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Streamflow disaggregation: a nonlinear deterministic approach

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Abstract. This study introduces a nonlinear deterministic approach for streamflow disaggregation. According to this approach, the streamflow transformation process from one scale to another is treated as a nonlinear deterministic process, rather than a stochastic process as generally assumed. The approach follows two important steps: (1) reconstruction of the scalar (streamflow) series in a multi-dimensional phase-space for representing the transformation dynamics; and (2) use of a local approximation (nearest neighbor) method for disaggregation. The approach is employed for streamflow disaggregation in the Mississippi River basin, USA. Data of successively doubled resolutions between daily and 16 days (i.e. daily, 2-day, 4-day, 8-day, and 16-day) are studied, and disaggregations are attempted only between successive resolutions (i.e. 2-day to daily, 4-day to 2-day, 8-day to 4-day, and 16-day to 8-day). Comparisons between the disaggregated values and the actual values reveal excellent agreements for all the cases studied, indicating the suitability of the approach for streamflow disaggregation. A further insight into the results reveals that the best results are, in general, achieved for low embedding dimensions (2 or 3) and small number of neighbors (less than 50), suggesting possible presence of nonlinear determinism in the underlying transformation process. A decrease in accuracy with increasing disaggregation scale is also observed, a possible implication of the existence of a scaling regime in streamflow.

Vogel, 1984; Bras and Rodriguez-Iturbe, 1985; Grygier and Stedinger, 1988; Lin, 1990; Santos and Salas, 1992; Maheepala and Perera, 1996). The essence of such models is to develop a staging framework (e.g. Santos and Salas, 1992), where streamflow sequences are generated at a given level of aggregation and then disaggregated into component flows.

Traditionally, streamflow disaggregation approaches have involved some variant of a linear model of the form

$$X_t = \mathbf{A}Z_t + \mathbf{B}V_t \quad (1)$$

where X_t is the vector of disaggregate variables at time t , Z_t is the aggregate variable, V_t is a vector of independent random innovations (usually drawn from a Gaussian distribution), and \mathbf{A} and \mathbf{B} are parameter matrices. The matrix \mathbf{A} is estimated to reproduce the correlation between aggregate and disaggregate flows, whereas the matrix \mathbf{B} is estimated to reproduce the correlation between individual disaggregate components. The many model variants that have been made available in the literature make different assumptions on the structure and sparsity of these matrices. They also apply, prior to use of Eq. (1), a variety of normalizing transformations to the data to account for the fact that (monthly) streamflow data are seldom normally distributed. Summability (i.e. the requirement that disaggregate variables should add up to the aggregate quantity) has also been an issue in these models, though a few studies have effectively handled this problem in some ways (e.g. Bras and Rodriguez-Iturbe, 1985; Grygier and Stedinger, 1988).

An important aspect that has to be recognized from the above models is that they present a mathematical framework where a joint distribution of disaggregate and aggregate variables is specified. However, the specified model structure is parametric. It is imposed by the form of Eq. (1) and the normalizing transformations applied to the data to represent the marginal distributions. Even though, the parametric approach has been shown to be effective for streamflow disaggregation purposes, they also possess certain important drawbacks, such as the following (Tarboton et al., 1998):

1 Introduction

Streamflow disaggregation has been and continues to be a challenging problem in hydrology. The past few decades have witnessed numerous studies addressing the streamflow disaggregation problem and, consequently, a large number of mathematical models (e.g. Harms and Campbell, 1967; Valencia and Schaake, 1972; Salas et al., 1980; Stedinger and

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1. As Eq. (1) involves linear combinations of random variables, it is compatible mainly with Gaussian distributions (with only a few exceptions). Therefore, if the marginal distribution of the streamflow variables involved is not Gaussian, normalizing transformations are required for each streamflow component, in which case Eq. (1) would be applied to the normalized flow variables. It is often difficult to find a general normalizing transformation and retain statistical properties of the streamflow process in the untransformed multi-variable space; and
2. The linear nature of Eq. (1) limits it from representing any nonlinearity in the dependence structure between variables, except through the normalizing transformation used.

In view of such limitations with the parametric approach, Tarboton et al. (1998) developed a nonparametric approach for streamflow disaggregation. Such a study, in fact, followed the studies by Lall and Sharma (1996) and Sharma et al. (1997), which proposed and demonstrated the use of the nonparametric approach for streamflow simulation. The nonparametric approach eliminates the drawbacks of the parametric approach (Tarboton et al., 1998), since: (1) the necessary joint probability density functions are estimated directly from the historic data using kernel density estimates; (2) the procedures are data driven and relatively automatic and, therefore, nonlinear dependence can be incorporated to the extent suggested by the data; and (3) difficult subjective choices as to appropriate marginal distributions and normalizing transformations are avoided.

With regards to disaggregation in particular, since the basic purpose is to determine the proportions of the aggregate flow to allocate to each subset, the real difference between the parametric and the nonparametric approaches is the following. The parametric approach deals with the allocation problem through a “global” prescription of the associated density function and correlation structure in a transformed data domain, whereas in the nonparametric approach this problem is approached by looking at the relative proportions of the subset variables in a “local” sense. As a result, the nonparametric approach has the ability to better capture (any) variations that may lead to heterogeneous density functions and to adaptively model complex relationships between aggregate and disaggregate flows.

The nonparametric approach, presented by Tarboton et al. (1998), is certainly a significant step forward in the context of streamflow disaggregation (or any other hydrologic analysis), because it not only recognizes the possible nonlinear behavior of the streamflow (disaggregation) phenomenon but also attempts to incorporate the nonlinear dependence of the data. In regards to the issue of nonlinearity, it is appropriate to note that the topic of “nonlinear hydrology” has already witnessed a significant progress in the last decade or so. Among the notable advances that have been made within the area of nonlinear hydrology, the finding of the possible nonlinear deterministic nature of hydrologic phenom-

ena (e.g. Rodriguez-Iturbe et al., 1989) has received arguably the widest attention (both positively and negatively). This is particularly the case in streamflow studies (e.g. Jayawardena and Lai, 1994; Porporato and Ridolfi, 1997; Krasovskaia et al., 1999; Jayawardena and Gurung, 2000; Sivakumar et al., 2001a, 2002a, b; Lisi and Villi, 2001; Islam and Sivakumar, 2002). For further details, the reader is referred to the articles by Sivakumar (2000, 2004).

The above studies have brought encouraging news for hydrologists, in particular streamflow modelers, as they revealed the possible presence of nonlinear determinism in the seemingly highly irregular hydrologic phenomena, suggesting the possibility of accurate short-term predictions. This has further been verified and supported by the near-accurate predictions achieved for streamflow data observed at different river systems (e.g. Porporato and Ridolfi, 1997; Jayawardena and Gurung, 2000; Sivakumar et al., 2001a, 2002a, b; Lisi and Villi, 2001; Islam and Sivakumar, 2002) and also for other hydrologic and geomorphic data, such as lake volume (e.g. Abarbanel and Lall, 1996) and suspended sediment concentration (e.g. Sivakumar, 2002).

In the spirit of such studies, an attempt is made in the present study to use the relevant ideas for streamflow disaggregation purposes. It is appropriate to note that such an attempt is not entirely new to hydrology, as the first author and his colleagues have previously used such ideas for rainfall disaggregation (Sivakumar et al., 2001b). As the present study is, in a way, an extension of the study by Sivakumar et al. (2001b) as far as the field of hydrology is concerned, its originality must only be assessed from a hydrologic problem (i.e. streamflow disaggregation) point of view, rather than from a methodological perspective. Having said that, the study by Sivakumar et al. (2001b) encountered an important problem in implementing the disaggregation procedure, essentially due to the presence of zero rainfall values. It is the authors’ opinion that such a problem is either completely or largely overcome (depending upon the river system) when one is dealing with streamflow data, since the probability of occurrence of no flow is almost zero for large rivers and significantly low for others (compared to the probability of no rain situation). This is particularly the case for annual and monthly streamflow data, used in most of the previous streamflow disaggregation studies.

For the purpose of streamflow disaggregation in the present study, data observed in the Mississippi River basin (at St. Louis, Missouri), USA, are considered (recent research on the flow series from the basin has provided clues to the possible presence of nonlinear deterministic behavior in the underlying dynamics; Sivakumar and Jayawardena, 2002). Streamflow data of successively doubled resolutions (i.e. scales) between daily and 16 days, i.e. daily, 2-day, 4-day, 8-day, and 16-day, are studied. Disaggregations are made only between successive resolutions, i.e. 2-day to daily, 4-day to 2-day, 8-day to 4-day, and 16-day to 8-day. The nonlinear local approximation disaggregation procedure proposed by Sivakumar et al. (2001b), with required modifications for streamflow data, is employed. The accuracy of

disaggregation is measured using four different indicators: (1) correlation coefficient; (2) root mean square error; (3) direct time series plots; and (4) scatter diagrams.

The organization of this paper is as follows. Section 2 presents a brief account of the nonlinear deterministic disaggregation procedure, originally proposed by Sivakumar et al. (2001b). Section 3 presents the details of the Mississippi River basin and the streamflow data considered in this study. Details of the disaggregation analysis carried out, results obtained and their discussion are reported in Sect. 4. Conclusions from the present study and the scope for further research are presented in Sect. 5.

2 Nonlinear deterministic disaggregation procedure

In a recent study, Sivakumar et al. (2001b) proposed a nonlinear deterministic disaggregation approach for rainfall and also demonstrated its effectiveness on the rainfall data observed in the Leaf River basin in Mississippi, USA. As this approach is employed in the present study for streamflow disaggregation, the procedure is described below.

Let us assume that we have a streamflow series X_i , $i=1, 2, \dots, N$, at a certain resolution T_1 , and the task is to obtain the (disaggregated) streamflow values $(Z_i)_k$, $k=1, 2, \dots, p$, at a higher (or finer) resolution T_2 , where $p=T_1/T_2$. Let us also assume that the values of X_i are distributed into $(Z_i)_k$ according to $(Z_i)_k=(W_i)_k * X_i$, where $(W_i)_k$ are the distributions of weights of X_i to $(Z_i)_k$ and $\sum_{k=1}^p (W_i)_k = 1$. As the present study considers, for the purpose of convenience, only streamflow data at successively doubled temporal resolutions for disaggregation purposes, the parameter p is given by $p=T_1/T_2=2$. A schematic diagram depicting such a disaggregation situation is presented in Fig. 1.

As the purpose is streamflow disaggregation (rather than prediction), the procedure is simplified by working with only the available streamflow series (rather than predicting/generating the future streamflow values and disaggregating them). Let us now assume that information is available about the history of distributions of weights $(W_i)_k$ (or X_i and $(Z_i)_k$), $i=1, 2, \dots, n$, where $n < N$, and the task at hand is to obtain the distributions of weights $(W_i)_k$ and, hence, the streamflow values $(Z_i)_k$ at a finer resolution, where $i=n+1, n+2, \dots, N$ and $k=1, 2, \dots, p$. In other words, streamflow values X_i , $i=1, 2, \dots, n$, are used as the “training set” for the model to learn the dynamics of disaggregation (or transformation), whereas streamflow values X_i , $i=n+1, n+2, \dots, N$ are used as the testing test to assess the model performance. Based on these information, the nonlinear deterministic disaggregation approach is developed as follows. The procedure adopted in the model is somewhat similar to the one generally used for prediction of nonlinear deterministic time series (e.g. Farmer and Sidorowich, 1987; Casdagli, 1989, 1991).

As the basic problem is to understand the dynamic changes that take place in the streamflow transformation process, it is first of all necessary to represent the evolution of the un-

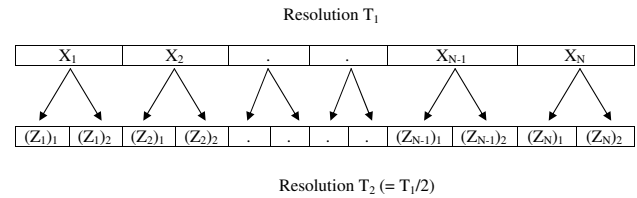


Fig. 1. Schematic representation of distributions of weights of streamflow transformation from one resolution to another.

derlying mechanism(s). This can be done by reconstructing the multi-dimensional phase-space from the available single-dimensional series, X_i , where $i=1, 2, \dots, N$, as follows (e.g. Takens, 1981):

$$Y_j = (X_j, X_{j+\tau}, X_{j+2\tau}, \dots, X_{j+(m-1)\tau}) \tag{2}$$

where $j=1, 2, \dots, N - (m - 1)\tau/\Delta t$, m is the dimension of the vector Y_j , called as embedding dimension, and τ is the delay time taken to be some suitable multiple of the sampling time Δt . Such a reconstruction (in a correct m dimension) allows making connection between the current state (i.e. Y_j) and the future state (i.e. Y_{j+T}) through a functional relationship

$$Y_{j+T} = f_T(Y_j) \tag{3}$$

An appropriate expression for f_T (i.e. F_T) is found using a local approximation technique (e.g. Farmer and Sidorowich, 1987), which entails the subdivision of the f_T domain into many subsets (neighborhoods), each of which identifies some approximations F_T , valid only in that subset.

With the above information, let us now consider determining how the streamflow data X_{n+1} (i.e. the value at time $n+1$) at resolution T_1 is disaggregated to values at resolution T_2 , i.e. determining the distributions of weights $(W_{n+1})_k$. The phase-space for this case can be reconstructed using the series X_i , $i=1, 2, \dots, n+1$, according to Eq. (2), where $j=1, 2, \dots, (n+1) - (m - 1)\tau/\Delta t$. Then, the disaggregation of X_{n+1} is made based on Y_j , $j=(n+1) - (m - 1)\tau/\Delta t$, and its neighbors Y'_j for all $j' < j$. The neighbors of Y_j are found on the basis of the minimum values of $\|Y_j - Y'_j\|$. If only one neighbor is considered, then the distributions of weights $(W_{n+1})_k$ of X_{n+1} would be the distributions of weights of the corresponding element X_j in the nearest vector Y'_j . This is called the zeroth-order approximation. An improvement to this is the first-order approximation, which considers k' number of neighbors, and the distributions of weights $(W_{n+1})_k$ of X_{n+1} is taken as an average of the k' values' distributions of weights of the corresponding elements X_j in the nearest vectors. The optimal value of k' (i.e. k'_{opt}) is determined by trial and error (e.g. Casdagli, 1991). Having determined the weights, the disaggregation of flow value X_{n+1} observed at the resolution T_1 to flow values $(Z_{n+1})_k$ at resolution T_2 is obtained according to $(Z_{n+1})_k=(W_{n+1})_k * X_{n+1}$.

The above procedure is repeated to obtain the distributions of weights of streamflow values $X_{n+2}, X_{n+3}, \dots, X_N$, i.e. $(W_{n+2})_k, (W_{n+3})_k, \dots, (W_N)_k$, and hence the streamflow values at the resolution T_2 , i.e. $(Z_{n+2})_k, (Z_{n+3})_k, \dots, (Z_N)_k$. The

Table 1. Statistics of Streamflow Data of Different Temporal Resolutions in the Mississippi River Basin at St. Louis, Missouri (Unit= $\text{m}^3\text{s}^{-1}\text{d}_s$, where d_s is the scale of observation in days).

Statistic	Daily	2-day	4-day	8-day	16-day
Number of data	8192	4096	2048	1024	512
Mean	5513.9	11027.7	22055.4	44110.8	88221.6
Standard deviation	3462.6	6908.1	13713.4	26995.2	52251.5
Maximum value	24100	48100	94300	183300	338500
Minimum value	980	1990	4030	8280	17430
Coefficient of variation	0.6280	0.6264	0.6218	0.6120	0.5923
Skew	1.4779	1.4771	1.4704	1.4559	1.4122
Kurtosis	2.5031	2.5081	2.5078	2.5066	2.3898

accuracy of disaggregation can be evaluated by comparing the actual and the modeled disaggregated values using any of the standard statistical measures. In the present study, the disaggregation accuracy is evaluated using correlation coefficient (CC) and root mean square error (RMSE). Time series plots and scatter diagrams are also used to choose the best disaggregation results, among a large combination of results achieved with varying number of neighbors and embedding dimensions.

3 Study area and data used

In the present study, river flow data observed in the Mississippi River basin is studied to evaluate the performance of the nonlinear deterministic disaggregation approach. The Mississippi River, because of its enormous size and quantity of flow, plays a major role in fulfilling various water demands in a number of states in the United States and also in parts of Canada. However, the river's size and quantity of flow are also primary reasons for the flooding and sediment transport problems faced in these regions. The frequent floods in the Mississippi River cause extensive losses of life and property. The river is also a dominant mover of sediment and transports more sediment than any other river in North America (e.g. Meade and Parker, 1985), in spite of the large dams that have been built across its major tributaries. Discharging as large as about 230 million tons of suspended sediment per year to the coastal zone, the Mississippi River ranks sixth in the world in suspended sediment transport to the oceans (e.g. Milliman and Meade, 1983). The extensive flooding and sediment transport problems caused by the Mississippi River, often within the order of a few days, necessitate accurate flow data at much higher resolutions than that are currently available, in order for flood forecasting and emergency measures to be effective. For this reason, in the present study, flow data observed in the Mississippi River basin is studied for streamflow disaggregation purposes, in order to evaluate the performance of the nonlinear deterministic disaggregation approach.

Flow data in the Mississippi River basin are measured at a large number of locations throughout the basin. For the

present study, flow data observed in a sub-basin station of the Mississippi River basin at St. Louis in the State of Missouri (US Geological Survey station no. 07010000) are considered. The sub-basin is situated at $38^{\circ}37'03''$ latitude and $90^{\circ}10'47''$ longitude, on downstream side of west pier of Eads Bridge at St. Louis, 24.1 km downstream from Missouri River. The drainage area of this sub-basin is $251\,230\text{ km}^2$ (e.g. Chin et al., 1975). The natural flow of stream at this gaging station is affected by many reservoirs and navigation dams in the upper Mississippi River basin and by many reservoirs and diversions for irrigation in the Missouri River basin.

For the above station, daily flow measurements have been made available from April 1948. However, there were some missing data before 1960. As the use of continuous data eliminates the possible uncertainties on data quality (that could arise from interpolation and other schemes if the record were to contain missing data), it is decided to use only the data measured starting from 1 January 1961. The data considered in this study are those measured over a period of about 22.5 years from 1961 to 1983 (amounting to 8192 values).

To evaluate the effectiveness of the disaggregation approach, an aggregation-disaggregation scheme (aggregation followed by disaggregation) is used. First, the above daily flow values are aggregated (by simple addition) to obtain flow data at four successively doubled lower resolutions (i.e. 2-day, 4-day, 8-day, and 16-day). The nonlinear deterministic disaggregation approach is then employed to disaggregate these aggregated data series to obtain flow data at the successively doubled finer resolutions (i.e. from 16-day to 8-day, from 8-day to 4-day, from 4-day to 2-day, and from 2-day to daily). Table 1 presents some of the important statistics of these five flow series. As the minimum values indicate, there are no zero values in the flow series. This eliminates the problems faced by Sivakumar et al. (2001b) in their study of disaggregation of rainfall series observed in the Leaf River basin, even though this cannot be generalized for every streamflow series.

Each of the above five series is used as follows in the implementation of the disaggregation procedure. The entire series is divided into two halves. The first half of the series

Table 2. 2-day to Daily Streamflow Disaggregation Results in the Mississippi River Basin at St. Louis, Missouri.

Embedding dimension (m)	Correlation coefficient (CC)	Root mean square error (RMSE)	Optimal number of neighbors (k'_{opt})
1	0.9981	260.867	150
2	0.9990	187.025	10
3	0.9991	183.801	3
4	0.9989	196.865	5
5	0.9988	207.081	10
6	0.9987	216.099	10
7	0.9986	227.645	5
8	0.9985	230.183	5
9	0.9985	234.772	10
10	0.9984	238.474	10

is used for phase-space reconstruction to represent the dynamics of the disaggregation process. As the phase-space reconstruction is, in a way, done as a “training” or “learning” procedure to understand how the coarser (i.e. lower) resolution series is disaggregated into the next finer resolution series, such a set is called “training set” or “learning set.” Based on such a training procedure, the disaggregation is made only for one-fourth of the second half of the series (that immediately follows the first half). This latter set, essentially used to verify the effectiveness of the disaggregation procedure through comparison between actual and modeled disaggregated values, is called the “testing set.” Therefore, the training and testing sets are selected in such a way that disaggregation is made for the same period, irrespective of the disaggregation resolution. This is done, as it would allow useful and consistent comparisons between the disaggregation results obtained for the four disaggregation cases. This, in turn, could provide important information about the performance or effectiveness of the nonlinear deterministic disaggregation scheme with respect to changing (increasing/decreasing) scales.

4 Analysis, results and discussion

4.1 Analysis and results

The nonlinear deterministic disaggregation approach is now employed to the above flow series. As mentioned above, disaggregation between only successively doubled resolutions (i.e. $p=2$) is considered. For each of the four disaggregation cases, the flow series is reconstructed in phase-spaces or embedding dimensions (m) from 1 to 10 to represent the transformation dynamics, and the number of neighbors (k') used in the disaggregation procedure is varied from 1 to 200. However, to reduce the computational time, only nine different combinations of numbers of neighbors (i.e. 1, 2, 5, 10, 20, 50, 100, 150, and 200) are considered. These combinations

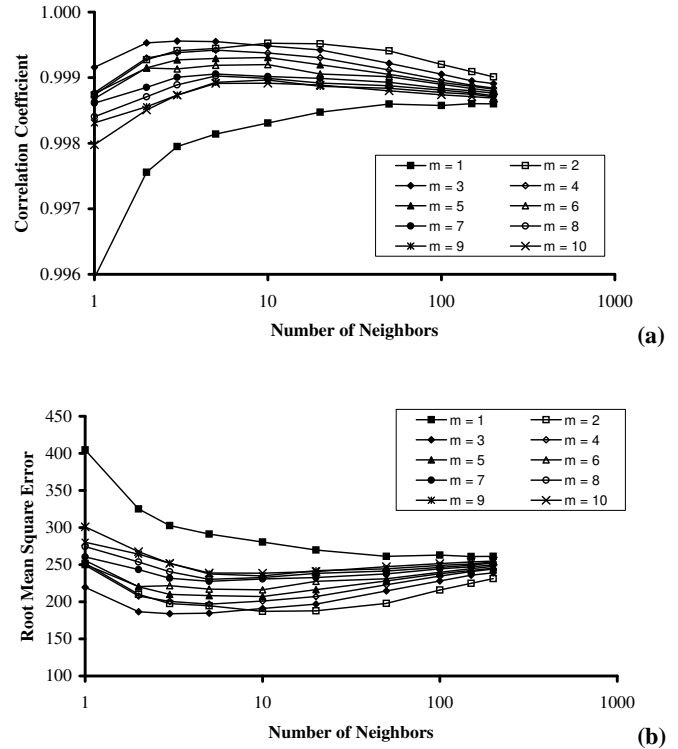


Fig. 2. Effect of number of neighbors on the performance of disaggregation of 2-day streamflow to daily streamflow in the Mississippi River basin at St. Louis, Missouri: (a) correlation coefficient; and (b) root mean square error.

are chosen (at different, but appropriate, intervals) in such a way that the results would be able to reflect the sensitivity of the disaggregation results to the number of neighbors used in the disaggregation procedure.

With the above general information, the streamflow disaggregation results obtained for each of the four disaggregation cases using the nonlinear deterministic procedure are presented in this section. However, for the purpose of brevity, detailed results are presented only for the case of disaggregation of flow from 2-day to daily, and for the remaining three cases, only the important results are highlighted.

4.1.1 Disaggregation of flow from 2-day to daily

Figures 2a and b present the accuracy of disaggregation (in terms of correlation coefficient (CC) and root mean square error (RMSE)) against the number of neighbors (for each of the ten embedding dimensions) when the 2-day flow series is disaggregated into daily flow series. As can be seen, in general, for any embedding dimension, the disaggregation accuracy increases with increasing number of neighbors up to a certain point and then saturates (or even decreases) beyond that point. The minimum number of neighbors that corresponds to the above saturation point is called as the “optimal number of neighbors”, k'_{opt} . These results are presented in a different form in Table 2, which includes also the optimal number of neighbors. As can be seen, different k'_{opt} values

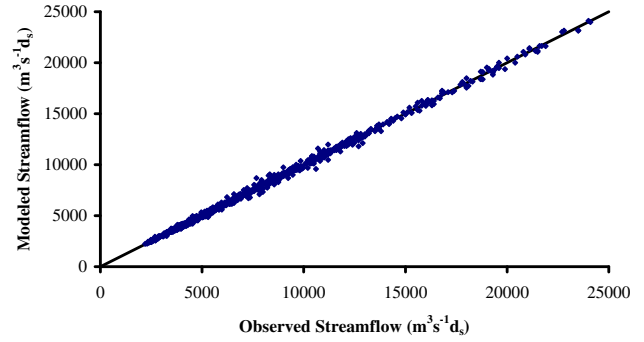
Table 3. 4-day to 2-day Streamflow Disaggregation Results in the Mississippi River Basin at St. Louis, Missouri.

Embedding dimension (m)	Correlation coefficient (CC)	Root mean square error (RMSE)	Optimal number of neighbors (k'_{opt})
1	0.9941	920.248	200
2	0.9961	745.735	20
3	0.9966	702.532	5
4	0.9958	770.948	5
5	0.9951	833.635	10
6	0.9948	860.788	10
7	0.9947	871.089	20
8	0.9945	882.668	20
9	0.9945	885.667	20
10	0.9945	882.886	20

are obtained for different embedding dimensions. Again, the disaggregation results show a trend of increase in accuracy with increasing embedding dimension up to a certain point and then saturation (or even decrease) in accuracy beyond that point. The smallest embedding dimension corresponding to such a saturation point is called as the “optimal embedding dimension”, m_{opt} .

Figure 2 and Table 2 indicate that, even though almost all of the ten combinations of m and nine combinations of k' yield very good results, the best disaggregation results (with $CC=0.9991$, $RMSE=183.801$) are achieved when the embedding dimension is 3 and the number of neighbors is 3, i.e. $m_{opt}=3$ and $k'_{opt}=3$ (indicated in bold in Table 2). For this case, Fig. 3 presents comparisons, using scatter diagram (with the solid 1:1 diagonal line shown for reference), of the actual daily flow series and the daily flow series disaggregated from the 2-day series (time series and scatter diagram comparisons for different combinations of m and k' (figures not shown) also indicate that the best results are indeed achieved for $m=3$ and $k'=3$). As can be seen, the disaggregated flow values are in excellent agreement with the actual flow values, as the points are lying on an almost perfect diagonal line.

The fact that the best disaggregation results are achieved for $m=3$ could be an indication that a three-dimensional phase-space is essential to represent the important dynamics involved in the flow transformation process between 2-day and daily scales. In other words, the transformation dynamics may be governed by only three dominant variables or mechanisms. This seems to suggest that the disaggregation dynamics can be understood and modeled through a low-dimensional approach. The near-accurate disaggregation results achieved using such an approach seem to provide further support to the above. The observations of low m_{opt} ($=3$) and small k'_{opt} ($=3$) values also seem to present clues to the presence of low-dimensional deterministic behavior in the underlying transformation dynamics (e.g. Casdagli, 1989, 1991).

**Fig. 3.** Comparison between modeled and observed disaggregated values of 2-day streamflow to daily streamflow in the Mississippi River basin at St. Louis, Missouri. The results are for embedding dimension (m)=3 and number of neighbors (k')=3.

At this stage, it is relevant to discuss the decrease in disaggregation accuracy beyond m_{opt} and k'_{opt} . If the underlying transformation dynamics is low-dimensional deterministic, then, conceptually, the disaggregation accuracy should increase with increase in embedding dimension up to a certain point (i.e. m_{opt}) and attain saturation beyond that point. A similar conceptual definition also applies for the number of neighbors, where saturation in disaggregation accuracy should be attained beyond k'_{opt} . However, the results obtained for the case of flow disaggregation from 2-day to daily, presented in Table 2 and Fig. 2, reveal a slightly different story. While, as expected, an increase in disaggregation accuracy up to m_{opt} (Table 2) and k'_{opt} (Fig. 2) is observed, there is no saturation in disaggregation accuracy beyond m_{opt} and k'_{opt} , but a decrease in accuracy is observed. This is surprising considering the fact that any dimension beyond m_{opt} (for a particular k') or any number of neighbors beyond k'_{opt} (for a particular m) potentially include only additional information about the dynamics in the phase-space reconstruction or in the disaggregation procedure, as the case may be.

Having said that, the above pure theoretical explanation and expectation is valid only for noise-free data series, such as artificially generated ones. As noise is a prominent limiting factor in the phase-space reconstruction and neighbor searching procedures (e.g. Schreiber and Kantz, 1996), such a theoretical expectation is difficult to meet with when one deals with real data, which are always contaminated with noise. The effect of noise (on prediction/disaggregation) with respect to embedding dimension and number of neighbors are discussed in detail in Sivakumar et al. (1999, 2001a, b, 2002a, b) and, therefore, are not reported herein.

4.1.2 Disaggregation of flow from 4-day to 2-day, 8-day to 4-day, and 16-day to 8-day

Tables 3, 4, and 5 summarize the results of disaggregation of flow from 4-day to 2-day, from 8-day to 4-day, and from 16-day to 8-day, respectively. The results presented therein, for each case, are the best results achieved for each of the

Table 4. 8-day to 4-day Streamflow Disaggregation Results in the Mississippi River Basin at St. Louis, Missouri.

Embedding dimension (m)	Correlation coefficient (CC)	Root mean square error (RMSE)	Optimal number of neighbors (k'_{opt})
1	0.9892	2470.02	200
2	0.9902	2350.76	50
3	0.9899	2381.36	50
4	0.9898	2401.80	100
5	0.9897	2411.06	100
6	0.9896	2425.62	100
7	0.9896	2418.28	150
8	0.9896	2425.59	200
9	0.9894	2440.80	200
10	0.9894	2445.44	200

Table 5. 16-day to 8-day Streamflow Disaggregation Results in the Mississippi River Basin at St. Louis, Missouri.

Embedding dimension (m)	Correlation coefficient (CC)	Root mean square error (RMSE)	Optimal number of neighbors (k'_{opt})
1	0.9747	7441.11	100
2	0.9750	7398.83	150
3	0.9754	7342.64	20
4	0.9753	7358.45	20
5	0.9744	7478.98	200
6	0.9751	7388.94	10
7	0.9755	7325.99	10
8	0.9759	7258.19	10
9	0.9743	7493.35	200
10	0.9756	7315.44	5

ten embedding dimensions used in the phase-space reconstruction, with the optimal number of neighbors for each dimension is also presented. From these results, the following general observations may be made:

1. The disaggregation accuracy is very high for all of the three disaggregation cases (with $CC > 0.974$), irrespective of the embedding dimension used for the phase-space reconstruction;
2. The best disaggregation results are near-accurate (with $CC > 0.975$);
3. The best disaggregation results are achieved when the embedding dimension is low, i.e. typically 2 or 3 (the only exception to this is the case of disaggregation from 16-day to 8-day, where $m=8$ yields the best results, and $m=10$ and $m=3$ yield, in order, the next best results) (indicated in bold in Tables 4, 5, and 6);
4. The best disaggregation results are achieved when the number of neighbors is small, i.e. typically below 20 (an exception to this is the case of disaggregation from 8-day to 4-day, for which $k'=50$ yields the best results); and
5. The disaggregation accuracy decreases with increasing scale of aggregation, with the best results for the case of disaggregation from 4-day to 2-day and the worst for the case of disaggregation from 16-day to 8-day.

The first four of these observations are consistent with the observations made earlier for the case of disaggregation from 2-day to daily. Also, a comparison of the results for all of the above four disaggregation cases supports the fifth observation, with the best results obtained for the case of disaggregation from 2-day to daily (see below more further details).

Figures 4, 5, and 6 compare, through scatter diagrams, the actual and modeled disaggregated values for the cases of disaggregation from 4-day to 2-day, from 8-day to 4-day, and

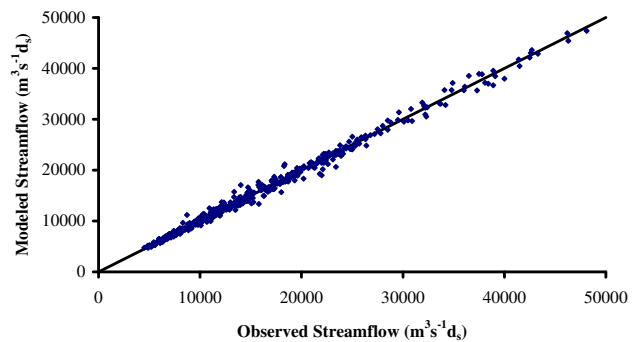


Fig. 4. Comparison between modeled and observed disaggregated values of 4-day streamflow to 2-day streamflow in the Mississippi River basin at St. Louis, Missouri. The results are for embedding dimension (m)=3 and number of neighbors (k')=5.

from 16-day to 8-day, respectively. The results shown are the best results achieved for each of these three cases (except for the last case), and are chosen from Tables 3, 4, and 5, respectively. In the case of disaggregation from 16-day to 8-day, results corresponding to two different combinations: (1) $m=3$ and $k'=20$ (Fig. 6a); and (2) $m=8$ and $k'=10$ (Fig. 6b), are shown. This is done because these two combinations yield almost similar results but represent phase-space reconstructions at low and high embedding dimensions, respectively, and, therefore, might provide interesting observations and facilitate better comparisons and interpretations.

As can be seen from Figs. 4, 5, and 6, there are, in general, excellent agreements between the actual and modeled disaggregated flow values for each of the three cases (as for Fig. 6, while an unambiguous identification of the better combination is not easy, the combination of $m=8$ and $k'=10$ seems to have an edge over that of $m=3$ and $k'=20$). This indicates the suitability of the nonlinear deterministic approach for understanding and modeling the flow disaggregation dynamics at these disaggregation scales. Also, a decrease in disaggregation accuracy with increasing scale of aggregation is clearly evident from the scatter diagrams.

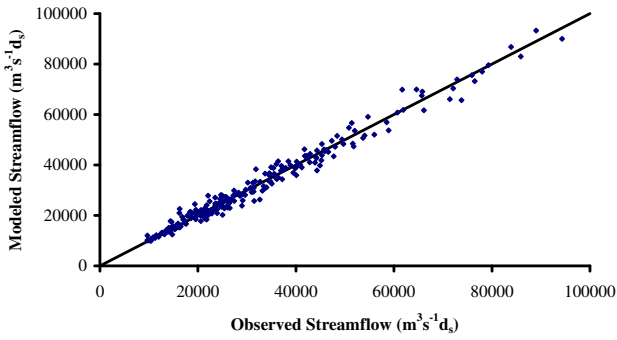


Fig. 5. Comparison between modeled and observed disaggregated values of 8-day streamflow to 4-day streamflow in the Mississippi River basin at St. Louis, Missouri. The results are for embedding dimension (m)=2 and number of neighbors (k')=50.

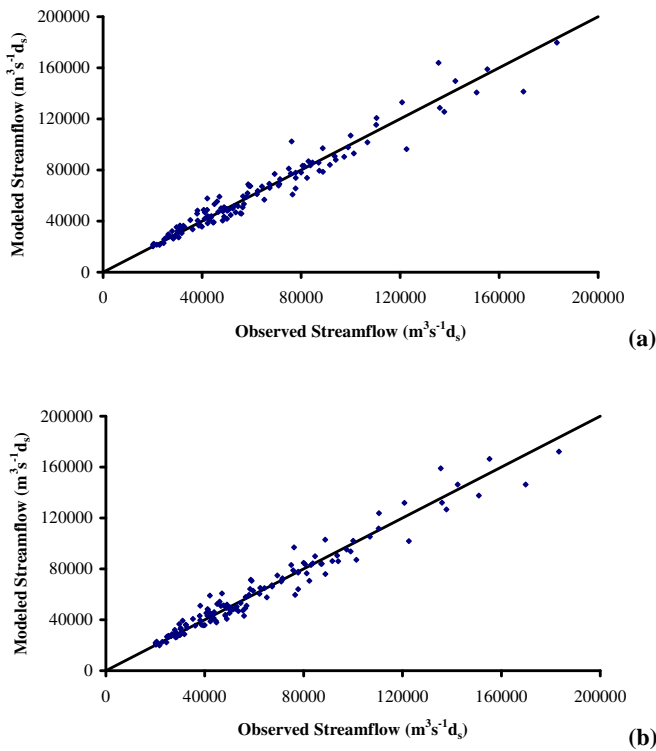


Fig. 6. Comparison between modeled and observed disaggregated values of 16-day streamflow to 8-day streamflow in the Mississippi River basin at St. Louis, Missouri: (a) embedding dimension (m)=3 and number of neighbors (k')=20; and (b) embedding dimension (m)=8 and number of neighbors (k')=10.

4.2 Discussion of results

The decrease in disaggregation accuracy with increasing scale of aggregation may seem contradictory, since it is generally (but not necessarily) believed that data at coarser resolutions are less irregular when compared to that at finer resolutions and, thus, are easier to deal with. Even if this belief/expectation exists, it should be noted, in the present case, that the transformation process between any two scales

is entirely different from the evolution process at the two individual scales. Therefore, when the task at hand is disaggregation, “coarser resolutions” do not necessarily mean less irregular than “finer resolutions.” In view of this, the present results could indeed be an actual reflection of the reality of the flow transformation process at the four disaggregation resolutions considered. On the other hand, the above results could also be an indication of the “scaling range” present in the river flow process. That is, a clear scaling range may exist between daily and 8-day scales (where the disaggregation procedure is much more effective), and may disappear gradually beyond such a resolution. Whether or not this is indeed true needs to be investigated by studying resolutions coarser than 16 days.

Having said that, it is also possible that the aggregation procedure used to obtain data sets at different (coarser) resolutions could hamper the ability of the present disaggregation procedure in providing accurate results. This is essentially because the coarser resolution data series (e.g. 16-day) contain, in all probability, higher levels of noise than the finer resolution series (e.g. daily), considering the facts that the finest resolution series (i.e. daily) itself is contaminated with noise and that data at other resolutions are obtained by simply adding the appropriate (number of) daily values. The presence of noise in the data series of two different resolutions certainly brings noise to the transformation (i.e. distributions of weights) between the two resolutions. As a result, the distributions of weights at coarser resolutions would, in all probability, contain higher level of noise than that at finer resolutions. The issue of noise on the outcomes of the disaggregation procedure has already been discussed and, therefore, is not reiterated at this stage.

One other observation that is worthy of mention is concerned with the pattern of behavior (or lack thereof) in the disaggregation accuracy with respect to the embedding dimension and the number of neighbors, for the four flow disaggregation cases studied. For the case of disaggregation from 2-day to daily (Table 2 and Fig. 3), there is a definite pattern of increase in disaggregation accuracy with an increase in m and k' and then a decrease with further increase in m and k' (except for $m=1$, in which case the phase-space is largely inadequate). There is also some consistency in the k'_{opt} for each m (k'_{opt} typically below 10). These patterns are observed also for the cases of disaggregation from 4-day to 2-day (with k'_{opt} typically below 50) (Table 3) and from 8-day to 4-day (with k'_{opt} typically above 100, except for $m=2$ and $m=3$) (Table 4). In other words, clear m_{opt} and k'_{opt} exist for these three cases. However, no definite pattern is observed for the case of disaggregation from 16-day to 8-day (Table 5), where the disaggregation accuracy fluctuates, in an irregular manner, with respect to both m and k' . The k'_{opt} for each m also fluctuates significantly, ranging from 5 to 200. What causes this situation is not clear at this moment, and further investigations are needed in this area.

However, the fluctuation with respect to m seems to start at $m=4$, and there still seems to be a trend of increase in disaggregation accuracy up to $m=3$, suggesting that a three-dimensional phase-space could still be sufficient for this case, just as it is for the other cases (for which m_{opt} is typically 2 or 3). The fact that there is no significant difference between the results obtained at $m=3$ and at $m=8$ only seems to support the above. However, one has to be cautious in providing such interpretations and conclusions, since there is always a possibility of getting trapped into a “local optimum” rather than finding a “global optimum”. The determination of m_{opt} and k'_{opt} in the present disaggregation procedure (or any phase-space reconstruction and neighbor searching procedure for that matter) is in itself an important problem to be addressed, details of which are not discussed herein (the interested reader is referred to, for instance, Jayawardena et al. (2002) and Phoon et al. (2002) for details).

5 Summary, conclusions and future research potential

The present study introduced a nonlinear deterministic approach for streamflow disaggregation that treats the dynamics of flow transformation between (two) scales as a deterministic chaotic process. As per this approach, the flow transformation dynamics was represented first using a phase-space reconstruction procedure and then disaggregation was made using a local approximation (nearest neighbor) method. The performance of the approach was tested on the streamflow series observed in the Mississippi River basin (at St. Louis, Missouri), USA. Specifically, flow series of successively doubled resolutions between daily and 16 days (i.e. daily, 2-day, 4-day, 8-day, and 16-day) were studied, and disaggregations were made only between successive resolutions (i.e. 2-day to daily, 4-day to 2-day, 8-day to 4-day, and 16-day to 8-day). The results revealed the appropriateness of the nonlinear deterministic approach for streamflow disaggregation, as there were excellent agreements between the actual values and the modeled values for all of the four disaggregation cases studied. In general, phase-space reconstruction in lower dimensions (typically 2 or 3) yielded the best disaggregation results, a possible implication that the underlying transformation dynamics could be dominated by only a few variables or mechanisms. The results also indicated a decrease in accuracy with a change of disaggregation scale from finer to coarser. While this could imply the existence of a particular “scaling range,” (probably between daily and 8 days in this case) where the disaggregation procedure is expected to be effective, further verification is necessary in light of the potential limitations of the present approach for noisy time series, among others.

The present study was different from the previous streamflow disaggregation studies in two important aspects: (1) The study treated the dynamics of streamflow transformation as a nonlinear deterministic process, whereas the previous studies assumed the underlying process as stochastic (through parametric or non-parametric procedures); and (2) Whereas

most, if not all, of the past studies focused on streamflow disaggregation between very coarse resolutions (e.g. annual and monthly scales), the present study attempted disaggregation between relatively much finer resolutions (e.g. daily and weekly scales). In regards to (1), even though a direct comparison between the present study and the past studies could not be made, due essentially to the different disaggregation scales studied, the near-accurate results achieved in the present study indicate the suitability of the nonlinear deterministic approach for streamflow disaggregation. A comparison of the performance of stochastic and nonlinear deterministic approaches is expected to shed some light on the usefulness and appropriateness of these approaches for the specific disaggregation scale (finer or coarser) at hand, and on the selection of the better approach for that scale. Efforts are being made in this direction, details of which will be reported elsewhere.

It is the authors’ opinion that the present study has equal practical relevance and significance when compared to the previous studies because of the finer disaggregation scales studied, as mentioned in (2). Obtaining streamflow data at much finer resolutions (e.g. daily scales and even finer) is as equally important as that at coarser resolutions (e.g. monthly). This is because (the availability of) finer resolution data plays an important role in effectively forecasting flood events and efficiently improving and implementing flood warning and emergency measures, which normally (must) happen within a few days or even a few hours.

A final remark on the possibility of dealing with streamflow disaggregation problem over all (or at least a large range of) scales is in order. As the stochastic streamflow disaggregation schemes have been found to provide very good results for coarser resolutions (e.g. Lin, 1990; Maheepala and Perera, 1996; Tarboton et al., 1998) and as the present deterministic disaggregation procedure has been found to perform extremely well for finer resolutions, coupling of these two approaches could potentially yield better results than those that can be achieved when the two are performed independently. In this regard, the coupling of nonlinear deterministic approach and nonparametric approach (e.g. Tarboton et al., 1998) could be a first step. As these two approaches possess important commonalities, such as: (1) they use historic data in the analysis (to reconstruct the phase-space or to estimate the necessary joint probability density functions); (2) they are data driven; (3) they restore summability; (4) they view the allocation problem from a “local” sense rather than a “global” sense; and (5) they are able to incorporate the nonlinear dependency that is present in the underlying dynamics, it is hoped that their coupling could be done without much difficulty. Whether this is indeed the case remains to be seen.

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