [AJT<sup>+</sup>05]) generated the devastating Indian Ocean tsunami refered in the literature as the Tsunami Boxing Day 2004 (cf. C. Synolakis & E. Bernard (2006), [SB06]). The local tsunami runups from this event exceeded the height of 34 m at Lhoknga in the western Aceh Province. This and other observations led many researchers to ask whether this tsunami was unusualy large for this specific earthquake size. Later it was shown by E.L. Geist et al. (2006) [GBAT06] that the Great Sumatra-Andaman earthquake is very similar in terms of local tsunami magnitude to past events of the same size, for example appeares similar scales scales with the 1964 Great Alaska earthquake ( $M_w = 9.2$ , cf. H. Kanamori (1970) [Kan70]).

On March 28, 2005 another earthquake occured approximately 110 km to the SE from the Great Sumatra-Andaman earthquake's epicenter. The magnitude of this earthquake was estimated to be  $M_w = 8.6 \sim 8.7$  (cf. [LKA+05, Bil05, WIS05]). This event triggered a massive evacuation in the surrounding Indian Ocean countries. However, the March 28, 2005 Northern Sumatra earthquake failed to generate a significant tsunami event. The survey teams reported maximum tsunami runup of 4 m. [GBAT06]. This event can be compared with the 1957 Aleutian earthquake ( $M_w = 8.6$ , cf. J.M. Johnson and K. Satake (1993) [JS93]) which produced a maximum tsunami runup of 15 m (see J.F. Lander (1996)

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[Lan96]). The deficiency of the March 2005 tsunami is related to the slip concentration in the down-dip part of the rupture zone and to the fact that a substantial part of the vertical displacement occured in shallow waters or on the ground [GBAT06].

By contrast, the smaller July 16, 2006 Java earthquake ( $M_w = 7.8$ , cf. C.J. Ammon et al. (2006), [AKLV06]) generated an unexpectedly destructive tsunami wave which affected over 300 km of south Java coastline and killed more than 600 people. Field measurements of runup distributions range uniformly from 5 to 7 m in most inundated areas. However, unexpectably high runup vanues were reported at Nusakambangan Island exceeding the value of 20 m. This tsunami focusing effect could be seemingly ascribed to local site effects. July 16, 2006 Java tsunami can be compared to a similar event occured on June 2, 1994 at the East Java ( $M_w = 7.6$ ) (see Y. Tsuji et al. (1995) [TIM+95]). This 1994 Java tsunami produced more than 200 casualties with local runup at Rajekwesi slightly exceeding 13 m.

All these examples of recent and historical tsunami events show that there is a big variaty of local tsunami height and runup with respect to the earthquake magnitude  $M_w$ . It is almost obvious that the seismic moment  $M_0$  is adequate to estimate far-field tsunami amplitudes. However, tsunami wave energy reflects better the local tsunami severity, while the specific runup distribution depends on bathymetric propagation paths and other site-specific effects. One of the central challenges in the tsunami science is to rapidly assess a local tsunami severity from very first rough earthquake estimations. In the current state of our knowledge false alarms for local tsunamis appear to be unavoidable. The tsunami generation modeling attempts to improve our understanding of tsunami behaviour in the vicinity of the genesis region.

Tsunami generation modeling was initiated in the early sixties by the prominent work of K. Kajiura [Kaj63], who proposed the use of the static vertical sea bed displacement for the initial condition of the free surface elevation. Classically, the celebrated Okada [Oka85, Oka92] and sometimes Mansinha & Smylie<sup>1</sup> [MS67, MS71] or Gusiakov [Gus78] solutions are used to compute the coseismic sea bed displacements. This approach is still widely used by the tsunami wave modeling community. However, some progress has been recently made in this direction [OTM01, DD07c, Dut07, DD09b, RLF+08, SF09, DPD10].

In tsunami wave modeling community the horizontal displacements are often disregarded in the tsunami genesis process. We can quote here a few publications devoted to tsunami waves such as one by D. Stevenson (2005) [Ste05]:

"Although horizontal displacements are often larger, they are unimportant for tsunami generation except to the extent that the sloping ocean floor also forces a vertical displacement of the water column."

Or another one by G.A. Ichinose et al. (2000) [ISAL00]:

"The initial lake level values were specified by assuming that the lake surface instantaneously conformed to the vertical displacement of the lake bottom while horizontal velocities were set to zero. The effect of horizontal deformation on the initial condition is neglected here and left for future work."

<sup>&</sup>lt;sup>1</sup>In fact, Mansinha & Smylie solution is a particular case of the more general Okada solution.

The authors of this article also tended to neglect the horizontal displacements field in previous tsunami generation studies [Dut07, Mit09]. Perhaps, this situation can be ascribed to the work of E. Berg (1970) [Ber70] who showed in the case of the 1964 Alaska's earthquake  $(M_w = 9.2)$  that the input into the potential energy from the horizontal motion is less than 1.5% of that from the vertical movement.

The attitude to horizontal displacements changed after the prominent work by Y. Tanioka and K. Satake (1996) [TS96]. According to their computations for the 1994 Java earthquake, the inclusion of horizontal displacements contribution results in 43% increase in maximum initial vertical ground displacement and an increase in the wave amplitudes at the shoreline by 30%. The predicted tsunami runup increases by a factor of two. These striking results incited researchers to reconsider the role of the horizontal sea bed motion. Hereafter, various tsunami generation techniques which involve only the vertical displacement field are referred to as the incomplete scenario. On the contrary, the complete tsunami generation takes also into account the horizontal displacements field.

In the current study we shed some light onto the energy transfer process from an underwater earthquake to the implied tsunami wave (in the spirit of the study by D. Dutykh & F. Dias (2009), [DD09a]), while taking into account and quantifying the horizontal displacements contribution into tsunami energy balance. We focus only on the generation stage since the propagation phase and runup techniques are well understood nowadays [Ima96, DKK08, KCY07, DPD10, DKM10].

The present study is organized as follows. In Section 2 we present mathematical models used in this study. Specifically, in Sections 2.1 and in Section 2.2 a description of the bottom motion and the fluid layer solution is presented respectively. Some rationale on tsunami wave energy computations is discussed in Section 2.2.1. Numerical results are presented in Section 3. Finally, some important conclusions of this study are outlined in Section 4.

#### 2. Mathematical models

Tsunami waves are caused by rapid motion of the seafloor due to an underwater earthquake over broad areas in comparison to water depth. There are some other mechanisms of tsunami genesis such as underwater landslides, for example. However, in this study we focus on the purely seismic mechanism which occurs most frequently in nature.

Hydrodynamics and seismology are only weakly coupled in the tsunami generation problem. Namely, the released seismic energy is partially transmitted to the ocean through the sea bed deformation while the ocean has obviously no influence onto the rupturing process. Consequently, our problem is reduced to two relatively distinct questions:

- (1) Reconstruct the sea bed deformation  $h = h(\vec{x}, t)$  caused by the seismic event under consideration
- (2) Compute the resulting free surface motion

The way how these questions are answered in our study is explained below in Sections 2.1 and 2.2 respectively.

- 2.1. **Dynamic bottom displacements reconstruction.** Traditionally, the free surface initial condition for various tsunami propagation codes (see [TG97, Ima96, IYO06]) is assumed to be identical to the static vertical deformation of the ocean bottom [Kaj70]. This assumption is classically justified by the three following reasons:
  - (1) Tsunamis are long waves. In this regime the vertical acceleration is neglected with respect to the gravity force
  - (2) The sea bed deformation is assumed to be instantaneous. It is based on the comparison of gravity wave speed (200 m/s for water depth of 4 km) and the seismic wave celerity ( $\approx 3600 \text{ m/s}$ )
  - (3) The effect of horizontal bottom motion is negligible for tsunami generation since the bathymetry is considered as a perturbation of the flat bottom [Ber70]

We have to say that nonhydrostatic effects as well as finite time source duration have been modeled in several recent studies [TT01, DD07c, FG07, KDD07, DD09b, FM09]. Some attempts have also been made to take into account the horizontal displacements contribution [TS96, GBAT06, SFZ<sup>+</sup>08].

In the present study we relax all three classical assumptions. The bottom deformation is reconstructed using the finite fault solution as it was suggested in our previous study [DMG10]. However, we extend the previous construction to take into account the horizontal displacements contribution as well. The finite fault solution is based on the multi-fault representation of the rupture [BLM00, JWH02]. The rupture complexity is reconstructed using a joint inversion of the static and seismic data. Fault's surface is parametrized by multiple segments with variable local slip, rake angle, rise time and rupture velocity. The inversion is performed in an appropriate wavelet transform space. The objective function is a weighted sum of  $L_1$ ,  $L_2$  norms and some correlative functions. With this approach seismologists are able to recover rupture slip details [BLM00, JWH02]. This available seismic information is exploited hereafter to compute the sea bed displacements produced by an underwater earthquake with better geophysical resolution.

Let us describe the way of how the sea bed displacements are reconstructed. In this reconstruction procedure we follow the great lines of our previous study [DMG10], while adding new ingredients concerning the horizontal displacements field reconstruction. We illustrate the proposed approach in the case of the July 17, 2006 Java tsunami for which the finite fault solution is available [Ji06].

The fault is considered to be the rectangle with vertices located at  $(109.20508^{\circ} \text{ (Lon)}, -10.37387^{\circ} \text{ (Lat)}, 6.24795 \, km \text{ (Depth)}), <math>(106.50434^{\circ}, -9.45925^{\circ}, 6.24795 \, km), (106.72382^{\circ}, -8.82807^{\circ}, 19.79951 \, km), (109.42455^{\circ}, -9.74269^{\circ}, 19.79951 \, km) \text{ (see Figure 1)}. The fault's plane is conventionally divided into <math>N_x = 21$  subfaults along strike and  $N_y = 7$  subfaults down the dip angle, leading to the total number of  $N_x \times N_y = 147$  equal segments. Parameters such as subfault location  $(x_c, y_c)$ , depth  $d_i$ , slip  $u_i$  and rake angle  $\phi_i$  for each segment can be found at [Ji06] (see also Appendix II, [DMG10]). The elastic constants and parameters such as dip and slip angles which are common to all subfaults are given in Table 1. We underline that the slip angle is measured conventionally in the counter-clockwise direction from the North. The relations between the elastic wave celerities  $c_p$ ,  $c_s$  and Lamé

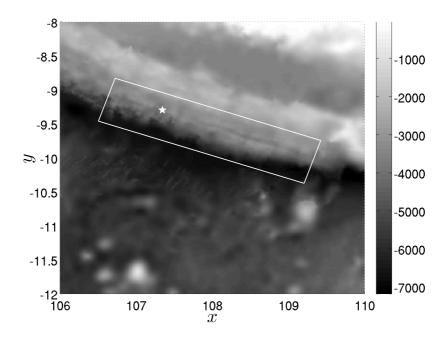


FIGURE 1. Surface projection of the fault's plane and the ETOPO1 bathymetric map of the region we study. The symbol  $\star$  indicates the hypocenter's location at  $(107.345^{\circ}, -9.295^{\circ})$ . The local Cartesian coordinate system is centered at the point  $(108^{\circ}, -10^{\circ})$ . This region is located between  $(106^{\circ}, -8^{\circ})$  and  $(110^{\circ}, -12^{\circ})$ .

$P$ -wave celerity $c_p, m/s$	6000
S-wave celerity $c_s$ , $m/s$	3400
Crust density $\rho$ , $kg/m^3$	2700
Dip angle, $\delta$	10.35°
Slip angle (CW from N)	288.94°

Table 1. Geophysical parameters used to model elastic properties of the subduction zone in the region of Java.

coefficients  $\lambda$ ,  $\mu$  used in Okada's solution are classical and can also be found in Appendix III, [DMG10].

One of the main ingredients in our construction is the so-called Okada solution [Oka85, Oka92] which is used in the case of an active fault of small or intermidiate size. The success of this solution may be ascribed to the closed-form analytical expressions which can be effectively used for various modeling purposes involving coseismic deformation.

Remark 1. The celebrated Okada solution [Oka85, Oka92] is based on two main ingredients—the dislocation theory of Volterra [Vol07] and Mindlin's fundamental solution for an

elastic half-space [Min36]. Particular cases of this solution were known before Okada's work, for example the well-known Mansinha & Smylie's solution [MS67, MS71]. Usually all these particular cases differ by the choice of the dislocation and the Burger's vector orientation [Pre65]. We recall the basic assumptions behind this solution:

- Fault is immersed into the linear homogeneous and isotropic half-space
- Fault is a Volterra's type dislocation
- Dislocation has a rectangular shape

For more information on Okada's solution we refer to [DD07c, DD07a, Dut07].

The trace of the Okada's solution at the sea bottom (substituting z=0 in the geophysical coordinate system) for each subfault will be denoted  $\mathcal{O}_i^{(j)}(\vec{x};\delta,\lambda,\mu,\ldots)$ , where  $\delta$  is the dip angle,  $\lambda$ ,  $\mu$  are the Lamé coefficients and dots denote the dependence of the function  $\mathcal{O}_i^{(j)}(\vec{x})$  on other 8 parameters, cf. [DD07c]. The index i takes values from 1 to  $N_x \times N_y$  and denotes the corresponding subfault segment, while the superscript j is equal to 1 or 2 for the horizontal displacements and to 3 for the vertical component. Hereafter we will adopt the short-hand notation  $\mathcal{O}_i^{(j)}(\vec{x})$  for the  $j^{\text{th}}$  displacement component of the Okada's solution for the  $i^{\text{th}}$  segment having in mind its dependence on various parameters.

Taking into account the dynamic characteristics of the rupturing process. We make some further assumptions on the time dependence of displacement fields. The finite fault solution provides us two additional parameters concerning the rupture dynamics — the rupture velocity  $v_r$  and the rise time  $t_r$  which are equal to 1.1 km/s and 8 s for July 17, 2006 Java event respectively. The epicenter is located at the point  $\vec{x}_e = (107.345^{\circ}, -9.295^{\circ})$  (cf. [Ji06]). Given the origin  $\vec{x}_e$ , the rupture velocity  $v_r$  and  $i^{\text{th}}$  subfault location  $\vec{x}_i$ , we define the subfault activation times  $t_i$  needed for the rupture to achieve the corresponding segment i by the formulas:

$$t_i = \frac{||\vec{x}_e - \vec{x}_i||}{v_r}, \quad i = 1, \dots, N_x \times N_y.$$

We will also follow the pioneering idea of J. Hammack [Ham72, Ham73] developed later in [TT01, THT02, DD07c, DDK06, KDD07] where the maximum bottom deformation is achieved during some finite time (known as the *rise time*) according to a specifically chosen dynamic scenario. Various scenarios used in practice (instantaneous, linear, trigonometric, exponential, etc) can be found in [Ham73, DDK06, DD07c]. In this study we adopt the trigonometric scenario which is given by the following formula:

(2.1) 
$$T(t) = \mathcal{H}(t - t_r) + \frac{1}{2}\mathcal{H}(t)\mathcal{H}(t_r - t)(1 - \cos(\pi t/t_r)),$$

where  $\mathcal{H}(t)$  is the Heaviside step function. This scenario has an advantage to have also the first derivative continuous at the activation time t = 0. For illustrative purposes this dynamic scenario is represented on Figure 2.

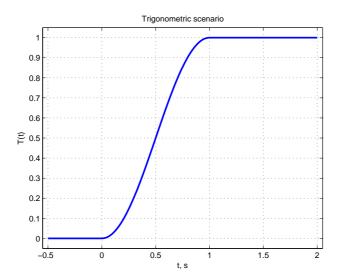


FIGURE 2. Trigonometric dynamic scenario for  $t_r = 1$  s (see J. Hammack (1973), [Ham73]).

We sum up together all ingredients proposed above to reconstruct dynamic displacements field  $\vec{u} = (u_1, u_2, u_3)$  at the sea bottom:

$$u_j(\vec{x}, t) = \sum_{i=1}^{N_x \times N_y} T(t - t_i) \mathcal{O}_i^{(j)}(\vec{x}).$$

**Remark 2.** We would like to underline here the asymptotic behaviour of the sea bed displacements. By definition of the trigonometric scenario (2.1) we have  $\lim_{t\to +\infty} T(t) = 1$ . Consequently, the sea bed deformation will attain fast its state which consists of the linear superposition of subfaults contributions:

$$u_j(\vec{x}, t) = \sum_{i=1}^{N_x \times N_y} \mathcal{O}_i^{(j)}(\vec{x}).$$

Finally, we can predict the sea bed motion by taking into account horizontal and vertical displacements:

(2.2) 
$$h(\vec{x},t) = h_0(\vec{x} + \vec{u}_{1,2}(\vec{x},t)) - u_3(\vec{x},t),$$

where  $h_0(\vec{x})$  is a function which interpolates<sup>2</sup> the static bathymetry profile given e.g. by the ETOPO1 database (see Figures 1 and 4).

**Remark 3.** In some studies where horizontal displacements were taken into account (cf. [TS96, BCHK06, SFZ<sup>+</sup>08, BCD<sup>+</sup>09]), the first order Taylor expansion was permanently

<sup>&</sup>lt;sup>2</sup>In our numerical simulations presented below we use the MATLAB TriScatteredInterp class to interpolate the static bathymetry values given by ETOPO1 database.

applied to the bathymetry representation formula (2.2) to give:

(2.3) 
$$h(\vec{x},t) \approx h_0(\vec{x}) + \vec{u}_{1,2}(\vec{x},t) \cdot \nabla_{\vec{x}} h_0(\vec{x}) - u_3(\vec{x},t)$$

We prefer not to follow this approximation and to use the exact formula (2.2) since it is valid for all slopes (see Figure 4 for Java bathymetry). Another difficulty lies in the estimation of the bathymetry gradient  $\nabla_{\vec{x}}h_0(x)$  required by Taylor formula (2.3). The application of any finite differences scheme to the measured  $h_0(\vec{x})$  leads to an ill-posed problem. Consequently, one needs to apply extensive smoothing procedures to the raw data  $h_0(\vec{x})$  which induces an additional loss in accuracy.

In the present study we do not completely avoid the computation of the static bathymetry  $qradient \nabla_{\vec{x}} h_0(\vec{x})$  since the kinematic bottom boundary condition (2.7) involves the time derivative of the bathymetry function:

$$\partial_t h = \nabla_{\vec{x}} h_0(\vec{x}) \cdot \partial_t \vec{u}_{1,2}(\vec{x},t) - \partial_t u_3(\vec{x},t).$$

The main difference with formula (2.3) is that the former is approximate while the latter is exact.

Another possibility could be to consider static horizontal displacements  $\vec{u}_{1,2}(\vec{x})$  thus keeping dynamics only in the vertical component  $u_3(\vec{x},t)$ . However, we do not choose this option in this work.

In the following section we will present our approach in coupling this dynamic deformation with the hydrodynamic problem to predict waves induced on the Ocean free surface.

2.2. The water wave problem with moving bottom. We consider the incompressible flow of an ideal fluid with constant density  $\rho$  in the domain  $\Omega \subseteq \mathbb{R}^2$ . The horizontal independent variables will be denoted by  $\vec{x} = (x, y)$  and the vertical one by z. The origin of the Cartesian coordinate system is traditionally chosen such that the surface z=0corresponds to the still water level. The fluid domain is bounded below by the bottom  $z=-h(\vec{x},t)$  and above by the free surface  $z=\eta(\vec{x},t)$ . Usually we assume that the total depth  $H(\vec{x},t) := h(\vec{x},t) + \eta(\vec{x},t)$  remains positive  $H(\vec{x},t) \ge h_0 > 0$  under the system dynamics  $\forall t \in [0, T]$ . The sketch of the physical domain is shown in Figure 3.

**Remark 4.** Classically in water wave modeling, we make the assumption that the free surface is a graph  $z = \eta(\vec{x},t)$  of a single-valued function. It means in practice that we exclude some interesting phenomena, (e.g. wave breaking phenomena) which are out of the scope of this modeling paradigm.

The governing equations of the classical water wave problem are the following [Lam32, Sto58, Mei94, Whi99]:

(2.4) 
$$\Delta \phi = \nabla^{2} \phi + \partial_{zz}^{2} \phi = 0, \qquad (\vec{x}, z) \in \Omega \times [-h, \eta],$$
(2.5) 
$$\partial_{t} \eta + \nabla \phi \cdot \nabla \eta - \partial_{z} \phi = 0, \qquad z = \eta(\vec{x}, t),$$
(2.6) 
$$\partial_{t} \phi + \frac{1}{2} |\nabla \phi|^{2} + \frac{1}{2} (\partial_{z} \phi)^{2} + g \eta = 0, \qquad z = \eta(\vec{x}, t),$$
(2.7) 
$$\partial_{t} h + \nabla \phi \cdot \nabla h + \partial_{z} \phi = 0, \qquad z = -h(\vec{x}, t),$$

(2.5) 
$$\partial_t \eta + \nabla \phi \cdot \nabla \eta - \partial_z \phi = 0, \qquad z = \eta(\vec{x}, t),$$

(2.6) 
$$\partial_t \phi + \frac{1}{2} |\nabla \phi|^2 + \frac{1}{2} (\partial_z \phi)^2 + g \eta = 0, \qquad z = \eta(\vec{x}, t),$$

(2.7) 
$$\partial_t h + \nabla \phi \cdot \nabla h + \partial_z \phi = 0, \qquad z = -h(\vec{x}, t),$$

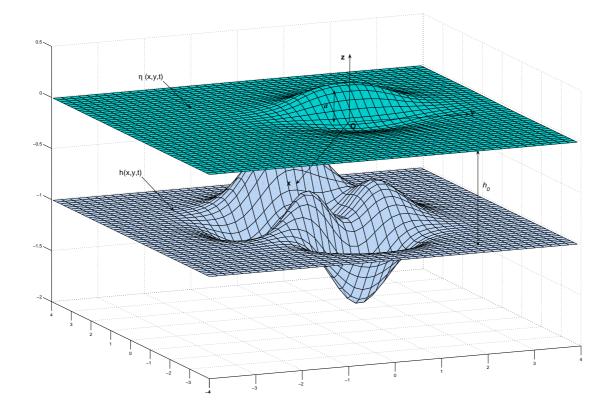


FIGURE 3. Sketch of the physical domain.

with  $\phi$  the velocity potential, g the acceleration due to gravity force and  $\nabla = (\partial_x, \partial_y)$  denotes the gradient operator in horizontal Cartesian coordinates.

Fluid incompressibility and flow irrotationality assumptions lead to the Laplace equation (2.4) for the velocity potential  $\phi(\vec{x}, z, t)$ . The main difficulty of the water wave problem lies on the boundary conditions. Equations (2.5) and (2.7) express the free surface and bottom impermeability, while the Bernoulli condition (2.6) expresses the free surface isobarity respectively.

Function  $h(\vec{x}, t)$  represents the ocean's bathymetry (depth below the still water level, see Figure 3) and is assumed to be known. The dependence on time is included in order to take into account the bottom motion during an underwater earthquake [DD07a, DD07c, DDK06, KDD07, Dut07].

Remark 5. Recently, some weak dissipative effects have also been included into the classical water wave problem (2.4) – (2.7). For more details on the visco-potential formulation we refer to [DDZ08, DD07b, Dut07, Dut09b, Dut09a].

For the exposition below we will need also to compute unitary exterior normals to the fluid domain. It is straightforward to obtain the following expressions for the normals at

the free surface  $S_f$  and the bottom  $S_b$  respectively:

$$\hat{n}_f = \frac{1}{\sqrt{1+|\nabla \eta|^2}} \begin{vmatrix} -\nabla \eta \\ 1 \end{vmatrix}, \qquad \hat{n}_b = \frac{1}{\sqrt{1+|\nabla h|^2}} \begin{vmatrix} -\nabla h \\ -1 \end{vmatrix}.$$

In 1968 V. Zakharov proposed a different formulation of the water wave problem based on the trace of the velocity potential at the free surface [Zak68]:

$$\varphi(\vec{x},t) := \phi(\vec{x},\eta(\vec{x},t),t).$$

This variable plays a role of the generalized momentum in the Hamiltonian description of water waves [Zak68, DB06]. The second canonical variable is the free surface elevation  $\eta$ .

Another important ingredient is the normal velocity at the free surface  $v_n$  which is defined as:

(2.8) 
$$v_n(\vec{x},t) := \sqrt{1 + |\nabla \eta|^2} \frac{\partial \phi}{\partial \hat{n}_f} \Big|_{z=\eta} = (\partial_z \phi - \nabla \phi \cdot \nabla \eta)|_{z=\eta}.$$

Dynamic boundary conditions (2.5) and (2.6) at the free surface can be rewritten in terms of  $\varphi$ ,  $v_n$  and  $\eta$  [CSS92, CS93, FCKG05]:

(2.9) 
$$\partial_t \eta - \mathcal{D}_{\eta}(\varphi) = 0,$$

$$\partial_t \varphi + \frac{1}{2} |\nabla \varphi|^2 + g\eta - \frac{1}{2(1+|\nabla \eta|^2)} \left[ \mathcal{D}_{\eta}(\varphi) + \nabla \varphi \cdot \nabla \eta \right]^2 = 0.$$

Here we introduced the so-called Dirichlet-to-Neumann operator (D2N) [CM85, CS93] which maps the velocity potential at the free surface  $\varphi$  to the normal velocity  $v_n$ :

$$\mathcal{D}_{\eta}: \qquad \varphi \mapsto v_{n} = \sqrt{1 + |\nabla \eta|^{2}} \frac{\partial \phi}{\partial \hat{n}_{f}} \Big|_{z=\eta}$$

$$\nabla^{2} \phi + \partial_{zz}^{2} \phi = 0, \quad (\vec{x}, z) \in \Omega \times [-h, \eta],$$

$$\phi = \varphi, \quad z = \eta,$$

$$\sqrt{1 + |\nabla h|^{2}} \frac{\partial \phi}{\partial \hat{n}_{h}} = \partial_{t} h, \quad z = -h.$$

The name of this operator comes from the fact that it makes a correspondence between Dirichlet data  $\varphi$  and Neumann data  $\sqrt{1+|\nabla\eta|^2} \frac{\partial \varphi}{\partial \hat{n}_f}\Big|_{z=\eta}$  at the free surface.

So, the water wave problem can be reduced to a system of two PDEs (2.9) governing the evolution of the canonical variables  $\eta$  and  $\varphi$ . In order to solve this system of equations we have to be able to compute efficiently the D2N map  $\mathcal{D}_{\eta}(\varphi)$ . We will use the Weakly Nonlinear (WN) method described in [DMG10]. It relies on the approximate solution of the 3D Laplace equation in a perturbed strip-like domain using the Fourier transform  $(\hat{\varphi} := \mathcal{F}[\varphi], \, \eta = \mathcal{F}^{-1}[\hat{\eta}])$ :

$$\hat{\mathcal{D}}_{\eta}(\varphi) = \hat{\varphi}|\vec{k}|\tanh(|\vec{k}|H) + \hat{f}\operatorname{sech}(|\vec{k}|H) - \mathcal{F}\Big[\mathcal{F}^{-1}\big[i\vec{k}\hat{\varphi}\big]\cdot\mathcal{F}^{-1}\big[i\vec{k}\hat{\eta}\big]\Big],$$

where  $\vec{k}$  is the wavenumber and f is the bathymetry forcing term

$$f(\vec{x},t) := -\partial_t h - \nabla \phi|_{z=-h} \cdot \nabla h.$$

Several details on the time integration procedure can be also found in [DMG10]. Basically, the main idea to integrate exactly in time the linear terms is due to D. Clamond and his collaborators [FCKG05]. The resulting method is only weakly nonlinear and analogous at some point to the first order approximation model proposed in [GN07].

This method was validated in [DMG10] by comparisons with the numerically exact Tanaka solution [Tan86]. Recently it was shown that a tsunami generation process is essentially linear [KDD07, SF09]. Consequently, our WN approach will allow us to observe some nonlinear effects if locally they are important. This is possible when the generation region involves a wide range of depths from deep to shallow regions (see Figures 1 and 4).

2.2.1. Tsunami wave energy. In this study we are more particularly interested in the evolution of the generated wave energy [DD09a]. In free surface incompressible flows, the kinetic and potential energies (denoted K and  $\Pi$  correspondingly) are completely determined by the velocity field and the free surface elevation:

$$K(t) := \frac{\rho}{2} \int_{-h}^{\eta} \iint_{\Omega} |\nabla \phi|^2 d\vec{x} dz, \qquad \Pi(t) := \frac{\rho g}{2} \iint_{\Omega} \eta^2 d\vec{x}.$$

The definition of the kinetic energy K involves an integral over the three dimensional physical domain  $\Omega \times [-h, \eta]$ . We can reduce the integral dimension using the fact that the velocity potential  $\phi$  is a harmonic function:

$$|\nabla \phi|^2 = \nabla \cdot (\phi \nabla \phi) - \phi \underbrace{\Delta \phi}_{=0} \equiv \nabla \cdot (\phi \nabla \phi).$$

Consequently, the kinetic energy can be rewritten as follows:

$$K(t) = \underbrace{\frac{\rho}{2} \iint_{S_f + S_b} \nabla \cdot (\phi \nabla \phi) \ d\sigma}_{S_f + S_b} = \underbrace{\frac{\rho}{2} \iint_{\Omega} \varphi \mathcal{D}_{\eta}(\varphi) \ d\vec{x}}_{(I)} + \underbrace{\frac{\rho}{2} \iint_{\Omega} \check{\phi} \partial_t h \ d\vec{x}}_{(II)},$$

where  $\check{\phi}$  denotes the trace of the velocity potential at the bottom  $\phi|_{z=-h}$  (see D. Clamond & D. Dutykh (2010), [CD10]). The first integral (I) is classical and represents the change of kinetic energy under the free surface motion while the second one (II) is the forcing term due to the bottom deformation. The total energy<sup>3</sup> is defined as the sum of kinetic and potential ones:

$$E(t) := K(t) + \Pi(t) = \frac{\rho}{2} \iint_{\Omega} \varphi \mathcal{D}_{\eta}(\varphi) \ d\vec{x} + \frac{\rho}{2} \iint_{\Omega} \check{\phi} \partial_{t} h \ d\vec{x} + \frac{\rho g}{2} \iint_{\Omega} \eta^{2} \ d\vec{x}.$$

Below we will compute the evolution of the kinetic, potential and total energies beneath moving bottom.

<sup>&</sup>lt;sup>3</sup>The total energy is normally a conserved quantity but in this study we have an external forcing term (II) coming from the bottom kinematic boundary condition.

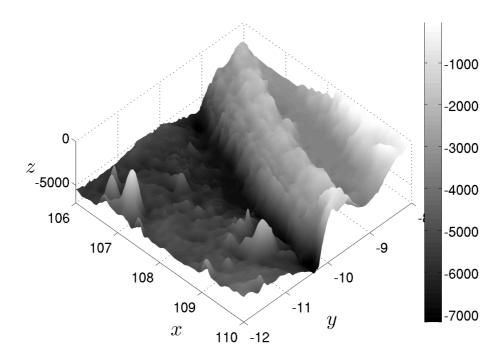


FIGURE 4. Side view of the bathymetry, cf. also Figure 1.

#### 3. Numerical results

The proposed approach will be directly illustrated on the Java 2006 event. The July 17, 2006 Java earthquake involved thrust faulting in the Java's trench and generated a tsunami wave that inundated the southern coast of Java [AKLV06, FKM<sup>+</sup>07]. The estimates of the size of the earthquake (cf. [AKLV06]) indicate a seismic moment of  $6.7 \times 10^{20} \ N \cdot m$ , which corresponds to the magnitude  $M_w = 7.8$ . Later this estimation was refined [Ji06] to  $M_w = 7.7$ . Like other events in this region, Java's event had an unusually low rupture speed of  $1.0 - 1.5 \ \text{km/s}$  (we take the value of  $1.1 \ \text{km/s}$  according to the finite fault solution [Ji06]), and occurred near the up-dip edge of the subduction zone thrust fault. According to C. Ammon et al [AKLV06], most aftershocks involved normal faulting. The rupture propagated approximately 200 km along the trench with an overall duration of approximately 185 s. The fault's surface projection along with ocean ETOPO1 bathymetric map are shown on Figures 1 and 4. We note that Indian Ocean's bathymetry considered in this study varies between 7186 and 20 meters in the shallowest region which may imply local importance of nonlinear effects.

**Remark 6.** We have to mention that the finite fault inversion for this earthquake was also performed by the Caltech team [OK06]. They estimated the July 17, Southern Java earthquake magnitude to be  $M_w = 7.9$ . In this study we do not present numerical simulations

Ocean water density, $\rho$ , $kg/m^3$	1027.0
Gravity acceleration, $g, m/s^2$	9.81
Epicenter location (Lon, Lat)	$(107.345^{\circ}, -9.295^{\circ})$
Rupture velocity, $v_r$ , $km/s$	1.1
Rise time, $t_0$ , $s$	8.0
Number of Fourier modes in $x$	256
Number of Fourier modes in $y$	256

Table 2. Physical and numerical parameters used in simulations.

involving their data but it is straightforward to apply the algorithms [DMG10] to this case as well.

The numerical solutions given by the Weakly Nonlinear (WN) model are computed on a uniform grid of  $256 \times 256$  points<sup>4</sup>. The time step  $\Delta t$  is chosen adaptively according to the RK(4,5) method proposed in [DP80]. The problem is integrated numerically during T=255 s which is a sufficient time interval for the bottom to take its final shape (< 220 s) and of the resulting tsunami wave to enter into the propagation stage. The values of various physical and numerical parameters used in simulations are given in Table 2.

We begin our numerical investigations by quantifying the contribution of horizontal displacements into the sea bed deformation process. For this purpose we consider the difference  $d_h(\vec{x})$  between the deformed bottom under the action of only horizontal displacements in their steady state  $(t \to +\infty)$  and the initial configuration:

$$d_h(\vec{x}) := \frac{h_0(\vec{x} + \vec{u}_{1,2}(\vec{x}, t)|_{t \to +\infty}) - h_0(\vec{x})}{\max_{\vec{x}, t \to +\infty} |u_3(\vec{x}, t)|}.$$

On Figure 5 we plot the quantitative effect of horizontal displacements relative to the maximum vertical displacement  $\max_{\vec{x},t\to+\infty}|u_3(\vec{x},t)|$ . The computations performed showed that the maximum amplitude of the bottom variation due to the action of horizontal displacements reaches 21% of the maximum amplitude of the vertical displacement. In practice it means that locally (depending on the bathymetry shape and the slip distribution) we cannot completely neglect the effect of horizontal motion.

The next step consists in quantifying the impact of horizontal displacements onto free surface motion. We put six numerical wave gauges at the following locations: (a)  $(107.2^{\circ}, -9.388^{\circ})$ , (b)  $(107.4^{\circ}, -9.205^{\circ})$ , (c)  $(107.6^{\circ}, -9.648^{\circ})$ , (d)  $(107.7^{\circ}, -9.411^{\circ})$ , (e)  $(108.3^{\circ}, -10.02^{\circ})$ , (f)  $(108.2^{\circ}, -9.75^{\circ})$ . The locations of the wave gauges are represented by the symbol  $\diamond$  on Figure 6 along with the static sea bed displacement. Wave gauges are intentionally put in places where the largest waves are expected.

Numerical wave gauge records are presented in Figure 7. It can be seen that in most cases horizontal displacements lead to an increase in wave amplitude but not necessarily as

<sup>&</sup>lt;sup>4</sup>Since we use a pseudo-spectral method, the convergence is expected to be exponential and this number of harmonics should be sufficient to capture all scales important for our study.

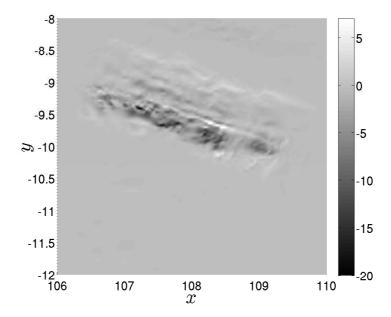


FIGURE 5. Contribution of horizontal displacements field into the sea bed deformation expressed in percentage of the maximum vertical displacement. The grey color corresponds to negligible values while white and black zones show most important contributions.

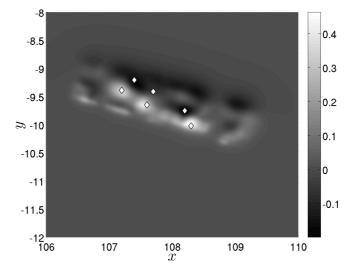


FIGURE 6. Location of the six numerical wave gauges (indicated by the symbol  $\diamond$ ) superposed with the steady state coseismic bottom displacement.

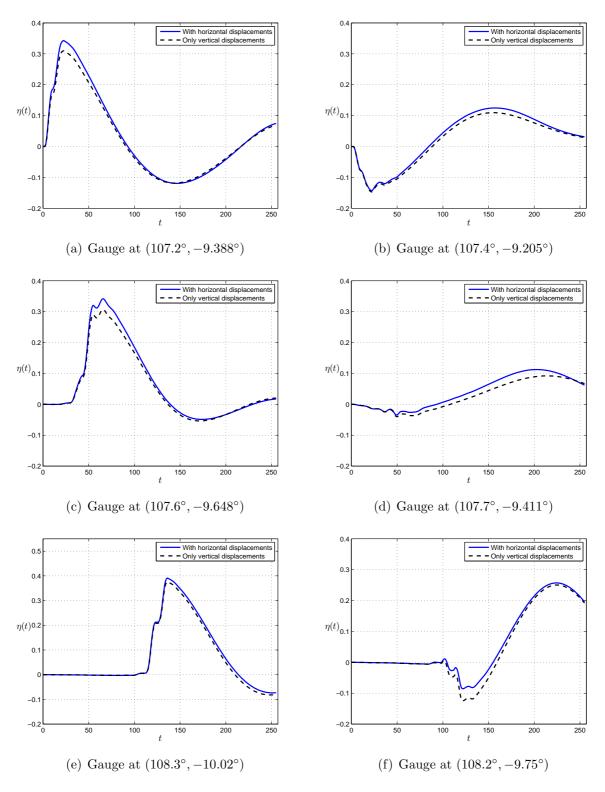


FIGURE 7. Free surface elevations computed numerically at six wave gauges located approximately in local extrema of the static bottom displacement. The vertical axis is represented in meters and time is given in seconds. The black dashed line corresponds to the wave generated only by vertical displacements (incomplete generation) while the blue solid line takes also into account the horizontal displacements contribution (complete scenario). Note a change in vertical scale in Figure (e).

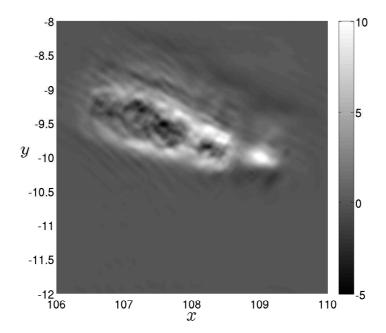


FIGURE 8. Relative difference between the free surface elevation at  $t_e = 220$  seconds computed according to the complete  $\eta_c(\vec{x}, t_e)$  (with horizontal displacements) and incomplete  $\eta_i(\vec{x}, t_e)$  (only vertical component) tsunami generation scenarios. The vertical scale is given in percents of the maximum amplitude of the incomplete scenario —  $\max_{\vec{x}} \eta_i(\vec{x}, t_e)$ .

it is shown in Figure 7(f) (the last wave gauge located at (108.2°, -9.75°)). We investigated more thoroughly this question. Figure 8 shows the relative difference between free surface elevations computed according to complete  $\eta_h(\vec{x}, t_e)$  and incomplete  $\eta(\vec{x}, t_e)$  scenarios. More precisely, we plot the following quantity:

$$d(\vec{x}) := \frac{\eta_h(\vec{x}, t_e) - \eta(\vec{x}, t_e)}{\max_{\vec{x}} |\eta(\vec{x}, t_e)|}, \qquad t_e = 220 \ s.$$

The time  $t_e = 220$  s is chosen just when the bottom motion finishes and the wave enters in the free propagation regime. In Figure 8 the grey color corresponds to the zero value of the difference  $d(\vec{x})$ , while the white color shows regions where the wave is amplified by horizontal displacements by approximately 10%. On the contrary, black zones show an attenuation effect of horizontal sea bed motion (about -5%). Recall that all values are given in terms of the maximum amplitude  $\max_{\vec{x}} |\eta(\vec{x}, t_e)|$  percentage of the incomplete generation approach. Some correlation with the results presented in Figure 5 can be noticed.

Finally, we study the evolution of kinetic and potential energies during the tsunami generation process described in Section 2.2.1. Specifically we are interested in the contribution

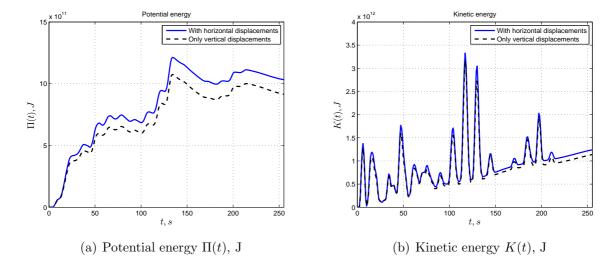


FIGURE 9. Energy evolution during our simulations in complete (blue solid line) and incomplete (black dotted line) scenarios. Note that scales are different on left and right images.

of horizontal displacements into tsunami energy balance. Figure 9 shows the evolution of potential (9(a)) and kinetic (9(b)) energies in complete (blue solid line) and incomplete (black dashed line) scenarios. Consecutive peaks in the kinetic energy come from activation of new fault segments in accordance with the rupture propagation along the fault.

Despite some local attenuation effects of horizontal displacements on the free surface elevation, the complete generation scenario produces a tsunami wave with more important energy content. Namely, at the end of the simulation  $(T=255~\mathrm{s})$  the total energy  $\Pi(T)+K(T)$  computed according to the complete scenario is equal to  $2.27\times10^{12}~\mathrm{J}$ . The incomplete scenario leads to the total energy of  $2.05\times10^{12}~\mathrm{J}$ . Consequently, horizontal displacements in our particular case of July 17, Java 2006 event contribute approximately 10% into the total tsunami energy value. This value is consistent with our previous results concerning the differences in wave amplitudes (see Figure 8).

It is also interesting to compare the tsunami energy with energy of the underlying seismic event. USGS Energy and Broadband solution indicates that the radiated seismic energy of the Java 2006 earthquake is equal to  $3.2 \times 10^{14}$  J. Hence, according to our computations with the complete generation approach, about 0.71% of the seismic energy was transmitted to the tsunami wave. The ratio of the total tsunami and seismic radiated energies can be used as a measure of the tsunami generation efficiency of a specific earthquake.

This parameter can be also estimated for the great Sumatra-Andaman earthquake of December 26, 2004. The total energy of the Great Indian Ocean tsunami 2004 was estimated by T. Lay et al. (2005) [LKA+05] to be equal to  $4.2 \times 10^{15}$  J. According to the same USGS Energy and Broadband solution the radiated seismic energy was equal to  $1.1 \times 10^{17}$  J. Thus, the tsunamigenic efficiency of the Great Sumatra-Andaman earthquake is 3.8% which is bigger than that of Java 2006 event but remains in the same order of magnitude

around 1%. It is probable that we do not still take into account all important factors in Java 2006 tsunami genesis.

#### 4. Discussion and conclusions

In the present study the process of tsunami generation is further investigated. Main ingredients have already been presented in our previous study [DMG10]. Namely, the current tsunami genesis model relies on a combination of the finite fault solution [JWH02] and recently proposed Weakly Nonlinear (WN) solver for the water wave problem with moving bottom [DMG10]. Consequently, in our model we incorporate recent advances in seismology and computational hydrodynamics.

This study is focused on the role of the horizontal displacements in the real world tsunami genesis process. By our intuition we know that a horizontal motion of the flat bottom will not cause any significant disturbance on the free surface. However, the real bathymetry is far from being flat. The question which arises naturally is how to quantify the effect of horizontal coseismic displacements during real world events?

The primary goal of this study was to propose relatively simple, efficient and accurate procedures to model tsunami generation process in realistic environments. Special emphasis was payed to the role of horizontal displacements which should be also taken into account. Thus, the dynamics of horizontal coseismic displacements were reconstructed and their effect on free surface motion was quantified. The evolution of kinetic and potential energies were also investigated in our study. In the case of July 17, 2006 Java event our simulations indicate that 10% of the energy input can be seemingly ascribed to effects of the horizontal bottom motion.

The results presented in this study does not provide any evidence for the extreme runup values caused by July 17, 2006 Java tsunami [FKM<sup>+</sup>07], since but illustrate the proposed methodology on this important real world event. In our opinion the successful theory should incorporate various generation mechanisms.

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