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On the derivation of structural models with general thermomechanical prestress

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Abstract

The vibrating behaviour of thin structures is affected by prestress states. Hence, the effects of thermal prestress are important research subjects in view of ambient vibration monitoring of civil structures. The interaction between prestress, geometrically non-linear behaviour, as well as damping and its coupling with the aforementioned phenomena has to be taken into account for a comprehensive understanding of the structural behaviour. Since the literature on this subject lacks a clear procedure to derive models of thin prestressed and damped structures from 3D continuum mechanics, this paper presents a new derivation of models for thin structures accounting for generic prestress, moderate rotations and viscous damping. Although inspired by classical approaches, the proposed procedure is quite different, because of (i) the definition of a modified Hu-Washizu (H-W) functional, accounting for stress constraints associated with Lagrange multipliers, in order to derive lower-dimensional models in a convenient way; (ii) an original definition of a (mechanical and thermal) strain measure and a rotation measure enabling one to identify the main terms in the strain energy and to derive a cascade of lower-dimensional models (iii) a new definition of "strain-rotation domains" providing a clear interpretation of the classical assumptions of "small perturbations" and "small strains and moderate rotations"; (iv) the introduction of a pseudo-potential with stress constraints to account for viscous damping. The proposed procedure is applied to thin beams.

Key words: prestress state, thermal effects, small strains and moderate rotations, geometric non-linearity, damping

PACS:

1 Introduction

Ambient vibration monitoring (*e.g.* Wenzel and Pichler (2005), Basseville et al. (2004)) has now become widely accepted as an important tool for Structural Health Monitoring (SHM). But structural vibrations are affected by prestress states. In particular, thermal variations may cause very significant changes in a bridge spectrum (Peeters and De Roeck, 2001; Farrar et al., 1994). Since temperature effects may be orders of magnitude larger than the effect of a damage, overlooking them prevents from a reliable damage detection based on vibration monitoring. Most attempts to eliminate thermal effects have favoured blind approaches not taking advantage of predictive models (Sohn et al., 2003). However, successful endeavours to eliminate the temperature from subspace-based damage detection algorithms prove the relevance of relying on predictive thermomechanical models yielding the prestress state due to temperature (Nasser, 2006; Basseville et al., 2006). This suggests to deeper understand the way temperature interacts with structural dynamics and to revisit associated models. This paper steps forward in this direction.

On the other hand, identification of the geometrically non-linear behaviour of beams or plates has recently given rise to numerous papers (*e.g.* (Kerschen et al., 2003), (Argoul and Le, 2003), (Perignon et al., 2003)), often using the notion of non-linear modes (Rosenberg, 1962) (Vakakis, 1997). Non-linear dynamics of prestressed beams or plates undergoing small strains and moderate rotations is simulated in (Ribeiro, 2001), (Perignon et al., 2003), (Amabili, 2005). However, the dynamics used in these contributions seems to be based on some "historical" assumptions, whose justification is often skipped.

The purpose of this paper is then to provide a new viewpoint on this classical subject, where a very large number of contributions, sometimes not clearly related, have been superposed over the years. We make clear the series of simplifying assumptions leading from the 3D continuous thermo-elasticity theory to the equations governing the dynamics of thin structures with prestress and thermal field, under the assumption of "small strains and moderate rotations". Attention is paid to the definition of the range of validity of these classical equations, introducing the notion of strain-rotation domains. More precisely: (i) a Hu-Washizu functional including suitable stress constraints and the associated Lagrange multipliers is defined: the constitutive law, the strain-displacement relationship as well as the dynamic equilibrium equations are then derived by imposing the stationarity conditions. Stress constraints characterizing thin body theories are introduced in the 3D models in view of a

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convenient derivation of 1D beam models. **(ii)** The most important configurations characterizing the structural behaviour are clearly identified. **(iii)** The assumption that the *prestressed* configuration has a known geometry, adopted as *Lagrangian reference* configuration (see *e.g.* (Gérardin and Rixen, 1995)), is removed. When the pre-stress field and the original geometry of the structure are very simple, this hypothesis is convenient. However, for more general conditions, *e.g.* when the prestress state is time-dependent or related to a general thermal field, this assumption appears to be over-simplified. Hence, a different analysis, where prestressed and Lagrangian reference configurations are *distinct*, seems to be more suitable. The equilibrium equation governing the dynamics of a structure subjected to a generic prestress is defined as the *difference* between the Lagrangian equations at the dynamic configuration and at the statically prestressed configuration. **(iv)** A rigorous formalization of the assumption of "small strain and moderate rotations" is provided. To this end, measures of the strain amplitude (symmetric part of the displacement gradient) and of the rotations (skew-symmetric part) are introduced in the 3D continuous framework, whereas standard approaches use the aspect ratio as the governing parameter (*e.g.* (Ciarlet, 1980)). The new solution-dependent measures enables one to define strain-rotation domains in which the leading terms of the strain energy are clearly identified.

Damping, due to internal friction or other dissipative phenomena, should carefully be taken into account in the structural analysis. Several damping models exist, like general linear damping or viscous proportional or non-proportional linear damping; see *e.g.* (Adhikari Woodhouse, 2001). However, the link between these structural damping models and the corresponding dissipative material behaviour, described by a given strain-stress law, does not seem to be clearly established in the literature. Here the beam dynamic equations with linear viscous damping are derived for the case of "small strains and moderate rotations", thus filling the gap.

The different configurations characterizing a vibrating structure subjected to static and dynamic loads and to a thermal field are defined in Section 2. Then, the thermo-elastic constitutive rule is presented in Section 3. Physical linearization is considered. Section 4 introduces a modified Hu-Washizu functional accounting for stress constraints. The corresponding stationarity conditions are discussed. In Section 5, two global measures for the strains and the rotations are introduced and used to suggest approximated expressions of the strain energy, each approximation being valid in a strain-rotation domain. 2D finite element simulations substantiate the approximation of the strain energy. Section 6 introduces a dissipative stress tensor in view of modelling damping effect. Section 7 leads to a general expression of the weak equilibrium of a continuum subjected to a general static prestress. In Section 8 the previous general procedure is used to derive the Euler-Bernoulli beam equations for small strains and moderate rotations in the undamped case, while

the damped case is treated in Section 9. After the Conclusions, the Appendix explains how to compute the Lagrange multipliers.

2 Configurations of a structure

The following configurations (see Figure 1 and Table 1) may be distinguished:

- (1) V_{00} is the *geometric reference configuration*, where the *displacements are assumed to vanish*. For a beam, V_{00} is the straight configuration. Let \mathbf{X} be the reference position of a material point.
- (2) V_{0i} is the initial configuration of the body, where the temperature field T_{0i} is constant and where no external force is applied. This configuration is relevant in the case of geometric imperfections, where V_{0i} differs from V_{00} . In this paper, however, we assume $V_{0i} = V_{00}$, *i.e.* $\mathbf{U}_{0i} = \mathbf{x}_{0i} - \mathbf{X} = \mathbf{0}$. The stress may not vanish since a self-equilibrated stress $\mathbf{\Pi}_{0i}$ may exist.
- (3) V_0 is the equilibrium configuration under *static external* loads \mathbf{f}_0 , \mathbf{g}_0 and a temperature field T_0 resulting in a displacement \mathbf{U}_0 , a relative displacement between V_{0i} and V_0 equal to $\mathbf{U}_0 - \mathbf{U}_{0i}$ and a stress field $\mathbf{\Pi}_0 - \mathbf{\Pi}_{0i}$.
- (4) V_1 is the instantaneous configuration of a vibrating structure subjected to the static loads \mathbf{f}_0 , \mathbf{g}_0 , to the temperature field T_0 and to *dynamic* volume and surface forces \mathbf{f}_1 and \mathbf{g}_1 . The relative displacement field between V_0 and V_1 is indicated by $\mathbf{U}_{01} = \mathbf{U}_1 - \mathbf{U}_0 = \mathbf{x}_1 - \mathbf{x}_0$. In practical situations \mathbf{f}_0 , \mathbf{g}_0 and T_0 may vary with time, *e.g.* on a daily period, but their variations are supposed to be very slow with respect to free vibrations of the structure. *This decomposition of a given external force into a static part and dynamic part* is convenient but *not unique*, *e.g.* it may depend on the time interval considered if two different static loads are successively imposed. Therefore, the total *response* of the structure does not split uniquely in a static component and a dynamic response.

3 Constitutive law and physical linearization

Based the objectivity principle, (see *e.g.* Mandel (1966, p. 602)), the Helmholtz free energy Ψ can be defined as a function of the Green-Lagrange strain tensor

$$\mathbf{E}(\mathbf{U}) = \frac{1}{2} \left(\frac{\partial \mathbf{U}}{\partial \mathbf{X}} + \left(\frac{\partial \mathbf{U}}{\partial \mathbf{X}} \right)^T \right) + \frac{1}{2} \left(\frac{\partial \mathbf{U}}{\partial \mathbf{X}} \right)^T \cdot \left(\frac{\partial \mathbf{U}}{\partial \mathbf{X}} \right) = \nabla^s(\mathbf{U}) + \frac{1}{2} \nabla(\mathbf{U})^T \cdot \nabla(\mathbf{U}) \quad (1)$$

and of the temperature T , *viz.* $\Psi = \Psi(\mathbf{E}, T)$. As usual, \cdot indicates the dot product and \mathbf{U} is the displacement measured with respect to the given reference configuration V_{00} . When $\|\mathbf{E}\| \ll 1$ and the temperature variations are small,

$\Psi(\mathbf{E}, T)$ can be approximated by a truncated series expansion. Truncating at the second order around the given initial configuration V_{00} , characterized by $\mathbf{E} = \mathbf{0}$ and $T = T_{0i}$ (physical linearization) leads to

$$\begin{aligned} \Psi(\mathbf{E}, T) = & \Pi_{0i} : \mathbf{E} + \frac{1}{2} \mathbf{E} : \mathbf{D} : \mathbf{E} - \mathbf{E} : \mathbf{A} (T - T_{0i}) \\ & - s_{0i} (T - T_{0i}) - \frac{1}{2} \frac{c_\varepsilon}{T_{0i}} (T - T_{0i})^2 \end{aligned} \quad (2)$$

where $:$ denotes the doubly contracted inner product, \mathbf{D} the fourth order tensor of the elastic constants, $T - T_{0i}$ the temperature variation and \mathbf{A} a diagonal second order tensor accounting for thermal expansion, s_{0i} the initial volume entropy and c_ε the specific volume heat [$J m^{-3} K^{-1}$]. When a *St. Venant-Kirchhoff* material is considered, one has

$$\mathbf{D} = \lambda \mathbf{1} \otimes \mathbf{1} + 2\mu \mathbf{I} \quad \text{and} \quad \mathbf{A} = \alpha (3\lambda + 2\mu) \mathbf{1} \quad (3)$$

where \otimes is the outer tensor product; $\mathbf{1}$ is the second order identity tensor; \mathbf{I} is the fourth order identity tensor; λ and μ are the Lamé coefficients, α is the thermal dilation coefficient. As it is well-known, the following identities hold:

$$\lambda = \frac{E\nu}{(1+\nu)(1-2\nu)}, \quad \mu = \frac{E}{2(1+\nu)}, \quad (4)$$

where E is the Young modulus, ν is the Poisson ratio. In this case, the Helmholtz energy (2) becomes

$$\begin{aligned} \Psi(\mathbf{E}, T) = & \Pi_{0i} : \mathbf{E} + \frac{1}{2} \lambda (tr(\mathbf{E}))^2 + \mu \mathbf{E} : \mathbf{E} - \alpha (3\lambda + 2\mu) (T - T_{0i}) tr(\mathbf{E}) \\ & - s_{0i} (T - T_{0i}) - \frac{1}{2} \frac{c_\varepsilon}{T_{0i}} (T - T_{0i})^2 \end{aligned} \quad (5)$$

Eq. (2) leads to a constitutive law linear with respect to \mathbf{E} and $T - T_{0i}$:

$$\mathbf{\Pi} = \frac{\partial \Psi}{\partial \mathbf{E}} = \mathbf{\Pi}^{nd} = \Pi_{0i} + \mathbf{D} : \mathbf{E} - \mathbf{A} (T - T_{0i}) \quad (6)$$

where $\mathbf{\Pi}$ is the (*second*) *Piola-Kirchhoff symmetric stress tensor*, the index *nd* means non-dissipative. Eq. (6) also reads

$$\mathbf{\Pi} = \mathbf{\Pi}^{nd} = \Pi_{0i} + \lambda tr(\mathbf{E}) \mathbf{1} + 2\mu (\mathbf{E}) - \alpha (3\lambda + 2\mu) (T - T_{0i}) \mathbf{1} \quad (7)$$

For isotropic materials having elastic non-linear constitutive behaviour, a direct generalization of (7) can be defined. It is called *second order elasticity*

(Mandel, 1966, p. 607), supplemented here by the self-equilibrated stress $\mathbf{\Pi}_{0i}$:

$$\begin{aligned} \mathbf{\Pi} = \mathbf{\Pi}^{nd} = \mathbf{\Pi}_{0i} &+ \left[\lambda \operatorname{tr}(\mathbf{E}) + \frac{A}{2} \operatorname{tr}(\mathbf{E}^2) + 3B (\operatorname{tr}(\mathbf{E}))^2 - \alpha (3\lambda + 2\mu) (T - T_{0i}) \right] \mathbf{1} \\ &+ \left[\lambda' \operatorname{tr}(\mathbf{E}) (T - T_{0i}) + a' (T - T_{0i})^2 \right] \mathbf{1} + 2\mu \mathbf{E} + A \operatorname{tr}(\mathbf{E}) \mathbf{E} + 2\mu' \mathbf{E} (T - T_{0i}) + C \mathbf{E} \cdot \mathbf{E} \end{aligned} \quad (8)$$

where A, B, C, λ', μ' and a' are the material parameters introduced in addition to the usual ones λ, μ and α .

Remark: the temperature variations can be considered small when

$$\left\| \mathbf{D}^{-1} : \mathbf{A} (T - T_{0i}) \right\| \ll 1 \quad (9)$$

viz. when strains associated with them are small with respect to unity, similarly to strains fulfilling condition $\|\mathbf{E}\| \ll 1$. For isotropic materials, the temperature variations are small if $|\alpha (T - T_{0i})| \ll 1$. This condition is consistent with the assumption of physical linearization.

4 A Hu-Washizu functional with additional stress constraints

In this section, a special representation of the problem at hand is introduced, in view of taking into account stress constraints in a three-dimensional framework without forgetting about compatibility issues. A Hu-Washizu (H-W) functional depending on the stress $\mathbf{\Pi}$, the strain measure $\bar{\mathbf{E}}$ and the displacement \mathbf{U} , considered independent is supplemented with constraints on the stress, in order to model thin bodies. Let \mathbb{U} , \mathbb{T} and \mathbb{F} denote spaces of smooth enough displacement fields, of symmetric second order tensor fields and scalar fields of Lagrange multipliers, respectively. The proposed H-W functional reads

$$\begin{aligned} J_{H-W}(\mathbf{\Pi}^*, \bar{\mathbf{E}}^*, \mathbf{U}^*, \mathbf{\Lambda}^*) &= \int_{V_{00}} \Psi(\bar{\mathbf{E}}^*, T) dV - \int_{V_{00}} \mathbf{\Pi} : (\bar{\mathbf{E}}^* - \mathbf{E}(\mathbf{U}^*)) dV \\ &- \int_{V_{00}} \mathbf{f} \cdot \mathbf{U}^* dV - \int_{\partial V_{00,\sigma}} \mathbf{g} \cdot \mathbf{U}^* dA - \int_{\partial V_{00,u}} [(\mathbf{1} + \nabla(\mathbf{U}^*)) \cdot \mathbf{\Pi}^*] \cdot \mathbf{N} \cdot (\mathbf{U}^* - \bar{\mathbf{U}}) dA \\ &- \int_{V_{00}} \sum_{k=1}^{n_\Lambda} \Lambda_k^* \mathbf{R}_k : \mathbf{\Pi}^* dV \end{aligned} \quad (10)$$

with $\mathbf{\Pi}^* \in \mathbb{T}$, $\bar{\mathbf{E}}^* \in \mathbb{T}$, $\mathbf{U}^* \in \mathbb{U}$ and $\mathbf{\Lambda}^* \in \mathbb{F}^{n_\Lambda}$, where n_Λ is the number of stress constraints and $\bar{\mathbf{U}}$ is the given displacement on the boundary $\partial V_{00,u} = \partial V_{00} - \partial V_{00,\sigma}$. Note that J_{H-W} is a functional defined on $3 + n_\Lambda$ fields. At a solution $(\mathbf{\Pi}, \bar{\mathbf{E}}, \mathbf{U}, \mathbf{\Lambda})$ of the static problem, the functional above satisfies the stationarity with respect to the $3 + n_\Lambda$ fields. The last term leads to n_Λ linear stress constraints:

$$\mathbf{R}_k : \mathbf{\Pi} := (\mathbf{N}_k \otimes \mathbf{N}_k) : \mathbf{\Pi} = (\mathbf{\Pi} \cdot \mathbf{N}_k) \cdot \mathbf{N}_k = 0 \quad (11)$$

where \mathbf{N}_k is a vector and \mathbf{R}_k is a second order constant tensor. Eq. (11) imposed on $\mathbf{\Pi}$ carries over to an equivalent condition on the Cauchy stress $\boldsymbol{\sigma}$. The use of this kind of constraints to derive beam equations from 3D elasticity is illustrated in Section 7. The case of a general 3D structure with no constraint can be addressed by formally setting $\Lambda_k = 0$ and $\mathbf{R}_k = \mathbf{0}$. The well-known impossibility to derive beam or plate equations from purely kinematical assumptions in the three-dimensional equations of elasticity in pure displacement motivate the introduction of internal constraints as in (Nardinocchi and P. Podio Guidugli, 1994), (Lembo and Podio Guidugli, 2001). These constraints enable one to use a pure displacement approach while mimicking the averaging process underlying the convergence of the equations of elasticity when the aspect ratio tends to zero. The derivation of lower-dimensional models without assumptions can rely on Γ -convergence (Acerbi Buttazzo, 1986), asymptotic analysis (Ciarlet, 1980), or energy methods (Babuska et al., 1992). This paper aims at deriving the equations governing the evolution of thin structures subject to prestress states without above mathematical apparatus.

4.1 Action functional and stationarity conditions

While the stationarity of the H-W functional leads to statics, the action functional leads to elastodynamics over a time interval $[0, t_f]$. Let us set $\mathbb{V} = \{\mathbf{U}(\mathbf{X}, t) : \mathbf{U}(\cdot, t) \in \mathbb{U} \ \forall t \in [0, t_f]\}$. The kinetic energy $\mathcal{T}(\dot{\mathbf{U}}^*) = \frac{1}{2} \int_{V_{00}} \rho_{00} \dot{\mathbf{U}}^* \cdot \dot{\mathbf{U}}^* dV$, combined with the H-W functional enables one to define the action

$$\mathcal{D}(\mathbf{\Pi}^*, \bar{\mathbf{E}}^*, \mathbf{U}^*, \boldsymbol{\Lambda}^*) = \int_0^{t_f} (\mathcal{T}(\dot{\mathbf{U}}^*) - J_{H-W}(\mathbf{\Pi}^*, \bar{\mathbf{E}}^*, \mathbf{U}^*, \boldsymbol{\Lambda}^*)) dt \quad (12)$$

Hamilton's principle is given by $\delta\mathcal{D} = 0$, where small variations $(\delta\mathbf{\Pi}, \delta\bar{\mathbf{E}}, \delta\mathbf{U}, \delta\boldsymbol{\Lambda}) \in \mathbb{T} \times \mathbb{T} \times \mathbb{V} \times \mathbb{F}^{n_\Lambda}$ are considered. The stationarity operator $\delta(\cdot)$ is intended as *isochronous*, viz. $\delta t = 0$ and $\delta \int_0^{t_f} \mathcal{L} dt = \int_0^{t_f} \delta \mathcal{L} dt$ (see also Quadrelli and Atluri (1999)). Hamilton's principle leads to the stationarity conditions

$$\begin{aligned} 1. \quad & \forall k = 1, n_\Lambda \quad \mathbf{R}_k : \mathbf{\Pi} = 0 \quad \text{in } V_{00} \\ 2. \quad & \begin{cases} \bar{\mathbf{E}} = \mathbf{E}(\mathbf{U}) - \sum_{k=1}^{n_\Lambda} \Lambda_k \mathbf{R}_k & \text{in } V_{00} \\ \mathbf{U} = \bar{\mathbf{U}} & \text{on } \partial V_{00,u} \end{cases} \quad (13) \\ 3. \quad & \mathbf{\Pi} = \left. \frac{\partial \Psi(\bar{\mathbf{E}}^*, T)}{\partial \mathbf{E}^*} \right|_{\bar{\mathbf{E}}^* = \bar{\mathbf{E}}} \quad \text{in } V_{00} \end{aligned}$$

In the physically linear case, Eq. (13-3) corresponds to (6). If isotropy is assumed, Eq. (13-3) is equal to (7). The first equation expresses the stress con-

straints, the second one defines the strain $\bar{\mathbf{E}}$, which differs from $\mathbf{E}(\mathbf{U})$ due to the stress constraints. Moreover, $\mathbf{\Pi}$ depends on the strain $\bar{\mathbf{E}}$ accounting for the Lagrange multipliers and not on the standard strain measure $\mathbf{E}(\mathbf{U})$. The stationarity condition of the action \mathcal{D} with respect to displacements leads to

$$\begin{aligned} \forall \delta \mathbf{U} \in \mathbb{V}, \quad & \int_0^{t_f} \left[\int_{V_{00}} \mathbf{\Pi} : \delta \mathbf{E}(\mathbf{U}, \delta \mathbf{U}) dV - \int_{V_{00}} \mathbf{f} \cdot \delta \mathbf{U} dV - \int_{\partial V_{00, \sigma}} \mathbf{g} \cdot \delta \mathbf{U} dA \right] dt \\ & - \int_0^{t_f} \left[\int_{\partial V_{00, u}} \left([(\mathbf{1} + \nabla(\mathbf{U})) \cdot \mathbf{\Pi}] \cdot \mathbf{N} \right) \cdot \delta \mathbf{U} dA + \int_{V_{00}} \rho_{00} \dot{\mathbf{U}} \cdot \delta \dot{\mathbf{U}} dV \right] dt = 0 \end{aligned} \quad (14)$$

where it has been used the identity $\mathbf{U} = \bar{\mathbf{U}}$ on $\partial V_{00, u}$, given in (13). Moreover, the virtual strain is defined by

$$\begin{aligned} \delta \mathbf{E}(\mathbf{U}, \delta \mathbf{U}) &= \left. \frac{\partial \mathbf{E}(\mathbf{U}^*)}{\partial \mathbf{U}^*} \right|_{\mathbf{U}^* = \mathbf{U}} \cdot \delta \mathbf{U} \\ &= \nabla^s(\delta \mathbf{U}) + \frac{1}{2} \nabla(\delta \mathbf{U})^T \cdot \nabla(\mathbf{U}) + \frac{1}{2} \nabla(\mathbf{U})^T \cdot \nabla(\delta \mathbf{U}) \end{aligned} \quad (15)$$

Integrating by parts in time leads to the virtual works principle at every t

$$\mathcal{W}_i(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) + \mathcal{W}_e(\mathbf{f}, \mathbf{g}, \delta \mathbf{U}) = \mathcal{W}_a(\ddot{\mathbf{U}}, \delta \mathbf{U}) \quad \forall \delta \mathbf{U} \in \mathbb{V} \quad (16)$$

where

$$\begin{aligned} \mathcal{W}_i(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &:= - \int_{V_{00}} \mathbf{\Pi} : \delta \mathbf{E}(\mathbf{U}, \delta \mathbf{U}) dV, \quad \mathcal{W}_a(\ddot{\mathbf{U}}, \delta \mathbf{U}) := \int_{V_{00}} \rho_{00} \ddot{\mathbf{U}} \cdot \delta \mathbf{U} dV \\ \mathcal{W}_e(\mathbf{f}, \mathbf{g}, \delta \mathbf{U}) &:= \int_{V_{00}} \mathbf{f} \cdot \delta \mathbf{U} dV + \int_{\partial V_{00, \sigma}} \mathbf{g} \cdot \delta \mathbf{U} dA + \int_{\partial V_{00, u}} \left([(\mathbf{1} + \nabla(\mathbf{U})) \cdot \mathbf{\Pi}] \cdot \mathbf{N} \right) \cdot \delta \mathbf{U} dA \end{aligned} \quad (17)$$

denote the virtual work of internal, inertia and external forces, respectively. Finally, by integration by parts, one obtains the corresponding strong form equation and the boundary conditions on $\partial V_{00, \sigma}$:

$$4. \quad \begin{cases} \operatorname{div} \left((\mathbf{1} + \nabla(\mathbf{U})) \cdot \mathbf{\Pi} \right) + \mathbf{f} = \rho_{00} \ddot{\mathbf{U}} & \text{in } V_{00} \\ (\mathbf{1} + \nabla(\mathbf{U})) \cdot \mathbf{\Pi} \cdot \mathbf{N} = \mathbf{g} & \text{on } \partial V_{00, \sigma} \end{cases} \quad (18)$$

with the initial conditions $\mathbf{U}(t=0) = \tilde{\mathbf{U}}$ and $\dot{\mathbf{U}}(t=0) = \tilde{\mathbf{V}}$ in V_{00} .

5 Physical linearization, strain-rotation domains and dominant terms in the strain energy

Let \mathbf{U} , $\bar{\mathbf{E}}$, $\mathbf{\Pi}$ and Λ_k denote the fields defining the equilibrium at time t . The strain energy density associated with the strain field $\bar{\mathbf{E}} = \bar{\mathbf{E}}(t)$ reads

$$\mathcal{F}(\bar{\mathbf{E}}, T) = \int_{V_{00}} \Psi(\bar{\mathbf{E}}, T) dV \quad (19)$$

where $\Psi(\bar{\mathbf{E}}, T)$ is given by (5). The displacement gradient writes

$$\nabla(\mathbf{U}) = \nabla^s(\mathbf{U}) + \nabla^{sk}(\mathbf{U}) := \boldsymbol{\varepsilon}(\mathbf{U}) + \mathbf{r}(\mathbf{U}) = \boldsymbol{\varepsilon} + \mathbf{r} \quad (20)$$

where the symmetric tensor $\boldsymbol{\varepsilon}$ is called *small strain tensor*, while the skew-symmetric part \mathbf{r} is called the *small rotation tensor*. For brevity, hereinafter they will be referred to as *strain tensor* and *rotation tensor*. In the same way, the strain increment (15) writes

$$\delta\mathbf{E}(\mathbf{U}, \delta\mathbf{U}) = \nabla^s(\delta\mathbf{U}) + \frac{1}{2}\nabla(\delta\mathbf{U})^T \cdot (\boldsymbol{\varepsilon} + \mathbf{r}) + \frac{1}{2}(\boldsymbol{\varepsilon} + \mathbf{r})^T \cdot \nabla(\delta\mathbf{U}) \quad (21)$$

5.1 A global measure for strains and for rotations and a hierarchy for the strain energy terms

In view of finding out the dominant terms in (19), let us introduce the solution-dependent scalar parameters η and p as follows:

$$\eta = \eta_\varepsilon + \eta_{\Delta T}, \quad \eta_\varepsilon = \frac{1}{\sqrt{V_{00}}} \|\boldsymbol{\varepsilon}\|_{L_2(V_{00})}, \quad \eta_{\Delta T} = \frac{1}{\sqrt{V_{00}}} \|\alpha(T - T_{0i}) \mathbf{1}\|_{L_2(V_{00})} \quad (22)$$

$$\eta^p := \frac{1}{\sqrt{2}} \frac{1}{\sqrt{V_{00}}} \|\mathbf{r}\|_{L_2(V_{00})} = \left(\frac{1}{V_{00}} \int_{V_{00}} \frac{1}{2} r_{ij} r_{ij} dV \right)^{\frac{1}{2}}$$

where $\|\cdot\|_{L_2(V_{00})}$ is the L_2 -norm associated with the domain V_{00} . The strain measure η is supposed to be small. Observe that: (i) p is only defined whenever $\eta > 0$. Thus infinitesimal rigid body motions at initial temperature are excluded from the following analysis; (ii) If the rotations are not large, \mathbf{r} is a rotation tensor and the quantity η^p is a global measure of the rotations. Hence, p can be interpreted as a global index of the relative amplitude of rotations with respect to strains. As a result (see Eq. (1))

$$\mathbf{E}(\mathbf{U}) = O(\eta) + O(\eta^{2p}) + O(\eta^{1+p}) + O(\eta^2) \quad (23)$$

Under dynamic conditions and provided that the vibration frequency is bounded, one also has $\left\| \frac{d\boldsymbol{\varepsilon}}{dt} \right\|_{L_2(V_{00})} = O(\eta)$ and $\left\| \frac{d\mathbf{r}}{dt} \right\|_{L_2(V_{00})} = O(\eta^p)$. In addition, let us make a consistent hypothesis on the self-equilibrated prestress $\boldsymbol{\Pi}_{0i}$:

$$\boldsymbol{\Pi}_{0i} = \boldsymbol{\Pi}_{0i}^{(1)} + \boldsymbol{\Pi}_{0i}^{(2p)} + \boldsymbol{\Pi}_{0i}^{(2)} + \boldsymbol{\Pi}_{0i}^{(1+p)} = O(\eta) + O(\eta^{2p}) + O(\eta^{1+p}) + O(\eta^2) \quad (24)$$

Since the Lagrange multipliers Λ_k fulfill the relationship for all $k = 1, n_\Lambda$

$$\mathbf{R}_k : \boldsymbol{\Pi} = \mathbf{R}_k : \left[\boldsymbol{\Pi}_{0i} + \mathbf{D} : \left(\mathbf{E}(\mathbf{U}) - \sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l \right) - \mathbf{A} (T - T_{0i}) \right] = 0 \quad (25)$$

then, provided the stress constraints \mathbf{R}_k are linearly independent, $\boldsymbol{\Lambda} = (\Lambda_k)$, $k = 1, n_\Lambda$, is the solution of a linear Gram system and splits as follows (see Appendix):

$$\boldsymbol{\Lambda} = \boldsymbol{\Lambda}^{(1)} + \boldsymbol{\Lambda}^{(2p)} + \boldsymbol{\Lambda}^{(1+p)} + \boldsymbol{\Lambda}^{(2)} = O(\eta) + O(\eta^{2p}) + O(\eta^{1+p}) + O(\eta^2) \quad (26)$$

where $\boldsymbol{\Lambda}^{(1)}$, $\boldsymbol{\Lambda}^{(2p)}$, $\boldsymbol{\Lambda}^{(1+p)}$ and $\boldsymbol{\Lambda}^{(2)}$ can be evaluated analytically in simple cases. This implies that the Lagrange multipliers, $\bar{\mathbf{E}}$ (Eq. (13-2)) and $\mathbf{E}(\mathbf{U})$ split in terms of the same order.

From Eqs. (13) and (26), the stress splits in four terms:

$$\boldsymbol{\Pi} = \boldsymbol{\Pi}^{(phys.lin.)} = \boldsymbol{\Pi}^{(1)} + \boldsymbol{\Pi}^{(2p)} + \boldsymbol{\Pi}^{(1+p)} + \boldsymbol{\Pi}^{(2)} \quad (27)$$

where

$$\begin{aligned} \boldsymbol{\Pi}^{(1)} &= \boldsymbol{\Pi}_{0i}^{(1)} + \mathbf{D} : \left(\boldsymbol{\varepsilon} - \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l \right) - \mathbf{A} (T - T_{0i}) = O(\eta) \\ \boldsymbol{\Pi}^{(2p)} &= \boldsymbol{\Pi}_{0i}^{(2p)} + \mathbf{D} : \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} - \mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l = O(\eta^{2p}) \\ \boldsymbol{\Pi}^{(1+p)} &= \boldsymbol{\Pi}_{0i}^{(1+p)} + \mathbf{D} : \left(\frac{1}{2} \mathbf{r}^T \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \mathbf{r} \right) - \mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1+p)} \mathbf{R}_l = O(\eta^{1+p}) \\ \boldsymbol{\Pi}^{(2)} &= \boldsymbol{\Pi}_{0i}^{(2)} + \mathbf{D} : \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon} - \mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2)} \mathbf{R}_l = O(\eta^2) \end{aligned} \quad (28)$$

Finally, assume that

$$0 < \eta < 1 \quad p > 0 \quad (29)$$

Since $\eta < 1$, the terms with the smallest exponent are the largest ones. The comparison between different terms must be done for a given p -value. If $p \geq 1$, viz. when \mathbf{r} is of the same order of magnitude as, or smaller than, $\boldsymbol{\varepsilon}$ (case of *small* rotations), all non-linear terms in the Green-Lagrange strain \mathbf{E} and

in the Lagrange multipliers Λ_k can be discarded and the standard condition of small transformation is retrieved. The exponent $p < 1$ is introduced in order to account for the possibility of having rotations *larger* than strains. The complete expression of the strain energy reads (recall Eq. (2), where the terms that depends *only* on the temperature are omitted for brevity):

$$\begin{aligned}\mathcal{F}(\bar{\mathbf{E}}, T) &= \int_{V_{00}} \Psi(\bar{\mathbf{E}}, T) dV = \int_{V_{00}} \left(\mathbf{\Pi}_{0i} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \bar{\mathbf{E}} \right) : \bar{\mathbf{E}} dV \\ &= \int_{V_{00}} \left(\mathbf{\Pi}_{0i} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : (\mathbf{E} - \sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l) \right) : (\mathbf{E} - \sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l) dV\end{aligned}$$

i.e. $\mathcal{F}(\bar{\mathbf{E}}, T) = \mathcal{F}(\mathbf{E}, T) + \mathcal{F}_\Lambda(\mathbf{E}, T)$, where

$$\begin{aligned}\mathcal{F}(\mathbf{E}, T) &= \int_{V_{00}} \left(\mathbf{\Pi}_{0i} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \mathbf{E} \right) : \mathbf{E} dV \\ \mathcal{F}_\Lambda(\mathbf{E}, T) &= - \int_{V_{00}} \left(\mathbf{\Pi}_{0i} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \mathbf{E} \right) : (\sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l) dV \\ &\quad - \int_{V_{00}} \frac{1}{2} \mathbf{D} : (\sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l) : \mathbf{E} dV + \int_{V_{00}} \frac{1}{2} \mathbf{D} : (\sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l) : (\sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l) dV\end{aligned}$$

More precisely,

$$\begin{aligned}\mathcal{F}(\mathbf{E}, T) &= \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1)} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \boldsymbol{\varepsilon} \right) : \left(\boldsymbol{\varepsilon} + \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) dV \\ &\quad + \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1)} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \boldsymbol{\varepsilon} \right) : \left(\frac{1}{2} \mathbf{r}^T \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \mathbf{r} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon} \right) dV \\ &\quad + \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(2p)} + \frac{1}{2} \mathbf{D} : \left(\frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) \right) : \left(\boldsymbol{\varepsilon} + \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} + \frac{1}{2} \mathbf{r}^T \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \mathbf{r} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon} \right) dV \\ &\quad + \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1+p)} + \frac{1}{2} \mathbf{D} : \left(\frac{1}{2} \mathbf{r}^T \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \mathbf{r} \right) \right) : \left(\boldsymbol{\varepsilon} + \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) dV \\ &\quad + \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1+p)} + \frac{1}{2} \mathbf{D} : \left(\frac{1}{2} \mathbf{r}^T \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \mathbf{r} \right) \right) : \left(\frac{1}{2} \mathbf{r}^T \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \mathbf{r} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon} \right) dV \\ &\quad + \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(2)} + \frac{1}{2} \mathbf{D} : \left(\frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon} \right) \right) : \left(\boldsymbol{\varepsilon} + \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} + \frac{1}{2} \mathbf{r}^T \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \mathbf{r} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon} \right) dV\end{aligned} \quad (30)$$

Eq. (30) shows all terms of the strain energy not depending on Λ . The exponents s_i of these energy terms are indicated in Figure 2 and in Table 2. The strain and rotation measures η and η^p lead to a hierarchy between the different terms. For every p -value, the smallest exponents are $s_1 = 2$, $s_2 = 1 + 2p$, $s_3 = 4p$ (see Fig. 2). Then, the dominant terms are

$$\begin{aligned}\mathcal{F}^{(2)}(\mathbf{E}, T) &= \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1)} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \boldsymbol{\varepsilon} \right) : \boldsymbol{\varepsilon} dV = O(\eta^2) \\ \mathcal{F}^{(1+2p)}(\mathbf{E}, T) &= \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1)} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \boldsymbol{\varepsilon} \right) : \left(\frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) dV \\ &\quad + \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(2p)} + \frac{1}{2} \mathbf{D} : \left(\frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) \right) : \boldsymbol{\varepsilon} dV = O(\eta^{1+2p}) \\ \mathcal{F}^{(4p)}(\mathbf{E}, T) &= \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(2p)} + \frac{1}{2} \mathbf{D} : \left(\frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) \right) : \left(\frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) dV = O(\eta^{4p})\end{aligned} \quad (31)$$

Likewise, the dominant contribution to \mathcal{F}_Λ splits in three terms:

$$\begin{aligned} \mathcal{F}_\Lambda^{(2)}(\mathbf{E}, T) &= - \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1)} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \boldsymbol{\varepsilon} \right) : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l dV \\ &- \int_{V_{00}} \frac{1}{2} \left(\mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l \right) : \boldsymbol{\varepsilon} dV + \int_{V_{00}} \frac{1}{2} \left(\mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l \right) : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l dV = O(\eta^2) \end{aligned}$$

$$\begin{aligned} \mathcal{F}_\Lambda^{(1+2p)}(\mathbf{E}, T) &= - \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(1)} - \mathbf{A}(T - T_{0i}) + \frac{1}{2} \mathbf{D} : \boldsymbol{\varepsilon} \right) : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l dV \\ &- \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(2p)} + \frac{1}{2} \mathbf{D} : \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l dV \\ &- \int_{V_{00}} \frac{1}{2} \left(\mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l \right) : \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} dV - \int_{V_{00}} \frac{1}{2} \left(\mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l \right) : \boldsymbol{\varepsilon} dV \\ &+ \int_{V_{00}} \frac{1}{2} \left(\mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(1)} \mathbf{R}_l \right) : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l dV = O(\eta^{1+2p}) \end{aligned}$$

$$\begin{aligned} \mathcal{F}_\Lambda^{(4p)}(\mathbf{E}, T) &= - \int_{V_{00}} \left(\mathbf{\Pi}_{0i}^{(2p)} + \frac{1}{2} \mathbf{D} : \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} \right) : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l dV \\ &- \int_{V_{00}} \frac{1}{2} \left(\mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l \right) : \frac{1}{2} \mathbf{r}^T \cdot \mathbf{r} dV \\ &+ \int_{V_{00}} \frac{1}{2} \left(\mathbf{D} : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l \right) : \sum_{l=1}^{n_\Lambda} \Lambda_l^{(2p)} \mathbf{R}_l dV = O(\eta^{4p}) \end{aligned}$$

In summary, one can write

$$\mathcal{F}(\bar{\mathbf{E}}, T) = \mathcal{F}^{(b)}(\mathbf{E}, T) + \mathcal{F}_\Lambda^{(b)}(\mathbf{E}, T) + \sum_{i=4}^n O(\eta^{si}) \quad (32)$$

with

$$\begin{aligned} \mathcal{F}^{(b)}(\mathbf{E}, T) &= \mathcal{F}^{(2)}(\mathbf{E}, T) + \mathcal{F}^{(1+2p)}(\mathbf{E}, T) + \mathcal{F}^{(4p)}(\mathbf{E}, T) \\ \mathcal{F}_\Lambda^{(b)}(\mathbf{E}, T) &= \mathcal{F}_\Lambda^{(2)}(\mathbf{E}, T) + \mathcal{F}_\Lambda^{(1+2p)}(\mathbf{E}, T) + \mathcal{F}_\Lambda^{(4p)}(\mathbf{E}, T) \end{aligned}$$

A strain energy containing only the three dominant terms, viz. $\mathcal{F} = \mathcal{F}^{(b)}(\mathbf{E}, T) + \mathcal{F}_\Lambda^{(b)}(\mathbf{E}, T)$, can be directly derived starting from the Hu-Washizu functional (10), provided that the strain measure is chosen as follows:

$$\mathbf{E}(\mathbf{U}) \rightarrow \mathbf{E}^{(b)}(\mathbf{U}) = \boldsymbol{\varepsilon}(\mathbf{U}) + \boldsymbol{\chi}(\mathbf{U}), \quad \boldsymbol{\chi}(\mathbf{U}) = \frac{1}{2} \mathbf{r}(\mathbf{U})^T \cdot \mathbf{r}(\mathbf{U}) \quad (33)$$

The virtual works of the internal forces (see (17), (21) and (28)) read

$$\begin{aligned} -\mathcal{W}_i^{(2)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= \int_{V_{00}} \mathbf{\Pi}^{(1)} : \nabla^s(\delta \mathbf{U}) dV \\ -\mathcal{W}_i^{(1+2p)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= \int_{V_{00}} \mathbf{\Pi}^{(1)} : \delta \boldsymbol{\chi}(\mathbf{U}, \delta \mathbf{U}) dV + \int_{V_{00}} \mathbf{\Pi}^{(2p)} : \nabla^s(\delta \mathbf{U}) dV \\ -\mathcal{W}_i^{(4p)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= \int_{V_{00}} \mathbf{\Pi}^{(2p)} : \delta \boldsymbol{\chi}(\mathbf{U}, \delta \mathbf{U}) dV \end{aligned} \quad (34)$$

where $\delta\chi(\mathbf{U}, \delta\mathbf{U}) = \frac{1}{2} \left(\mathbf{r}(\delta\mathbf{U})^T \cdot \mathbf{r}(\mathbf{U}) + \mathbf{r}(\mathbf{U})^T \cdot \mathbf{r}(\delta\mathbf{U}) \right)$.

5.2 Definition of "strain-rotation domains" and of approximated expressions of the strain energy

Eq. (32) reveals the dominant energy terms. However, it is also interesting to find the *strain-rotation domains* (depending on η and p) where the approximation of the exact strain energy by a simplified expression is acceptable. *Three* domains are of special interest:

(a) The domain \mathbb{H}_a where

$$\mathcal{F}(\bar{\mathbf{E}}, T) \simeq \mathcal{F}^{(2)}(\mathbf{E}, T) + \mathcal{F}_\Lambda^{(2)}(\mathbf{E}, T) = O(\eta^2) \quad (35)$$

Eq. (35) represents the situation usually called of *small perturbations*. The corresponding virtual work of internal forces reduces to $\mathcal{W}_i^{(2)}(\mathbf{\Pi}, \mathbf{U}, \delta\mathbf{U})$; see Eq. (34). As it is seen hereafter, a suitable name for \mathbb{H}_a is *domain of small strains and relatively small squared rotations (the ratio η^{2p-1} is small)*.

(b) The domain \mathbb{H}_b where

$$\mathcal{F}(\bar{\mathbf{E}}, T) \simeq \mathcal{F}^{(b)}(\mathbf{E}, T) + \mathcal{F}_\Lambda^{(b)}(\mathbf{E}, T) = O(\eta^2) + O(\eta^{1+2p}) + O(\eta^{4p}) \quad (36)$$

i.e. three energy terms are considered together, of order 2, $1 + 2p$ and $4p$, after the discussion of the previous Section. The virtual work reduces to

$$\mathcal{W}_i^{(b)}(\mathbf{\Pi}, \mathbf{U}, \delta\mathbf{U}) = \mathcal{W}_i^{(2)}(\mathbf{\Pi}, \mathbf{U}, \delta\mathbf{U}) + \mathcal{W}_i^{(1+2p)}(\mathbf{\Pi}, \mathbf{U}, \delta\mathbf{U}) + \mathcal{W}_i^{(4p)}(\mathbf{\Pi}, \mathbf{U}, \delta\mathbf{U})$$

This region is the *domain of small strains and relatively moderate squared rotations (the ratio $\eta^{2p-1} \simeq 1$)*, called *moderate rotation domain* for brevity.

(c) The domain \mathbb{H}_c where

$$\mathcal{F}(\bar{\mathbf{E}}, T) \simeq \mathcal{F}^{(4p)}(\mathbf{E}, T) + \mathcal{F}_\Lambda^{(4p)}(\mathbf{E}, T) = O(\eta^{4p}) \quad (37)$$

and $\mathcal{W}_i(\mathbf{\Pi}, \mathbf{U}, \delta\mathbf{U}) \rightarrow \mathcal{W}_i^{(4p)}(\mathbf{\Pi}, \mathbf{U}, \delta\mathbf{U}) = O(\eta^{4p})$, *i.e.* the energy term of order $4p$ is larger than all the others (*domain of small strains and relatively large squared rotations, i.e. the ratio η^{2p-1} is large*).

In order to draw these domains, let us introduce a small positive number $\zeta \ll 1$, for instance $\zeta = 0.01$. Then, the situations $p \leq \frac{1}{2}$ and $p \geq \frac{1}{2}$ can be distinguished, since for $p \leq \frac{1}{2}$ the dominant term is of order $4p$ and in the other case the term of order 2 is the largest (Figure 2). When $p \geq \frac{1}{2}$, look for the conditions on η and p such that all energy terms are small compared with

the second order term. The following system of inequalities defines \mathbb{H}_a :

$$\mathbb{H}_a : \quad \frac{\eta^{s_i}}{\eta^2} \leq \zeta \quad \text{for all } i = 2, n \quad (38)$$

Every condition enjoys a simple log-log representation, ($x = \log \eta^p, y = \log \eta$), since $\eta^{s_i-2} = \eta^{c_i p + d_i} \rightarrow \log \eta^{s_i-2} = \log \eta^{c_i p + d_i} = c_i x + d_i y \leq \log \zeta$, where c_i and d_i are integer numbers. For instance, when $s_i = s_2 = 1 + 2p \leq \log \zeta$, one has $2x - y \leq \log \zeta$, which relates $y = \log \eta$ with $x = \log \eta^p$. This condition is associated with a line which is the bottom limit of the domain \mathbb{H}_a ; see Figure 3a. If $s_i = 3$, one has $c_i = 0, d_i = 1$, leading to the condition $y = \log \eta \leq \log \zeta$, associated with an horizontal line as shown in same Figure. This means that the ratio between the energy term of order 3 and that of order 2 is smaller or equal to ζ , provided that $\eta \leq \zeta$. Still assuming $p \geq \frac{1}{2}$, one can define the conditions such that all the energy terms are small compared with that of order 2, except those of order $1 + 2p$ and $4p$:

$$\mathbb{H}'_b : \frac{\eta^{s_i}}{\eta^2} \leq \zeta \quad \forall i = 4, n, \quad \zeta < \frac{\eta^{1+2p}}{\eta^2} := \rho_1 \leq 1, \quad \zeta \lesseqgtr \frac{\eta^{4p}}{\eta^2} := \rho_2 \leq 1 \quad (39)$$

The third relationship shows that inside this domain, the ratio between the term of order $4p$ and that of order 2 may be either small, equal or greater than ζ . When $p \leq \frac{1}{2}$, the conditions such that all the energy terms are small compared with that of order $4p$ read:

$$\mathbb{H}_c : \quad \frac{\eta^{s_i}}{\eta^{4p}} \leq \zeta \quad \text{for all } i = 1, 2 \text{ and } 4, n \quad (40)$$

This set is associated with the approximated energy (37). If the terms of order $1 + 2p$ and 2 are not small, one has

$$\mathbb{H}''_b : \frac{\eta^{s_i}}{\eta^{4p}} \leq \zeta \quad \forall i = 4, n, \quad \zeta < \frac{\eta^{1+2p}}{\eta^{4p}} = \frac{1}{\rho_1} \leq 1, \quad \zeta \lesseqgtr \frac{\eta^2}{\eta^{4p}} = \frac{1}{\rho_2} \leq 1 \quad (41)$$

The set $\mathbb{H}_b = \mathbb{H}'_b \cup \mathbb{H}''_b$ is associated to the approximated energy (36). Inside this domain, three energy terms (of order 2, $1 + 2p$ and $4p$) are retained. The ratios ρ_1 and ρ_2 and their inverses (Eqs. (39), (41)) give an estimation of the relative amplitude of these dominant terms.

Observe that \mathbb{H}_c is extended to large rotations (η^{4p} close to 1) because we have compared the energy terms of the first four rows of Table 2 characterizing the *physically linear* and isotropic constitutive law (7). However, it is of interest here to determine the conditions under which the physical linearization is a reasonable approximation of any real material behaviour. It is expected

that this is the case when the rotations are not too large. In order to derive sharp bounds on the rotations, the quadratic constitutive law (8) has to be considered, and the associated strain energy must be computed. This leads to (see also (27))

$$\begin{aligned}\mathbf{\Pi}^{(phys.non-lin.)} &= \mathbf{\Pi}^{(phys.lin.)} + (O(\eta) + O(\eta^{2p}) + O(\eta^{1+p}) + O(\eta^2))^2 \\ \mathcal{F} &= O(\eta^2) + O(\eta^{1+2p}) + O(\eta^{4p}) + \sum_{i=4}^{n_{lin}} O(\eta^{s_i})\end{aligned}\quad (42)$$

i.e. some energy terms have to be added to those of the physically linear case, as indicated in Table 2. Then, the same conditions as in (38)-(41) are considered, with n substituted by n_{lin} , in order to compute the conditions under which all the physically non-linear terms are small compared with the dominant ones, either of order 2 or $4p$. These conditions define the regions depicted in Figure 3b: they are the *strain-rotation domains* where the physical linearization is admissible. An important difference with respect to Figure 3a is that \mathbb{H}_b and \mathbb{H}_c are bounded by the vertical line $x \leq \frac{1}{2} \log \zeta$, equivalent to $\eta^{2p} \leq \zeta$, viz. the squared rotations, not only the strains, must be small. It can be proven that this limitation derives from the condition $\frac{\eta^{6p}}{\eta^{4p}} \leq \zeta$, imposing that the term of order $6p$ associated with the physically non-linear law remains small compared with the term of order $4p$, dominant for $p \leq 1/2$.

The condition of having small Green-Lagrange strain reads: $\|\mathbf{E}\| = O(\eta) + O(\eta^{2p}) + O(\eta^{1+p}) + O(\eta^2) \leq \zeta \ll 1$. It is easy to identify in Figure 3 the domains in the strain-rotation plane where the first, the second and the fourth term of \mathbf{E} are less or equal to ζ . It can be also proven that the condition of having a small third term, i.e. $\eta^{1+p} \leq \zeta$, is fulfilled in the three sets \mathbb{H}_a , \mathbb{H}_b and \mathbb{H}_c .

Other approximations retaining at least four energy terms are possible. However, (a), (b) and (c) define situations often discussed in the literature and for this reason the present analysis is restricted to them. Case (b), collecting three terms, is formally more complex than the others and is discussed in detail hereinafter.

5.3 Numerical example: "exact" values of η and p and comparison of the exact and "simplified" energy maps

Consider a problem of plain stress elasticity (n_{dim} , the dimension of the problem, is equal to 2). The corresponding conditions on the Piola-Kirchhoff tensor are $\Pi_{XZ} = \Pi_{YZ} = \Pi_{ZZ} = 0$. A St.Venant- Kirchhoff material is chosen with

$\mathbf{\Pi}_{0i} = \mathbf{0}$ and $T = T_{0i}$. Then

$$\begin{aligned}\Psi &= \Psi(\mathbf{E}) = \frac{1}{2}\mathbf{E} : (\tilde{\lambda}\mathbf{1} \otimes \mathbf{1} + 2\mu\mathbf{I}) : \mathbf{E} \\ \mathbf{\Pi} &= \mathbf{\Pi}(\mathbf{E}) = 2\mu\mathbf{E} + \tilde{\lambda} \operatorname{tr}(\mathbf{E})\mathbf{1} = \begin{bmatrix} \Pi_{XX} & \Pi_{XY} \\ \Pi_{XY} & \Pi_{YY} \end{bmatrix}\end{aligned}\quad (43)$$

where $\tilde{\lambda} = \frac{E\nu}{1-\nu^2} \neq \lambda$ due to plain stress assumption and

$$\mathbf{E}(\mathbf{U}) = \begin{bmatrix} \frac{\partial u}{\partial X} & \frac{1}{2}\left(\frac{\partial u}{\partial Y} + \frac{\partial v}{\partial X}\right) \\ \frac{1}{2}\left(\frac{\partial u}{\partial Y} + \frac{\partial v}{\partial X}\right) & \frac{\partial v}{\partial Y} \end{bmatrix} + \frac{1}{2} \begin{bmatrix} \frac{\partial u}{\partial X} \frac{\partial u}{\partial X} + \frac{\partial v}{\partial X} \frac{\partial v}{\partial X} & \frac{\partial u}{\partial X} \frac{\partial u}{\partial Y} + \frac{\partial v}{\partial X} \frac{\partial v}{\partial Y} \\ \frac{\partial u}{\partial X} \frac{\partial u}{\partial Y} + \frac{\partial v}{\partial X} \frac{\partial v}{\partial Y} & \frac{\partial u}{\partial Y} \frac{\partial u}{\partial Y} + \frac{\partial v}{\partial Y} \frac{\partial v}{\partial Y} \end{bmatrix} \quad (44)$$

is the 2D Green-Lagrange strain with $\mathbf{U} = [u, v]^T$. The relevant H-W functional is given by (10), without the term depending on Λ_k^* and with Ψ given by (43-1). For a static problem with the external volume force $\mathbf{f}_0 = [f_{X,0}, f_{Y,0}]^T$ and surface force $\mathbf{g}_0 = [g_{X,0}, g_{Y,0}]^T$, the weak form of the equilibrium equation reads:

$$\begin{aligned}\mathbf{R}(\mathbf{U}, \delta\mathbf{U}) &= - \int_{V_{00}} \mathbf{\Pi}(\mathbf{E}(\mathbf{U})) : \delta\mathbf{E}(\mathbf{U}, \delta\mathbf{U}) \, dV \\ &+ \int_{V_{00}} \mathbf{f}_0 \cdot \delta\mathbf{U} \, dA + \int_{\partial V_{00,\sigma}} \mathbf{g}_0 \cdot \delta\mathbf{U} \, dA = 0 \quad \text{for all } \delta\mathbf{U} \in \mathbb{V}\end{aligned}\quad (45)$$

with $\delta\mathbf{U} = [\delta u, \delta v]^T$ and $\delta\mathbf{E}(\mathbf{U}, \delta\mathbf{U})$ defined by the right-hand side of Eq. (15). Eq. (45) is a non-linear partial differential equation, which can be discretized by a standard finite element method. The standard Newton algorithm has been implemented in the code FreeFEM++ (Danaila et al., 2003): given the displacement \mathbf{U}_n at iteration n , the increment \mathbf{w} is computed by

$$\mathbf{w} \in \mathbb{U} \text{ such that } \mathbf{R}(\mathbf{U}_n, \delta\mathbf{U}) + \delta\mathbf{R}(\mathbf{U}_n, \delta\mathbf{U}, \mathbf{w}) \simeq \mathbf{0} \quad \text{for all } \delta\mathbf{U} \in \mathbb{V}$$

with

$$\begin{aligned}\delta\mathbf{R}(\mathbf{U}_n, \delta\mathbf{U}, \mathbf{w}) &= - \int_{\Omega} \mathbf{\Pi}(\delta\mathbf{E}(\mathbf{U}_n, \mathbf{w})) : \delta\mathbf{E}(\mathbf{U}_n, \delta\mathbf{U}) \, d\Omega \\ &\quad - \int_{\Omega} \mathbf{\Pi}(\mathbf{E}(\mathbf{U}_n)) : \delta^2\mathbf{E}(\delta\mathbf{U}, \mathbf{w}) \, d\Omega \\ \delta^2\mathbf{E}(\delta\mathbf{U}, \mathbf{w}) &= \frac{1}{2}\nabla(\delta\mathbf{U})^T \cdot \nabla(\mathbf{w}) + \frac{1}{2}\nabla(\mathbf{w})^T \cdot \nabla(\delta\mathbf{U})\end{aligned}$$

Set $\mathbf{U}_{n+1} = \mathbf{U}_n + \mathbf{w}$. Repeat until $\frac{\|\mathbf{w}\|_{L_2(V_{00})}}{\|\mathbf{U}_n\|_{L_2(V_{00})}}$ is small enough.

The structure examined in this example is a parallelepiped beam lying in the XY -plane, the dimensions are $b = h = 1\text{cm}$ and $L_{00} = 50\text{cm}$. The beam

is clamped at both ends and the volume load is $[f_{X,0}, f_{Y,0}] = [0, -|f_{Y,0}|]$ daN/cm^3 . Exploiting the symmetry of the problem, only a half-beam is meshed, with the boundary conditions $[u = 0, v = 0]$ for $Y \in [-h/2, h/2]$ and $X = 0$ and $u = 0$ for $Y \in [-h/2, h/2]$ and $X = L_{00}/2$. The material parameters (steel) read $E = 2100000 daN/cm^2$, $\nu = 0.28$, $\mu = 820312.5 daN/cm^2$, $\lambda = 638020.8 daN/cm^2$. A mesh of triangular elements has been chosen, with two elements inside every cell of a regular grid of 15×375 squares. The finite element space is of P1 type. The numerical simulations give the results collected in Tables 3 and 4, where $v_{\max} = v(X = L_{00}/2)$, $x = \log_{10}(\eta^p)$, $y = \log_{10}(\eta)$, $p = \frac{x}{y}$, $\rho_2 = \frac{\eta^{4p}}{\eta^2}$, $\rho_1 = \frac{\eta^{1+2p}}{\eta^2}$ and

$$\eta = \frac{1}{\sqrt{V_{00}}} \|\boldsymbol{\varepsilon}\|_{L_2(V_{00})} = \sqrt{\frac{\int_{V_{00}} (\varepsilon_{XX}^2 + \varepsilon_{YY}^2 + 2\varepsilon_{XY}^2) dV}{bhL_{00}/2}}, \quad \eta_{xx} = \sqrt{\frac{\int_{V_{00}} \varepsilon_{XX}^2 dV}{bhL_{00}/2}} \quad (46)$$

$$\eta^p = \frac{1}{\sqrt{2V_{00}}} \|\mathbf{r}\|_{L_2(V_{00})} = \sqrt{\frac{\int_{V_{00}} r_{XY}^2 dV}{bhL_{00}/2}}$$

The last column of Table 4 provides a global estimation of the energy error between the exact and the approximated energies $\mathcal{F} = \int \Psi(\mathbf{E}) dV$ and $\mathcal{F}^{(b)} = \int \Psi(\mathbf{E}^{(b)}) dV$. Observe the maximum absolute value of the strain ε_{xx} , reported in the third column of Table 3: in the first four cases it is less than 0.002, which is the limit elastic strain for a steel having yielding stress approximately equal to $4200 daN/cm^2 = 420 MPa$. For these situations, the material is truly physically linear. Conversely, when $f_{Y0} = -6 daN/cm^3$, see the last row of Table 3, the maximum absolute value of ε_{xx} is larger than 0.002. Hence, the physical linearity is truly fulfilled only for steels having a greater yielding stress. The $x - y$ coordinates of the *strain-rotation points* associated with each f_{Y0} value are reported in the seventh and eighth columns of Table 3. The corresponding graphical representation is given in Figure 3-b, depicted assuming $\zeta = 0.01$. The map $\Psi(\mathbf{E})$, *viz.* the strain energy density at the static equilibrium \mathbf{U}_0 is shown in Figure 4. The energy density corresponding to the approximated strain energy $\Psi(\mathbf{E}^{(b)})$ is given in Figure 5. The relative difference of energy density between the two cases is illustrated in Figure 6. With a surface load $\mathbf{g}_0 = [g_{X,0}, g_{Y,0}] = [0, -|g_{Y,0}|]$ daN/cm^2 and $\mathbf{f}_0 = \mathbf{0}$, the results are similar, as one can see from Tables 5, 6 and Figure 7. This confirms that η and p are not too sensitive to the load distribution.

6 Dissipative stress, Hu-Washizu functional and damping pseudo-potential with stress constraints

The linear law (6) can be generalized by adding to $\boldsymbol{\Pi}^{nd}$ a dissipative term:

$$\boldsymbol{\Pi} = \boldsymbol{\Pi}^{nd} + \boldsymbol{\Pi}^d \quad (47)$$

The index d indicates the dissipative part of the stress. Let

$$\begin{aligned} J^d \left(\mathbf{\Pi}^{d*}, \frac{d\tilde{\mathbf{E}}^*}{dt}, \lambda_k^*; \frac{d\bar{\mathbf{E}}}{dt} \right) &= \int_{V_{00}} \phi \left(\frac{d\tilde{\mathbf{E}}^*}{dt} \right) dV \\ - \int_{V_{00}} \mathbf{\Pi}^{d*} : \left(\frac{d\tilde{\mathbf{E}}^*}{dt} - \frac{d\bar{\mathbf{E}}}{dt} \right) dV &- \int_{V_{00}} \sum_{k=1}^{n_\Lambda} \lambda_k^* \mathbf{R}_k : \mathbf{\Pi}^{d*} dV \end{aligned} \quad (48)$$

be the functional associated with a dissipative stress. It is assumed that it depends on the dissipative stress $\mathbf{\Pi}^{d*} \in \mathbb{T}$, the generic strain flow $\frac{d\tilde{\mathbf{E}}^*}{dt} \in \mathbb{T}$ and $\lambda_k^* \in \mathbb{F}$, i.e. the Lagrange multipliers associated with the constraints imposed on $\mathbf{\Pi}^{d*}$. Moreover, the *actual* strain flow $\frac{d\bar{\mathbf{E}}}{dt}$ plays the role of additional parameter: for this reason it is separated from the main variables by the semi-colon ";", instead of the comma. The actual strain flow is computed from the problem associated with the Hu-Washizu functional defined in Eq. (54). The scalar non-negative and convex function ϕ is called *pseudo-potential* or *dissipation potential*. A classical definition is $\phi = \frac{1}{2} \frac{d\tilde{\mathbf{E}}}{dt} : \mathbf{F} : \frac{d\tilde{\mathbf{E}}}{dt}$, i.e. a quadratic function. For an isotropic material, one has $\mathbf{F} = \lambda_d \mathbf{1} \otimes \mathbf{1} + 2\mu_d \mathbf{I}$, where λ_d and μ_d are analogous to the Lamé constants λ and μ . The stationarity conditions imposed on (48) lead to following strong form expressions

$$\begin{aligned} a. \quad \forall k = 1, n_\Lambda \quad \mathbf{R}_k : \mathbf{\Pi}^d &= 0 && \text{in } V_{00} \\ b. \quad \frac{d\tilde{\mathbf{E}}}{dt} &= \frac{d\bar{\mathbf{E}}}{dt} - \sum_{k=1}^{n_\Lambda} \lambda_k \mathbf{R}_k && \text{in } V_{00} \\ c. \quad \mathbf{\Pi}^d &:= \left. \frac{\partial \phi \left(\frac{d\tilde{\mathbf{E}}^*}{dt} \right)}{\partial \frac{d\tilde{\mathbf{E}}^*}{dt}} \right|_{\frac{d\tilde{\mathbf{E}}^*}{dt} = \frac{d\bar{\mathbf{E}}}{dt}} = \mathbf{F} : \frac{d\bar{\mathbf{E}}}{dt} && \text{in } V_{00} \end{aligned} \quad (49)$$

The first equation indicates the stress constraints imposed on $\mathbf{\Pi}^d$, the second one shows that the strain flow governing the dissipative behaviour is not equal to the time derivative of the strain when $\lambda_k \neq 0$. Finally, the third equation is the constitutive law for the dissipative stress, obtained from the pseudo-potential ϕ . The constraints on the dissipative stress read

$$\mathbf{R}_k : \mathbf{\Pi}^d = \mathbf{R}_k : \left[\mathbf{F} : \left(\frac{d\bar{\mathbf{E}}}{dt} - \sum_{l=1}^{n_\Lambda} \frac{d\Lambda_l}{dt} \mathbf{R}_l - \sum_{l=1}^{n_\Lambda} \lambda_l \mathbf{R}_l \right) \right] = 0 \quad (50)$$

where Λ_l , $l = 1, n_\Lambda$ are known from the analysis of the non-dissipative part of the stress. Following the procedure indicated in the Appendix, one can prove that $\boldsymbol{\lambda} = (\lambda_k)$, $k = 1, n_\Lambda$ is the solution of a linear Gram system. Therefore

$$\boldsymbol{\lambda} = \boldsymbol{\lambda}^{(1)} + \boldsymbol{\lambda}^{(2p)} + \boldsymbol{\lambda}^{(1+p)} + \boldsymbol{\lambda}^{(2)} = O(\eta) + O(\eta^{2p}) + O(\eta^{1+p}) + O(\eta^2) \quad (51)$$

Using Eqs. (49) and (51), one obtains

$$\mathbf{\Pi}^d = \mathbf{\Pi}^{d(1)} + \mathbf{\Pi}^{d(2p)} + \mathbf{\Pi}^{d(1+p)} + \mathbf{\Pi}^{d(2)} \quad (52)$$

where the following four terms of different orders are distinguished:

$$\begin{aligned}
\Pi^{d(1)} &= \mathbf{F} : \left(\frac{d\boldsymbol{\varepsilon}}{dt} - \sum_{k=1}^{n_\Lambda} \frac{d\Lambda_k^{(1)}}{dt} \mathbf{R}_k - \sum_{k=1}^{n_\Lambda} \lambda_k^{(1)} \mathbf{R}_k \right) \\
\Pi^{d(2p)} &= \mathbf{F} : \left(\frac{1}{2} \frac{d\mathbf{r}^T}{dt} \cdot \mathbf{r} + \frac{1}{2} \mathbf{r}^T \cdot \frac{d\mathbf{r}}{dt} - \sum_{k=1}^{n_\Lambda} \frac{d\Lambda_k^{(2p)}}{dt} \mathbf{R}_k - \sum_{k=1}^{n_\Lambda} \lambda_k^{(2p)} \mathbf{R}_k \right) \\
\Pi^{d(1+p)} &= \mathbf{F} : \left(\frac{1}{2} \frac{d\mathbf{r}^T}{dt} \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \mathbf{r}^T \cdot \frac{d\boldsymbol{\varepsilon}}{dt} + \frac{1}{2} \boldsymbol{\varepsilon}^T \cdot \frac{d\mathbf{r}}{dt} + \frac{1}{2} \frac{d\boldsymbol{\varepsilon}^T}{dt} \cdot \mathbf{r} \right) \\
&\quad - \mathbf{F} : \left(\sum_{k=1}^{n_\Lambda} \frac{d\Lambda_k^{(1+p)}}{dt} \mathbf{R}_k + \sum_{k=1}^{n_\Lambda} \lambda_k^{(1+p)} \mathbf{R}_k \right) \\
\Pi^{d(2)} &= \mathbf{F} : \left(\frac{1}{2} \frac{d\boldsymbol{\varepsilon}^T}{dt} \cdot \boldsymbol{\varepsilon} + \frac{1}{2} \boldsymbol{\varepsilon} \cdot \frac{d\boldsymbol{\varepsilon}^T}{dt} - \sum_{k=1}^{n_\Lambda} \frac{d\Lambda_k^{(2)}}{dt} \mathbf{R}_k - \sum_{k=1}^{n_\Lambda} \lambda_k^{(2)} \mathbf{R}_k \right)
\end{aligned} \tag{53}$$

In order to define the non-dissipative part of the stress, as well as the equilibrium equation, the following Hu-Washizu type functional is introduced:

$$\begin{aligned}
J_{H-W} \left(\Pi^{nd*}, \bar{\mathbf{E}}^*, \mathbf{U}^*, \Lambda_k^*; \Pi^d \right) &= \int_{V_{00}} \Psi \left(\bar{\mathbf{E}}^*, T \right) dV - \int_{V_{00}} \Pi^{nd*} : \left(\bar{\mathbf{E}}^* - \mathbf{E}(\mathbf{U}^*) \right) dV \\
&+ \int_{V_{00}} \Pi^d : \mathbf{E}(\mathbf{U}^*) dV - \int_{V_{00}} \mathbf{f} \cdot \mathbf{U}^* dV - \int_{\partial V_{00,\sigma}} \mathbf{g} \cdot \mathbf{U}^* dA \\
&- \int_{\partial V_{00,u}} \left(\left[(\mathbf{1} + \nabla(\mathbf{U}^*)) \cdot (\Pi^{nd*} + \Pi^d) \right] \cdot \mathbf{N} \right) \cdot (\mathbf{U}^* - \bar{\mathbf{U}}) dA \\
&- \int_{V_{00}} \sum_{k=1}^{n_\Lambda} \Lambda_k^* \mathbf{R}_k : \Pi^{nd*} dV
\end{aligned} \tag{54}$$

Eq. (54) should be compared with (10). An attentive reader can see that the functional depends on the non-dissipative part of the stress $\Pi^{nd*} \in \mathbb{T}$ instead of on the total stress Π^* . Moreover, an additional dependence on Π^d is introduced, where Π^d is the stationary solution of (48). Stationarity imposed on J_{H-W} leads to the strong form expressions:

$$\begin{aligned}
1. \quad & \mathbf{R}_k : \Pi^{nd} = 0 \quad , \quad k = 1, n_\Lambda \quad \text{in } V_{00} \\
2. \quad & \begin{cases} \bar{\mathbf{E}} = \mathbf{E}(\mathbf{U}) - \sum_{k=1}^{n_\Lambda} \Lambda_k \mathbf{R}_k & \text{in } V_{00} \\ \mathbf{U} = \bar{\mathbf{U}} & \text{on } \partial V_{00,u} \end{cases} \tag{55} \\
3. \quad & \Pi^{nd} = \left. \frac{\partial \Psi(\bar{\mathbf{E}}^*, T)}{\partial \bar{\mathbf{E}}^*} \right|_{\bar{\mathbf{E}}^* = \bar{\mathbf{E}}} \quad \text{in } V_{00}
\end{aligned}$$

Moreover, recalling (12) and imposing stationarity in the displacements, one obtains the following weak form equilibrium equation:

$$\mathcal{W}_i \left(\Pi^{nd}, \mathbf{U}, \delta \mathbf{U} \right) + \mathcal{W}_i \left(\Pi^d, \mathbf{U}, \delta \mathbf{U} \right) + \mathcal{W}_e \left(\mathbf{f}, \mathbf{g}, \delta \mathbf{U} \right) = \mathcal{W}_a \left(\ddot{\mathbf{U}}, \delta \mathbf{U} \right), \quad \forall \delta \mathbf{U} \in \mathbb{V}$$

where $\mathcal{W}_i(\boldsymbol{\Pi}^{nd}, \mathbf{U}, \delta\mathbf{U}^*)$ is defined in (17), $\mathcal{W}_e(\mathbf{f}, \mathbf{g}, \delta\mathbf{U})$ is also given in (17), with $\boldsymbol{\Pi} = \boldsymbol{\Pi}^{nd} + \boldsymbol{\Pi}^d$ and

$$\mathcal{W}_i^d = \mathcal{W}_i(\boldsymbol{\Pi}^d, \mathbf{U}, \delta\mathbf{U}) = - \int_{V_{00}} \boldsymbol{\Pi}^d : \delta\mathbf{E}(\mathbf{U}, \delta\mathbf{U}) dV \quad (56)$$

The virtual work of inertia forces is the same as in the previous non-dissipative case. The dynamics of the system is ruled by (18), where $\boldsymbol{\Pi}$ is given by (47). The case of moderate rotations is obtained by just substituting $\mathbf{E}(\mathbf{U}^*)$ with $\mathbf{E}^{(b)}(\mathbf{U}^*)$ in Eqs. (48) and (54). This corresponds to the substitutions $\mathcal{W}_i \rightarrow \mathcal{W}_i^{(b)}$ and $\mathcal{W}_i^d \rightarrow \mathcal{W}_i^{d(b)}$, where (see Eq. (34))

$$\mathcal{W}_i^{d(b)} = \mathcal{W}_i^{(2)}(\boldsymbol{\Pi}^d, \mathbf{U}, \delta\mathbf{U}) + \mathcal{W}_i^{(1+2p)}(\boldsymbol{\Pi}^d, \mathbf{U}, \delta\mathbf{U}) + \mathcal{W}_i^{(4p)}(\boldsymbol{\Pi}^d, \mathbf{U}, \delta\mathbf{U}) \quad (57)$$

7 Accounting for a static prestress

As already discussed, when a static prestress due *external* mechanical and/or thermal loading occurs, the structure passes from the state $V_{00} = V_{0i}$ to a state V_0 . It is interesting to write the equations governing the equilibrium at the generic configuration V_1 as a function of the unknown displacement $\mathbf{U}_{01} = \mathbf{U}_1 - \mathbf{U}_0$, expressing the motion with respect to V_0 , as illustrated in Figure 1. This can be easily done subtracting the equilibrium equations established in the previous sections, written at V_1 and at V_0 . Both equilibrium conditions at V_0 and V_1 should be written, together with the other expressions coming from the stationarity of the relevant H-W type functional. For the sake of simplicity, only the weak form of the dynamic equilibrium is reported in the analysis of this section. As seen above, the virtual work of the internal forces is indicated by \mathcal{W}_i in the general case, and by $\mathcal{W}_i^{(b)}$ in the case of moderate rotations. All the equations of this section are written as function of \mathcal{W}_i , and then refer to the general case. However, the formal substitution of $\mathcal{W}_i^{(b)}$ at the place of \mathcal{W}_i gives the equations for the moderate rotation case.

The static problem defining the prestressed configuration V_0 reads

$$\left\{ \begin{array}{l} \text{Find } \mathbf{U}_0 \in \mathbb{U} \text{ such that for all } \delta\mathbf{U} \in \mathbb{V} \\ \mathcal{W}_i(\boldsymbol{\Pi}_0, \mathbf{U}_0, \delta\mathbf{U}) + \mathcal{W}_e(\mathbf{f}_0, \mathbf{g}_0, \delta\mathbf{U}) = 0, \quad \boldsymbol{\Pi}_0 = \boldsymbol{\Pi}^{nd}(\mathbf{U}_0, \Lambda_{k,0}) \\ \mathbf{U}_0 = \bar{\mathbf{U}}_0 \quad \text{on } \partial V_{00,u} \end{array} \right. \quad (58)$$

where $\Lambda_{k,0}$ are the Lagrange multipliers associated with the static problem and the prestress $\boldsymbol{\Pi}_0$ accounts for the temperature field T_0 . The dynamic problem

defining the generic configuration V_1 reads

$$\left\{ \begin{array}{l} \text{Find } \mathbf{U}_1 \in \mathbb{U} \text{ such that for all } \delta \mathbf{U} \in \mathbb{V} \\ \mathcal{W}_i(\boldsymbol{\Pi}_1^{nd}, \mathbf{U}_1, \delta \mathbf{U}) + \mathcal{W}_i(\boldsymbol{\Pi}_1^d, \mathbf{U}_1, \delta \mathbf{U}) \\ + \mathcal{W}_e(\mathbf{f}_0 + \mathbf{f}_1(t), \mathbf{g}_0 + \mathbf{g}_1(t), \delta \mathbf{U}) = \mathcal{W}_a(\ddot{\mathbf{U}}_1, \delta \mathbf{U}) \\ \boldsymbol{\Pi}_1 = \boldsymbol{\Pi}^{nd}(\mathbf{U}_1, \Lambda_{k,1}) + \boldsymbol{\Pi}^d(\mathbf{U}_1, \lambda_{k,1}) \\ \mathbf{U}_1 = \bar{\mathbf{U}}_0 + \bar{\mathbf{U}}_{01}(t) \quad \text{on } \partial V_{00,u} \end{array} \right. \quad (59)$$

where $\Lambda_{k,1}$ are the Lagrange multipliers computed for the dynamic problem. The *difference* between (59) and (58) leads to

$$\left\{ \begin{array}{l} \text{Find } \mathbf{U}_{01} \in \mathbb{U} \text{ such that for all } \delta \mathbf{U} \in \mathbb{V} \\ \Delta \mathcal{W}_i(\boldsymbol{\Pi}_0, \boldsymbol{\Pi}_1^{nd}, \mathbf{U}_{01}, \mathbf{U}_0, \delta \mathbf{U}) + \mathcal{W}_i(\boldsymbol{\Pi}_1^d, \mathbf{U}_0 + \mathbf{U}_{01}, \delta \mathbf{U}) \\ + \mathcal{W}_e(\mathbf{f}_1(t), \mathbf{g}_1(t), \delta \mathbf{U}) = \mathcal{W}_a(\ddot{\mathbf{U}}_{01}, \delta \mathbf{U}) \\ \boldsymbol{\Pi}_0 = \boldsymbol{\Pi}(\mathbf{U}_0, \Lambda_{k,0}), \quad \boldsymbol{\Pi}_1^d = \boldsymbol{\Pi}(\mathbf{U}_0 + \mathbf{U}_{01}, \lambda_{k,1}) \\ \boldsymbol{\Pi}_1 = \boldsymbol{\Pi}^{nd}(\mathbf{U}_0 + \mathbf{U}_{01}, \Lambda_{k,1}) + \boldsymbol{\Pi}_1^d \\ \mathbf{U}_{01} = \bar{\mathbf{U}}_1(t) \quad \text{on } \partial V_{00,u} \end{array} \right. \quad (60)$$

with

$$\Delta \mathcal{W}_i(\boldsymbol{\Pi}_0, \boldsymbol{\Pi}_1^{nd}, \mathbf{U}_{01}, \mathbf{U}_0, \delta \mathbf{U}) = \mathcal{W}_i(\boldsymbol{\Pi}_1^{nd}, \mathbf{U}_0 + \mathbf{U}_{01}, \delta \mathbf{U}) - \mathcal{W}_i(\boldsymbol{\Pi}_0, \mathbf{U}_0, \delta \mathbf{U}) \quad (61)$$

knowing that $\ddot{\mathbf{U}}_1 = \ddot{\mathbf{U}}_{01}$. Eq. (60) describes the dynamics around a statically prestressed configuration for the general case. Observe that all the equations are defined in the Lagrangian configuration $V_{00} = V_{0i}$, free of any external prestress effect by definition. The case of moderate rotations is retrieved introducing in the same equation $\mathcal{W}_i^{(b)}$ instead of \mathcal{W}_i and $\mathcal{W}_i^{d(b)}$ instead of \mathcal{W}_i^d . From Eq. (31) and recalling (36), (52) and (53), Eq. (61) becomes

$$\begin{aligned} & -\Delta \mathcal{W}_i^{(b)}(\boldsymbol{\Pi}_0, \boldsymbol{\Pi}_1^{nd}, \mathbf{U}_{01}, \mathbf{U}_0, \delta \mathbf{U}) \\ & = \int_{V_{00}} (\boldsymbol{\Pi}_1^{nd(1)} + \boldsymbol{\Pi}_1^{nd(2p)}) : (\nabla^s(\delta \mathbf{U}) + \delta \boldsymbol{\chi}(\mathbf{U}_0 + \mathbf{U}_{01}, \delta \mathbf{U})) dV \\ & \quad - \int_{V_{00}} (\boldsymbol{\Pi}_0^{(1)} + \boldsymbol{\Pi}_0^{(2p)}) : (\nabla^s(\delta \mathbf{U}) + \delta \boldsymbol{\chi}(\mathbf{U}_0, \delta \mathbf{U})) dV \end{aligned} \quad (62)$$

8 Moderate rotations and Bernoulli-Navier kinematic assumptions: undamped case

In this section, the stationarity conditions (13) with suited stress constraints are used in conjunction with the strain measure (33) for moderate rotations and the so-called Navier kinematic assumptions for beams, in order to obtain the corresponding strong form of the dynamic equilibrium equations. This analysis will enable a better understanding of the general equations previously presented. In particular, a simple way of estimating η and p is suggested with reference to the example of a clamped-clamped beam. In the non deformed configuration $V_{00} = V_{0i}$, the beam axis coincides with the cartesian axis X and the beam motion is supposed to be limited to the plane $X - Y$. A quadratic Helmholtz energy is adopted, leading to a linear constitutive law depending on the tensors \mathbf{D} and \mathbf{A} :

$$\mathbf{\Pi} = \mathbf{\Pi}_{0i} + \mathbf{D} : \bar{\mathbf{E}} - \mathbf{A} (T - T_{0i}) \quad (63)$$

with

$$\bar{\mathbf{E}} = \mathbf{E}^{(b)}(\mathbf{U}) - \sum_{k=1}^{n_\Lambda} \Lambda_k \mathbf{R}_k = \boldsymbol{\varepsilon}(\mathbf{U}) + \boldsymbol{\chi}(\mathbf{U}) - \sum_{k=1}^{n_\Lambda} \Lambda_k \mathbf{R}_k \quad (64)$$

For an isotropic material, one has $\mathbf{D} = \frac{E}{(1+\nu)(1-2\nu)} [\nu \mathbf{1} \otimes \mathbf{1} + (1-2\nu) \mathbf{I}]$. Moreover, since $\mathbf{A} = \frac{\alpha E}{1-2\nu} \mathbf{1}$, one has $\mathbf{D}^{-1} : \mathbf{A} (T - T_{0i}) = \alpha (T - T_{0i}) \mathbf{1}$. The Navier kinematic assumption reads

$$\mathbf{u} = [u - Y v', v, 0]^T \quad (65)$$

where $u = u(X, t)$, $v = v(X, t)$ are the X- and Y-displacement fields; the apex ' indicates the derivation with respect to X . The strains $\boldsymbol{\varepsilon}(\mathbf{U})$ and $\boldsymbol{\chi}(\mathbf{U})$ read:

$$\boldsymbol{\varepsilon}(\mathbf{U}) = \begin{bmatrix} u' - Y v'' & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}, \quad \boldsymbol{\chi}(\mathbf{U}) = \frac{1}{2} \begin{bmatrix} v'^2 & 0 & 0 \\ 0 & v'^2 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

For the prestress $\mathbf{\Pi}_{0i}$, we assume:

$$\mathbf{\Pi}_{0i} = \begin{bmatrix} \Pi_{0i,1} & 0 & 0 \\ 0 & \Pi_{0i,2} & 0 \\ 0 & 0 & \Pi_{0i,3} \end{bmatrix} = \begin{bmatrix} \Pi_{0i}^{(x)} + Y \Pi_{0i,x}^{(b)} & 0 & 0 \\ 0 & \Pi_{0i}^{(y)} + Y \Pi_{0i,y}^{(b)} & 0 \\ 0 & 0 & \Pi_{0i}^{(z)} + Y \Pi_{0i,z}^{(b)} \end{bmatrix}$$

Off-diagonal terms of $\mathbf{\Pi}_{0i}$ may not vanish. Nonetheless, they have no influence on the following analysis, since they are associated with zero virtual strain components in the virtual work product. The thermal and load fields write

$$\mathbf{f} := \begin{bmatrix} f_x(X, t) - Y f_b(X, t) \\ f_y(X, t) \\ 0 \end{bmatrix}, \quad \mathbf{g} = \begin{bmatrix} g_x(X, t) - Y g_b(X, t) \\ g_y(X, t) \\ 0 \end{bmatrix}$$

$$T - T_{0i} = [T_x(X) - T_{0i,x}(X)] - Y [\gamma(X) - \gamma_{0i}(X)] = \Delta T_x - Y \Delta \gamma$$

The stress constraints usually imposed to retrieve beam equations are $\Pi_{YY} = \Pi_{ZZ} = 0$, formally expressed by the conditions

$$\mathbf{R}_1 : \mathbf{\Pi} = \mathbf{R}_2 : \mathbf{\Pi} = 0 \quad \text{with} \quad \mathbf{R}_1 = \begin{bmatrix} 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 0 \end{bmatrix}, \quad \mathbf{R}_2 = \begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix} \quad (66)$$

Eqs. (22),(24) and (29) written for this case become

$$\begin{aligned} \eta &= \left(\frac{1}{V_{00}} \int_{V_{00}} [u' - Y v'']^2 dV \right)^{\frac{1}{2}} + \left(\frac{1}{V_{00}} \int_{V_{00}} 3 [\alpha (\Delta T_x - Y \Delta \gamma)]^2 dV \right)^{\frac{1}{2}} \\ &= \left(\frac{1}{V_{00}} \int_0^{L_{00}} (A (u')^2 + J (v'')^2) dX \right)^{\frac{1}{2}} + \left(\frac{3\alpha^2}{V_{00}} \int_0^{L_{00}} (A (\Delta T_x)^2 + J (\Delta \gamma)^2) dX \right)^{\frac{1}{2}} \\ \eta^p &= \left(\frac{1}{V_{00}} \int_{V_{00}} v'^2 dV \right)^{\frac{1}{2}} = \left(\frac{1}{V_{00}} \int_0^{L_{00}} A v'^2 dX \right)^{\frac{1}{2}} \\ \mathbf{\Pi}_{0i} &= O(\eta) + O(\eta^{2p}) \\ 0 &< \eta < 1 \quad p > 0 \end{aligned} \quad (67)$$

where A is the area of the generic beam section; J is the inertia moment and L_{00} is the beam length. Then, the same procedure as in the general case can be applied here, in order to determine the strain-rotation domains \mathbb{H}_a , \mathbb{H}_b and \mathbb{H}_c . As it is well-known, the Navier-Bernoulli kinematic assumptions entails that all shear strains, *i.e.* the off-diagonal elements of $\boldsymbol{\varepsilon}$, are equal to *zero*. As a result, one can easily prove that also the terms of order $3 + p$, $2 + p$ and $1 + 3p$ in the strain energy

$$\mathcal{F} = O(\eta^2) + O(\eta^{1+2p}) + O(\eta^{4p}) + \sum_{i=4}^{n_{NB}} O(\eta^{s_i}) \quad (68)$$

become zero (see Figure 8 and compare to Figure 2). As a result, the strain-rotation domains are not the same as in the general case, as illustrated in

Figure 9. The difference is highlighted by the small region excluded in the general case and admitted by the Navier kinematic conditions. Inside this region, the pertinence of the Navier assumptions (65) should be further investigated. Other kinematic assumptions, like for instance those of Timoshenko, appear to be more sound.

8.1 Strong form equations

Eqs. (63), (66) and (65) lead to the Lagrange multipliers

$$\begin{aligned}\Lambda_1 &= \frac{\Pi_{0i,2}(1-\nu^2)-\nu(1+\nu)\Pi_{0i,3}}{E} + \nu \left(u' - Y v'' + \frac{1}{2} v'^2 \right) \\ &\quad + \frac{1}{2} v'^2 - \alpha (\Delta T_x - Y \Delta \gamma) (1 + \nu) \\ \Lambda_2 &= \frac{\Pi_{0i,3}(1-\nu^2)-\nu(1+\nu)\Pi_{0i,2}}{E} + \nu \left(u' - Y v'' + \frac{1}{2} v'^2 \right) - \alpha (\Delta T_x - Y \Delta \gamma) (1 + \nu)\end{aligned}\tag{69}$$

from which (see (64)) $\bar{E}_{XX} = E_{XX} = u' - Y v'' + \frac{1}{2} v'^2$, $\bar{E}_{YY} = E_{YY} - \Lambda_1 = \frac{1}{2} v'^2 - \Lambda_1$, $\bar{E}_{ZZ} = E_{ZZ} - \Lambda_2 = -\Lambda_2$ and

$$\Pi_{XX} = \Pi_{0i,x} + Y \Pi_{0i,b} + E \left(u' - Y v'' + \frac{1}{2} v'^2 - \alpha (\Delta T_x - Y \Delta \gamma) \right)$$

where $\Pi_{0i,x} = \Pi_{0i}^{(x)} - \nu \left(\Pi_{0i}^{(y)} + \Pi_{0i}^{(z)} \right)$ and $\Pi_{0i,b} = \Pi_{0i,x}^{(b)} - \nu \left(\Pi_{0i,y}^{(b)} + \Pi_{0i,z}^{(b)} \right)$, while the other stress components are zero. Moreover, the virtual works read

$$\mathcal{W}_i^{(b)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) + \mathcal{W}_e(\mathbf{f}, \mathbf{g}, \delta \mathbf{U}) = \mathcal{W}_a(\ddot{\mathbf{U}}, \delta \mathbf{U})$$

$$\begin{aligned}\mathcal{W}_i^{(b)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= - \int_{V_{00}} \Pi_{XX} [\delta u' - Y \delta v'' + v' \delta v'] dV \\ &= \mathcal{W}_i^{(2)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) + \mathcal{W}_i^{(1+2p)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) + \mathcal{W}_i^{(4p)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) \\ -\mathcal{W}_i^{(2)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= \int_0^{L_{00}} [EA(u' - \alpha \Delta T_x)] \delta u' dX + \int_0^{L_{00}} [EJ(v'' - \alpha \Delta \gamma)] \delta v'' dX \\ -\mathcal{W}_i^{(1+2p)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= \int_0^{L_{00}} [EA \frac{1}{2} v'^2] \delta u' dX + \int_0^{L_{00}} [EA(u' - \alpha \Delta T_x)] [v' \delta v'] dX \\ -\mathcal{W}_i^{(4p)}(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= \int_0^{L_{00}} [EA \frac{1}{2} v'^2] [v' \delta v'] dX\end{aligned}\tag{70}$$

$$\begin{aligned}\mathcal{W}_e(\mathbf{f}, \mathbf{g}, \delta \mathbf{U}) &= \int_{V_{00}} [(f_x - Y f_b) (\delta u - Y \delta v') + f_y \delta v] dV \\ &\quad + \int_{\partial V_{00,\sigma}} [(g_x - Y g_b) (\delta u - Y \delta v') + g_y \delta v] dA \\ &\quad + [R_x \delta u + C_z \delta v' + R_y \delta v]_{X=0}^{X=L_{00}}\end{aligned}$$

$$\begin{aligned}\mathcal{W}_a(\ddot{\mathbf{U}}, \delta \mathbf{U}) &= \int_{V_{00}} \rho_{00} [(\ddot{u} - Y\ddot{v}')(\delta u - Y\delta v') + \ddot{v}\delta v] dV \\ &= \int_0^{L_{00}} \rho_{00} A \ddot{u} \delta u dX + \int_0^{L_{00}} \rho_{00} A \ddot{v} \delta v dX - \int_0^{L_{00}} \rho_{00} J \ddot{v}'' \delta v dX\end{aligned}$$

where R_x , R_y and C_z are the reaction forces and the reaction moment at the boundary $\partial V_{00,u}$. Observe the second and third term in \mathcal{W}_a : it can be proven that the ratio r_1 between the third term (rotational inertia) and the second term (translational inertia) reads $r_1 = O\left(\frac{J}{AL_{00}^2}\right) := O\left(\frac{ch^2}{L_{00}^2}\right)$, where h is the beam width. When the squared aspect ratio $(h/L_{00})^2$ is small, r_1 is small too. Note that this ratio can be easily expressed in terms of η and η^p when $\Delta T_x = \Delta \gamma = 0$ and $u' = O(\eta^{2p})$. In this case, Eqs. (67-1,2) entail $r_1 = O(\eta^{2p}) + O(\eta^{2-2p})$. Hence, under these assumptions $\eta^{2p} < \zeta$ and $\eta^{2-2p} < \zeta$ suffice to have r_1 small. The first condition is always fulfilled due to physical linearization assumption, while the second one is equivalent to $\log \eta^{2-2p} < \log \zeta$ and $y < x + \log \zeta / 2$ (see Figure 9) and is satisfied in a large portion of \mathbb{H}_b . For simplicity, the rotational inertia is always omitted hereinafter. The strong form equations corresponding to (70) are derived using the standard procedure:

$$\begin{cases} \left[A\Pi_{0i,x} + EA \left(u' + \frac{1}{2}v'^2 - \alpha\Delta T_x \right) \right]' = -p_x + \rho_{00}A\ddot{u} \\ (EJ(v'' - \alpha\Delta\gamma) + J\Pi_{0i,b})'' \\ - \left(v' \left[A\Pi_{0i,x} + EA \left(u' + \frac{1}{2}v'^2 - \alpha\Delta T_x \right) \right] \right)' = -q' + p_y - \rho_{00}A\ddot{v} \end{cases} \quad (71)$$

where $p_x = Af_x$ and $p_y = Af_y$ are the horizontal and vertical loading per unit beam length, respectively; $q = Jf_b$ is a couple per unit length. The boundary conditions of type $\partial V_{00,\sigma}$ involving the external forces $P_X = Ag_x$, $P_Y = Ag_y$ and the external couple $M = Jg_b$ at the ends of the beam, are not reported for brevity. At the configuration $V_{00} = V_{0i}$, one has $u = v = \Delta T_x = \Delta \gamma = 0$, $\partial V_{00} = \partial V_{00,\sigma}$ with zero loads, entailing $(A\Pi_{0i,x})' = 0$ and $(J\Pi_{0i,b})'' = 0$. This implies that $A\Pi_{0i,x}$ is constant along the length of the beam. Since zero force is applied on ∂V_{00} , *i.e.* $P_X = Ag_x(0) = 0 = A\Pi_{0i,x}$, this constant is equal to zero and the same holds for $J\Pi_{0i,b}$. This means that the initial *self-equilibrated* stress is zero and the equilibrium equations become

$$\begin{cases} - \left(EA \left(u' - \alpha\Delta T_x + \frac{1}{2}v'^2 \right) \right)' = p_x - \rho_{00}A\ddot{u} \\ (EJv'' - \alpha EJ\Delta\gamma)'' \\ - \left(v' \left[EA \left(u' + \frac{1}{2}v'^2 - \alpha\Delta T_x \right) \right] \right)' = -q' + p_y - \rho_{00}A\ddot{v} \end{cases} \quad (72)$$

This is the general expression of the beam equation with temperature field. The corresponding expression of the strain energy (see (31) and (36)) becomes

$$\begin{aligned}
\mathcal{F} &= \mathcal{F}^{(b)} = \mathcal{F}^{(2)} + \mathcal{F}^{(1+2p)} + \mathcal{F}^{(4p)} \\
\mathcal{F}^{(2)} &= \int_0^{L_{00}} EA \left(\frac{1}{2}u' - \alpha \Delta T_x \right) u' dX + \int_0^{L_{00}} EJ \left(\frac{1}{2}v'' - \alpha \Delta \gamma \right) v'' dX \\
\mathcal{F}^{(1+2p)} &= \int_0^{L_{00}} EA \left(\frac{1}{2}u' - \alpha \Delta T_x \right) \frac{1}{2}v'^2 dX + \frac{1}{2} \int_0^{L_{00}} EA \left(\frac{1}{2}v'^2 \right) u' dX \\
\mathcal{F}^{(4p)} &= \frac{1}{2} \int_0^{L_{00}} EA \left(\frac{1}{2}v'^2 \right)^2 dX
\end{aligned} \tag{73}$$

8.2 The geometric interpretation of η and η^p

In this Section, an interpretation of η and η^p in terms of suitable deflection and shape ratios, and as functions of the temperature field is provided for the case of a homogeneous beam. A first example concerns a beam with very small bending stiffness, *i.e.* $J/A \simeq 0$. A static vertical load F is applied at the midspan, where it induces a transversal displacement v_{\max} . Moreover, $\ddot{u} = \ddot{v} = p_x = p_y = q = 0$ and an axial temperature field is introduced. Then, Eq. (72) becomes

$$\begin{cases} EA \left(u' - \alpha \Delta T_x + \frac{1}{2}v'^2 \right) = R_x \\ R_x v'' = 0 \end{cases} \tag{74}$$

where R_x is the constant horizontal reaction at $X = L_{00}$. Boundary conditions write $u(0) = 0$, $u(L_{00}) = \bar{u} \geq 0$, $v(0) = v(L_{00}) = 0$ and $v''(0) = v''(L_{00}) = 0$. Since v' is piecewise constant, with a discontinuity at the midspan, integration of the first equation in (74) yields

$$\frac{R_x}{EA} = \frac{\bar{u}}{L_{00}} + \frac{1}{2}v'^2 - \frac{1}{L_{00}} \int_0^{L_{00}} \alpha \Delta T_x dX = \frac{\bar{u}}{L_{00}} + \frac{1}{2}v'^2 - \alpha \Delta \bar{T}_x$$

where $\Delta \bar{T}_x$ is the averaged temperature variation. It follows $u' = \alpha \Delta T_x - \alpha \Delta \bar{T}_x + \frac{\bar{u}}{L_{00}}$ *i.e.* $u' = \frac{\bar{u}}{L_{00}}$ when the axial temperature field is constant, even if non-zero. In this case $\Delta T_x = \Delta \bar{T}_x = \text{const.}$ and by using (67), one obtains

$$\eta = \eta_\varepsilon + \eta_{\Delta T} = \frac{\bar{u}}{L_{00}} + \sqrt{3} \alpha |\Delta T_x| \quad \text{and} \quad \eta^p = |v'| = 2 \frac{|v_{\max}|}{L_{00}} \tag{75}$$

which provide a simple interpretation of η and η^p in terms of temperature difference and of ratios between the maximum displacements and the beam length. Hence, the strain-rotation domains of Figure 9, which depend on η and

η^p , can also be interpreted using these ratios. For instance, consider the case of zero temperature field and given \bar{u} value, such that $\eta = \eta_\varepsilon = \bar{u}/L_{00} = 10^{-8}$: the *strain-rotation points* corresponding to this situation and for different values of $|v_{\max}|$ are depicted in Figure 9-b: they have the same y -value (constant η) and different x -values. The larger $|v_{\max}|$, the larger η^p : then, according to the value of $|v_{\max}|$, the point representing the structural state may belong to any of the sets \mathbb{H}_a , \mathbb{H}_b or \mathbb{H}_c and the relevant equilibrium equation is different in each case. Since the strain-rotation domains depend on ζ , Figure 9 refers to the case $\zeta = 0.01$. Observe in addition that $A_\alpha := \frac{1}{\sqrt{3}} \frac{\eta_{\Delta T}}{\eta_\varepsilon} = \frac{\alpha |\Delta T_x|}{\frac{\bar{u}}{L_{00}}}$ gives an estimate of the relative importance of the thermal and mechanical strains.

Let us now consider a homogeneous beam with a distributed vertical static load p_y , a generic temperature field and with $\ddot{u} = \ddot{v} = p_x = q = 0$. The same structure has been studied in the numerical examples of Section 5.3. Eq. (72) becomes

$$\begin{cases} EA \left(u' - \alpha \Delta T_x + \frac{1}{2} v'^2 \right) = R_x \\ EJ \left(v'''' - \alpha \Delta \gamma'' \right) - R_x v'' = p_y \end{cases} \quad (76)$$

For the X -direction, the boundary conditions (b.c.) are $u(0) = u(L_{00}) = 0$ and in the vertical direction one has $v(0) = v(L_{00}) = v'(0) = v'(L_{00}) = 0$. Integrating the first equation with the b.c. at $X = 0$ and $X = L_{00}$ leads to

$$\frac{R_x}{EA} = -\frac{1}{L_{00}} \int_0^{L_{00}} \alpha \Delta T_x dX + \frac{1}{L_{00}} \int_0^{L_{00}} \frac{1}{2} v'^2 dX \quad (77)$$

It follows, according to (76-1)

$$u' - \alpha \Delta T_x = \frac{1}{L_{00}} \int_0^{L_{00}} \frac{1}{2} v'^2 dX - \frac{1}{2} v'^2 - \alpha \Delta \bar{T}_x \quad (78)$$

where $\Delta \bar{T}_x$ has the same definition as in the previous example. Assume that ΔT_x and $\Delta \gamma$ are constant and substitute (78) into the definition (67) of η . Hence

$$\begin{aligned} \eta &= \eta_\varepsilon + \eta_{\Delta T}, \quad \eta_\varepsilon = \left(\frac{1}{V_{00}} \int_0^{L_{00}} \left(A \left(\frac{1}{2} \eta^{2p} - \frac{1}{2} v'^2 \right)^2 + J (v'')^2 \right) dX \right)^{\frac{1}{2}} \\ \eta_{\Delta T} &= \left(3\alpha^2 \left((\Delta T_x)^2 + \frac{J}{A} (\Delta \gamma)^2 \right) \right)^{\frac{1}{2}} \end{aligned}$$

In order to have a better understanding of the geometrical meaning of η , the solution $v(X)$, depending on p_y , ΔT_x and $\Delta \gamma$ should be analytically expressed.

However, this is not a simple task in general. Hence, accounting for the b.c., we assume here that the deformed shape is approximately co-sinusoidal:

$$v(X) = \frac{v_{\max}}{2} \left[1 - \cos\left(\frac{2\pi X}{L_{00}}\right) \right] \quad (79)$$

Hence, by using the definitions (67), one obtains

$$\begin{aligned} \eta^p &= \frac{\pi}{\sqrt{2}} \frac{|v_{\max}|}{L_{00}}, & \eta &= \eta_\varepsilon + \eta_{\Delta T} \\ \eta_\varepsilon &= \frac{\pi^2}{4\sqrt{2}} \frac{|v_{\max}|}{L_{00}} \sqrt{\frac{v_{\max}^2}{L_{00}^2} + 64c \frac{h^2}{L_{00}^2}}, & \eta_{\Delta T} &= \sqrt{3}\alpha \sqrt{\Delta T_x^2 + c(\Delta\gamma h)^2} \end{aligned} \quad (80)$$

where $c = \frac{J}{Ah^2}$, with h the beam width. When the transversal displacement is different from zero, the ratio between the thermal and mechanical contributions in η is equal to

$$A_\alpha := \frac{1}{\sqrt{3}} \frac{\eta_{\Delta T}}{\eta_\varepsilon} = \frac{\alpha}{\frac{\pi^2}{4\sqrt{2}} \frac{|v_{\max}|}{L_{00}}} \sqrt{\frac{\Delta T_x^2 + c(\Delta\gamma h)^2}{\frac{v_{\max}^2}{L_{00}^2} + 64c \frac{h^2}{L_{00}^2}}} \quad (81)$$

The interest of Eq. (80) is that it gives a geometrical interpretation for the case of beams of the quantities η and η^p defined in (22) for the general case and in (67) when the Navier-Bernoulli kinematic assumptions are adopted. They can be easily related to geometrical ratios involving the maximum deflection, the width and the length of the beam. These geometrical ratios are known to be important for beam analysis, but they are clearly related here to the tensorial quantities of a full 3D formulation of the structural problem. When both maximum deflection and geometry of the beam are known (or estimated), it is easy to find the corresponding point in strain-rotation domains and to establish how many terms need to be taken into account for the computation of the solution.

In order to find the explicit expression of the strain energy, note that ΔT_x constant entails from Eq. (78)

$$u' = \frac{1}{L_{00}} \int_0^{L_{00}} \frac{1}{2} v'^2 dX - \frac{1}{2} v'^2 = \frac{1}{2} \eta^{2p} - \frac{1}{2} v'^2 \quad (82)$$

and $\delta u' = \eta^p \delta \eta^p - v' \delta v'$. Hence, using (79) and the expression of η^p given in (80), the strain energy contributions (73) read

$$\begin{aligned}\mathcal{F}^{(2)} &= EAL_{00} \left(\frac{\pi^4}{64} \left(\frac{v_{\max}}{L_{00}} \right)^4 + \pi^4 c \left(\frac{h}{L_{00}} \right)^2 \left(\frac{v_{\max}}{L_{00}} \right)^2 \right) \\ \mathcal{F}^{(1+2p)} &= EAL_{00} \left(-\frac{\pi^4}{32} \left(\frac{v_{\max}}{L_{00}} \right)^4 - \frac{\pi^2}{4} \alpha \Delta T_x \left(\frac{v_{\max}}{L_{00}} \right)^2 \right) \\ \mathcal{F}^{(4p)} &= EAL_{00} \frac{3\pi^4}{64} \left(\frac{v_{\max}}{L_{00}} \right)^4\end{aligned}$$

The ratio between the energy terms of order $4p$ and 2 reads

$$\frac{\mathcal{F}^{(4p)}}{\mathcal{F}^{(2)}} = 3 \frac{\left(\frac{v_{\max}}{h} \right)^2}{64c + \left(\frac{v_{\max}}{h} \right)^2} = \frac{3}{8} \eta_\varepsilon^{4p-2}$$

and it only depends on the ratio v_{\max}/h . The virtual works of internal and external forces (see (70)) read

$$\begin{aligned}-\mathcal{W}_i(\mathbf{\Pi}, \mathbf{U}, \delta \mathbf{U}) &= EAL_{00} \left(\frac{\pi^4}{8} \left(\frac{v_{\max}}{L_{00}} \right)^3 + 2\pi^4 c \left(\frac{h}{L_{00}} \right)^2 \left(\frac{v_{\max}}{L_{00}} \right) - \frac{\pi^2}{2} \alpha \Delta T_x \left(\frac{v_{\max}}{L_{00}} \right) \right) \frac{\delta v_{\max}}{L_{00}} \\ \mathcal{W}_e(\mathbf{f}, \mathbf{g}, \delta \mathbf{U}) &= \int_0^{L_{00}} p_y \frac{\delta v_{\max}}{2} \left[1 - \cos \left(\frac{2\pi X}{L_{00}} \right) \right] dX = \frac{p_y L_{00}}{2} \delta v_{\max}\end{aligned}$$

with p_y supposed constant, and by the virtual work principle, one obtains

$$\left(2\pi^4 c \left(\frac{h}{L_{00}} \right)^2 - \frac{\pi^2}{2} \alpha \Delta T_x \right) \frac{v_{\max}}{L_{00}} + \frac{\pi^4}{8} \left(\frac{v_{\max}}{L_{00}} \right)^3 = \frac{p_y L_{00}}{2EA} \quad (83)$$

which provides a simple nonlinear relationship between v_{\max} and p_y . Eq. (83) does not depend on $\Delta\gamma$, since this quantity is assumed constant on the beam length. Using Eq. (83) with $\Delta T_x = 0$ and supposing $h/L_{00} = 0.01/0.5 = 0.02$, $c = 1/12$, $A = 1\text{cm}^2$ and $p_y = Af_{Y0}$ daN/cm like in Section 5.3, one obtains the v_{\max} values collected in the first column of Table 7. Moreover, by means of (80), it is easy to compute $\eta = \eta_\varepsilon$ and η^p . These values are good estimations of the corresponding quantities computed from the numerical tests (Table 3). The difference between η estimated as in (80) and in the numerical analysis is due to the shear stress as well as the $\Pi_{Y\gamma}$ component. On the other hand, η_{xx} issued by the numerical analysis and the analytical estimation of $\eta = \eta_\varepsilon$ according to (80) are very close. The corresponding co-ordinates in the strain-rotation domain are reported in the fifth and the sixth column of Table 7 (see also Figure 9). The influence of a thermal field with $\alpha = 10^{-5} \text{ }^\circ\text{C}^{-1}$, $\Delta T_x = 20^\circ\text{C}$ and $h\Delta\gamma = 10^\circ\text{C}$ on the strain can be estimated as follows: Eq. (83) with the true temperature field is used to compute v_{\max} , then η_ε and $\eta_{\Delta T}$ are separately evaluated, as well as their ratio A_α (see the seventh column of

Table 8). A more useful comparison may be done by the ratio between v_{\max} of this last case and v_{\max} for the case $\Delta T_x = 0$ at the same level of p_y (see the last column of Table 8). This ratio correctly neglects the influence of $\Delta\gamma$.

8.3 Equilibrium around a prestressed configuration: strong form equations

By subtracting Eq. (72) written at the state V_1 from the same equation at the state V_0 , one obtains

$$\begin{cases} - \left(EA \left(u'_{01} + \frac{1}{2} v'^2_{01} + v'_{01} v'_0 \right) \right)' = p_{1,x} - \rho_{00} A \ddot{u}_{01} \\ (EJv''_{01})'' - (v'_{01} A \Pi_{0x})' \\ - \left((v'_{01} + v'_0) \left[EA \left(u'_{01} + \frac{1}{2} v'^2_{01} + v'_{01} v'_0 \right) \right] \right)' = -q'_1 + p_{1,y} - \rho_{00} A \ddot{v}_{01} \end{cases} \quad (84)$$

where u_0, v_0 are the axial and transversal displacements characterizing the statically prestressed configuration, measured between V_{00} and V_0 ; $T_{0,x} - T_{0i,x}$ is the axial temperature field; $\Pi_{0,x} = EA \left(u'_0 - \alpha (T_{0,x} - T_{0i,x}) + \frac{1}{2} v'^2_0 \right)$ is the static axial prestress; the unknowns $u_{01} = u_1 - u_0$, $v_{01} = v_1 - v_0$ are the displacements between the dynamic configuration V_1 and the static one. The first and the second term of the bending equation (84-2) are linear. The second one is related to the static configuration V_0 (u_0, v'_0 and $T_{0,x}$): it is the effect of the static prestress due to *external* static loads, *i.e.* it is not self-equilibrated and is associated with a body configuration V_0 different from $V_{0i} = V_{00}$. The bending equation in (84) contains a term coupling u'_{01} with the static and the dynamic rotations v'_0 and v'_{01} . This term seems to be of paramount importance since experimental investigations (Treysède, 2006) prove that pre-bending effect v'_0 play a crucial role, and not only prestress. When u'_{01} is different from zero, this term cannot be neglected in front of the static prestress contribution associated with Π_{0x} . This situation occurs when a pulsating axial loading $P_X(t)$ is applied at one end of the beam, inducing parametric resonance. In this case, the first equation in (84) becomes

$$EA \left(u'_{01} + \frac{1}{2} v'^2_{01} + v'_{01} v'_0 \right) = -P_X(t) + \int_X^{L_{00}} \rho_{00} A \ddot{u}_{01} dX$$

where $p_{1x} = 0$ by assumption. The influence of the axial inertia on the solution is discussed, e.g., by Ribeiro (2001).

9 Moderate rotations and Bernoulli-Navier kinematic assumptions: viscous damping case

The stress when linear viscous damping occurs is given by (see (47), (49) and (55)):

$$\begin{aligned}\boldsymbol{\Pi} &= \boldsymbol{\Pi}^{nd} + \boldsymbol{\Pi}^d & \boldsymbol{\Pi}^{nd} &= \boldsymbol{\Pi}_{0i} + \mathbf{D} : (\mathbf{E} - \sum_{k=1}^{n_\Lambda} \Lambda_k \mathbf{R}_k) - \mathbf{A} (T - T_{0i}) \\ \boldsymbol{\Pi}^d &= \mathbf{F} : \frac{d\tilde{\mathbf{E}}}{dt} = \mathbf{F} : \left(\frac{d\mathbf{E}}{dt} - \sum_{k=1}^{n_\Lambda} \dot{\Lambda}_k \mathbf{R}_k - \sum_{k=1}^{n_\Lambda} \lambda_k \mathbf{R}_k \right)\end{aligned}$$

where the Lagrange multipliers can be computed imposing the constraints $\mathbf{R}_k : \boldsymbol{\Pi}^{nd} = 0$ and $\mathbf{R}_k : \boldsymbol{\Pi}^d = 0$ $k = 1, 2$, where \mathbf{R}_1 and \mathbf{R}_2 are given in (66). Since the two parts of the stress must be separately equal to zero, the Lagrange multipliers Λ_1, Λ_2 do not change with respect to the non-dissipative case, and they are given in Eq. (69). Hence, their time derivatives can be computed under the assumption, previously discussed, that the time variations of the temperature field are very small compared to those of displacements. Hence, $\dot{\Lambda}_1 = \nu s + v' \dot{v}'$ and $\dot{\Lambda}_2 = \nu s$, with $s = \dot{u}' - Y \dot{v}'' + \dot{v}' v'$. As a result, one has

$$\frac{d\tilde{\mathbf{E}}}{dt} = \frac{d\bar{\mathbf{E}}}{dt} - \sum_{k=1}^{n_\Lambda} \lambda_k \mathbf{R}_k = \begin{bmatrix} s & 0 & 0 \\ 0 & -\nu s - \lambda_1 & 0 \\ 0 & 0 & -\nu s - \lambda_2 \end{bmatrix}$$

Let us assume that damping is proportional to the stiffness and to the mass, *viz.* $\mathbf{F} = \alpha_\xi \rho_{00} \mathbf{I} + \beta_\xi \mathbf{D} = \alpha_\xi \rho_{00} \mathbf{I} + \beta_\xi (\lambda_1 \otimes \mathbf{1} + 2\mu \mathbf{I}) = \lambda_d \mathbf{1} \otimes \mathbf{1} + 2\mu_d \mathbf{I}$, with $\lambda_d = \beta_\xi \lambda$ and $2\mu_d = \alpha_\xi \rho_{00} + 2\mu \beta_\xi$. Then, the constraints on the dissipative stress read

$$\begin{aligned}\left(\mathbf{F} : \frac{d\tilde{\mathbf{E}}}{dt} \right)_{YY} &= \alpha_\xi \rho_{00} (-\nu s - \lambda_1) + \beta_\xi \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)(-\lambda_1) + \nu(-\lambda_2)] = 0 \\ \left(\mathbf{F} : \frac{d\tilde{\mathbf{E}}}{dt} \right)_{ZZ} &= \alpha_\xi \rho_{00} (-\nu s - \lambda_2) + \beta_\xi \frac{E}{(1+\nu)(1-2\nu)} [\nu(-\lambda_1) + (1-\nu)(-\lambda_2)] = 0\end{aligned}$$

It entails $\lambda_1 = \lambda_2 = \frac{-\alpha_\xi \rho_{00} \nu}{\alpha_\xi \rho_{00} + \beta_\xi \frac{E}{(1+\nu)(1-2\nu)}} s$, $\Pi_{YY}^d = \Pi_{ZZ}^d = 0$ and $\Pi_{XX}^d = \left(\mathbf{F} : \frac{d\tilde{\mathbf{E}}}{dt} \right)_{XX} = c_\xi s$ with

$$c_\xi = \frac{(\beta_\xi E)^2 + \alpha_\xi \rho_{00} \beta_\xi E (2-\nu) + (1+\nu)(1-2\nu)(\alpha_\xi \rho_{00})^2}{\beta_\xi E + (1+\nu)(1-2\nu)\alpha_\xi \rho_{00}}$$

This expression relates the "structural" viscous damping coefficient c_ξ and the "material" parameters $\alpha_\xi, \beta_\xi, \rho_{00}, E$ and ν . The virtual work of dissipative

forces becomes (see Eq. (56))

$$\mathcal{W}_i^d = \mathcal{W}_i(\mathbf{\Pi}^d, \mathbf{U}, \delta \mathbf{U}) = - \int_{V_{00}} \Pi_{XX}^d (\delta u' - Y \delta v'' + v' \delta v') dV$$

and the strong form equations read

$$\begin{cases} - \left[EA \left(u' - \alpha (T_x - T_{0i,x}) + \frac{1}{2} v'^2 + c_\xi (\dot{u}' + v' \dot{v}') \right) \right]' = p_x - \rho_{00} A \ddot{u} \\ (EJ (v'' - \alpha (\gamma - \gamma_{0i}) + c_\xi \dot{v}''))'' \\ - \left(v' \left[EA \left(u' - \alpha (T_x - T_{0i,x}) + \frac{1}{2} v'^2 + c_\xi (\dot{u}' + v' \dot{v}') \right) \right] \right)' = -q' + p_y - \rho_{00} A \ddot{v} \end{cases} \quad (85)$$

When $\mathbf{F} = \beta_\xi \mathbf{D}$, i.e. damping proportional to the stiffness, one has $\lambda_1 = \lambda_2 = 0$ and $c_\xi = \beta_\xi E$. Moreover, around a static configuration with prestress $\Pi_{0x} = E \left(u'_0 - \alpha (T_{0x} - T_{0i,x}) + \frac{1}{2} v_0'^2 \right)$, one has

$$\begin{cases} - \left[EA \left(u'_{01} + \frac{1}{2} v_{01}'^2 + v'_0 v'_{01} + c_\xi (\dot{u}'_{01} + (v'_0 + v'_{01}) \dot{v}'_{01}) \right) \right]' = p_{1x} - \rho_{00} A \ddot{u}_{01} \\ (EJ (v''_{01} + c_\xi \dot{v}''_{01}))'' - (v'_{01} A \Pi_{0x})' \\ - \left((v'_{01} + v'_0) \left[EA \left(u'_{01} + \frac{1}{2} v_{01}'^2 + v'_0 v'_{01} + c_\xi (\dot{u}'_{01} + (v'_0 + v'_{01}) \dot{v}'_{01}) \right) \right] \right)' \\ = -q'_1 + p_{1y} - \rho_{00} A \ddot{v}_{01} \end{cases} \quad (86)$$

10 Conclusions

Based on a Hu-Washizu functional accounting for stress constraints, an original derivation of the dynamic equations for thin prestressed and prestrained structures from continuum mechanics has been presented. The assumption of small strains and moderate rotations, as well as the physical linearization, have been formalized by the notion of strain-rotation domains associated with new strain and rotation global measures. The corresponding approximated expressions for the strain energy have been justified by 2D non-linear finite element investigations. The presence of a generic mechanical or thermal prestress and physically linear viscous damping has been discussed in detail. In particular, damping is introduced by the original definition of a pseudo-potential with stress constraints. Coupled beam equations governing traction and bending for small strains and moderate rotations have been derived by using the proposed general procedure.

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11 Appendix: Lagrange multipliers

In this Section, we show how to compute the Lagrange multipliers. The stress constraints write

$$\mathbf{R}_k: \boldsymbol{\Pi} = \mathbf{R}_k: \left(\boldsymbol{\Pi}_{0i} + \mathbf{D} : \left(\mathbf{E}(\mathbf{U}) - \sum_{l=1}^{n_\Lambda} \Lambda_l \mathbf{R}_l \right) - \mathbf{A} \Delta T \right) = 0 \quad (87)$$

with $k = 1, n_\Lambda$. Eq. (87) is equivalent to

$$\sum_{k=1}^{n_\Lambda} \mathbf{R}_k: \mathbf{D} : \mathbf{R}_l \Lambda_l = \mathbf{R}_k: (\boldsymbol{\Pi}_{0i} + \mathbf{D} : \mathbf{E}(\mathbf{U}) - \mathbf{A} \Delta T)$$

or $\mathbf{K} \cdot \boldsymbol{\Lambda} = \mathbf{B}$, where $\boldsymbol{\Lambda} = (\Lambda_i)_{i=1, n_\Lambda}$, $\mathbf{K} = (k_{ij})_{i,j=1, n_\Lambda}$, $\mathbf{B} = (B_i)_{i=1, n_\Lambda}$ and

$$k_{ij} = \mathbf{R}_i: \mathbf{D} : \mathbf{R}_j, \quad B_i = \mathbf{R}_i: (\boldsymbol{\Pi}_{0i} + \mathbf{D} : \mathbf{E}(\mathbf{U}) - \mathbf{A} \Delta T)$$

By virtue of the symmetry and positivity of \mathbf{D} it is easy to prove that \mathbf{K} is symmetric positive definite and therefore invertible, provided that tensors \mathbf{R}_k are linearly independent. Hence, the Lagrange multipliers read $\mathbf{\Lambda} = \mathbf{K}^{-1} \cdot \mathbf{B}$. Then, the assumptions (22), (23) and (24) prove that $\mathbf{\Lambda} = \mathbf{\Lambda}^{(1)} + \mathbf{\Lambda}^{(2p)} + \mathbf{\Lambda}^{(1+p)} + \mathbf{\Lambda}^{(2)}$. In particular, the constraints (66) leads to

$$\mathbf{K} = \begin{pmatrix} \lambda + 2\mu & \lambda \\ \lambda & \lambda + 2\mu \end{pmatrix}$$

and

$$\begin{bmatrix} \Lambda_1 \\ \Lambda_2 \end{bmatrix} = \frac{1}{(\lambda + 2\mu)^2 - \lambda^2} \begin{pmatrix} \lambda + 2\mu & -\lambda \\ -\lambda & \lambda + 2\mu \end{pmatrix} \cdot \begin{bmatrix} (\mathbf{\Pi}_{0i} + \mathbf{D} : \mathbf{E}(\mathbf{U}) - \mathbf{A}\Delta T)_{YY} \\ (\mathbf{\Pi}_{0i} + \mathbf{D} : \mathbf{E}(\mathbf{U}) - \mathbf{A}\Delta T)_{ZZ} \end{bmatrix}$$

For the dissipative stress, one has analogous constraints

$$\mathbf{R}_k : \mathbf{\Pi}^d = \mathbf{R}_k : \mathbf{F} : \left(\frac{d\mathbf{E}(\mathbf{U})}{dt} - \sum_{l=1}^{n_\Lambda} \frac{d\Lambda_l}{dt} \mathbf{R}_l - \sum_{l=1}^{n_\Lambda} \lambda_l \mathbf{R}_l \right) = 0$$

Then $\boldsymbol{\lambda} = \bar{\mathbf{K}}^{-1} \cdot \bar{\mathbf{B}} = \boldsymbol{\lambda}^{(1)} + \boldsymbol{\lambda}^{(2p)} + \boldsymbol{\lambda}^{(1+p)} + \boldsymbol{\lambda}^{(2)}$, where $\boldsymbol{\lambda} = (\lambda_i)_{i=1, n_\Lambda}$ and

$$\begin{aligned} \bar{\mathbf{K}} &= (\bar{k}_{ij})_{i,j=1, n_\Lambda}, & \bar{k}_{ij} &= \mathbf{R}_i : \mathbf{F} : \mathbf{R}_j \\ \bar{\mathbf{B}} &= (\bar{B}_i)_{i=1, n_\Lambda}, & \bar{B}_i &= \mathbf{R}_i : \mathbf{F} : \left(\frac{d\mathbf{E}(\mathbf{U})}{dt} - \sum_{l=1}^{n_\Lambda} \frac{d\Lambda_l}{dt} \mathbf{R}_l \right) \end{aligned}$$

$\bar{\mathbf{K}}$ is symmetric positive definite. For a beam

$$\bar{\mathbf{K}} = \begin{pmatrix} \lambda_d + 2\mu_d & \lambda_d \\ \lambda_d & \lambda_d + 2\mu_d \end{pmatrix}$$

and

$$\begin{bmatrix} \lambda_1 \\ \lambda_2 \end{bmatrix} = \frac{1}{(\lambda_d + 2\mu_d)^2 - \lambda_d^2} \begin{pmatrix} \lambda_d + 2\mu_d & -\lambda_d \\ -\lambda_d & \lambda_d + 2\mu_d \end{pmatrix} \cdot \begin{bmatrix} \left(\mathbf{F} : \left(\frac{d\mathbf{E}(\mathbf{U})}{dt} - \sum_{l=1}^{n_\Lambda} \frac{d\Lambda_l}{dt} \mathbf{R}_l \right) \right)_{YY} \\ \left(\mathbf{F} : \left(\frac{d\mathbf{E}(\mathbf{U})}{dt} - \sum_{l=1}^{n_\Lambda} \frac{d\Lambda_l}{dt} \mathbf{R}_l \right) \right)_{ZZ} \end{bmatrix}$$

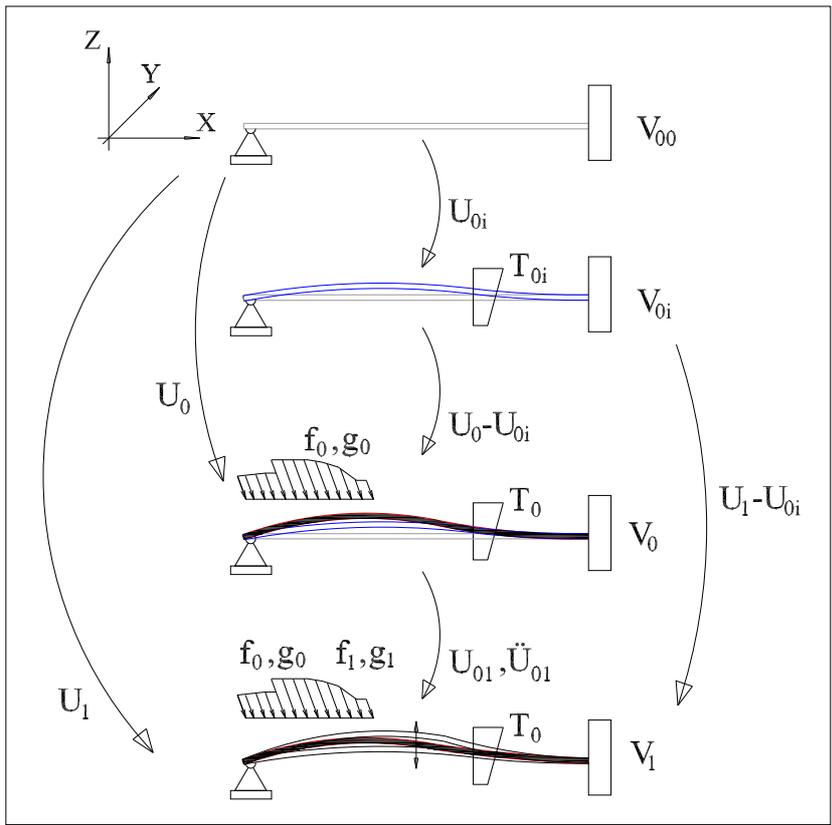


Fig. 1. The configurations of a structure.

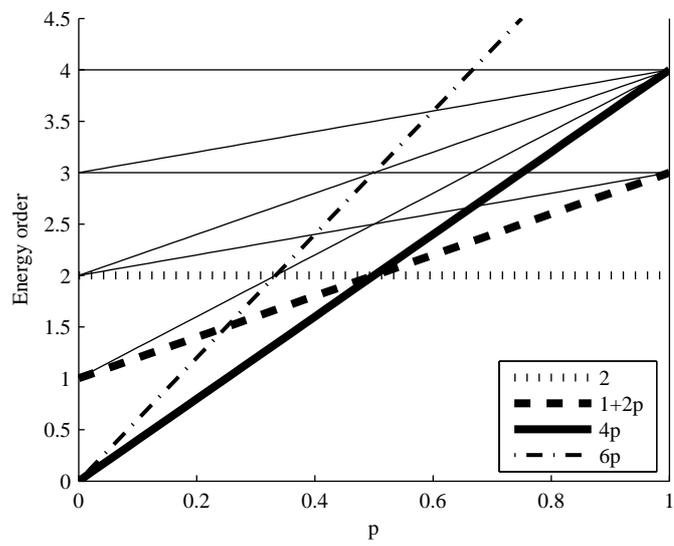


Fig. 2. Orders of the strain energy terms. The dominant terms (2 , $1+2p$ and $4p$) are highlighted. The line corresponding to the order $6p$ represents the most important energy term associated with the non-linear constitutive law (8).

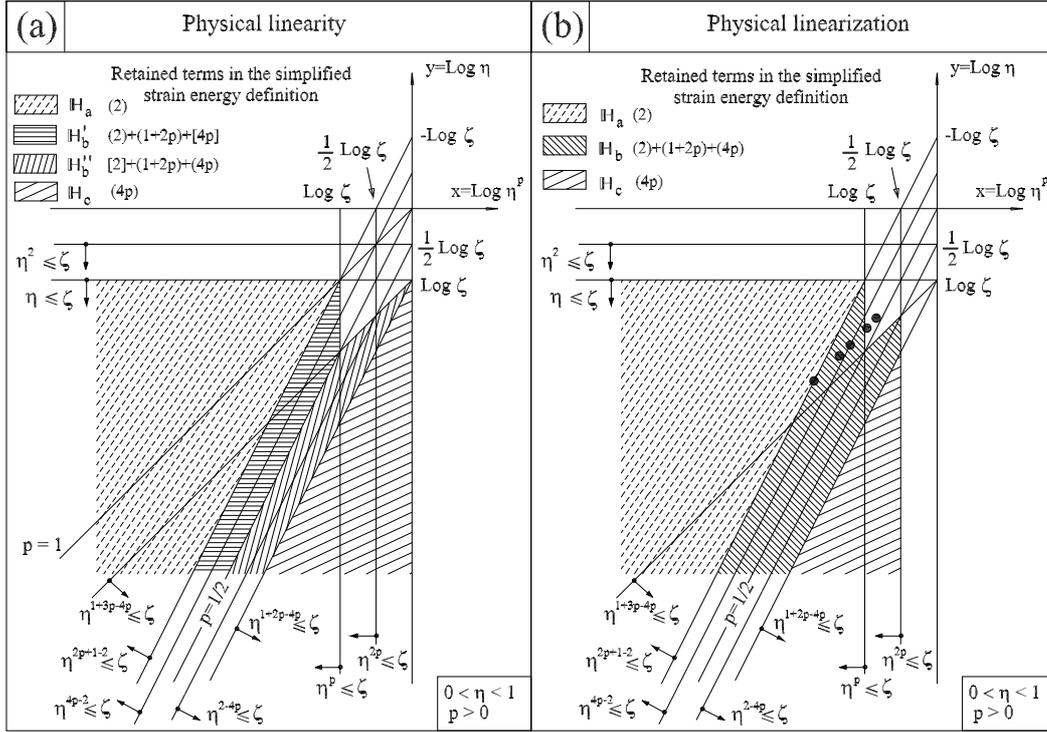


Fig. 3. (a) Strain-rotation domains for a physically linear material. (b) Strain-rotation domains under the assumption of physical linearization. The sets \mathbb{H}'_b and \mathbb{H}''_b are merged into the set \mathbb{H}_b . The "strain-rotation points" represent the deformed configurations of the structure of the numerical example discussed in Section 5.3 . In both Figures (a) and (b), the dominant terms of the strain energy are indicated.

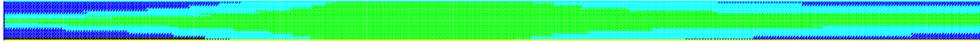
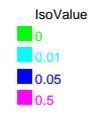


Fig. 4. Volume vertical force $f_{Y0} = -1 \text{ daN/cm}^3$. Map of the strain energy density $\Psi(\mathbf{E}) [\text{daNcm cm}^{-3}]$, with $\mathbf{E} = \mathbf{E}(\mathbf{U})$.

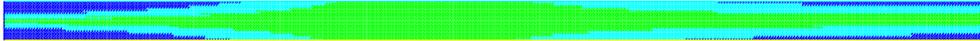
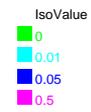


Fig. 5. Volume vertical force $f_{Y0} = -1 \text{ daN/cm}^3$. Map of the *approximated* strain energy density $\Psi(\mathbf{E}) [\text{daNcm cm}^{-3}]$, with $\mathbf{E} = \mathbf{E}^{(b)}(\mathbf{U})$.

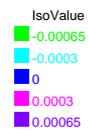


Fig. 6. Volume vertical force $f_{Y0} = -1 \text{ daN/cm}^3$. Map of the relative difference between the exact and the approximated strain energy density, viz. $\frac{\Psi(\mathbf{E}(\mathbf{U})) - \Psi(\mathbf{E}^{(b)}(\mathbf{U}))}{\Psi(\mathbf{E}(\mathbf{U}))}$.



Fig. 7. Surface vertical force $g_{Y0} = -1 \text{ daN/cm}^2$. Map of the relative difference between the exact and the approximated strain energy density, viz. $\frac{\Psi(\mathbf{E}(\mathbf{U})) - \Psi(\mathbf{E}^{(b)}(\mathbf{U}))}{\Psi(\mathbf{E}(\mathbf{U}))}$.

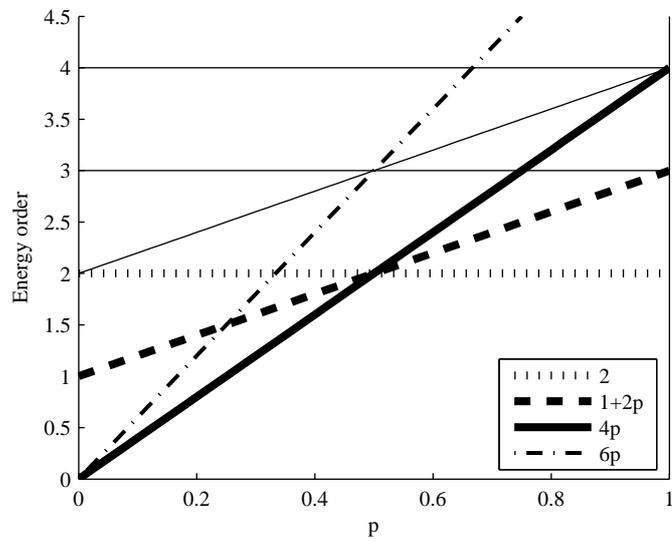


Fig. 8. Navier kinematic assumptions and orders of the strain energy terms. The dominant terms (2 , $1 + 2p$ and $4p$) are highlighted. The line corresponding to the order $6p$ represents the most important energy term associated with the non-linear constitutive law (8). Note that the terms of order $3 + p$, $2 + p$ and $1 + 3p$ no longer appear (compare with Figure 2).

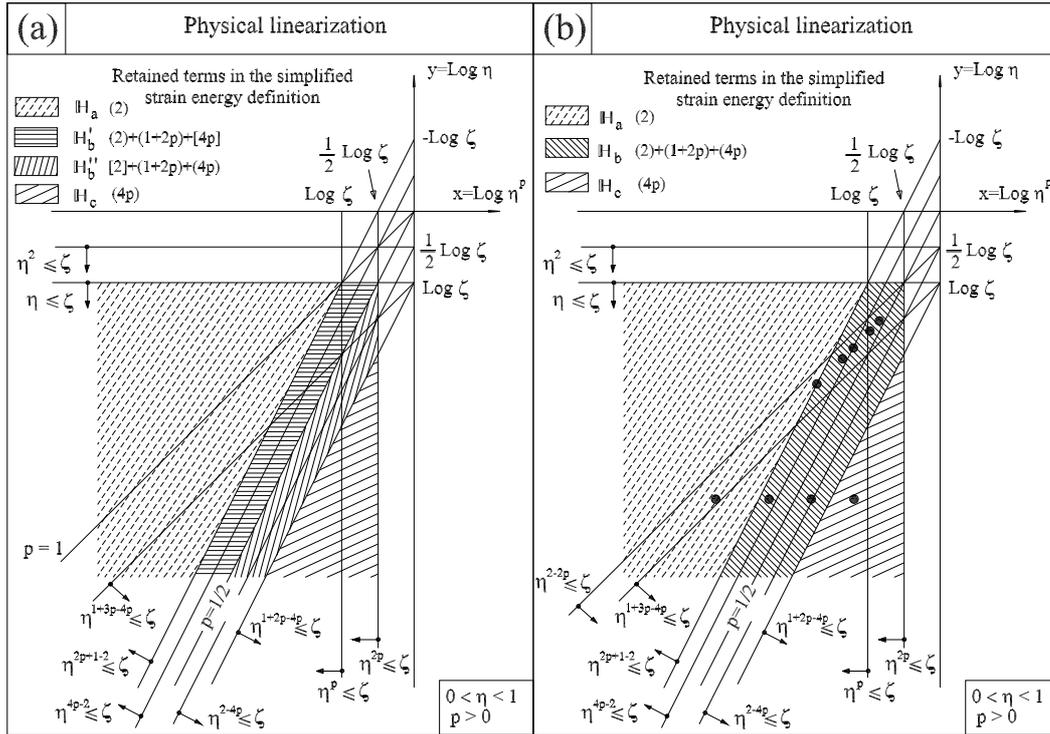


Fig. 9. Physical linearization and Navier kinematic assumptions. (a) Strain rotation domains. (b) The strain-rotation domains, where \mathbb{H}'_b and \mathbb{H}''_b are merged. The "strain-rotation points" represent the deformed configurations of the structures of the examples discussed in Section 8.2 .

	V_{00}	V_{0i}	V_0	V_1
\mathbf{U}	$\mathbf{0}$	$\mathbf{0}$	\mathbf{U}_0	$\mathbf{U}_1 = \mathbf{U}_0 + \mathbf{U}_{01}$
\mathbf{E}	–	$\mathbf{0}$	$\mathbf{E}(\mathbf{U}_0)$	$\mathbf{E}(\mathbf{U}_1)$
$\delta\mathbf{E}$	–	$\delta\mathbf{E}(\mathbf{0}, \delta\mathbf{U}) = \delta\boldsymbol{\varepsilon}$	$\delta\mathbf{E}(\mathbf{U}_0, \delta\mathbf{U})$	$\delta\mathbf{E}(\mathbf{U}_1, \delta\mathbf{U})$
T	–	T_{0i}	T_0	T_0
$\boldsymbol{\Pi}$	–	$\boldsymbol{\Pi}_{0i}$	$\boldsymbol{\Pi}_0 = \boldsymbol{\Pi}_{0i} + \mathbf{D} : \mathbf{E}(\mathbf{U}_0)$ $-\mathbf{A}(T_0 - T_{0i})$	$\boldsymbol{\Pi}_{0i} + \mathbf{D} : \mathbf{E}(\mathbf{U}_1)$ $-\mathbf{A}(T_0 - T_{0i})$
\mathbf{f}	–	$\mathbf{0}$	\mathbf{f}_0	$\mathbf{f}_0 + \mathbf{f}_1(t)$
\mathbf{g}	–	$\mathbf{0}$	\mathbf{g}_0	$\mathbf{g}_0 + \mathbf{g}_1(t)$
$\bar{\mathbf{U}}$	$\mathbf{0}$	$\mathbf{0}$	$\bar{\mathbf{U}}_0$	$\bar{\mathbf{U}}_0 + \bar{\mathbf{U}}_1(t)$
$\ddot{\mathbf{U}}$	–	$\mathbf{0}$	$\mathbf{0}$	$\ddot{\mathbf{U}}_1 = \ddot{\mathbf{U}}_{01}$

Table 1

Displacements, strains, temperatures and stresses associated to each structural configuration. $\mathbf{U}_{0i} = \bar{\mathbf{U}}_{0i} = \mathbf{0}$ in the analyses of this paper, since no geometric imperfection is considered.

$(\mathbf{\Pi}) \setminus (\mathbf{E})$	$\boldsymbol{\varepsilon}$	$\mathbf{r}^T \cdot \mathbf{r}$	$(\boldsymbol{\varepsilon}^T \cdot \mathbf{r})$ $(\mathbf{r}^T \cdot \boldsymbol{\varepsilon})$	$\boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon}$
$\boldsymbol{\varepsilon}, \alpha\Delta T$	2	$1 + 2p$	$2 + p$	3
$\mathbf{r}^T \cdot \mathbf{r}$	$1 + 2p$	$4p$	$1 + 3p$	$2 + 2p$
$\boldsymbol{\varepsilon}^T \cdot \mathbf{r}$ $\mathbf{r}^T \cdot \boldsymbol{\varepsilon}$	$2 + p$	$1 + 3p$	$2 + 2p$	$3 + p$
$\boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon}$	3	$2 + 2p$	$3 + p$	4
$\boldsymbol{\varepsilon} \cdot \boldsymbol{\varepsilon}, (\alpha\Delta T)^2$	3	$2 + 2p$	$3 + p$	4
$(\mathbf{r}^T \cdot \mathbf{r}) \cdot (\mathbf{r}^T \cdot \mathbf{r})$	$1 + 4p$	$6p$	$1 + 5p$	$2 + 4p$
$(\boldsymbol{\varepsilon}^T \cdot \mathbf{r}) \cdot (\boldsymbol{\varepsilon}^T \cdot \mathbf{r})$ $(\mathbf{r}^T \cdot \boldsymbol{\varepsilon}) \cdot (\mathbf{r}^T \cdot \boldsymbol{\varepsilon})$	$3 + 2p$	$2 + 4p$	$3 + 3p$	$4 + 2p$
$(\boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon}) \cdot (\boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon})$	5	$4 + 2p$	$5 + p$	6
$(\boldsymbol{\varepsilon}) \cdot (\mathbf{r}^T \cdot \mathbf{r}), \alpha\Delta T (\mathbf{r}^T \cdot \mathbf{r})$	$2 + 2p$	$1 + 4p$	$2 + 3p$	$3 + 2p$
$\boldsymbol{\varepsilon} \cdot (\boldsymbol{\varepsilon}^T \cdot \mathbf{r}), \alpha\Delta T (\boldsymbol{\varepsilon}^T \cdot \mathbf{r})$ $\boldsymbol{\varepsilon} \cdot (\mathbf{r}^T \cdot \boldsymbol{\varepsilon}), \alpha\Delta T (\mathbf{r}^T \cdot \boldsymbol{\varepsilon})$	$3 + p$	$2 + 3p$	$3 + 2p$	$4 + p$
$\boldsymbol{\varepsilon} \cdot \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon}, \alpha\Delta T \boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon}$	4	$3 + 2p$	$4 + p$	5
$(\mathbf{r}^T \cdot \mathbf{r}) \cdot (\boldsymbol{\varepsilon}^T \cdot \mathbf{r})$ $(\mathbf{r}^T \cdot \mathbf{r}) \cdot (\mathbf{r}^T \cdot \boldsymbol{\varepsilon})$	$2 + 3p$	$1 + 5p$	$2 + 4p$	$3 + 3p$
$(\mathbf{r}^T \cdot \mathbf{r}) \cdot (\boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon})$	$3 + 2p$	$2 + 4p$	$3 + 3p$	$4 + 2p$
$(\boldsymbol{\varepsilon}^T \cdot \mathbf{r}) \cdot (\boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon})$ $(\mathbf{r}^T \cdot \boldsymbol{\varepsilon}) \cdot (\boldsymbol{\varepsilon}^T \cdot \boldsymbol{\varepsilon})$	$4 + p$	$3 + 3p$	$4 + 2p$	$5 + p$

Table 2

Orders of the terms appearing in the strain energy expressions (30)-(32) and (42-2), representing the physically linear and non-linear case, respectively.

$ f_{Y,0} $ [daN/cm ³]	$ v_{\max} $ [cm]	$\max \varepsilon_{xx} $	η_{xx}	η	η^p	x	y	p
0.1	0.0092	$6.46e-5$	$1.52e-5$	$1.59e-5$	$4.05e-4$	-3.4	-4.8	0.71
0.5	0.0459	$3.20e-4$	$7.58e-5$	$7.93e-5$	$2.02e-3$	-2.7	-4.1	0.66
1	0.0914	$6.34e-4$	$1.51e-4$	$1.58e-4$	$4.03e-3$	-2.39	-3.8	0.63
3	0.263	$1.85e-3$	$4.37e-4$	$4.66e-4$	$1.157e-2$	-1.94	-3.33	0.58
6	0.4745	$3.58e-3$	$8.03e-4$	$8.96e-4$	$2.09e-2$	-1.68	-3.05	0.55

Table 3

Clamped-clamped beam (2D plain stress analysis). Numerical results for different values of the vertical volume load f_{Y0} .

$ f_{Y,0} $ [daN/cm ³]	$\rho_1 = \eta^{2p-1}$	$\rho_2 = \eta^{4p-2}$	\mathcal{F} [daN cm]	$\mathcal{F}^{(b)}$ [daN cm]	$\frac{\mathcal{F}-\mathcal{F}^{(b)}}{\mathcal{F}}$
0.1	0.0103	0.00011	0.00613653	0.00613653	~ 0
0.5	0.0515	0.00265	0.153092	0.153091	$6.5e - 6$
1	0.1025	0.01052	0.608433	0.60842	$2.1e - 5$
3	0.2873	0.0825	5.15335	5.15244	$1.8e - 4$
6	0.4875	0.238	17.8727	17.8626	$5.7e - 4$

Table 4

Clamped-clamped beam (2D plain stress analysis). Energies for different values of the vertical volume load f_{Y0} .

$ g_{Y,0} $ [daN/cm ²]	$ v_{\max} $ [cm]	$\max \varepsilon_{xx} $	η_{xx}	η	η^p	x	y	p
0.1	0.0092	$6.46e-5$	$1.52e-5$	$1.59e-5$	$4.052e-4$	-3.39	-4.80	0.71
0.5	0.0459	$3.20e-4$	$7.58e-5$	$7.93e-5$	$2.02e-3$	-2.69	-4.10	0.66
1	0.09142	$6.34e-4$	$1.51e-4$	$1.58e-4$	$4.029e-3$	-2.39	-3.80	0.63
3	0.263	$1.85e-3$	$4.37e-4$	$4.67e-4$	$1.158e-2$	-1.94	-3.33	0.58
6	0.475	$3.58e-3$	$8.04e-4$	$8.98e-4$	$2.092e-2$	-1.68	-3.05	0.55

Table 5

Clamped-clamped beam (2D plain stress analysis). Numerical results for different values of the vertical surface load g_{Y0} .

$ g_{Y,0} $ [daN/cm ²]	$\rho_1 = \eta^{2p-1}$	$\rho_2 = \eta^{4p-2}$	\mathcal{F} [daN cm]	$\mathcal{F}^{(b)}$ [daN cm]	$\frac{\mathcal{F}-\mathcal{F}^{(b)}}{\mathcal{F}}$
0.1	0.01035	0.00011	0.0061368	0.0061368	~ 0
0.5	0.0515	0.00265	0.153124	0.153123	$6.53e - 6$
1	0.1025	0.0105	0.608689	0.608676	$2.14e - 5$
3	0.2870	0.0824	5.15922	5.15831	$1.76e - 4$
6	0.4876	0.2377	17.9055	17.8953	$5.70e - 4$

Table 6

Clamped-clamped beam (2D plain stress analysis). Energies for different values of the vertical surface load g_{Y0} .

$ p_y $ [daN/cm]	$ v_{\max, \Delta T=0} $ [cm]	η	η^p	x	y	p
0.1	0.009165	$1.477e-5$	$4.072e-4$	-3.39	-4.83	0.702
0.5	0.04576	$7.377e-5$	$2.033e-3$	-2.69	-4.13	0.651
1	0.0911	$1.469e-4$	$4.047e-3$	-2.39	-3.83	0.624
3	0.2616	$4.243e-4$	$1.162e-2$	-1.935	-3.37	0.574
6	0.4714	$7.754e-4$	$2.094e-2$	-1.68	-3.11	0.54

Table 7

Clamped-clamped beam with co-sinusoidal deformed shape. Numerical results for different values of the vertical load p_y .

$ p_y $ [daN/cm]	$ v_{\max} $ [cm]	η_ε	$\eta_{\Delta T}$	η	η^p	A_α	$\frac{ v_{\max} }{ v_{\max, \Delta T=0} }$
0.1	0.010808	$1.742e-5$	$6.083e-4$	$6.260e-4$	$4.802e-4$	34.9	1.180
0.5	0.053906	$8.690e-5$	$6.083e-4$	$6.952e-4$	$2.395e-3$	7.0	1.178
1	0.1070	$1.726e-4$	$6.083e-4$	$7.809e-4$	$4.754e-3$	3.52	1.174
3	0.3003	$4.8801e-4$	$6.083e-4$	$1.096e-3$	$1.334e-2$	1.25	1.148
6	0.52242	$8.632e-4$	$6.083e-4$	$1.472e-3$	$2.321e-2$	0.705	1.108

Table 8

Clamped-clamped beam with co-sinusoidal deformed shape. The vertical load is the same as in Table 3. Moreover, a thermal field with $\Delta T_x = 20^\circ C$ and $h\Delta\gamma = 10^\circ C$ has been added.