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Computer simulation of model cohesive powders: Plastic consolidation, structural changes and elasticity under isotropic loads.

F. A. Gilabert,^{1,*} J.-N. Roux,² and A. Castellanos¹

¹*Faculty of Physics, University of Seville, Avda. Reina Mercedes s/n, 41012 Seville, Spain.*

²*Université Paris-Est, Institut Navier,*

*Laboratoire des Matériaux et des Structures du Génie Civil[†],
2 Allée Kepler, Cité Descartes, 77420 Champs-sur-Marne, France.*

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The quasistatic behavior of a simple 2D model of a cohesive powder under isotropic loads is investigated by Discrete Elements Methods simulations. We ignore contact plasticity and focus on the effect of geometry and collective rearrangements on the material behavior. The loose packing states, as assembled and characterized in a previous numerical study [Gilabert, Roux and Castellanos, Phys. Rev. E **75**, 011303 (2007)], are observed, under growing confining pressure P , to undergo important structural changes, while solid fraction Φ irreversibly increases (typically, from 0.4–0.5 to 0.75–0.8). The system state goes through three stages, with different forms of the *plastic consolidation curve*, $\Phi(P^*)$, which is a function of the growing reduced pressure $P^* = Pa/F_0$, defined with adhesion force F_0 and grain diameter a . In the low-confinement regime (I), the system undergoes negligible plastic compaction, and its structure is influenced by the assembling process. In regime II the material state is independent of initial conditions, and the void ratio varies linearly with $\log P$ [i. e. $\Delta(1/\Phi) = \lambda \Delta(\log P^*)$], as described in the engineering literature. Plasticity index λ decreases as a function of contact rolling resistance (RR). In the last stage of compaction (III), Φ approaches an asymptotic, maximum solid fraction Φ_{\max} , as a power law, $\Phi_{\max} - \Phi \propto (P^*)^{-\alpha}$, with $\alpha \simeq 1$, and properties of cohesionless granular packs are gradually retrieved. Under consolidation, while the range ξ of fractal density correlations decreases, force patterns reorganize from self-balanced clusters to force chains, with correlative evolutions of force distributions, and elastic moduli increase by a large amount. Plastic deformation events correspond to very small changes in the network topology, while the denser regions tend to move like rigid bodies. Elastic properties are dominated by the bending of thin junctions in loose systems. For growing RR those tend to reduce to particle chains, the folding of which, rather than tensile ruptures, controls plastic compaction.

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I. INTRODUCTION

Cohesive granular materials are present in many natural or industrial processes, the understanding of which requires studies of their rheology under small confining pressures, when tensile intergranular forces play a major role. In such cases cohesive materials exhibit specific features that do not exist in cohesionless grain assemblies, such as the ability to form stable structures at low density and the sensitivity to stress *intensity*, as opposed to stress direction. Macroscopic constitutive laws and phenomenological tools have been developed and used in several engineering fields: mechanics of cohesive soils (clays and silts) [1, 2, 3, 4], metallic powder processing [5], handling of xerographic toners [6]. One simple material is the assembly of wet beads [7, 8, 9], in which some microscopic observations are possible [8, 9]. However, wet grain packs are only slightly less dense than dry ones, and do not enable the study of loose structures

obtained with powders. In general, the behavior of materials under proportional load (oedometric or isotropic compression) is characterized by the consolidation curve, which describes the irreversible compaction under growing stress [1]. Density can increase by factors of 3 or 4 under growing load.

Although numerical simulations have been widely used for several decades [10] to investigate microscopic mechanisms and classify mechanical properties of granular systems, studies of cohesive materials are still far less common, and almost exclusively limited to dense materials. Thus, the effects of capillary cohesion in wet sand or bead packs have been simulated [11, 12], as well as the compaction of metallic powders [13, 14, 15, 16] to states of very high density, or the behavior in shear tests of 2D dense cohesive packs with plastic deformation of contacts [17, 18]. Loose structures formed by particles packed under gravity and stabilized thanks to adhesion have been simulated [19]. Of particular relevance to the present study, among the very scarce numerical studies of *loose* packings [20] stabilized by cohesion and of their collapsing under growing loads, are the works by Bartels, Kadau, Wolf *et al.* [21, 22, 23, 24] on the oedometric compression of granular assemblies with initial low densities. This research group studied a dynamical compression regime, and observed a shock wave propagating

[†]LMSGC is a joint laboratory depending on Laboratoire Central des Ponts et Chaussées, École Nationale des Ponts et Chaussées and Centre National de la Recherche Scientifique.

*Electronic address: gilav@us.es

through the sample. Shear flows of cohesive granular materials have also been simulated [25, 26, 27, 28].

In a previous article [29], hereafter referred to as paper I, we studied by numerical simulation the assembling process, the structure and the force patterns of a model, two-dimensional (2D) cohesive granular material in loose equilibrium configurations. We now investigate the mechanical behavior of the same model granular material in isotropic compression and pressure cycles, as well as the evolution of various characteristics of intermediate equilibrium states as plastic compaction proceeds.

As in paper I, we keep the external pressure as the main control parameter. The adhesive strength F_0 in contacts sets a force scale in the material behavior, and hence (in 2D) the reduced pressure, defined as

$$P^* = \frac{aP}{F_0}, \quad (1)$$

in which a is a typical grain diameter, is a crucial dimensionless state parameter. The main objective of the present paper is the study of the process by which, as pressure is increased, cohesion-dominated loose structures, for which $P^* \ll 1$, get irreversibly compacted as P^* increases until pressure dominates ($P^* \gg 1$). Such a compaction was numerically observed *e.g.*, in Ref. [24]. However, our approach produces homogeneous, isotropic, equilibrium configurations under varying load and is therefore apt to provide more detailed information about the connections between macroscopic constitutive laws and microstructural or micromechanical features.

The simulations reported in paper I revealed the importance of the initial aggregation process taking place in a gas-like state prior to the application of an external confining pressure. The compression of the “granular gas” of isolated particles must be very slow and aggregates need to form in order to stabilize very loose systems, even under low reduced pressure P^* . This is the most notable aspect of the variability of initial states that were obtained in paper I – others being coordination numbers and self-balanced force patterns depending on initial “granular temperature”. On compressing those different configurations, will the final maximum density states differ under large P^* ? Another result of paper I is that loose systems have a fractal structure, with the fractal dimension $d_F \simeq 1.55$ of 2D ballistic aggregation in the presence of rolling resistance in the contacts, over some length scale ξ . How do such properties evolve under compression?

Such questions are addressed in the present paper, which is self-contained and can be understood without reading paper I, although some details about the model and the initial equilibrium states of low density will not be repeated. A summarized description of the material properties and of the initial configurations, the assembling process which is studied in paper I, is provided in Section II. The macroscopic material response in isotropic compression, with the possible influence of the initial state properties, is studied in Section III. The geometry and the density correlations, which were shown

in paper I to exhibit fractal behavior over some distance range, are studied in Section IV. Force networks, force distributions and elastic moduli are dealt with in Section V. Section VI discusses qualitatively some microscopic aspects of the consolidation behavior. Finally, in section VII we state the conclusion, pointing out briefly the next direction for future work.

II. MODEL MATERIAL AND SIMULATION PROCEDURES

A. Definitions and basic equations

The material and the simulation method are identical to those of paper I [29], which the reader might refer to for additional technical details, and for a physical discussion of some of the model ingredients. For the sake of completeness, we however provide a summarized description below. The contact law is an elaboration of the often employed spring-dashpot model with Coulomb friction, in which two additional ingredients are introduced: an attractive force and, possibly, some resistance to rolling at contacts. The model material is a 2D assembly of disks with diameters uniformly distributed between $a/2$ and a , enclosed in a rectangular cell with periodic boundary conditions in both directions. Both lengths L_1 , L_2 defining the cell size and shape are variable, and satisfy equations of motion designed to impose given values of diagonal stress components $\sigma_1 = \sigma_2 = P$. Stresses are controlled by a variant of the Parrinello-Rahman method [30]. In equilibrium, both diagonal stress components σ_α , ($\alpha = 1, 2$), with the convention that tensile stresses are negative, are given by the standard formula (A is the sample surface area):

$$\sigma_\alpha = \frac{1}{A} \sum_{1 \leq i < j \leq N} F_{ij}^{(\alpha)} r_{ij}^{(\alpha)}. \quad (2)$$

In (2), the r.h.s. sum runs over all interacting pairs i, j among the N disks of the system, \mathbf{F}_{ij} is the force transmitted from grain i to its neighbor j and vector \mathbf{r}_{ij} points from the center of i to the center of j (with the suitable nearest image convention to account for periodicity). The implementation of stress-controlled simulations is such that the cell length L_α along direction α increases or decreases if σ_α is larger (respectively: smaller) than its prescribed value.

As usual in molecular dynamics applied to granular materials (also known as the “discrete element method”) particles have rigid body kinematics and their motion is governed by Newton’s equations.

B. Interaction law

Grains interact with forces of elastic, adhesive, frictional and viscous origins. The static part of the normal

component N_{ij} of the force transmitted by grain i to its neighbor j is a function of h_{ij} , the distance separating disk perimeters. A negative h_{ij} means that the grains overlap, in which case they repel each other with a normal elastic force $F_N^{e,ij} = -K_N h_{ij}$. This force vanishes whenever $h_{ij} > 0$. (Overlap $h_{ij} < 0$ is, of course, a numerical representation of the physical contact deflection). The repulsive elastic force is supplemented with an attractive term $F_N^{a,ij}$, equal to $-F_0$ for contacting disks ($h_{ij} < 0$). $F_N^{a,ij}$ has a finite range D_0 , fixed to $10^{-3}a$, and varies linearly between $-F_0$ and zero as h_{ij} grows from 0 to D_0 . F_0 is the maximum tensile force a contact might support without breaking off. The normal contact law thus introduces a force scale, and a dimensionless parameter, the *stiffness parameter*, $\kappa \equiv aK_N/F_0$. κ characterizes the amount of elastic deflection h_0 under contact force F_0 , relative to grain size a ($h_0/a = \kappa^{-1}$). κ is set to a large value, $\kappa = 10^5$, so that the elastic deflections in contacts remain so small that they can be neglected in comparison to all other length scales in the problem (including interstices between neighbors [31]). The packing geometry can be regarded as that of an assembly of rigid grains (as formally dealt with in the “contact dynamics” simulation method used in [24]).

To the static contributions F_N^e and F_N^a to the normal force we add a viscous damping term opposing the relative normal velocity of i and j when the disks touch ($h_{ij} < 0$), corresponding to a constant, positive normal coefficient of restitution e_N in binary collisions if F_0 is set to zero. e_N is set to a low value, $e_N = 0.015$ in our simulations. In the presence of attractive forces the apparent restitution coefficient in a collision will depend on the initial relative velocity. For small kinetic energies the particles will eventually stick to each other. The minimum receding velocity for two particles of unit mass (the unit mass is chosen equal to the mass of a disk of diameter a) to separate is $V^*\sqrt{2}$, with

$$V^* = \sqrt{F_0 D_0}. \quad (3)$$

The elastic tangential force in contact i, j , F_T^{ij} , is to be evaluated incrementally. In case of no tangential sliding, it varies linearly with the relative tangential displacement at the contact point, involving a tangential stiffness constant, K_T . In the case of sliding, which occurs when the elastic law would cause T_{ij} to pass one of the Coulomb bounds $\pm\mu F_N^{e,ij}$, then T_{ij} stays equal to $\pm F_N^{e,ij}$. The relative tangential displacement at the contact point involves displacements of disk centers and rotations. The Coulomb condition introduces the *friction coefficient*, μ . It should be pointed out that it applies to the elastic repulsive part of the normal force only. Thus, a pair of contacting grains with h_{ij} equal to $F_0/K_N = h_0$, the equilibrium distance, such that the sum of elastic and adhesive terms vanishes, can transmit a tangential force T such that $|F_T| \leq \mu F_0$. (The importance of this feature of the contact law for collective properties macroscopic behavior of particle assemblies was stressed in paper I for

isotropic, static states, and in Ref. [27] in steady-state shear flows). All simulations reported here were carried out with $\mu = 0.5$.

We studied the influence of *rolling resistance* (RR) at contacts, which is modeled as in [32]. Two additional parameters are necessary: a *rolling spring constant*, K_R , with dimension of a moment, expressing proportionality between relative rotation and rolling moment (i. e., a torque concentrated at the contact point), as long as the rolling friction threshold is not reached; and a *rolling friction coefficient*, μ_R with the dimension of a length, setting the maximum absolute value of the rolling moment Γ_R to $\mu_R F_N^e$, proportional to the elastic part of the normal force. The implementation of this rolling law is analogous to that of the tangential one, with the rolling moment and the relative rotation respectively replacing the tangential force and the relative tangential displacement. A contact for which the total normal force is equal to zero in equilibrium, with $F_N^e = K_N h_0 = F_0$, may transmit a rolling moment Γ_R with $|\Gamma_R| \leq \mu_R F_N^e$. Since point contacts do not transmit torques, the rolling resistance stems from the irregularity of grain surface. Two contacting grains touch each other, in general, by two points (in 2D), which are separated by some microscopic distance l that is characteristic of the particle shape. μ_R should be proportional to l , and K_R proportional to l^2 . We set $\mu_R = \mu l$ and $K_R = K_N l^2$, with, in most calculations with RR, $l = a/100$.

Table I summarizes the values of parameters used in most simulations, in dimensionless form. Some calcu-

μ	e_N	κ	$\frac{K_T}{K_N}$	$\frac{D_0}{a}$	$\frac{K_R}{K_N a^2}$	$\frac{\mu_R}{a}$
0.5	0.015	10^5	1	10^{-3}	10^{-4}	0 or 0.005

TABLE I: Values of dimensionless model parameters used in most simulations.

tions were also performed with larger RR (up to $l = a$, $\mu_R = 0.5a$).

C. Initial states

In paper I, two extreme cases were studied in the assembling stage of cohesive packings under low P^* . First, an N -particle sample of hard-disk fluid is prepared at solid fraction Φ_I in a fixed cell. Then, in type 1 systems, velocities are set to zero and the external pressure control is started, until an equilibrium is reached under $P^* = 0.01$. The other procedure, by which type 2 samples are prepared, is meant to represent the opposite situation, in which aggregation is much faster than compression. Thus, while the cell size is fixed and the solid fraction stays equal to Φ_I , grains are attributed random (Maxwell-distributed) velocities and left to interact and aggregate until all N of them join to form one unique cluster. The system is then equilibrated at $P^* = 0$, and

Sample type	No cohesion	Type 1	Type 2		
N	1400	1400	1400	5600	10976
Number of samples	4	4	5	3	1
Lowest pressure	$P/K_N = 10^{-5}$	$P^* = 0.01$	$P^* = 0.01$		
Φ (no RR)	0.811 ± 0.001	0.723 ± 0.001	0.472 ± 0.008		
Φ (RR)	0.805 ± 0.002	0.688 ± 0.001	0.524 ± 0.008		

TABLE II: Set of granular samples used as initial equilibrated configurations in simulations of isotropic compression (with material parameters of Table I).

compressed to $P^* = 0.01$. To limit the influence of dynamical effects, the strain rate is requested not to exceed a maximum value $\dot{\epsilon}_{\max}$ during compression. We express this condition with the natural inertial time associated with the characteristic force F_0 : (m is the mass of a disk of diameter a)

$$T_0 = \sqrt{\frac{am}{F_0}}, \quad (4)$$

defining a dimensionless inertia parameter

$$I_a = \dot{\epsilon}_{\max} T_0. \quad (5)$$

I_a is set to 0.05 in our simulations. The main set of samples of types 1 and 2 (the latter coinciding with “series A” in paper I), to which some non-cohesive ones are added for comparison, is listed in Table II, in which the number of available configurations of different sizes is provided, along with solid fraction under the lowest nonzero pressure. All configurations are prepared both with ($\mu_R/a = 0.005$) and without ($\mu_R/a = 0$) RR, with the parameters of Table I. The initial solid fraction is $\Phi_I = 0.36$. Type 2 systems are also available under $P^* = 0$, right at the end of the aggregation stage [29], but we regard this intermediate stage as part of the initial packing process and focus our study on higher pressures (as apparent in Table II, the compression from zero pressure to $P^* = 0.01$ involves a large density increase, and important changes of the microstructure are reported in paper I). Distant interactions between grain pairs separated by a gap smaller than D_0 are scarce, and “rattlers”, *i.e.*, isolated, free grains with no interactions, are absent in cohesive systems because of the initial aggregation process. Coordination numbers under $P^* = 0.01$ are typically $z \simeq 3.1$ without RR, and $z \simeq 3.0$ with RR, for both type 1 and type 2 cohesive samples. Additional details about those equilibrium configurations under low pressure can be found in paper I.

The assembling stage of type 2 systems also depends on the initial velocities given to the grains before they form aggregates (the “granular temperature” of the original “granular gas”). The relevant dimensionless parameter is the ratio of the initial mean quadratic velocity V_0 to the characteristic velocity V^* defined in (3). V_0/V^* is set to 9.5 for the main sample series of Table II. The value of V_0/V^* was shown in paper I to have a strong influence on the initial coordination number z at $P^* = 0$ in samples

with RR: whereas z is larger than 3 for $V_0/V^* = 100$, it approaches 2 for small V_0 , of order $V^*/10$, in which case the loopless structures of geometric ballistic aggregation models are retrieved. However, this effect is strongly reduced after the compression step to $P^* = 0.01$.

In the following, unless otherwise specified, all results will pertain to the systems of Table II, and measurements will be averaged over all available samples, error bars on graphs extending to one sample to sample standard deviation on each side of the mean value.

D. Simulation procedures

1. Equilibrium conditions

One of the specificities of our simulations of cohesive packings under varying pressure is the approach, computing cost permitting, of the quasistatic material response, in which all configurations remain close to mechanical equilibrium. Equilibrium conditions have to be stringent enough to enable an unambiguous identification of the force-carrying contact network and a study of its elastic properties. Due to the frequent occurrence of small contact force values, this requires forces to balance with sufficient accuracy. We used similar criteria as in paper I, which, in agreement with other studies on cohesionless systems [31, 33], were observed to provide force values with sufficient accuracy. The tolerance levels on force and torque balance equations is expressed in terms of a typical intergranular force value $F_1 = \max(F_0, Pa)$. A configuration is deemed *equilibrated* when (1) the net force on each disk is lower than $10^{-5}F_1$; (2) the total moment on each disk is lower than $10^{-5}F_1a$; (3) the difference between imposed and measured stresses is less than $10^{-5}F_1/a$; and (4) the kinetic energy per grain is less than $5 \times 10^{-8}F_1a$.

2. Compression

The sample series of Table II are subjected to a step-wise compression cycle. In each compression step, external reduced pressure P^* is multiplied a constant factor $10^{1/8} \simeq 1.334$, and one waits until the new equilibrium configuration is reached, with the criteria stated in

Sec. IID 1. A condition of maximum strain rate is enforced, in order to approach the quasistatic compression curve, as in the preparation process, on setting (see Eqs. 5 and 4) $I_a = 0.05$. Parameter I_a , on replacing, in its definition, F_0 by the force scale aP (in 2D) corresponding to the confining pressure is analogous to inertia parameter I used to assess dynamical effects in steady shear flow [27, 28], or in the compression of non-cohesive granular packings [31, 34]. The compression program is pursued until P^* reaches the maximum value 13.33, above which negligible plastic collapse is observed. It should be noted that, thanks to the high value of stiffness parameter κ (see Sec. IIA), the typical contact deflection aP/K_N at this highest pressure level is still very small. Then, the effect of decreasing P^* back from its highest value to 0.01 is also simulated. As no large structural changes occur on decompressing the system, larger pressure jumps can be imposed on unloading.

The simulations are computationally costly, as in some pressure steps equilibration times of order $100T_0$ are required, while the time step for the integration of the equations of motion is a small fraction of $\sqrt{\frac{m}{K_N}} = \frac{T_0}{\sqrt{\kappa}}$. This limits the size and the number of samples, and the use of small strain rates. Some tests of statistical significance and rate dependence of the results will be reported in Section III.

As to non-cohesive samples, they are compressed from $P/K_N = 10^{-5}$ to $P/K_N = 1.334 \times 10^{-3}$ and back.

3. Computation of elastic moduli

We observe that once samples are equilibrated according to the conditions of Section IID 1, then the Coulomb criterion $|T| \leq \mu F_N^e$, as well as the rolling friction condition $|\Gamma_R| \leq \mu_R F_N^e$ are satisfied as *strict* inequalities in all contacts. No contact is ready to yield in sliding, and with RR no contact is ready to yield in rolling either. This ensures that the response to small enough external load increments about a well-equilibrated state will be elastic and reversible. Elastic moduli express elastic response, *i.e.*, with no effect of tangential or rotational sliding and no change in contact network topology and geometry. To compute elastic moduli, we build the stiffness matrix $\underline{\mathbf{K}}$ of the contact structure (also taking into account the distant interactions). As recalled in the appendix of paper I, $\underline{\mathbf{K}}$ is a square matrix of order $3N + 2$, the number of degrees of freedom in the system, containing stiffness coefficients K_N , K_T , K_R (with RR), and also $-F_0/D_0$ for the rare distant attractive bonds, as well as the coefficients of the rigidity matrix associated with the network geometry. $\underline{\mathbf{K}}$ is symmetric, positive definite in a situation of stable equilibrium (once the free translational motions of the whole grain assembly as one rigid body are eliminated). To compute elastic moduli, one has to solve a linear system of equations of the form

$$\underline{\mathbf{K}} \cdot \mathbf{U} = \mathbf{F}^{\text{ext}} \quad (6)$$

for the unknown displacement vector \mathbf{U} , containing all particle displacements and rotations, as well as strains $(\epsilon_\alpha)_{\alpha=1,2}$. The right-hand-side of (6) contains external forces and torques applied to the grains, which are set to zero, as well as stress increments $(\Delta\sigma_\alpha)_{\alpha=1,2}$ (the same procedure is followed in [35] with 2D disk packings and in [36] with 3D sphere packings). On setting $\Delta\sigma_1 = 1$, $\Delta\sigma_2 = 0$, or vice-versa, one thus gets two separate measurements of the compliance matrix in our (statistically) isotropic systems, from which moduli C_{11} and C_{12} are deduced, and hence the bulk modulus $B = (C_{11} + C_{12})/2$ and the shear modulus $G = (C_{11} - C_{12})/2$.

III. MATERIAL BEHAVIOR UNDER ISOTROPIC LOAD

A. Compression and pressure cycle with non-cohesive material

Non-cohesive systems of Table II, initially assembled on isotropically compressing a granular gas (a method used for 3D sphere packings in e.g. Refs. [37] and [31]), are subjected to a compression cycle, in which reduced pressure κ^{-1} increases from its initial value $P_0/K_N = 10^{-5}$, up to $P_1/K_N = 1.33 \times 10^{-3}$, and decreases back to 10^{-5} .

Typical results for the density of systems with and without RR are shown on Fig. 1. Changes of solid frac-

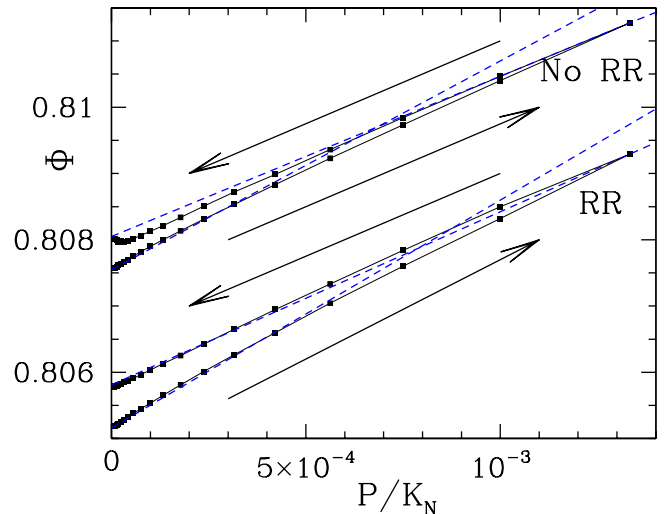


FIG. 1: (Color online) Φ versus P/K_N in pressure cycle with 1400 disk samples with and without RR. Blue dashed lines correspond to elastic response evaluated with the bulk modulus from initial and highest pressure states.

tion Φ which increase from its initial value Φ_0 to a maximum Φ_1 , and then decreases back, are very small (of order 10^{-3} , *i.e.*, of order κ^{-1} for the lowest values of κ), and nearly reversible (more than 90% of increment $\Phi_1 - \Phi_0$ is recovered on decompressing), as observed in Ref. [34] with 3D sphere packings. The predictions of lin-

ear elasticity about a reference state under pressure P_r , with solid fraction Φ_r and bulk modulus B_r given by

$$\Phi - \Phi_r = \Phi_r \frac{P - P_r}{B_r}, \quad (7)$$

provide a satisfactory approximation of the compression curve (Fig. 1) if applied with the lowest or the highest pressure states as reference.

The slight increase of bulk modulus as a function of Φ is due to the larger density of contacts under higher pressures, as reported in Section III A. One typical feature of frictional, cohesionless grain packs assembled by direct compression is the existence of a non-negligible population of “rattlers”, i.e., particles that transmit no force (as observed e.g. in Ref. [31] in 3D, or Ref. [35] in 2D systems). The fraction of rattlers x_0 thus exceeds 20% of the grains under P_0 in systems with RR in the present case, and reaches 17% without RR. x_0 is reduced to 14% under $P/K_N = 10^{-3}$. The *backbone* (force-carrying structure) is the set of non-rattler grains, characterized by coordination number $z^* = z/(1 - x_0)$ [31]. z^* increases with P , as rattlers get captured by the backbone and gaps separating neighboring grains close in compression.

Changes of x_0 and z^* are reversed on unloading (with some moderate hysteresis effect). The increase of z^* as a function of P , above a minimum value z_0^* , which would correspond to $P = 0$, is sometimes described by a power law [38], as

$$z^* = z_0^* + K \left(\frac{P}{K_N} \right)^\beta. \quad (8)$$

Such a fit can be used to describe our data in noncohesive samples. With RR we obtain z_0^* values close to 3 ($z_0^* = 3.031 \pm 0.005$ for the loading curve, $z_0^* = 3.066 \pm 0.006$ for the unloading one), while β increases from 0.34 ± 0.01 to about 0.47 from the loading ($K = 3.5 \pm 0.2$) to the unloading branch ($K = 7.6 \pm 0.9$). Without RR, parameters of (8) are $z_0^* = 3.123 \pm 0.006$, $\beta = 0.42 \pm 0.02$, $K = 4.9 \pm 0.6$ under growing P , $z_0^* = 3.115 \pm 0.004$, $\beta = 0.49 \pm 0.02$ and $K = 7.9 \pm 0.8$ on unloading. Thus z^* vary by about 10% in the studied pressure interval. As in other simulations [31, 39, 40], the minimum coordination numbers stay above the “critical” value for rigidity, which is equal to 3 without RR and to 2 with RR [29].

In conclusion, cohesionless systems under isotropic pressure cycles behave nearly elastically in an isotropic pressure cycle. As the pressure increases by more than 2 orders of magnitude, while remaining in the rigid limit of $\kappa \gg 1$, only small and nearly reversible changes in density and in other internal state variables are observed. (It should be recalled that this reversibility is only approximate, and that an irreversible *decrease* of coordination number is obtained in a compression cycle for systems initially prepared with a large z [34]). A small level of RR has little effect on density and material properties.

B. Compressing cohesive systems: general observations

Once subjected to a pressure cycle, as specified in Sec IID 2, the material prepared in initially loose states (type 2 of Table II) behaves as shown in Figs 2, 3 and 4. As the pressure increases, so does the density, and the large pores present under low P^* gradually disappear. The maximum packing fraction, $\Phi_{\max} = 0.774 \pm 0.001$

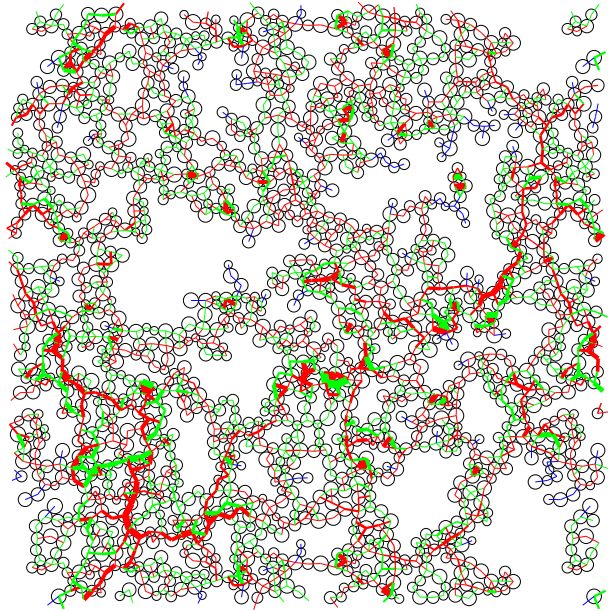


FIG. 2: (Color online) Equilibrium configuration of a sample of 1400 disks with RR in initial state, under $P^* = 0.01$, for which $\Phi = 0.5132$. Line thicknesses encode normal force intensities, red strokes depict compressive forces while tensile ones are colored in green, and forces equal to zero in blue.

in that case, is quite reproducible. Φ_{\max} is smaller than the solid fraction of cohesionless systems (for which $\Phi > 0.805$, see Fig. 1).

From the shape of $\Phi(P^*)$ curves at growing P^* , three regimes can be distinguished. At first, in a range of reduced pressure P^* of the order of the first nonzero value (10^{-2}), thereafter called regime I, Φ remain approximately constant: the contact network supports the growing pressure without rearranging. Then, in a second pressure interval which we shall refer to as regime II, a fast compression is observed. Density variations slow down in regime III, for P^* of order unity, as a maximum solid fraction Φ_{\max} is approached. On reducing the pressure, Φ then remains very close to Φ_{\max} : the compaction is irreversible.

The consolidation curve is similar to the ones obtained by numerical simulations in Refs. [22, 24], on imposing uniaxial strains to loose packings prepared by an anisotropic ballistic aggregation process, although our study differs from these works in several respects (see Section I). Refs. [22, 24] focus on regime III, and on dy-

namical compaction processes, with a shock wave propagating through the sample. The variations of solid

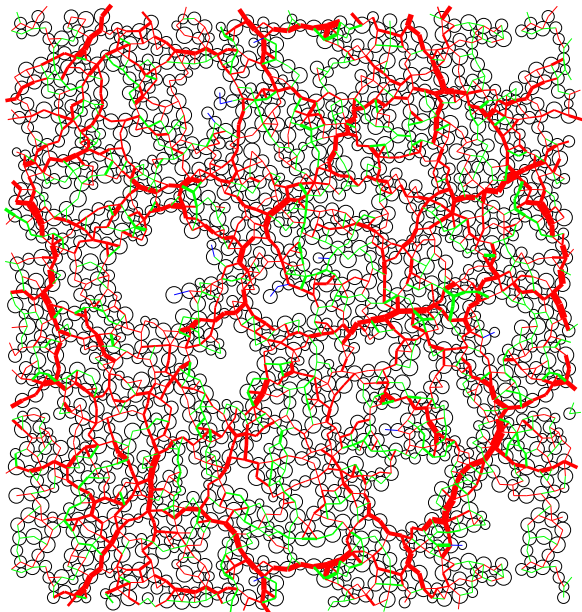


FIG. 3: (Color online) Sample of Fig. 2, with $\Phi = 0.6305$, equilibrated under $P^* = 0.178$ (different length and force units).

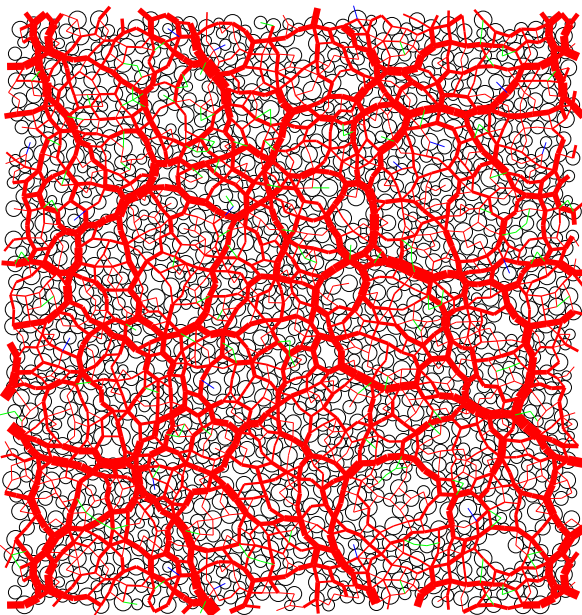


FIG. 4: (Color online) Same sample as on Figs. 2 and 3, under the maximum pressure $P^* = 13.3$. Solid fraction is $\Phi = 0.7778$.

fraction Φ versus P^* are shown in Fig. 5, for three samples of different sizes. Since all three curves are close to one another, we conclude that the macroscopic behavior is correctly captured in our simulations. Our results for $\Phi(P^*)$ also resemble experimental curves obtained on

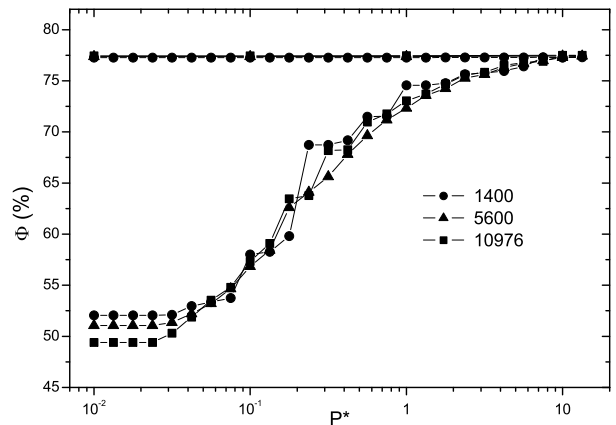


FIG. 5: Consolidation and decompression curves in 3 samples (with RR) with different numbers of grains, as indicated.

different materials, such as metallic powders [5], or xerographic toner [6, 41], at least in regimes I and II. Poquillon *et al.* [5], in particular, in an experimental study of a metallic powder, explicitly distinguish three compaction regimes, with the material elastically resisting compression in regime I, and then some plastic compaction, first attributed to particle rearrangement, as we observe, and later to contact plasticity. This latter effect, which is not included in our model, is likely to explain the difference under high P^* between many experiments and our results: experimental curves do not appear to approach an asymptotic density, but witness ongoing compaction up to the highest investigated pressure levels. In the case of metallic powders [5], quite high pressures are applied (hundreds of MPa), and, as revealed by direct microscopic observations, particles fusing or sintering gradually form compact solid. For metal particles with $d=10\ \mu\text{m}$ diameter, one can estimate the pressure F_0/d^2 corresponding to $P^* = 1$ to be in the 0.1 MPa range, so that the very large P^* values in the compaction experiment reveal a different physical origin of density increase. The stiffness parameter κ , is also significantly smaller in such experiments, with the consequence that plastic phenomena cannot be ignored (for a definition and discussion of κ in Hertzian sphere packings, see [34]). Contact plasticity dominates in the numerical studies of Martin *et al.* [13, 14, 15, 16], which focus on very high densities (beyond the random close packing value), when the material, due to sintering, turns into a porous compact. Hence only the early stages of metal powder compaction, in which densities are quite low [5] correspond to our simulations. In the case of xerographic toners, as investigated in the experiments reported in [6, 41, 42], $P^* = 1$, as discussed in [29], rather correspond to $P \sim 10\ \text{Pa}$. Nevertheless, the contact behavior, as investigated by atomic force microscopy, is likely to involve plastic effects [43, 44, 45].

C. Regime I: role of the initial assembling process

As shown in [29] (paper I), and briefly recalled in Sec. II C, assembling conditions have a considerable influence on packing density and microstructure under low P^* . It should be assessed to what extent those important differences in the initial configurations affect the plastic consolidation curve, and whether such a variability tends to disappear once the material undergoes significant compaction. This issue is investigated in this section, in which the effects of various features of the preparation process are observed. The role of some micromechanical parameters is also discussed.

1. Compaction and aggregation in the assembling stage

The most important feature of the assembling process is the competition between compression and aggregation, which leads to the difference between systems of type 1 and 2, as defined in [29] and recalled in Section II C. Type 1 samples reach a considerably higher densities from the beginning, under low P^* . Fig. 6 compares the subsequent consolidation curves. As type 1 systems are ini-

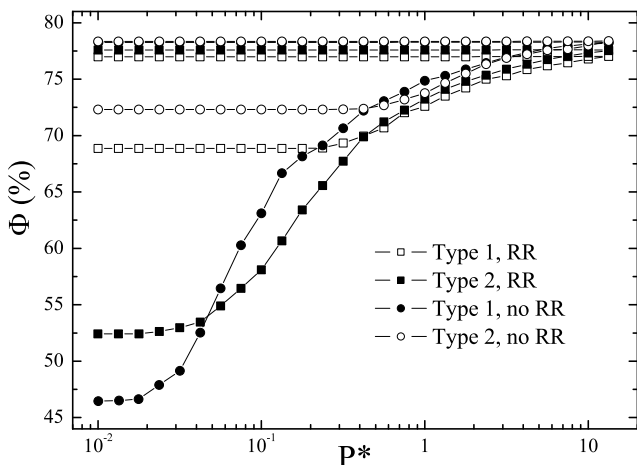


FIG. 6: Consolidation curve in type 1 and type 2 samples.

tially considerably denser, they are able to support larger pressures before rearranging, hence they have a wider regime I plateau. However, the pressure increase eventually reaches a high enough value to induce further compaction, and the consolidation curve is then very close to that of type 2 systems (the difference is actually smaller than the sample to sample r.m.s. fluctuation). Within the accuracy and statistical uncertainty of our simulations, the difference between initial states of types 1 and 2, although large, thus appears to disappear eventually upon plastically compacting the material.

2. Effects of first compression step and strain rate

In paper I [29] important changes between $P^* = 0$ and $P^* = 0.01$ in type 2 configurations were reported: this first compression step (in which Φ increases from $\Phi_I = 0.36$ to about 0.5, see Table II) causes the most dramatic change in the internal state of the system. One way to limit its effects is to reduce the strain rate, setting parameter I_a to a lower value. As shown on Fig. 7, displaying the consolidation curve obtained in $N = 1400$ systems with the usual value $I_a = 0.05$ and with the smaller one $I_a = 0.01$, lower inertial effects in the initial stage, while the equilibrium configuration at $P^* = 0.01$ is prepared, result in a lower density and tends to turn the initial plateau of the $\Phi(P^*)$ curve into a gentle ascending slope. Later on, as consolidation proceeds, very similar curves are obtained with both values of maximum dimensionless strain rate I_a (Fig. 7), although the smaller error bars (representing sample to sample r.m.s. fluctuations) witness smoother changes and better reproducibility for the slower compression. It may thus be concluded that the quasistatic consolidation curve is quite reasonably approached with the standard compression procedure detailed in Section II D 2, for which $I_a = 0.05$.

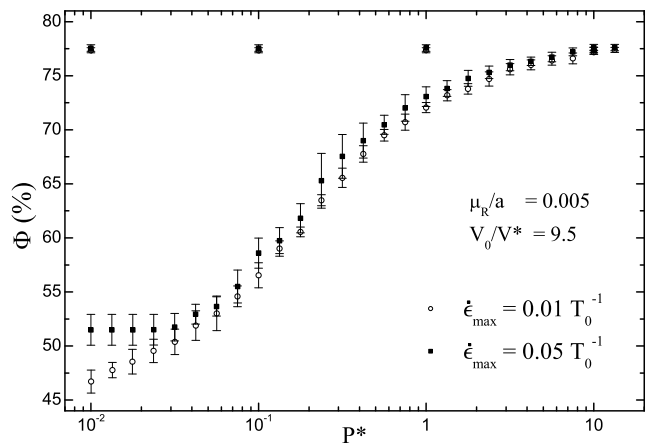


FIG. 7: Consolidation curve with two different values of I_a .

3. Effect of initial agitation and influence of RR

The initial agitation velocity (or “granular temperature”), as expressed by ratio V_0/V^* in the aggregation stage strongly influences the coordination number. Figs. 8 and 9 show how this initial influence affects the beginning of consolidation curves and, once again, fades out later on. Consolidation curves are shown in Fig. 8 for two different values of V_0/V^* , one tenfold larger than the standard value 9.5 used in the sample series of Table II, and the other one smaller by a factor of 100. Fig. 9 shows the effect of V_0 on coordination number. An increase of rolling resistance (with $\mu_R = 0.5$ instead of 0.005), similarly to a decrease of V_0 , stabilizes looser systems under

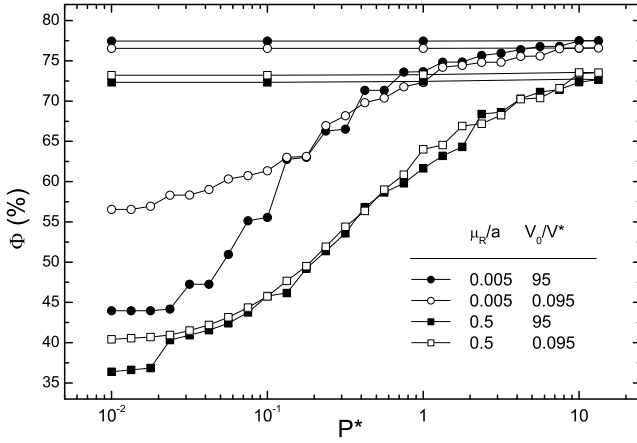


FIG. 8: Consolidation curve: effect of initial agitation level in aggregation stage, and influence of RR parameter.

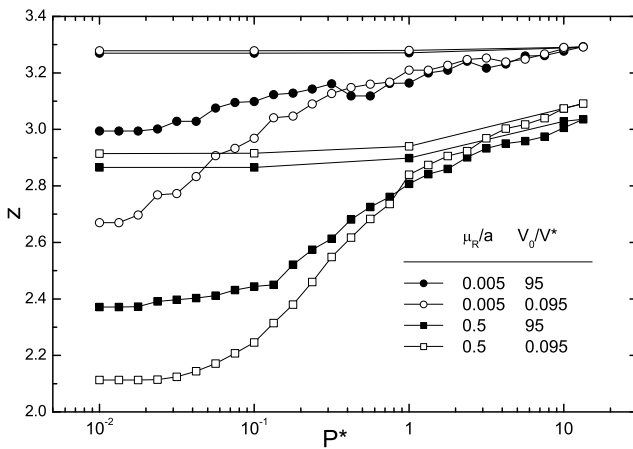


FIG. 9: Same as Fig. 8, for coordination number z as a function of P^* .

low P^* , with smaller coordination numbers. However, such a change in material properties does not only affect the initial, regime I part of the consolidation curve; it also alters the macroscopic mechanical behavior at larger densities: the slope of the consolidation curve is lower for larger RR.

4. Conclusion on initial states and regime I

Fragile tenuous structures due to aggregation are easily perturbed and sensitive to many factors in low consolidation states. In general, all perturbations favor some kind of preconsolidation effect, inducing denser, better coordinated structures. These effects are reduced in each one of the following situations: (1) if one waits until large aggregates form before applying a confining pressure; (2) if the initial agitation velocity V_0 is decreased; (3) for slower compression processes, especially when the very first non-vanishing pressure value is imposed; (4) with

larger RR levels. As the material is further compressed in (nearly) quasistatic conditions, the same macroscopic behavior is retrieved for given microscopic force laws [i.e., in cases (1) to (3)], irrespective of the initial perturbations affecting the beginning of the consolidation process. Although we did not vary restitution coefficient e_N governing energy dissipation in normal collisions, it is very likely that if increased, will induce larger inertial effects, similarly to a faster compression. On the other hand, viscous forces slowing down the motion of grains relatively to a surrounding fluid (often an important physical effect in fine powders) could reduce the effects of the initial agitation.

A regime I, with no plastic strain, is also observed in some experiments. For example, the response in uniaxial compression (i.e., $\sigma_1 > 0$, $\sigma_2 = \sigma_3 = 0$) of loose aggregates of micrometer-sized silica beads assembled by ballistic deposition – in that case, an anisotropic process in which particles are thrown onto a substrate – was studied by Blum and Schr  pler [46]. The deposit, with volume fraction $\Phi \simeq 0.15$, resists a stress of 500 Pa before plastic compaction is observed, which corresponds to a “reduced stress”, defined, in analogy with P^* , as $\sigma_1^* \equiv \sigma_1 a^2 / F_0$ of order 10^{-2} . In the simulations of Wolf *et al.* [24] (in the case of dynamic, *oedometric* compression) also some finite initial pressure increment has to be applied before plastic collapse is observed.

D. Regimes II and III: intrinsic consolidation behavior

Once the peculiarities of the sample preparation and first compression stage are erased, we refer to the material evolution, as the *intrinsic* consolidation behavior. In order to compare the shape of the consolidation curve to other observations more directly and quantitatively (and also for a more fundamental reason to be stated further) we subsequently describe it with $1/\Phi$, instead of Φ , as a function of $\log P^*$. This conforms to its traditional presentation in the literature [1, 3, 4, 5, 6], which often uses the *void ratio*, $e = (1/\Phi) - 1$.

Once the regime I ends, we obtain linear variations of e or $1/\Phi$ with $\log P^*$:

$$\frac{1}{\Phi} = \frac{1}{\Phi_0} - \lambda \ln \frac{P^*}{P_0^*} \quad (9)$$

where P_0^* and the corresponding solid fraction Φ_0 are the coordinates of the point where the system behavior joins the intrinsic consolidation curve in the available samples. The parameter λ , known as the plasticity index, is observed in our case to decrease as a function of μ_R (Fig. 8). We have also observed that the value of this index is not affected by the friction coefficient: in that sense, μ just displaces the whole consolidation curve vertically [47].

As the maximum solid fraction Φ_{\max} is approached, Eq. (9) is no longer valid, and the asymptotic regime is

better described with a power law, as in [24]:

$$\frac{1}{\Phi} = \frac{1}{\Phi_{\max}} + \frac{A}{(P^*)^\alpha}, \quad (10)$$

with a constant A and an exponent α (close to 1 in our results). In order to describe the consolidation curve in regimes II and III with a unique functional form, we use the following relation:

$$\frac{1}{\Phi} = \frac{1}{\Phi_0} - \lambda \ln \left\{ \frac{P^*}{P_0^*} \left[1 - \exp \left(- \left[\frac{P_1^*}{P^*} \right]^\alpha \right) \right] \right\}^{1/\alpha}, \quad (11)$$

which introduces additional parameters P_1^* and α , and crosses over from Eq. (9), for $P^* \ll P_1^*$, to Eq. (10), for $P^* \gg P_1^*$. Constant A in (10) is set to $\lambda/(2\alpha)$ on using (11) for large P^* values, and P_1^* is directly related to Φ_{\max} :

$$\ln \frac{P_1^*}{P_0^*} = \frac{1}{\Phi_0} - \frac{1}{\Phi_{\max}}.$$

Fig. 10 summarizes the definition and the role of all parameters of relation (11). A fit of our data to relation (11)

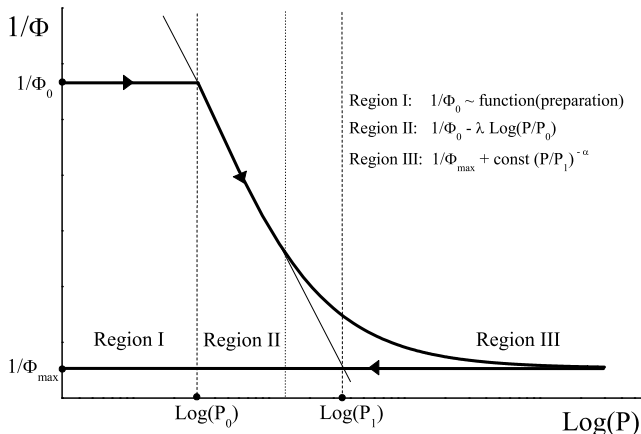


FIG. 10: Schematic view of intrinsic consolidation curve with regimes II and III, and role of parameters introduced in Eq. (11).

is shown in Fig. 11. It should be noted that even a small level of rolling resistance changes the plasticity index. Values of parameters are listed in Table III, where we also included the fit parameters for the sample with $\mu_R/a = 0.5$ corresponding to the data of Figs. 8 and 9.

As the consolidation curve in region II, defined by parameters λ and P_0^* , is observed not to depend on initial conditions, our simulations support the following interpretation: sooner or later in the process of quasistatic isotropic compression, the system joins, in the $P^* - \Phi$ plane, a certain locus, corresponding to compressive plastic yielding. This locus, which acts as an attractor in isotropic compression, is a straight line on using coordinates $\ln P^*$ and $1/\Phi$. The value of P_0^* simply signals where, depending on the preparation process, the yield

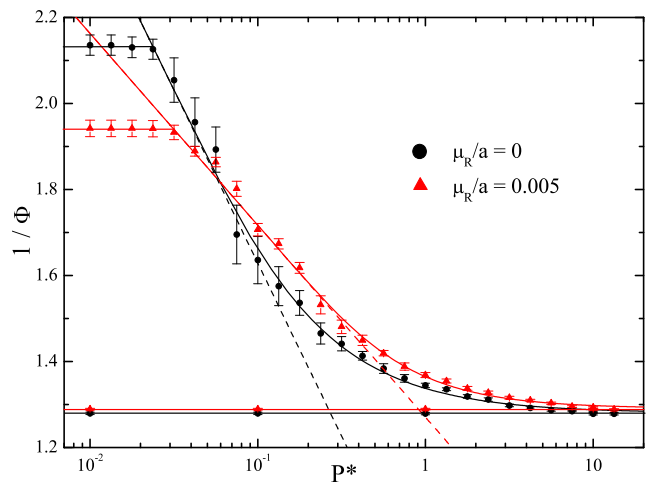


FIG. 11: (Color online) Consolidation data and fit to Eq. (11), for systems with and without (small) RR.

μ_R/a	P_0^*	Φ_0	λ	Φ_{\max}	α
0	0.0237	0.469	0.349 ± 0.019	0.7808	0.91 ± 0.10
0.005	0.0316	0.515	0.194 ± 0.004	0.7745	1.08 ± 0.16
0.5	0.0178	0.382	0.25 ± 0.01	0.724	0.86 ± 0.24

TABLE III: Values of parameters λ , Φ_{\max} and α used to fit the consolidation curve in systems of Table II, and in a sample with larger RR, with Eq. (11). Correspondingly, P_1^* values are 0.271 ± 0.033 without RR, 0.900 ± 0.064 for $\mu_R/a = 0.005$, and 2.6 ± 0.4 for $\mu_R/a = 0.5$.

locus is reached. Table I gives the values of 5 parameters, though one is redundant for the determination of this intrinsic curve (indeed Φ_{\max} is obtained as a result of the fitting).

Consequently, in a system prepared at a lower density, it should be possible to observe a wider interval of the intrinsic consolidation line. We could explicitly check this property in the case of one sample with $N = 5600$, for which the first nonzero equilibrium confining pressure in the loading history is equal to 2×10^{-3} instead of 10^{-2} . This sample appears to have reached regime II sooner (around $P_0^* = 10^{-2}$, or possibly below). The corresponding data points lie on the intrinsic consolidation curve (or, at least, within a distance smaller than error bars) identified on fitting the data of the main sample series, which had a larger first compression step (to $P^* = 10^{-2}$) and a larger value of P_0^* (about 3×10^{-2}). The yield locus can thus be *extrapolated* to lower pressures and densities, with the same plasticity index λ . On assembling cohesive aggregates with arbitrarily low densities, and on stabilizing them under very low initial pressures, it is conceivable (although increasingly difficult in numerical simulation because of the computational cost, as well as in experiments, because of the system sensitivity to perturbations) to create equilibrium structures with smaller and smaller densities and to explore an increas-

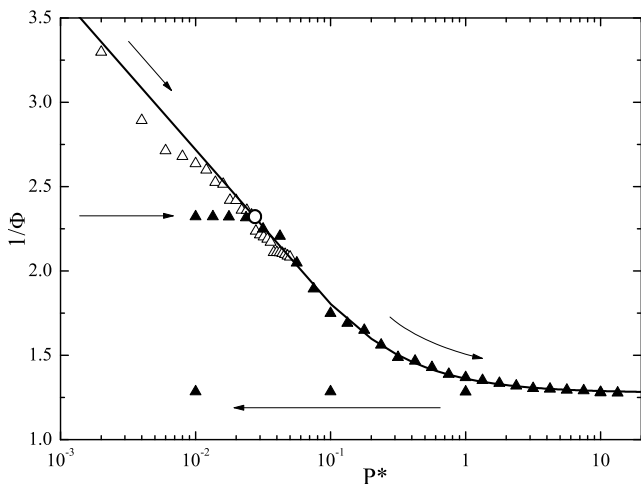


FIG. 12: Comparison of data obtained on the one low P_0^* sample (open triangles), and Eq. (9) (continuous line) with the parameters of Table III, as deduced from a fit of the data (black triangles) from the more systematic simulation series with larger P_0^* .

ingly larger interval of the intrinsic consolidation curve in the limit of $P_0^* \rightarrow 0$. The corresponding solid fraction Φ_0 would then also tend to zero. This limit is compatible with the functional form used in Eq. (9), while the use of the alternative form [6, 42],

$$\Phi - \Phi_0 = \nu \ln \frac{P^*}{P_0^*},$$

would lead to contradictions in the limit of $P_0^* \rightarrow 0$.

E. Unloading behavior

On the Φ versus P^* curves we have been showing so far, that the unloading branch, down to $P^* = 0.01$, shows very little density change. This property is actually satisfied on decreasing the pressure from other configurations in the compression process. Thus Fig. 13 shows that, if P^* is reduced to the initial level 0.01 from different states on the consolidation curve, density changes are hardly noticeable, and Φ stays very close to the maximum value reached at the largest imposed pressure P_c^* in the past. Furthermore, it is checked (in the case of sequence 4, drawn with open circles in Fig. 13) that the material might be reloaded, with no notable density change until pressure P_c^* is reached. P_c^* is known in soil mechanics as the *consolidation pressure*, and a material in a state such that $P^* < P_c^*$ is said to be *overconsolidated*. Upon increasing the pressure beyond the consolidation value P_c^* , the density irreversibly increases, and this compaction is described by the same curve as in the absence of intermediate pressure cycle: the recompression curve from C' retraces back the same evolution from D to E. Thus the material behavior conforms to the plasticity of clays

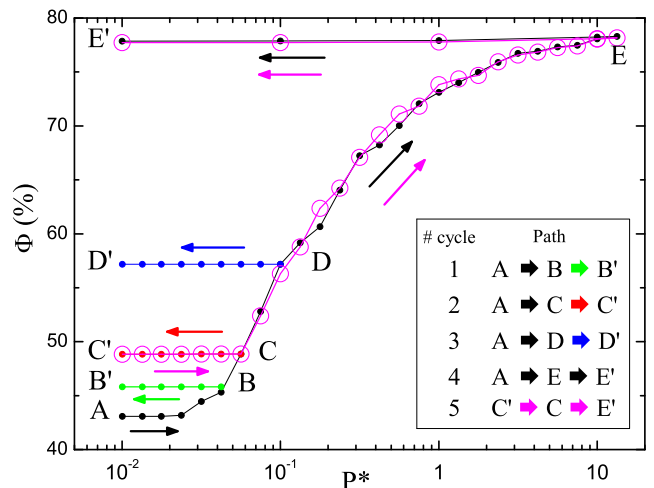


FIG. 13: (Color online) Effect of different (isotropic) unloading/reloading histories on solid fraction. The direct consolidation curve with decompression from the highest pressure, as shown in previous sections, is ABCDEE' (path 4). On unloading along lines BB', CC', DD', the system does not rearrange. Such paths are reversible and do not alter the material state, since paths 4 (small black dots) and 5 (large, open pink circles) superimpose in P^* , Φ plane.

in isotropic compression [1]. All decompressing paths in the P^* , Φ plane, along which $P^* < P_c^*$, are reversible. More precisely, they are similar to the pressure cycles applied to cohesionless systems (Fig. 1), and they do not depart much from the linear elastic response, as shown on Fig. 14.

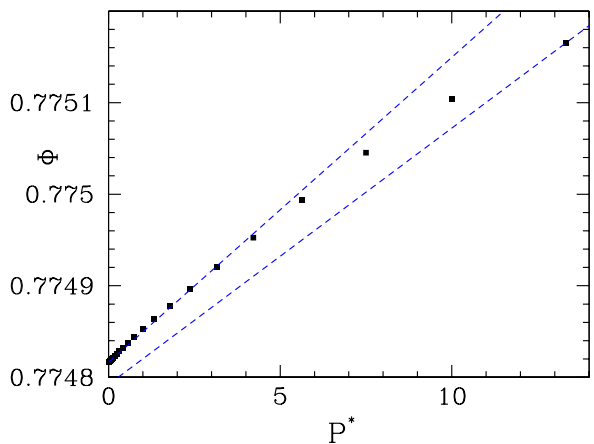


FIG. 14: (Color online) Analog of Fig. 1, for the unloading behavior of a sample with RR from $P^* = 13.3$ to $P^* = 0.01$. Dotted lines correspond to Eq. (7), applied with highest pressure state and with final state ($P^* = 0.01$) as references.

For the largest P^* values, adhesion forces are dominated by the confining stress and are nearly negligible: on setting F_0 to zero in equilibrated systems under

$P^* > 10$, we could check that the granular assembly finds a new equilibrium configuration with very small displacements and hardly any change in the contact network. The system, then rendered cohesionless, can be unloaded to small pressures, without notable plastic compression. This results in a configuration of packing fraction close to Φ_{\max} , and hence lower than in the systems assembled without cohesion. This suggests a method to prepare relatively loose, cohesionless configurations in the laboratory, as follows. First, one may compress a wet granular packing, in which adhesion is caused by capillary forces in intergranular menisci (so that $F_0 \sim \gamma a$, γ denoting the surface tension of the wetting fluid), up to some pressure P much larger than F_0/a^2 (in 3D). Then, cohesive forces can be suppressed, either on evaporating the liquid, or on saturating the pores with it. The pressure on the resulting cohesionless granular pack might then be decreased or increased with essentially elastic density changes (as shown in Section III A). The final configurations are then looser than the ones obtained on directly assembling and compressing cohesionless grains. Such an assembling method, known as *moist tamping*, is commonly used in soil mechanics laboratories to prepare sand samples in loose configurations [48].

IV. GEOMETRIC CHARACTERIZATION OF PRESSURE DEPENDENT STRUCTURES

The gradual collapse of the initially open structure of loose systems, as visually apparent on Figs. 2, 3, and 4 and witnessed by the consolidation curve studied in Section III, can be characterized by the density correlation indicators introduced in paper I.

The initial aggregation process was shown in paper I to result in a fractal structure of the density field over intermediate scales, between the grain diameter and some characteristic correlation length ξ . In the presence of rolling resistance, even with the small value $0.005a$ adopted for μ_R , the observed fractal dimension is compatible with the result of the ballistic aggregation model, $d_F \simeq 1.55$. The ballistic aggregation model is purely geometric, and corresponds to the irreversible bonding of particles or aggregates in each collision, with contacts that are rigid in translation and rotation. This limit case, for which the coordination number is equal to 2, is approached under low pressure [29] with large RR or small V_0/V^* . Better coordinated systems obtained with small RR and/or larger V_0/V^* have the same fractal dimension. Systems with no RR, on the other hand, are closer to dense objects with $d_F \simeq 1.9$ [29].

The limitation of the fractal behavior by an upper length scale ξ is a well-known geometric necessity in a large system with finite particle packing fraction Φ , because (in 2D) a fractal structure of dimension $d_F < 2$ within a square cell of edge length L exhibits an apparent density proportional to L^{d_F-2} . In physically relevant circumstances, systems with a finite packing fraction Φ

and a fractal structure over some distance range have a finite correlation length ξ above which the average value of Φ is observed. One then has $\Phi \propto \xi^{d_F-2}$ or

$$\xi \propto \Phi^{-1/(2-d_F)}, \quad (12)$$

the prefactor being specific to the particular system studied. Systems with size $L \gg \xi$ can then be regarded as homogeneous packings of fractal “blobs” of (linear) size ξ . Such ideas are quite generally used, and were applied to semi-dilute polymer solutions [49], to silica [50] or polymeric [51] gels, in computer simulations of aggregation models [52], and to various complex, supramolecular objects like fat crystals [53] or asphaltene aggregates [54].

One may expect that the density increase caused by the collapse, under growing load, of the tenuous structures formed by cohesive packings corresponds to a decrease in the fractal blob size ξ , while dimension d_F still describes the scaling of density correlation at smaller scale. One should then observe the scaling predicted in (12). This implicitly assumes that the small scale structure of the packing is not affected by the compaction process, which essentially breaks long, thin junctions and fills the largest pores. A clue in favor of such a scenario is provided by the results of Sec. III C, which suggest that the same structure is obtained if the material is directly prepared with some value of Φ , or if it is assembled first in a looser state and then isotropically compressed, up to solid fraction Φ .

To compute d_F and ξ , we measure the “scattering intensity” $I(k)$, *i.e.* the Fourier transform of the density autocorrelation function, as we briefly recall now (see paper I for more details). Density field $\chi(\mathbf{r})$, taking values 1 within particles and 0 outside, is first discretized on a regular mesh, then Fourier transformed, thereby obtaining $\hat{\chi}(\mathbf{k})$. We then evaluate $I(\mathbf{k}) = |\hat{\chi}(\mathbf{k})|^2/A$, A being the cell surface area. Invoking isotropy, it is a function of $k = ||\mathbf{k}||$ alone. $I(k)$ should then vary proportionally to k^{-d_F} for $a \ll 2\pi/k \ll \xi$, and reach some plateau for $k < 2\pi/\xi$.

This approach was used in paper I, and yielded the same fractal dimension, $d_F \simeq 1.52$ in systems with RR, under $P^* = 0$ (solid fraction $\Phi_I = 0.36$) and $\Phi = 0.01$ (solid fraction $\Phi_0 = 0.524 \pm 0.008$), while ξ decreased from $\xi_I = 9.3 \pm 0.4$ to $\xi_0 = 5.1 \pm 0.2$. It should be noted that these values are roughly compatible with relation (12) (as $(\xi_I/\xi_0)^{2-d_F} = 1.4 \pm 0.1$ is close to $\Phi_0/\Phi_I = 1.46 \pm 0.02$).

Fig. 15 shows the scattering function for similar consolidation states shown in Fig 2 ($P^* = 0.01$), in Fig. 3 ($P^* = 0.178$), and for $P^* = 1$. These results are averaged over the four largest samples (with RR) of Table II. In spite of the error bars, $I(k)$ exhibits the expected form, it is approximately constant below some crossover wavevector $2\pi/\xi$ which increases with Φ , and then decreases, with slope $-d_F$ on a logarithmic plot. Pressure $P^* = 0.178$ is the largest one for which this latter feature is clearly observed, and $I(k)$ data corresponding to smaller pressures are intermediate between $P^* = 0.01$ and $P^* = 0.178$ curves. The arrows on the plot signal the identified val-

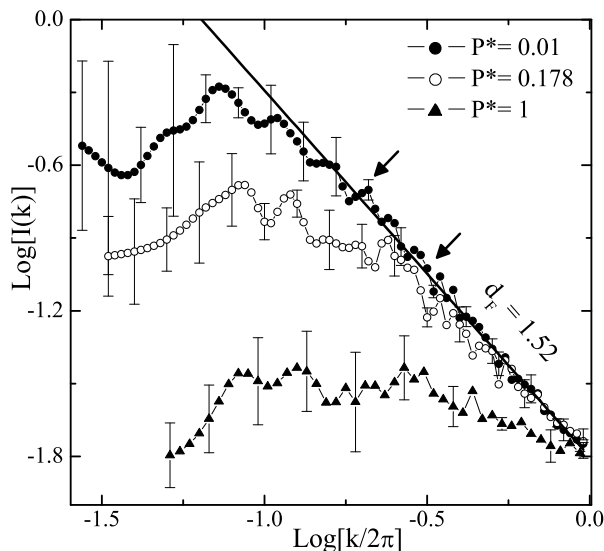


FIG. 15: Scattering intensity per unit area versus wave vector k . Results are averaged over the four largest samples (with RR) of Table II.

ues of wavevector $2\pi/\xi$, which values have been estimated by means of the fit function for $I(k)$ presented in paper I. The curve corresponding to $P^* = 1$ – a flat, low scattering signal – is typical of dense, homogeneous media with no fractal range for density correlations.

In view of the small value of ξ reached in the loosest configurations (those with $P^* = 0$ studied in paper I), relation (12) is difficult to test from density correlation data. Another characteristic length scale for density inhomogeneities, used in paper I, is the (mass) averaged radius of gyration of pores. It may provide an alternative definition of a blob size ξ' , proportional to ξ . We observed $\xi' \simeq \xi$ at $P^* = 0.01$. In fact, this equality works well under very low consolidations. However, under higher confining pressures we have observed that the definition of ξ' gives lower values than ξ .

Figure 16 shows this blob size ξ' as a function of pressure.

Despite the restricted fractal range, our observations therefore confirm the validity of the “fractal blob” model, with a constant d_F and a correlation length ξ decreasing as consolidation proceeds, until a final, homogeneous structure similar to that of cohesionless packings (albeit somewhat looser) is obtained. Other values of d_F are likely to be observed with other assembling processes (such as e.g., diffusion-limited cluster aggregation).

Values of ξ and d_F do not, however, entirely determine the mechanical properties of the system. The response of an aggregate to some mechanical perturbation should depend on its connectivity which, as explicitly shown in paper I, is independent of its fractal dimension (systems with different μ_R and/or prepared with different values of V_0/V^* have the same d_F , but very different coordination numbers – see also Section III C 3).

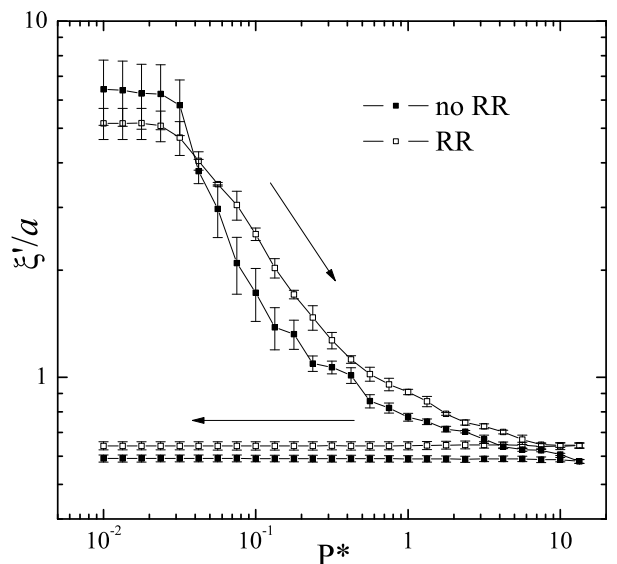


FIG. 16: Average radius of gyration of pores, ξ' , versus P^* .

Results concerning blob sizes in systems without RR, for which $d_F \simeq 1.9$ [29], are similar. Different stress states and mechanical conditions might also produce other types of loose structures. As an example, in the slow steady state shear flow of a very similar material simulated in [28] under normal reduced pressure $P^* = 1.25 \times 10^{-2}$ and shear stress $\sigma_{12} \simeq 1.5P$ anisotropic structures with $\Phi \simeq 0.6$ were observed.

V. PROPERTIES OF EQUILIBRIUM FORCE NETWORKS

A. Equilibrium forces

1. Average normal force

Formula (2), as explained in Ref. [11] and in paper I, leads to a simple relation between the average normal force $\langle F_N \rangle$ in equilibrium, pressure P , solid fraction Φ and coordination number z :

$$\langle F_N \rangle = \frac{\pi \langle d^2 \rangle P}{z \Phi \langle d \rangle} = \frac{7\pi a P}{9z \Phi}. \quad (13)$$

We observed formula (13) (involving the first and second moments of the diameter distribution) to be accurate in all simulated states despite some approximations involved [29]. However, as stressed in paper I, relation (13) fails to estimate the typical contact forces in the network under low P^* . Those reach values of order F_0 [12]. Normal forces of both signs (as visible on Fig 2 and Fig 3) coexist and, to a large extent, compensate under low P^* .

2. Coordination numbers

In initial low-pressure states, the coordination number, z , as shown on Fig. 17, is nearly equally shared between the contribution z_+ of compressive bonds and z_- of tensile bonds. A small population (z_0 per grain) of contacts carry forces equal to zero (within the numerical tolerance for force equilibrium). Those contacts, in which the normal deflection h takes the equilibrium value h_0 for isolated pairs [29], tend to be more numerous in the absence of applied stress, if the aggregation process avoids the building of hyperstatic (overbraced) structures. Their number is quickly reduced once aggregates made under $P = 0$ are subjected to some external stress and start rearranging.

The population of contacts loaded in compression increases along the consolidation curve until it dominates at large P^* . Upon unloading, the initial proportion of tensile forces is first retrieved, and z_- is eventually, under low P^* , larger than z_+ .

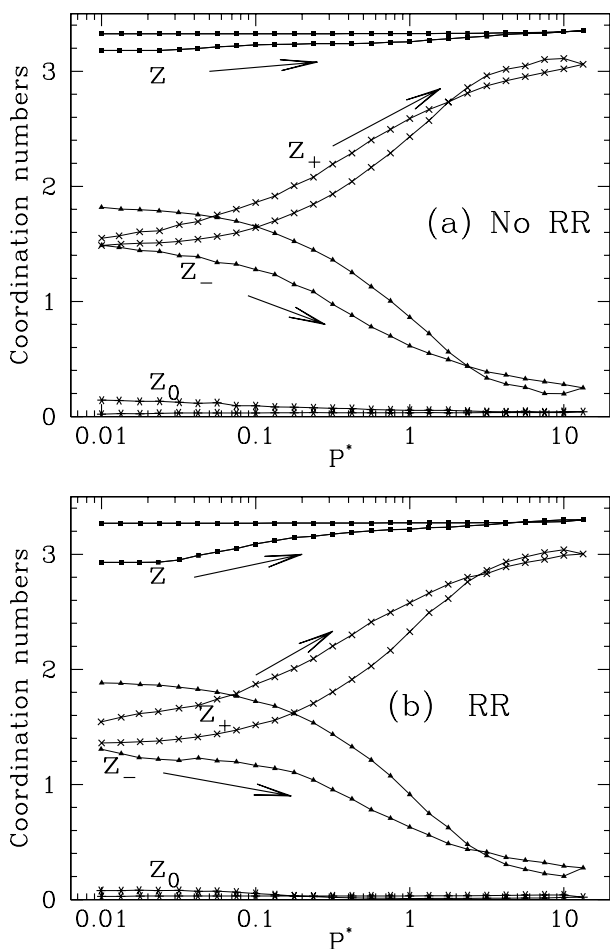


FIG. 17: Coordination numbers versus P^* in compression cycle (a) without and (b) with RR. Both plots display, from top to bottom, z , z_+ , z_- , and z_0 . The error bars (not shown) are about the size of symbols. Arrows close to the curves indicate the compression branch of the pressure cycle.

The *total* coordination number increases very little in the pressure cycle. Our observations thus contradict some statements in the literature [55, 56] relating z to Φ (even in cohesionless systems, Φ and z can vary independently [31]). In the course of plastic collapse of loose structures, as the solid fraction increases by more than 50%, we observe the number of contacts to increase by 5% in systems without RR, and by 12% with RR. Such a small variation of z in plastic compression contrasts with the comparatively very fast change of z in the quasielastic compression of cohesionless packings, as observed in Section III A, in which z increases by more than 10% for minute density increases.

As to the number of distant attractive interactions, i.e. pairs of neighboring grains separated by a gap smaller than D_0 (contributing to z_-), it is initially very low (typically 10 in a sample of 5600 particles), and then increases with P^* but remains below 2% of the total number of interactions.

3. Distribution of forces

Normal force distributions are (roughly) symmetric about zero in initial states under low P^* [12], as shown in Fig. 18. Under low P^* , tangential forces of order F_0 are also frequently observed [29], and the angle between the total contact force \mathbf{F} and the normal unit vector \mathbf{n} is not constrained by the Coulomb condition, which applies to $\mathbf{F} + F_0\mathbf{n}$ rather than to \mathbf{F} . This explains the typical patterns of self-balanced contact forces in small grain clusters, where compressive and tensile forces of order F_0 compensate locally, as might be observed on Fig. 2. The Coulomb condition applying to \mathbf{F} , on the other hand, favors alignments and “force chains”. Self-stressed small clusters form spontaneously when the disks aggregate, except for large RR and/or small V_0/V^* [29].

As consolidation proceeds, under growing P^* , normal force distributions develop a wider positive (compressive) side (Fig. 18), while the finite value for $N = -F_0$ is characteristic of the failure of bonds in traction. Forces eventually scale proportionally to P^* at large P^* , like in cohesionless systems [34, 35], as shown by Fig. 19. When P^* reaches values of several unities, the force distribution is similar to that of cohesionless packings, with an additional dwindling population of tensile contacts (Fig. 17). Force distributions in systems with RR are quite similar to those shown in Figs. 18 and 19.

4. Forces in dense, overconsolidated states

Upon decompressing to low pressure levels, some larger compressive forces (F_N/F_0 reaching 2 or 3) survive and the distribution is not symmetric (Fig. 18). Such effects of overconsolidation on contact forces are considerably larger than in cohesionless granular materials [34]. As in the case of cohesionless systems [34], we observed that

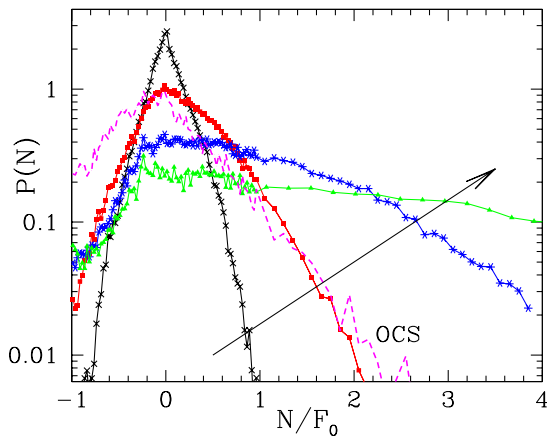


FIG. 18: (Color online) Probability distribution function $P(N)$ of static normal force in contacts, versus N/F_0 , in systems with no RR, for $P^* = 0.01$ (black), $P^* = 0.178$ (red), $P^* = 1$ (blue), $P^* = 2.37$ (green). Distribution widens as pressure increases as indicated by the arrow. $P(N)$ is also shown for $P^* = 0.01$ for the overconsolidated state (OCS) at the end of the pressure cycle (pink dashed line).

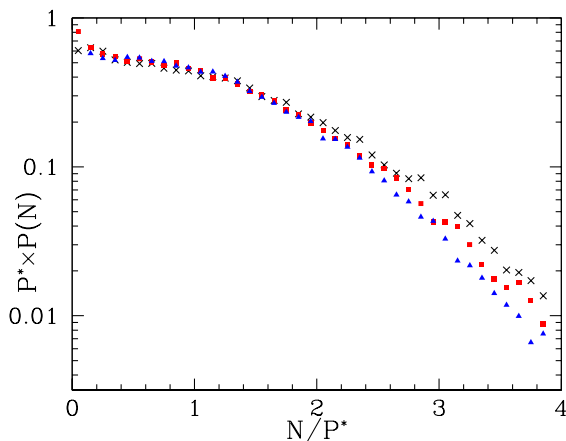


FIG. 19: (Color online) Positive wing of probability distribution function of rescaled normal forces, N/P^* , in systems with no RR, under $P^* = 2.37$ (black crosses), $P^* = 5.62$ (red square dots), $P^* = 13.3$ (blue triangles).

the decompression process tends to be affected by dynamical effects if it is too fast, and the overconsolidation effects on force distributions tend to be erased if too many contacts open in transient stages. The results pertaining to overconsolidated states shown in Figs. 18, 17 and 20 were obtained on simply reversing the stepwise compression program with the parameters indicated in Section IID 2 (i.e. with as many steps in decompression as in compression).

This final force distribution is similar to the one reported by Richefeu *et al.* [12] in simulations of packings of wet spherical beads, in which cohesion is due to capillary forces. After assembling the packing under a finite pressure and then decompressing to $P = 0$, these authors ob-

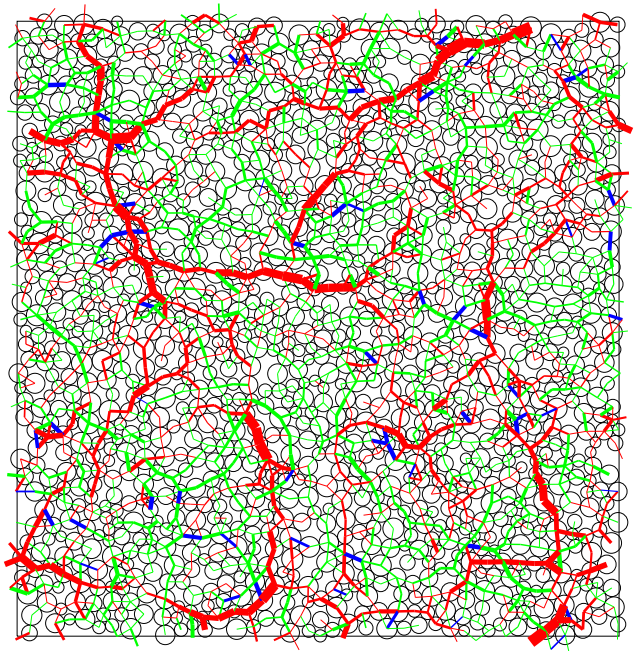


FIG. 20: (Color online) Dense overconsolidated state of a sample under $P^* = 0.01$ (with RR) at the end of the pressure cycle. Color code as on Fig. 2, with distant attractive forces (for which $0 < h < D_0$) in blue.

serve that the particles tend to form small domains with only compressive or only tensile forces. Fig. 20 reveals quite similar patterns in overconsolidated states under $P^* = 0.01$, with some predominance of the regions under tension, while compressive forces tend to organize more often in strong force chains. Tensile contacts are more numerous than compressive ones after the pressure cycle (Fig. 17).

To what extent overconsolidation effects on inner states influence the mechanical properties of cohesive granular materials (e.g., their response to shear stress) would deserve to be investigated.

B. Elastic moduli

Elastic moduli are used in experiments [57] and computer simulations [36, 58] to express the response of granular materials to small load increments. Their measurement, or that of wave velocities, is a non-destructive probe of the packing structure. Thus, in the case of cohesionless bead packings, the simulations of [36] showed that the moduli are sensitive to coordination number, which can vary independently of the solid fraction, and escapes direct observations [31].

In the present case of possibly loose and poorly connected cohesive systems, those moduli approximately describe the parts of the compression curves with no packing rearrangement (Fig. 14), like in cohesionless systems (Fig. 1).

1. Elastic moduli of cohesionless packings

We first quickly describe the variations of elastic moduli in the cohesionless systems of Table II, and how they can be related to their microstructural or micromechanical parameters. Fig. 21 is a plot of bulk and shear moduli versus pressure. Moduli should be proportional to con-

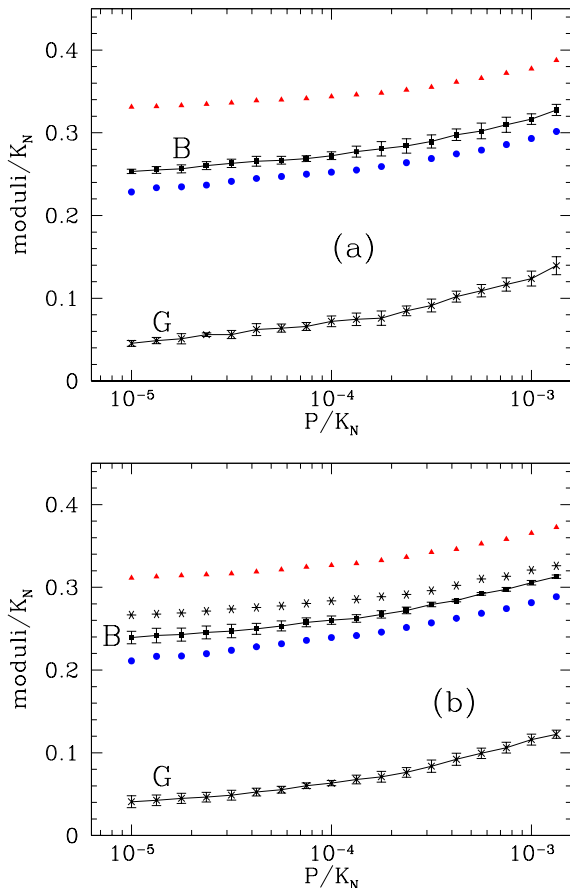


FIG. 21: (Color online) Bulk and shear moduli of cohesionless systems (a) without and (b) with RR, versus pressure in compression cycle. Voigt and Reuss bounds are shown as (red) triangles and (blue) round dots, respectively. Asterisks on plot (b) show values of B obtained on taking a larger rolling stiffness, $K_R = 10^{-2} K_N a^2$ instead of $K_R = 10^{-4} K_N a^2$.

tact stiffness K_N with no RR (as we set $K_T = K_N$). Unlike with Hertzian contacts, local stiffness constants K_N , K_T do not depend on forces. Consequently, the increase of moduli with pressure is moderate. The results of Fig. 21(a) are typical of cohesionless granular systems with small coordination number [35, 36, 40]. The evolution of bulk modulus is correctly described by the simple estimation formulae recalled below in Sec. VB 2, and it is explained by the increase of coordination number. Shear modulus G , on the other hand, is somewhat anomalously low, witnessing the propensity of a rather poorly connected contact network ($z^* \simeq 3.1$ under $P/K_N = 10^{-5}$, without RR) to rearrange under small stress increments,

if those are not proportional to the preexisting stresses.

The evolution of elastic moduli in the unloading part of the pressure cycle (not shown on the figures, for clarity) very nearly reverses the effect of the first compression.

Fig. 21(b) shows that systems with small RR have similar elastic properties, although slightly influenced by rolling stiffness K_R .

2. Simple estimation formulae

Bulk and shear moduli are traditionally estimated by the Voigt or mean field formula [37, 59], which give upper bounds [36] B^V , G^V in terms of contact stiffness constants and coordination number z , based on the assumption that particle centers move like points of a homogeneously strained continuum. In the present case one has:

$$\begin{aligned} B^V &= \frac{z\Phi [\langle d^2 \rangle + \langle d \rangle^2] K_N}{4\pi \langle d^2 \rangle} = \frac{55z\Phi K_N}{112\pi} \\ G^V &= \frac{K_N + K_T}{2K_N} B^V \end{aligned} \quad (14)$$

On deriving (14), similar approximations are used as for (13). The formulae are identical for systems with or without RR, and since we chose $K_T = K_N$ one has also $G^V = B^V$.

For the bulk modulus, one may also write down a lower bound B^R , the Reuss estimate [36], based on the evaluation of the elastic energy with trial forces in a load increment. The formula for B^R involves moments of the contact force distribution, specifically the following ratio:

$$\tilde{Z}_2 = \frac{\langle F_N^2 + \frac{K_N}{K_T} F_T^2 + \frac{K_N}{K_R} \Gamma^2 \rangle}{\langle F_N \rangle^2}, \quad (15)$$

in which averages are taken over all contacts carrying static normal force F_N , tangential force F_T and rolling moment Γ (to be set to zero in the absence of RR). Using (13), one has

$$B^R = \frac{z\Phi \langle d \rangle^2 K_N}{2\pi \langle d^2 \rangle \tilde{Z}_2} = \frac{27z\Phi K_N}{56\pi \tilde{Z}_2}. \quad (16)$$

This approximation of the bulk modulus becomes exact when the force increments caused by an isotropic pressure increase are proportional to the preexisting forces [36], and hence it tends to be accurate in systems with small degrees of force indeterminacy. The ratio of upper to lower bounds of the interval for B given by Eqs. (14) and (16) is $55\tilde{Z}_2/54$, and the bulk modulus is therefore especially well predicted when the force distribution is not too wide [36], and ratio \tilde{Z}_2 stays close to 1. Thus bulk moduli are rather successfully estimated (see Fig. 21) by B^R or B^V in the cohesionless case of Sections III A and VB 1. Force distributions have often been studied in cohesionless systems, in which they are strongly constrained by the no-tension condition. \tilde{Z}_2 cannot reach large values (it is smaller than 1.35 in the present case).

3. Elastic moduli in cohesive packings

Elastic moduli as functions of P^* during consolidation of cohesive systems are plotted in Fig. 22. Note the log-

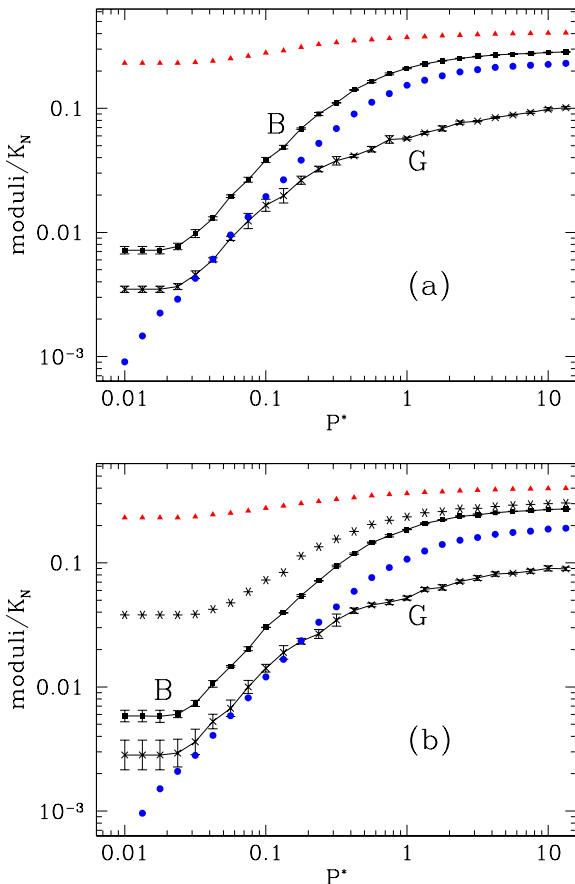


FIG. 22: (Color online) Bulk and shear moduli of cohesive systems (a) without and (b) with RR, versus (growing) pressure. Same symbols and colors as in Fig. 21.

arithmic scale used for elastic moduli (unlike in Fig. 21). Both bulk and shear moduli are very low at small P^* , which cannot be simply explained by the factor $z\Phi$ appearing in simple estimates (14) and (16) (z values, see Fig. 17, are similar to those of cohesionless systems while Φ is twice as small at most). Those anomalously low moduli witness the propensity of the system to rearrange *under isotropic as well as under deviatoric stress increments*.

On decompressing, the moduli (not shown in Fig. 22) stay close to the value reached at the consolidation pressure.

Moduli in samples with RR (Fig. 22b) have very similar values as in the absence of RR, although this may be partly coincidental, since they are quite sensitive to the value of rolling stiffness K_R .

The mean field estimates B^V , G^V are *both* too large by factors of 30 to 50 in loose states. By (13) the average normal force $\langle F_N \rangle$ vanishes as P^* tends to zero, while the

second moment is of order F_0^2 . Moreover, as tangential forces F_T are not limited by μF_N (but by $\mu(F_N + F_0)$ instead), their contribution to the elastic energy is important (and so is that of rolling moments in systems with RR). Hence coefficients \tilde{Z}_2 reach values of order 10^2 or 10^3 in low pressure states, and the ratio B^V/B^R is very large, unlike in cohesionless systems for which \tilde{Z}_2 is of order 1. The Reuss bound for B is first (in regime I) too small by a large factor. Then, it seems to capture the evolution of the bulk modulus in regimes II and III of the consolidation behavior: it increases nearly parallel to the evolution of B . Ratio B/B^R is reduced to ~ 2 for P^* of order 0.1, and slightly decreases as compression proceeds. It should be recalled, though, that the Reuss formula essentially relates the bulk modulus to another unknown quantity, \tilde{Z}_2 .

4. Elastic moduli and force indeterminacy

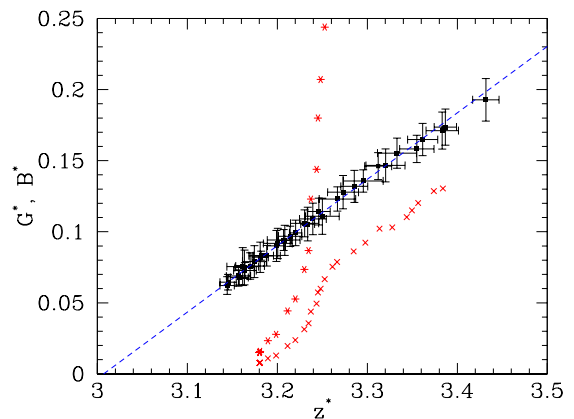


FIG. 23: (Color online) Elastic moduli (no RR) divided by $\Phi(1 - x_0)$, versus z^* . We recall that z^* is the coordination number without rattlers. Data with error bars correspond to G in cohesionless systems, for which the dashed line is a linear fit. G and B in the cohesive material are respectively shown as (red) crosses and asterisks.

The low value of the shear modulus in poorly coordinated cohesionless packings under isotropic stresses (see Fig. 21) has been observed [36, 40] and argued [60] to stem from its tendency to vary proportionally to the degree of force indeterminacy per unit volume (or area in 2D), as the latter approaches zero. The force indeterminacy per unit area without RR is proportional to $(z^* - 3)\Phi(1 - x_0)$, and thus one should have

$$G^* \equiv \frac{G}{\Phi(1 - x_0)} \propto z^* - 3. \quad (17)$$

Fig. 23 shows that our cohesionless packings abide by this law, as the linear variation of G^* with z^* would predict, within uncertainties, its vanishing for $z^* = 3$. However, it is also obvious from Fig. 23 that the anomalous behavior of both moduli in loose, cohesive grain assemblies are

not simply explained by the low coordination number, except perhaps for the shear moduli of the densest configurations (rightmost data points), which, after sufficient plastic compaction, become similar to cohesionless packings. The coordination number, when it is close enough to 3 (without RR) is characteristic of “barely rigid” contact networks, but it is a global, average quantity and does not account for the specific heterogeneities of loose cohesive packings.

5. Response to a small pressure increment

We now briefly discuss the response of contact forces in a small pressure increment ΔP probing the elastic behavior. Corresponding contact force increments are visualized in Fig. 24. Very strong compressive force chains

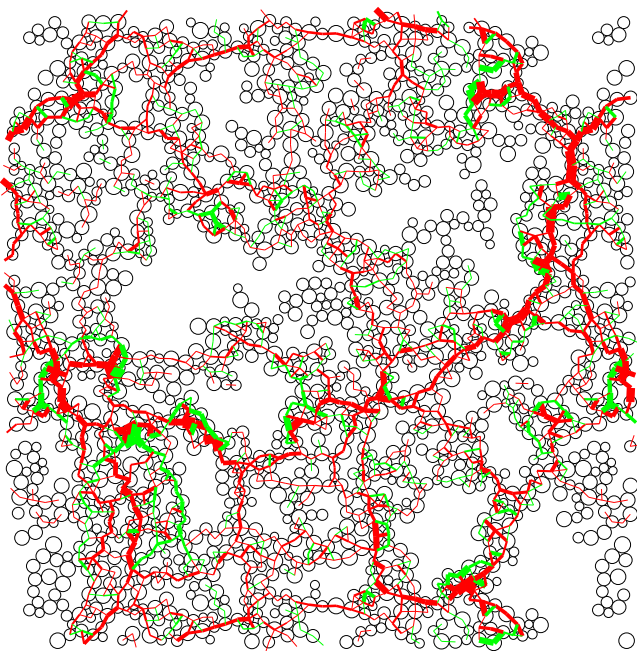


FIG. 24: (Color online) Distribution of force increments associated with elastic response in isotropic compression of system of Fig. 2. Contacts are ordered by decreasing contribution to elastic energy, and only the first 46% contact forces corresponding to 95% of the energy are drawn (colors as on Figs. 2 to 4).

appear, while large parts of the system carry very small forces. On sorting the contacts by decreasing contribution to the elastic energy of the force increments balancing ΔP , less than half of them (46%) contribute 95% of the energy. This proportion increases to about 65% in the densest configurations, to be compared to 68–70% in cohesionless systems. In fact, the configuration of Fig. 24, in a system with RR, has quite a few dead ends, i. e. sets of grains that are connected to the rest of the structure but do not belong to any percolating loop for force (or current) transport through the whole periodic cell. With

RR, the force-carrying structure coincides with the backbone in the sense of ordinary (scalar) percolation theory. The force patterns of Fig. 24 differ from those of Fig. 2, in which the equilibrium forces, prior to the application of ΔP are shown: some regions, especially the isolated, self-stressed clusters where compressions and tensions of order F_0 equilibrate, carry large forces but are bypassed in the transmission of the pressure increment ΔP . Dead ends may contain “islands” of self-balanced forces resulting from the aggregation process, as a direct visual inspection of Fig. 2 reveals, but they do not participate in the transmission of stress increments and they do not contribute to elastic moduli.

In the course of the consolidation process, the repeated application of pressure increments clearly favors force chains over localized self-stressed clusters, and the force pattern adapts to the external pressure. This corresponds to a closer similarity between the spatial distribution of equilibrium forces under pressure P on the one hand, and that of force increments caused by a small compression step ΔP , on the other hand. Hence the better performance of the Reuss approximation scheme relating the bulk modulus to the force distribution – although both quantities are difficult to predict.

6. Scaling with fractal blob size

The inability of the approaches used in cohesionless systems to predict the elastic moduli of loose cohesive packings can be attributed to their ignoring the peculiar network geometry, which is the origin of the strong force concentration shown in Fig. 24.

In view of the results of Section IV, it is tempting to relate the elastic moduli to the variations of blob size ξ . In scaling arguments about the density, the system can be regarded as a densely packed assembly of somewhat fuzzy ξ -sized objects, the blobs. To discuss elastic properties, the system is better represented as a network of “superbonds” of length ξ , or effective beams (with which the elongated structures carrying stress in Fig. 24 could be identified). In such a network, the dominant deformation mode is beam bending. The transverse deflection δ in bending of a beam of length ξ , caused by a force F , is proportional to $\xi^3 F$. Macroscopically, strains are of order $\epsilon = \delta/\xi$, while F corresponds to stress σ by $F \propto \sigma \xi$ in 2D. Consequently, the scaling of elastic moduli σ/ϵ with length ξ should involve a factor ξ^{-3} . (Some possible corrections to exponent 3 are possible, although the appropriate value in, e.g., the case of percolation networks of beams is very close to 3 [61, 62]). As ξ varies by a factor of 3 or 4 within the scaling range (see Fig. 16), relation $B \propto \xi^{-3}$ would predict an increase of moduli by a factor of a few tens.

Although this can be regarded as a fair estimate (see Fig. 22), it should be admitted that the fractal range is very likely too restricted for such scaling laws to apply without important corrections. With sufficiently large

rolling resistance, the “beams” can be reduced to single particle chains. Their bending stiffness is then simpler to analyze. Such cases are discussed below in Section V C.

C. The role of rolling resistance

As reported in the previous paragraphs, the small level of rolling resistance used in most simulations reported here ($\mu_R = 5 \cdot 10^{-3}$) has no notable effect on force distributions or moduli. Yet, such a small RR significantly affects plasticity index λ (see Table III) and fractal dimension d_F , which changes from 1.9 without RR to about 1.5 (Section IV).

Rolling resistance favors force transmission along thin strands of particles, each of them in contact with two neighbors. In order to understand the mechanisms by which RR affects macroscopic behavior and geometry, we investigated the effects of a large RR in one 1400-disk configurations. While single particle chains are eas-

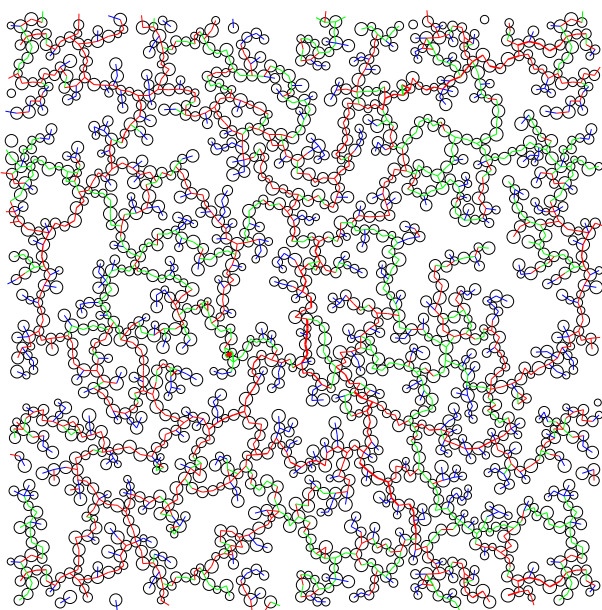


FIG. 25: (Color online) Configuration of loose sample with large RR under $P^* = 0.01$.

ily disrupted if μ_R/a is small, they become much more frequent for large rolling resistance, as shown in Fig. 25 (corresponding to rolling friction $\mu_R/a = 0.5$). Thus coordination numbers may approach 2 (see Fig. 9). The density and the length of such particle chains is also witnessed by the proportion x_{22} of the contacts that join 2-coordinated disks. Such contacts are impossible in an equilibrium structure without RR. x_{22} reaches 12% in large RR systems under low pressure (for Φ in the 0.4 to 0.5 range), down to 1-2% in the main sample series of Table II with small RR ($\mu_R/a = 0.05$). Thin, rigid strands of 2-coordinated disks might, however, be decorated by a side arm acting as a dead end for force transmission

(many examples can be seen in Fig. 25), and their mechanical role is thus only partially captured on simply recording fraction x_{22} . In the limit of $z \rightarrow 2$, which is approached under low pressure for large RR and/or low velocity V_0 in the assembling stage, the force network has a vanishing number of loops and approaches isostaticity, as discussed in paper I [29]. Consequently, as opposed to systems with small or no RR, force values are narrowly distributed for $P^* \ll 1$. For P^* of order 10^{-2} , normal forces above $F_0/5$ or below $-F_0/10$ are extremely scarce (with probability distribution function $P(N)$ in the 10^{-3} range). Furthermore, with $P^* \sim 1$, while compressive normal forces of order F_0 are frequently observed, $P(N)$ remains below 10^{-2} for $N \rightarrow -F_0$. This contrasts with the results shown on Fig. 18: the proportion of contacts on the verge of tensile rupture is much smaller in systems with large rolling resistance.

The prevalence of particle strands as force-transmitting structures should influence elastic properties. As noted in Section V B 6, linear structures tend to deform like bending beams, with a compliance proportional to the third power of their length. In the case of single linear strands, connections with contact properties are easily made more explicit. Consider, e.g., a straight, linear chain of n identical disks of radius R , with $n - 1$ contacts characterized by stiffness constants K_N , K_T and K_R . Then, in the elastic regime, all intermediate disks can be suppressed and the interaction between the extreme ones, numbers 1 and n along the chain, can be replaced by an effective one between two disks of radius $(n - 1)R$, and effective compliances $1/K_N^{(n)}$, $1/K_T^{(n)}$, and $1/K_R^{(n)}$ for normal, tangential and rolling relative motion, given by:

$$\begin{cases} \frac{1}{K_N^{(n)}} = \frac{n-1}{K_N} \\ \frac{1}{K_T^{(n)}} = \frac{n-1}{K_T} + \frac{(n-1)(4n^2 - 11n + 6)R^2}{3K_R} \\ \frac{1}{K_R^{(n)}} = \frac{n-1}{K_R} \end{cases} \quad (18)$$

For large n the tangential compliance is much larger than the longitudinal and rolling ones, so that long chains behave as beams, which essentially deform in bending. The local bending stiffness EI of the beam (i.e. the product of the material Young modulus by the moment of inertia of the beam section) corresponding to the chain of particles in the continuous limit is $EI = 2RK_R$. (This coefficient expresses the proportionality of bending moment to rotation angle gradient). For $n \gg 1$, the bending spring constant $3EI/l^3$ (expressing the transverse force to transverse deflection relationship) is correctly identified from $K_T^{(n)}$ given in (18), using the length $l = 2(n - 1)R$ of the straight n -particle strand.

Remarkably, the bending elasticity of small linear strands of micrometer-sized colloidal particles bound by adhesive forces has recently been measured by means of

optical tweezers [63]. Colloidal gels of polymer particles [64, 65, 66] should thus be modeled as cohesive particle assemblies with rather large RR level.

It is easy to check (just, for instance, by considering two such chains joining at their ends at some angle) that for all strand shapes other than straight lines, the extremities will be coupled by spring constants of order $K_T^{(n)}$ for both longitudinal (parallel to end-to-end vector) and transverse relative displacements. Consequently, the macroscopic elastic moduli should be proportional to rolling stiffness constant K_R . Fig. 26 shows that this proportionality is approximately satisfied in the loosest states of a system with rolling friction $\mu_R/a = 0.05$, in which three different values of K_R were used to evaluate the elastic response. Elastic moduli of denser states,

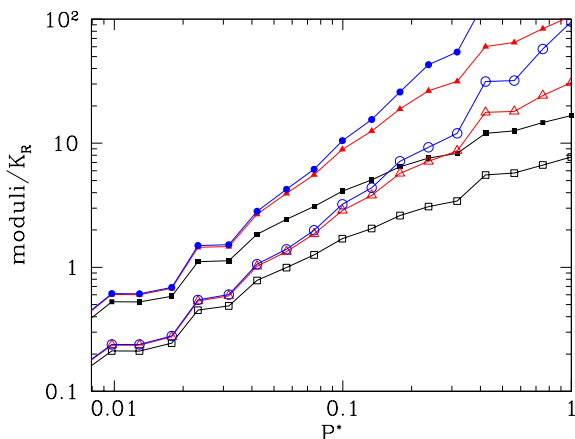


FIG. 26: (Color online) Bulk (filled symbols) and shear (open symbols) moduli, normalized by K_R , in low pressure states of a sample for which $\mu_R/a = 0.05$ and $K_R = 10^{-2} K_N a^2$ (black squares). Results obtained on evaluating moduli with $K_R = 10^{-3} K_N a^2$ and with $K_R = 10^{-4} K_N a^2$ are respectively shown as red triangles and blue circles.

however, depart from this behavior. Consequently, the scaling of elastic moduli with typical strand length (as suggested in Section V B 6) is limited to low consolidation states.

With small or vanishing RR, single particle strands are replaced by thicker junctions, which further restricts the consolidation pressure range for which elasticity is dominated by beam bending.

VI. PLASTIC CONSOLIDATION MECHANISM: QUALITATIVE ASPECTS

Cohesionless granular assemblies, if subjected to stress increments that are not proportional to initial stresses, essentially deform because the contact network gets repeatedly broken and repaired [33, 67]. Macroscopic strains, once they exceed the very small scales associated with the response of given contact networks [33, 36], thus result from a sequence of rearrangement events or micro-

scopic instabilities, during which the granular packing loses its coherence and gains some finite amount of kinetic energy, even for arbitrarily slow applied stress changes. Collisions and appearance of new contacts stabilize the packing at the end of each microscopic rearranging event. This process gradually changes the topology of the contact network, and produces specific evolution of its fabric (orientation anisotropy).

The mechanism of plastic collapse in isotropic compression of loose cohesive assemblies with small or vanishing RR in contacts, as observed in the present study, is similar. Just like in cohesionless systems [33], we expect the frequency of occurrence of rearrangements, along the loading path, to increase, and the corresponding strain jumps to decrease, as the size of samples grows, and thus the consolidation curve should be smooth in the thermodynamic limit. Due to the specific geometry of loose systems, in which dense zones are weakly connected through thin arms, better connected, solid-like regions tend to move like rigid bodies, while fragile junctions break and rearrange, so that initially large holes gradually fill up. Fig. 27 is an illustration of such a scenario. Displacements are depicted as arrows, pointing from the current positions to the ones reached in the next equilibrium configuration in the stepwise compression sequence. The affine term, corresponding to the overall cell shrinking deformation as Φ is increased by 0.05, only contribute a small part of the displacement field (typically 0.17%). Consequently, although this affine contribution is subtracted out from particle displacements on the figure, more densely packed, nearly rigid regions (marked with dotted lines) are easily identified by direct visual inspection. Fig. 27 shows that the contact network undergoes relatively small topological changes. More than 90% of contacts are conserved in this configurational change. The rate of contact change, and the evolution of coordination number with strain are significantly smaller than in cohesionless systems undergoing, e.g., shear deformation. During the compaction of loose samples those dense regions collide and slide past one another, along thin sheared zones where most of the broken contacts are found.

In the case of large RR, the peculiar microstructure involving single particle chains might lead to a slightly different deformation mechanism: unlike multiply connected junctions, simple strands can yield in bending without breaking: they fold at some contact, where the rolling friction threshold is reached, thereby releasing bending elasticity. This mechanism has directly been observed in experiments on single chains of colloidal particles [63, 65]. One might thus expect fewer contact losses in plastic compression.

To follow more closely the rearrangement sequences in the course of compaction, it is appropriate to monitor changes in the list of contacts in the motion between two equilibrium configurations. As an example, let us consider the evolution between equilibrated states as P^* increases from 0.177 to 0.237, and compare two samples,

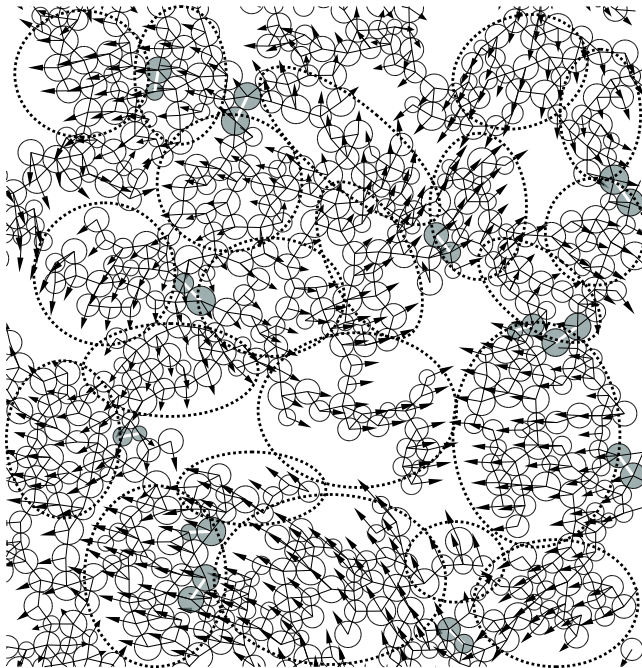


FIG. 27: Equilibrium particle positions in 1400 disk sample with small RR under $P^* = 0.032$. Particle displacements to new configuration equilibrated under $P^* = 0.042$ are shown as arrows (global density change $\Delta\Phi = 0.05$). Neighbor pairs for which contact opens are filled in grey. All other contacts (thin solid lines) are maintained. Dense regions moving approximately like rigid solids are circled within dotted lines. Most lost contacts are situated near the boundaries of such solid-like particle lumps. Although within some of these particle lumps some rigid sub-zones can be appreciated, as the structure evolves, their velocity fields converge to a unique direction moving as a whole.

one with small ($\mu_R/a = 0.005$) and the other with large ($\mu_R/a = 0.5$) RR. Table IV gives the changes in solid fraction and coordination number, and numbers of maintained, destroyed and created contacts in this compression step. Successive configurations separated by a fixed time interval $\Delta t = 0.16T_0$ are compared and Fig. 28 plots the number of destroyed and created contacts as functions of time. For the same strain increment, contact losses, as a function of global strain, are significantly less frequent in the sample with large RR. This fact is reflected both in the data of Table IV, where global changes are recorded, between the initial and final states, and in those of Fig. 28, where successive changes over time intervals Δt are detailed. As a consequence, while the coordination number hardly changes during consolidation in systems with small or vanishing RR (see Fig. 17), it gradually increases from an initial value close to 2 to nearly 3 in systems with large RR (Fig. 9). The lesser importance of tensile contact rupture in the plastic compression of assemblies with large RR is also witnessed by the normal force distribution (Section V C): forces approaching $-F_0$ are quite scarce, as opposed to the situation in samples

μ_R/a	$\Delta\Phi(\%)$	$\Delta z(\%)$	$N^{(=)}$	$N^{(-)}$	$N^{(+)}$
0.005	3.2	0.14	2084 (94.9 %)	112 (5.1 %)	115 (5.2 %)
0.5	3.1	1.2	1679 (98.5 %)	26 (1.5 %)	46 (2.7 %)

TABLE IV: Relative changes of solid fraction, $\Delta\Phi$, and of coordination number (Δz), and numbers of maintained ($N^{(=)}$), destroyed ($N^{(-)}$) and created ($N^{(+)}$) contacts in a 1400 disks sample, with small or large RR, in the compression step between $P^* = 0.177$ and $P^* = 0.237$.

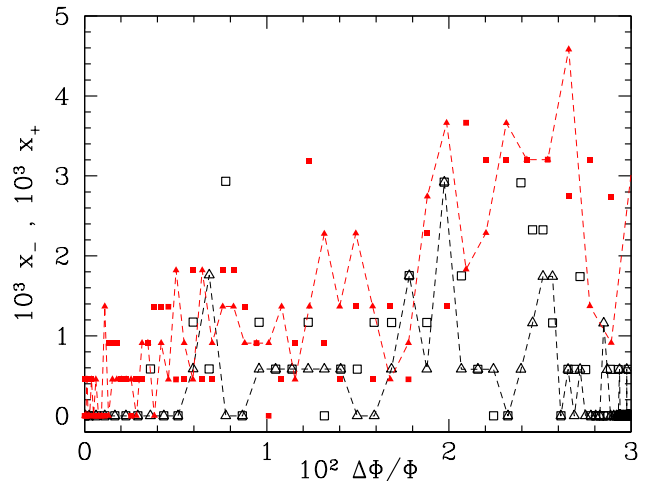


FIG. 28: (Color online) Evolution of the contact number as a function of relative density increase. In sample with $\mu_R/a = 5 \cdot 10^{-3}$ the proportions x_+ and x_- of gained and of lost contacts with respect to previous recorded list are respectively shown with red square dots and triangles – the latter being connected with a dashed line. A similar code is used for x_+ and x_- values in a sample with large RR ($\mu_R/a = 0.5$), but with open dots, and in black.

without RR (Fig. 18). With small RR, some single particle chains are also present, although shorter and less numerous. The sensitivity of plasticity index λ to the rolling friction is likely to be explained by different rupture mechanisms, the importance of folding rearrangements growing with the level of rolling resistance.

VII. CONCLUSION

To summarize, we have used numerical simulations to observe and characterize, at the macroscopic and microstructural levels, the consolidation behavior, in isotropic compression, of model cohesive powders. Macroscopic constitutive laws for quasistatic loading, unloading and elastic responses were shown to be reasonably well approached. The material behavior was investigated for a range of densities that is wider than in most simulation studies of cohesive granular materials. The consolidation process goes through three stages. In a first

regime, which is sensitive to the assembling procedure, no plastic collapse occurs, as the agitation in the assembling process has stabilized a strong enough microstructure to withstand a finite pressure increase. The normal force distribution widens until a significant fraction of contacts are on the verge of tensile rupture. The initial system geometry, which changes very little in regime I, is that of a dense assembly of fractal blobs, with dimension d_F taking the universal value associated with the aggregation process (here, ballistic) implemented in the sample preparation stage. The blob size ξ (at most, between 5 and 10 grain diameters in the present case) can be identified on studying density correlations. The subsequent consolidation behavior is remarkably independent on initial conditions, which merely determine where the intrinsic consolidation curve in the Φ - P^* plane is first met. The same curve is then followed whatever the initial conditions as the material is further compressed. This behavior corresponds, at the microscopic level, to a gradual change of the blob size. The curve in regime II has the same shape as reported in the soil mechanics literature, and the consolidation pressure is a plastic threshold below which the material response is approximately elastic (like the behavior of a cohesionless granular material under isotropic load). Elastic moduli increase rapidly with consolidation pressure or density. The cases of small RR or without RR should be distinguished from the situation of strong rolling resistance, although, in both cases, the microstructure of loose packings might be viewed as denser, better connected regions joined by thin arms. In the first case, loose packings collapse when the tensile strength of contacts is overcome by the externally imposed forces, preferentially within the fragile junctions between adjacent denser blobs. Systems with strong RR, on the other hand, contain single particle strands, which tend to fold without breaking in plastic compaction. While small RR systems gain very few contacts in the consolidation process, the coordination number might increase from nearly 2 to 3 with large RR. Eventually, the material approaches a limiting, maximum density (regime III), as the packing structure resembles that of a cohesionless system, for $P^* \gg 1$ (albeit, typically, somewhat looser). The absence of a similar upper limit of the density of cohesive packings in experiments for large P^* is due to plastic deformation of contacts. For large P^* cohesion forces are negligible, hence a procedure to produce loose packings by removing cohesive forces under large enough confining pressure.

The fractal blob size ξ , depending on solid fraction Φ , is a central microstructural feature, based on which some scaling laws for elastic properties can be attempted. It is also tempting, beyond the qualitative description of the microstructural changes associated with the consolidation process, to try to predict the consolidation curve from such geometric data. Yet scaling laws appear to apply only to a restricted part of the consolidation pressure interval.

Our results, in many respects, emphasize important

qualitative differences between cohesive and cohesionless granular assemblies. The existence of stable loose structures and the consolidation phenomenon are the most important differences in macroscopic behavior brought about by cohesion. At the microstructural level, unlike in cohesionless packings, the typical values of intergranular forces, or the force distribution, are not as simply estimated in cohesive systems, in which attractive and repulsive contact forces of the order of tensile strength F_0 tend to compensate under low pressure. In particular, compression cycles stabilize self-balanced force networks with large compression forces. Unlike in granular packings devoid of cohesion, the coordination number does not appear to be a significant state variable in cohesive systems with low RR, as it hardly changes along the consolidation curve. With large rolling resistance, it witnesses, however, the formation of loops under compression. While cohesionless assemblies with low coordination number usually contain many rattlers, all particles in cohesive packings are connected to the same contact structure, which is rigid, but comprises lots of “dead ends” or “side arms”, which might bear self-balanced forces but do not participate in the transmission of external stresses. Some of these new features can be summed up on remarking that loose powders are similar to gels as much as to granular packings with no cohesion.

Our investigations should be pursued in several directions. On the theoretical side, the connections between macroscopic properties and microstructure could be studied quantitatively. The behavior of loose cohesive packings under general stress states should be investigated. Thus one may determine whether such constitutive laws as the Cam-clay model [1] apply to the simulated material. And finally, more quantitative agreement with experiments and real materials should be sought. In spite of some obvious steps (e.g., one should simulate 3D systems), this latter objective looks daunting. One major difficulty is the importance of hydrodynamic effects at the assembling stage, when the microstructure and the fractal dimension of aggregates are determined. While we have bypassed this problem on implementing ballistic aggregation, it is necessary to investigate the behavior of other possible kinds of aggregates, by dealing with some tractable model for hydrodynamic effects. It is hopefully possible to introduce some mechanics and intergranular interactions within the models used with geometric aggregation rules (such as, e.g., diffusion-limited cluster-cluster aggregation). Then, another difficulty is that many parameters associated with the contact law (such as friction coefficient, rolling friction, rolling stiffness constant) should be identified for a real material to be investigated at the grain level. In this respect, the recent progress in microscopic observation and manipulation techniques seems quite promising, as formerly inaccessible parameters ruling interparticle contact mechanics are now beginning to be measured in model materials, thanks to particle-scale microscopy and micromanipulation techniques [43, 44, 63, 65, 68].

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- [1] D. M. Wood, *Soil Behaviour and Critical State Soil Mechanics*. (Cambridge University Press, England, 1990).
 - [2] J. K. Mitchell, *Fundamentals of soil behavior* (Wiley, New York, 1993).
 - [3] J. Biarez and P.-Y. Hicher, *Elementary Mechanics of Soil Behaviour* (A. A. Balkema, Rotterdam, 1993).
 - [4] J. H. Atkinson, *An introduction to mechanics of soils and foundations: Through critical state soil mechanics*. (McGraw-Hill, International Series in Civil Engineering, New York, 1993).
 - [5] D. Poquillon, J. Lemaitre, V. Baco-Carles, P. Tailhades, and J. Lacaze, *Powder Tech.* **126**, 65 (2002).
 - [6] A. Castellanos, *Advances in Physics* **54**, 263 (2005).
 - [7] P. Pierrat and H. S. Caram, *Powder Technology* **91**, 83 (1997).
 - [8] T. Gröger, U. Tüzün, and D. M. Heyes, *Powder Technology* **133**, 203 (2003).
 - [9] Z. Fournier, D. Geromichalos, S. Herminhaus, M. M. Kohonen, F. Mugele, M. Scheel, B. Schulz, C. Schier, R. Seemann, and A. Skudelný, *Journal of Physics Condensed Matter* **17**, 5477 (2005).
 - [10] P. A. Cundall and O. D. L. Strack, *Géotechnique* **29**, 47 (1979).
 - [11] V. Richefeu, M. S. El Youssoufi, and F. Radjai, *Physical Review E* **73** (2006).
 - [12] V. Richefeu, F. Radjai, and M. S. El Youssoufi, *Euro. Phys. J. E* **21**, 359 (2006).
 - [13] C. L. Martin and D. Bouvard, *Acta Materialia* **51**, 373 (2003).
 - [14] C. L. Martin, *Acta Materialia* **51**, 4589 (2003).
 - [15] C. L. Martin, D. Bouvard, and S. Shimai, *Journal of the Mechanics and Physics of Solids* **51**, 667 (2003).
 - [16] C. L. Martin, *Journal of the Mechanics and Physics of Solids* **52**, 1991 (2004).
 - [17] S. Luding, *Powder Technology* **158**, 45 (2005).
 - [18] R. Tykhoniuk, J. Tomas, S. Luding, M. Kappl, L. Heim, and H.-J. Butt, *Chemical Engineering Science* **62**, 504 (2007).
 - [19] K. Dong, R. Yang, R. Zou, and A. Yu, *Physical Review Letters* **96**, 145505 (2006).
 - [20] A. Misra, *Materials and manufacturing processes* **11**, 925 (1996).
 - [21] D. Kadau, G. Bartels, L. Brendel, and D. E. Wolf, *Computational Physics Communications* **147**, 190 (2002).
 - [22] D. Kadau, G. Bartels, L. Brendel, and D. E. Wolf, *Phase Trans.* **76**, 315 (2003).
 - [23] G. Bartels, T. Unger, D. Kadau, D. E. Wolf, and J. Kertész, *Granular Matter* **7**, 139 (2005).
 - [24] D. E. Wolf, T. Unger, D. Kadau, and L. Brendel, in *Powders and Grains 2005*, edited by R. García Rojo, H. J. Herrmann, and S. McNamara (Balkema, Lisse, 2005), pp. 525–533.
 - [25] R. Brewster, G. S. Grest, J. W. Landry, and A. J. Levine, *Physical Review E* **72**, 061301 (2005).
 - [26] L. Aarons and S. Sundaresan, *Powder Technology* **169**, 10 (2006).
 - [27] P. Rognon, J.-N. Roux, D. Wolf, M. Naaïm, and F. Chevoir, *Europhysics Letters* **74**, 644 (2006).
 - [28] P. G. Rognon, J.-N. Roux, M. Naaïm, and F. Chevoir, *J. Fluid Mech.* **596**, 21 (2008).
 - [29] F. A. Gilabert, J.-N. Roux, and A. Castellanos, *Physical Review E* **75**, 011303 (2007).
 - [30] M. Parrinello and A. Rahman, *Journal of Applied Physics* **52**, 7182 (1981).
 - [31] I. Agnolin and J.-N. Roux, *Physical Review E* **76**, 061302 (2007).
 - [32] A. Tordesillas and D. C. Stuart, *Powder Technology* **124**, 106 (2002).
 - [33] J.-N. Roux and G. Combe, *C. R. Académie des Sciences (Physique)* **3**, 131 (2002).
 - [34] I. Agnolin and J.-N. Roux, *Physical Review E* **76**, 061303 (2007).
 - [35] E. Somfai, J.-N. Roux, J. Snoeijer, M. van Hecke, and W. van Saarloos, *Physical Review E* **72**, 021301 (2005).
 - [36] I. Agnolin and J.-N. Roux, *Physical Review E* **76**, 061304 (2007).
 - [37] H. A. Makse, N. Gland, D. L. Johnson, and L. M. Schwartz, *Phys. Rev. Lett.* **83**, 5070 (1999).
 - [38] C. S. O'Hern, L. E. Silbert, A. J. Liu, and S. R. Nagel, *Phys. Rev. E* **68**, 011306 (2003).
 - [39] H. P. Zhang and H. A. Makse, *Physical Review E* **72**, 011301 (2005).
 - [40] E. Somfai, M. van Hecke, W. G. Ellenbroek, K. Shundyak, and W. van Saarloos, *Physical Review E* **75**, 020301 (2007).
 - [41] M. A. S. Quintanilla, Ph.D. thesis, Universidad de Sevilla (2003).
 - [42] A. Castellanos, J. M. Valverde, and M. A. S. Quintanilla, *Physical Review Letters* **94**, 075501 (2005).
 - [43] D. M. Schaefer, M. Carpenter, B. Gady, R. Reifengerger, L. P. DeMejo, and D. S. Rimai, *J. Adhes. Sci. Technol.* **8**, 197 (1994).
 - [44] M. Reitsma, V. Craig, and S. Biggs, *Int. J. Adhes. Adhes.* **20**, 445 (2000).
 - [45] M. A. S. Quintanilla, A. Castellanos, and J. M. Valverde, *Physical Review E* **64**, 031301 (2001).
 - [46] J. Blum and R. Schräpler, *Physical Review Letters* **93**, 115503 (2004).
 - [47] F. A. Gilabert, Ph.D. thesis, Universidad de Sevilla (2007).
 - [48] N. Benahmed, J. Canou, and J.-C. Dupla, *Comptes-Rendus Académie des Sciences, Mécanique* **332**, 887 (2004).
 - [49] P.-G. de Gennes, *Scaling Concepts in Polymer Physics* (Cornell University Press, Ithaca, 1979).
 - [50] G. Dietler, C. Aubert, D. Cannell, and P. Wiltzius, *Physical Review Letters* **57**, 3117 (1986).
 - [51] J.-P. Cohen-Addad, ed., *Physical properties of polymeric gels* (Wiley, New York, 1996).
 - [52] P. Meakin, *Journal of Sol-Gel Science and Technology*

- 15**, 97 (1999).
- [53] S. S. Narine and A. G. Marangoni, *Physical Review E* **59**, 1908 (1999).
 - [54] J.-N. Roux, D. Broseta, and B. Demé, *Langmuir* **17**, 5085 (2001).
 - [55] E. Jaraiz, S. Kimura, and O. Levenspiel, *Powder Tech.* **72**, 23 (1992).
 - [56] R. Y. Yang, R. P. Zou, and A. B. Yu, *J. Appl. Phys.* **94**, 3025 (2003).
 - [57] R. Kuwano and R. J. Jardine, *Géotechnique* **52**, 727 (2002).
 - [58] H. A. Makse, N. Gland, D. L. Johnson, and L. Schwartz, *Physical Review E* **70**, 061302 (2004).
 - [59] K. Walton, *Journal of Mechanics and Physics of Solids* **35**, 213 (1987).
 - [60] M. Wyart, *Annales de Physique Fr.* **30**, 1 (2006).
 - [61] A. Kantor and I. Webman, *Physical Review Letters* **52**, 1891 (1984).
 - [62] S. Roux, *Journal of Physics A* **19**, L351 (1986).
 - [63] J. P. Pantina and E. M. Furst, *Physical Review Letters* **94**, 138301 (2005).
 - [64] J. P. Pantina and E. M. Furst, *Langmuir* **20**, 3940 (2004).
 - [65] E. M. Furst and J. P. Pantina, *Physical Review E* **75**, 050402(R) (2007).
 - [66] J. P. Pantina and E. M. Furst, *Langmuir* **24**, 1141 (2008).
 - [67] L. Staron, J.-P. Vilotte, and F. Radjai, *Physical Review Letters* **89**, 204302 (2002).
 - [68] L.-O. Heim, H.-J. Butt, J. Blum, and R. Schräpler, *Granular Matter* **10**, 89 (2008).