



Wave propagation through soils in centrifuge testing.

J. F. Semblat, M. P. Luong

► To cite this version:

J. F. Semblat, M. P. Luong. Wave propagation through soils in centrifuge testing.. Journal of Earthquake Engineering, 1998, 2, pp.147-171. <10.1142/S1363246998000071>. <hal-00111600>

HAL Id: hal-00111600

<https://hal.science/hal-00111600v1>

Submitted on 23 Jan 2009

HAL is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.



HAL Authorization

Wave propagation through soils in centrifuge testing

J-F Semblat

Laboratoire Central des Ponts et Chaussées, Paris, France
jean-francois.semlat@lcpc.fr

M.P. Luong

Laboratoire de Mécanique des Solides, Ecole Polytechnique, Palaiseau, France
luong@athena.polytechnique.fr

ABSTRACT : Wave propagation phenomena in soils can be experimentally simulated using centrifuge scale models. An original excitation device (drop-ball arrangement) is proposed to generate short wave trains. Wave reflections on model boundaries are taken into account and removed by homomorphic filtering. Propagation is investigated through dispersion laws. For drop-ball experiments, spherical wave field analysis assuming linear viscoelasticity leads to a complete analytical description of wave propagation. Damping phenomena are examined and evaluated using this description.

1. INTRODUCTION

The dynamic behaviour of soils cannot generally be studied directly. However some special experimental methods allow direct analysis of dynamic soil behaviour [13, 15]. Nevertheless dynamic and vibratory excitations on soil often involve propagation phenomena. Such transient phenomena may prevail in the dynamic response of the medium, dealing with particular questions such as : wave dispersion, scattering and attenuation. This paper presents original wave propagation experiments in a centrifuged medium.

Wave propagation experiments in centrifuged scaled models [2, 4, 5, 10] generally concern earthquake engineering problems. Using such experiments, it is important to improve the representativeness of the source of excitation with respect to design considerations. Furthermore, the good control of wave reflection is sometimes difficult since the generated waves travel in a bounded medium of complex mechanical behaviour.

2. EXCITATION DEVICES

In this study, two different sources of excitation are used : an explosive earthquake simulator and a drop-ball arrangement to generate surface waves by the impact of a falling mass. The dimensions of the rigid container are the following : length $L=1.30$ m, width $l=0.80$ m and depth $h=0.40$ m. It is filled with dry Fontainebleau sand of unit weight $\gamma=16.5$ kN.m⁻³ and mean grain diameter 196 μ m.

2.1 Earthquake simulations

Earthquake simulation devices are often used to study soil-structure interaction problems. These generally consist of an explosion room coupled with a mechanical filter. The pressure wave created is transmitted to the soil mass by a flexible membrane. This kind of experimental device presents two important drawbacks :

- the long duration of the generated wave train makes wave reflection phenomena difficult to detect and to circumvent ;
- the excitation is not directly transmitted to the soil mass : there is a container-excitation interaction (part of the excitation travels in the container itself).

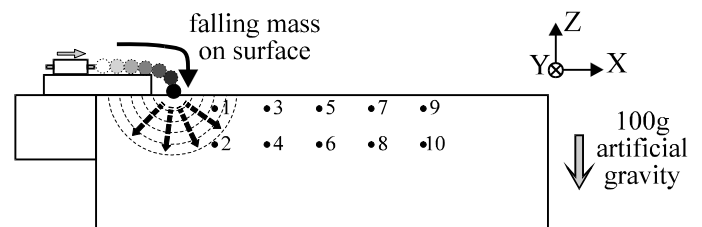


Figure 1 : Drop-ball arrangement for dynamic excitation on soil.

2.2 Drop-ball arrangement

Classical devices for wave propagation analysis in a centrifuged medium (explosive sources, vibratory loadings...) do not generally allow an easy control of the generated wave train (duration, transmission...). Our purpose is to generate a wave field of short

duration using a simple experimental system. As it is shown in Figure 1, a pneumatic actuator pushes a spherical ball which then falls onto the medium surface. All sensors have the same coordinate along the Y-axis. The whole experimental device is called « *drop-ball arrangement* ». It gives both short excitation duration and direct loading transmission [13,14].

2.3 Acceleration measurements

Three-dimensional acceleration measurements are performed at ten different locations positionned at two different depths (see Figure 1). Examples of horizontal acceleration signals (X-axis) are given in Figures 2 and 3. For earthquake simulations, the spectral content is rich, but the signal duration is long compared with the propagation delays through the container (see Figure 2 and [13]). For the drop-ball arrangement (Figure 3), acceleration signal duration is much shorter : it allows a good control of propagation phenomena and a precise analysis of dispersion and the attenuation process.

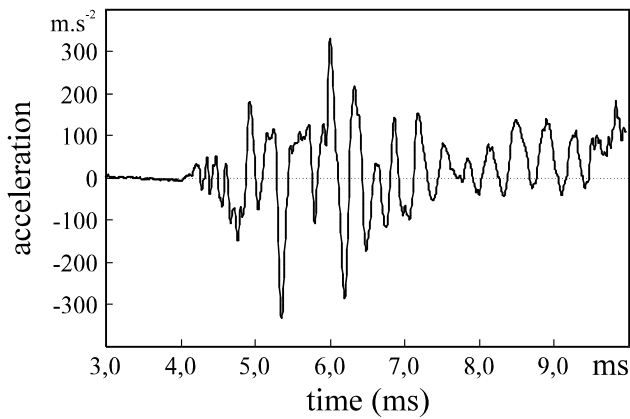


Figure 2 : Acceleration signal for explosive earthquake simulations.

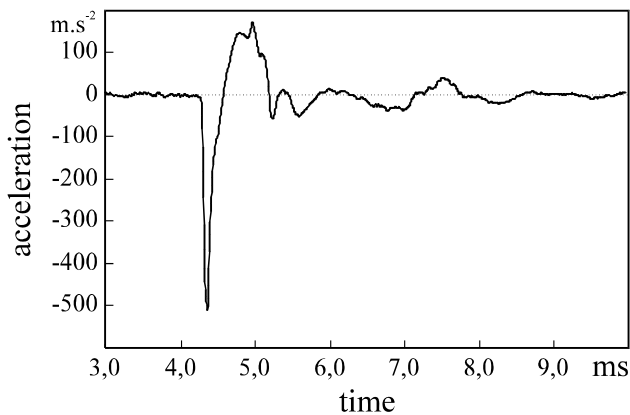


Fig.3: Acceleration signal for drop-ball experiments.

For drop-ball experiments, as the generated wave field is spherical, distances from the source to the

sensors have to be calculated. Table I gives values of all the source-sensor distances.

surface sensors					
number	1	3	5	7	9
distance	0.11 m	0.24 m	0.37 m	0.50 m	0.63 m
mid-depth sensors					
number	2	4	6	8	10
distance	0.19 m	0.28 m	0.40 m	0.52 m	0.65 m

Table I : Distance from the spherical wave source to the sensors.

2.4 Waves characterization

From the 3D measurements, it is possible to determine the acceleration vectors. For earthquake simulations, they are difficult to build because of the fast and irregular changes in their directions and magnitudes. For drop-ball experiments, acceleration vectors are quite easy to determine and to analyse because there is no interaction between the direct and reflected waves.

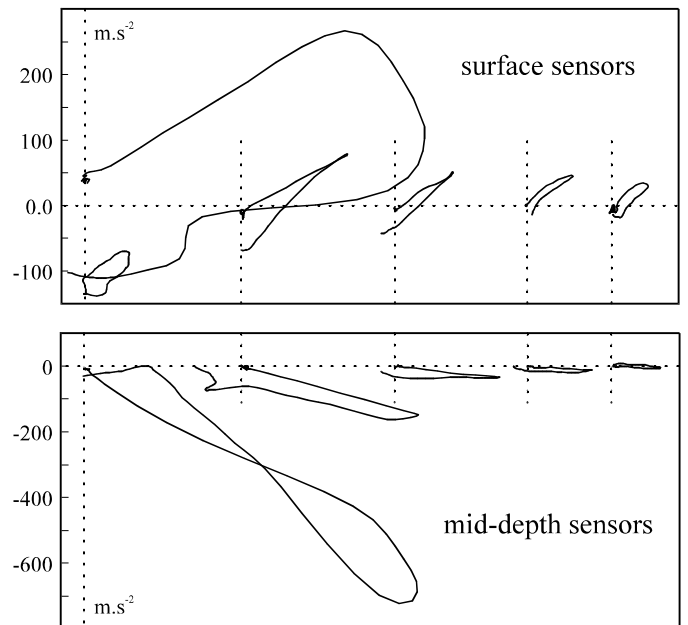


Figure 4 : Accelerations in XZ plane from experimental results.

Figure 4 gives acceleration values in the XZ plane for a drop-ball experiment : surface sensors at top (odd numbers) and mid-depth sensors at bottom (even numbers). It is then possible to determine

maximum accelerations and to build the corresponding acceleration vectors in XZ plane (acceleration along the Y axis is negligible).

For each sensor, the maximum acceleration magnitude and corresponding vector angle are estimated from the results of Figure 4. The angle between the vector direction and the horizontal axis X is denoted α_i for sensor « i ». Acceleration vectors are given in Figure 5 for all sensors.

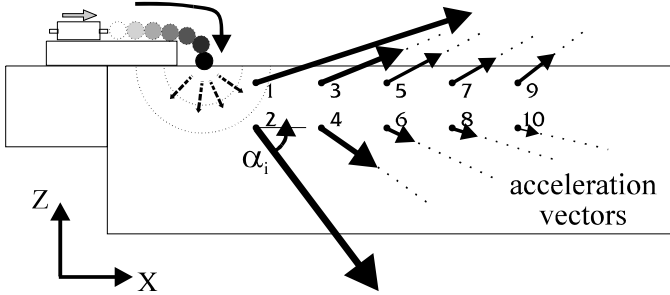


Figure 5 : Acceleration vectors in the XZ plane : pressure waves at mid-depth and different wave types for near-surface sensors.

Incident angles β_i give the direction of the line between the point of impact and the location of sensor i . The comparison between angles α_i and incident angles β_i shows an obvious coincidence between the incidence directions and acceleration vectors directions for mid-depth sensors ($\alpha_i = \beta_i$ for $i=2, 4, 6, 8$ and 10). For drop-ball experiments, the acceleration waves generated at mid-depth are clearly identified as pressure waves. This is a very important conclusion for the study of attenuation phenomena.

2.5 Some preliminary results

Wave velocity

Estimation of wave velocity is firstly made directly from peak delay measurements on time signals or using different signal processing methods [13] (crosscorrelation functions, time phase). A more precise evaluation of wave velocities (phase and group velocities) is subsequently proposed using time-frequency analysis of acceleration signals.

Values reached for explosive earthquake simulations lead to a mean wave velocity : $V_{\text{mean}} = 500 \text{ m.s}^{-1}$ [13]. For drop-ball experiments, values of wave velocities corresponding to different pairs of sensors are given in Table II (test n°13). The mean velocity is estimated to be : $V_{\text{mean}} = 423 \text{ m.s}^{-1}$. Note that there is no significant difference between

surface and mid-depth velocities (due to initial homogeneity of the medium). Wave velocities are thus nearly constant in the whole centrifuged model. For drop-ball experiments, they are slightly lower than those measured during explosive earthquake simulations.

wave velocities				
surface	1/3	3/5	5/7	7/9
	433 m/s	400 m/s	426 m/s	448 m/s
mid-depth	2/4	4/6	6/8	8/10
	409 m/s	421 m/s	429 m/s	419 m/s

Table II : Wave velocities for drop-ball test n°13

Simple numerical results

For drop-ball experiments, the generated wave field is assumed to be spherical. The numerical model is then chosen axisymetrical. Several numerical results (linear elasticity) are presented in Figure 6. They give interesting qualitative features of wave propagation phenomena in the centrifuged model : wave type, wave reflection on medium boundaries... These numerical computations are performed using civil engineering dedicated finite element system CESAR-LCPC. Numerical results given in Figure 6 correspond to four different time values :

- *time T_1* : wave propagation begins, the different wave types cannot be distinguished ;
- *time T_2* : pressure waves (the fastest ones) are well distinguished from shear waves and surface waves ;
- *time T_3* : reflection of pressure waves at the bottom of the container ;
- *time T_4* : direct pressure and shear waves keep travelling whereas the reflected P wave is propagated to the free surface.

From experimental results given in Figure 4 and 5, mid-depth sensors detect only pure pressure waves. As shown by experimental and numerical results (Figures 4, 5 and 6), different wave types appear near the free surface : shear waves, surface waves. In the following sections (§5 et 6), analytical models are proposed to study and precisely quantify attenuation and dispersive phenomena.

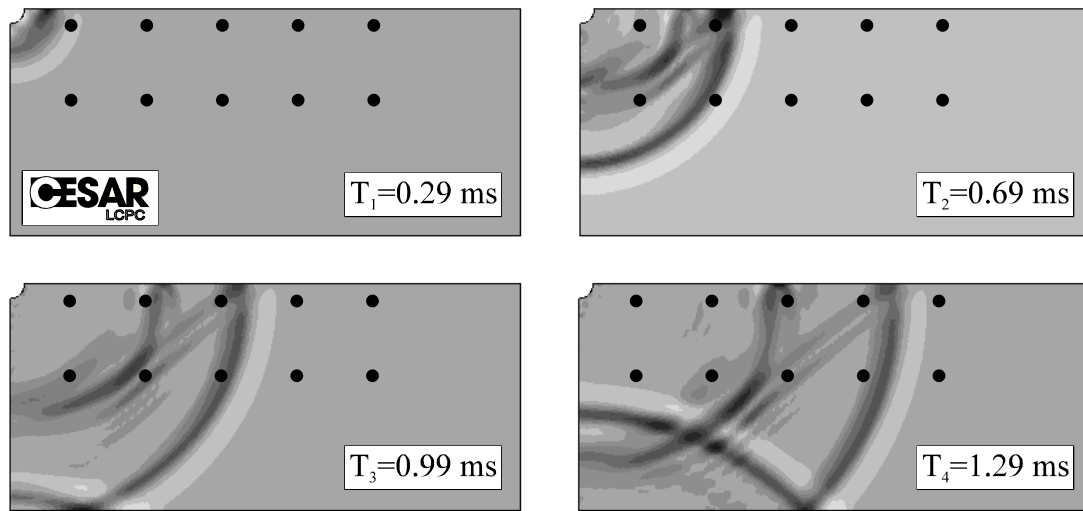


Figure 6 : Numerical modelling of spherical wave propagation (CESAR-LCPC).

2.6 Scale factors for dynamic problems

Centrifuge experiments are a realistic way of testing materials and structures, as stresses are identical in the centrifuge scaled model despite its dimensions. This is made possible thanks to artificial gravity forces allowing the scaling relations to be fulfilled. These scaling relations between full-scale structure and scaled model are determined considering physical relations characterising the problem (balance equations, constitutive relations...).

mechanical parameter	scale factor	numerical value
length	l^*	1/100
time	t^*	1/100
velocity	v^*	1
frequency	f^*	100
acceleration	a^*	100
viscosity	h^*	1/100
damping	a^*	100

Table III : Scaling relations for dynamic problems

In the experiments performed, sand is centrifuged under a 100g gravity (where g is 9.81 m.s^{-2}) and scaling relations for length, time, velocity are given in Table III. For dynamic testing, it is necessary to meet scaling relations considering transient excitation and propagation phenomena. Scaling

relations have to involve acceleration magnitude, frequency and also viscosity and damping for attenuation phenomena. These quantities could be usually evaluated by conventional resonant tests.

3. REFLECTED WAVES REMOVAL BY HOMOMORPHIC FILTERING

3.1 Wave reflections in drop-ball experiments

One of the main advantages of drop-ball experiments is the generation of short duration excitation. The detection and control of wave reflections are thereby made easier. As can be seen from Figure 7, for drop-ball experiments, direct and reflected waves appear in the acceleration signals.

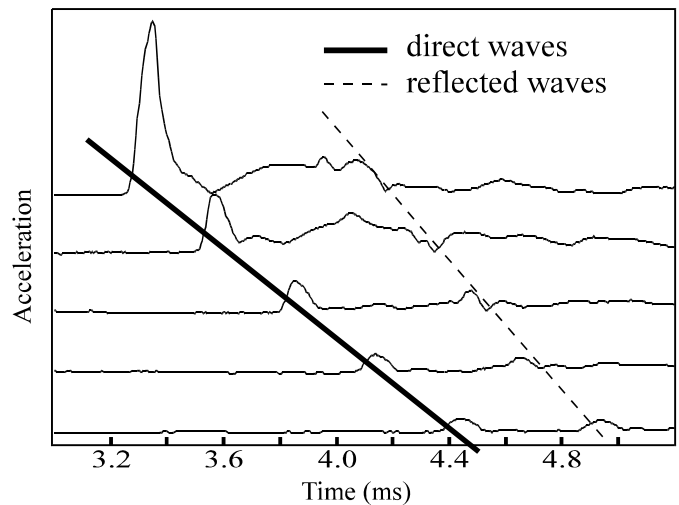


Figure 7 : Acceleration measurements (magnitude) for mid-depth sensors showing direct and reflected waves (drop-ball test).

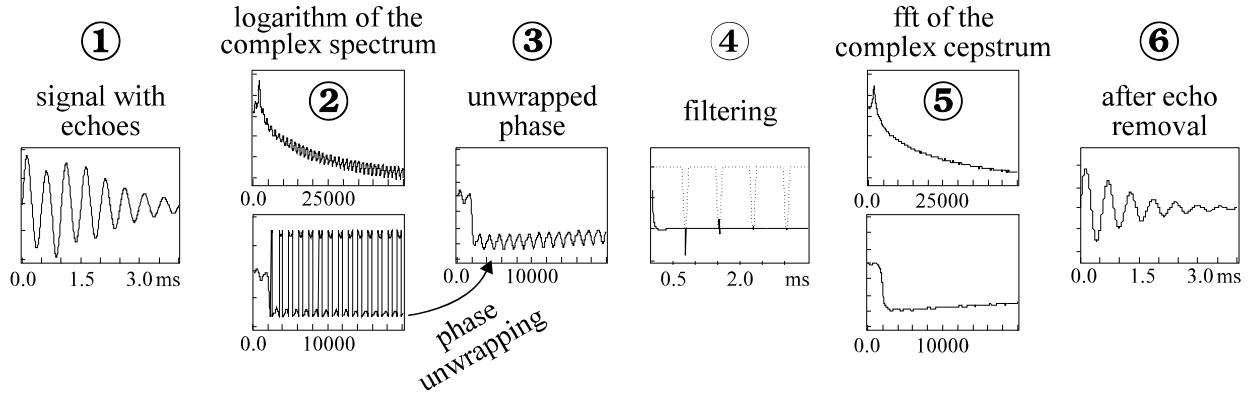


Figure 8 : *Complex cepstrum calculation procedure on a synthetic signal*

The detection of reflected waves is thus quite easy. It is then possible to estimate the delays between the direct and reflected waves (at the bottom of the container). Estimated experimental delays and calculated theoretical delays (assuming straight rays) are in agreement for all sensors [13]. The theoretical assumption considering straight rays is consequently acceptable. Furthermore, it would be interesting to remove reflected waves from the measured acceleration signal. For drop-ball experiments, it is not very difficult thanks to the short duration of the excitation. In a more general case, the method proposed in the following section (homomorphic filtering) works for every kind of excitation, even if it is of long duration.

3.2 Homomorphic filtering

3.2.1 Main principles

Homomorphic signal processing is based on a generalised linear systems theory [18, 19]. Homomorphic processing allows source excitation determination avoiding problems of echoes or wave reflection. This technique was used by Ulrych [19] to study attenuation and elastic wave dispersion by separating source terms from propagation terms.

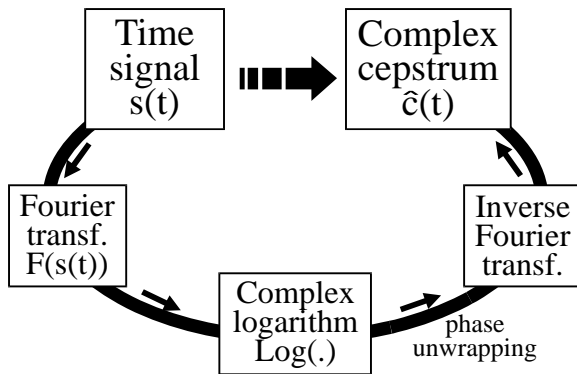


Figure 9 : *Main stages of homomorphic filtering.*

3.2.2 Complex cepstrum calculation

Complex cepstrum $\hat{c}(\tau)$ is calculated from magnitude and phase spectra. Figure 9 shows the

main calculation stages. If F denotes the Fourier transform and Log the complex logarithm, the complex cepstrum $\hat{c}(\tau)$ of the signal $s(t)$ can be written :

$$\hat{c}(\tau) = F^{-1}[\text{Log}(F(s(t)))]$$

Hopefully the phase spectrum $\text{Arg}[S(f)]$ is generally wrapped and must be converted to an unwrapped form. J.Tribolet [17] proposed a special unwrapping algorithm based on the phase derivative calculation. Its theoretical expression is the following :

$$\frac{d(\text{Arg}[S(f)])}{df} = \frac{S_R(f) \cdot S_I'(f) - S_I(f) \cdot S_R'(f)}{|S(f)|^2}$$

where $S(f)$ is the complex spectrum of $s(t)$. From this expression, it is possible to determine the complex cepstrum $\hat{c}(\tau)$. The different stages of the procedure are presented in Figure 8 for a synthetic signal. Phase unwrapping (stage 3) precedes the real filtering stage.

The complex cepstrum shows peaks characterizing echoes or reflected wave delays. To remove reflected waves, peaks in the complex cepstrum are suppressed by a weighting window (stage 4 of Figure 8). The « inverse cepstral transform » will then yield the original signal without any reflected waves [13, 18, 19].

3.2.3 Application to drop-ball experiments

Figure 10 gives an acceleration signal on which waves reflected at the bottom of the container are easy to detect (thin line). The same signal after homomorphic filtering is entirely free of any reflection (thick line).

This deconvolution technique is very useful for acceleration signals in a centrifuge. This method is also efficient for long-duration signals (see Figure 8) : even if direct and reflected waves are

thoroughly mixed, homomorphic filtering can separate both (or more) wave trains. It is a good alternative to isolation techniques (see J.A.Cheney et al. [4]).



Figure 10 : *Reflected wave removal by homomorphic filtering (drop-ball arrangement).*

4. ANALYSIS OF DISPERSION

Once reflection phenomena are detected and well controlled (for drop-ball experiments), a complete analysis of propagation calls for the study of wave dispersion in the medium. Signal processing by time-frequency analysis is proposed for that purpose.

4.1 Time-frequency analysis

4.1.1 Purpose

The decomposition of the acceleration signals in the time-frequency domain gives dispersive characteristics of propagation [6, 7, 8, 16]. This analysis can be performed using different methods : filtering, wavelet transform... In the different frequency bands, dispersion laws (phase and group velocities) of the centrifuged medium are then determined.

4.1.2 Wigner-Ville distribution

The Wigner-Ville distribution gives an overall theoretical approach to the time-frequency analysis of signal [7]. The main advantage of this method is that the choice of the weighting functions in the time and frequency domains is free. Even for strongly dispersive media, the Wigner-Ville distribution allows the determination of group velocity. Sessarego et al. used this method to study the dispersion of Lamb waves in a duraluminum plate [16].

4.1.3 Wavelet transform

The Wigner-Ville distribution includes many other types of transforms : shifted Fourier transform, wavelet transform, etc... The wavelet transform is built using a family of elementary functions $\psi_{ab}(t)$ defined from an analysing wavelet $\psi(t)$ [7, 16]. This family is expressed under the following form :

$$\psi_{ab}(t) = \frac{1}{\sqrt{a}} \cdot \psi\left(\frac{t-b}{a}\right), \quad b \in \mathbb{R}, a > 0$$

Wavelet coefficients of the signal s allow the characterization of s in both time and frequency domains. They are written in the following form :

$$C_s(a, b) = \langle s, \psi_{ab} \rangle = \int_{-\infty}^{+\infty} s(t) \cdot \overline{\psi_{ab}}(t) \cdot dt$$

Changing parameters a and b , the analysing wavelet $\psi(t)$ is simultaneously translated (parameter b) and expanded (or contracted, parameter a). The signal may then be analysed in the time-frequency domain after decomposition on the generated wavelet family.

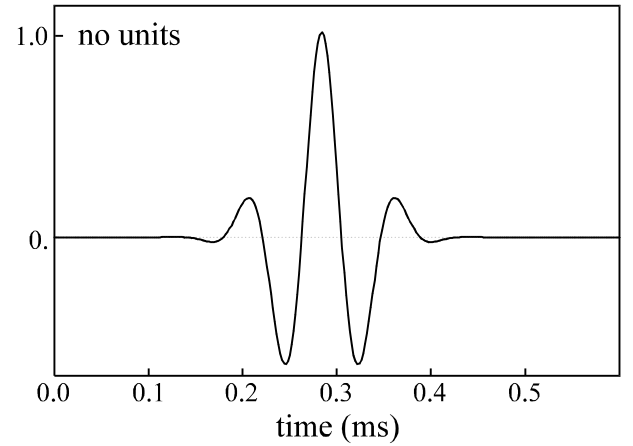


Figure 11 : *Analysing wavelet used for the time-frequency analysis of acceleration signals.*

An example of the analysing wavelet used in this study is given in Figure 11. One hundred wavelets are built from this analysing wavelet to determine $C_s(a, b)$ coefficients. Dispersive phenomena are then studied between 500 to 9500 Hz (see also [13]).

4.2 Analysis of dispersion

Acceleration signals are analyzed in different frequency bands (see some examples in Figure 12), and phase delays are estimated from these analyzed signals. The determination of group delays is made from the envelope curves of the signals [6, 8]. From phase and group delays, phase and group velocities are easily obtained [6, 8, 13].

The 3D-diagram presented in Figure 13 gives envelope curves of acceleration signals filtered in

different frequency bands. From this diagram, it is possible to determine group delays as a function of frequency. The thick line gives group delays for different frequencies.

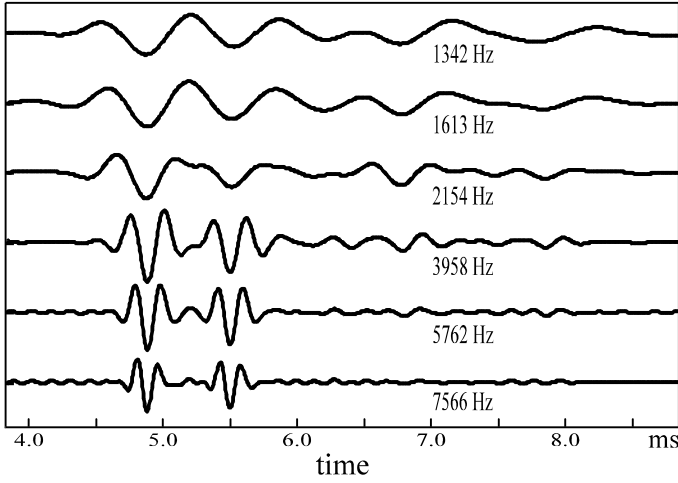


Figure 12 : Filtered signals for different frequency bands (drop-ball).

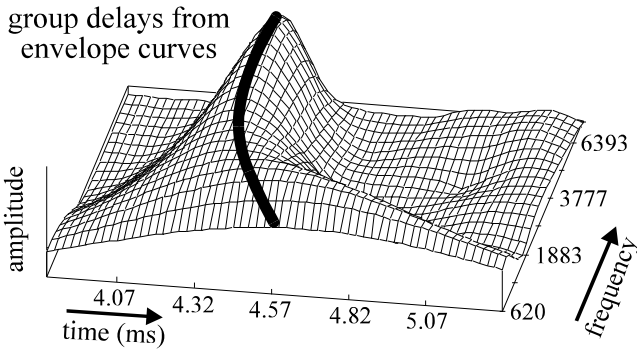


Figure 13 : 3D-diagram giving envelope curves for different frequency bands (drop-ball experiments).

For drop-ball experiments, wave reflection phenomena are easy to detect. As can be seen in Figure 12, direct and reflected waves are both present in filtered signals for medium and high frequencies, but for frequencies under 1500Hz, there is just one wave train. Actually, for low frequencies, the wavelengths are higher than the container dimensions and there is a strong geometrical dispersive effect. This can be seen in Figure 13 : group delays increase sharply below 1500 Hz :

- for low frequencies, there is a strong geometrical dispersive effect (waveguide effect). Group and phase velocities are largely underestimated (group and phase delays being overestimated) ;
- for medium and high frequencies, there is a material dispersive effect (weaker than the previous geometrical one). Reflected waves are not mixed with direct waves in the filtered

signals (see Figure 12) : phase and group velocities are correctly estimated.

frequency (in Hz)	drop-ball test n° 12 (velocities in m.s ⁻¹)		drop-ball test n° 13 (velocities in m.s ⁻¹)	
	V _Φ	V _g	V _Φ	V _g
440	350.8	266.5	353.2	267.4
975	389.7	433.4	368.3	453.8
1710	429.4	413.2	431.0	396.8
3175	427.3	424.3	422.9	420.1
5320	416.9	405.1	418.4	411.7
9280	415.4	416.5	418.4	426.9

Table IV : Experimental values of phase and group velocities.

Table IV gives experimental values of phase and group velocities for two drop-ball experiments and in six different frequency bands. These experimental values are compared further with theoretical phase and group velocities.

5. ATTENUATION

In this section, attenuation phenomena are investigated and related to the analysis of dispersion presented above with a viscoelastic formulation.

5.1 Geometrical attenuation

For drop-ball experiments, the generated wave field is spherical. There must be a strong geometrical contribution to the attenuation process which is generally considered (for an infinite medium) to be inversely proportional to the distance from the source of excitation. The acceleration wave magnitude at point M_i can then be written as : $A_i = A_0/r_i$. The comparison of acceleration magnitudes at two points M_i and M_j leads to the following expression :

$$A_j = A_i \cdot \left(\frac{r_i}{r_j} \right)$$

5.2 Material attenuation

Centrifuge experiments performed in the 80's by Luong suggested a physical (or material) attenuation of the form $a(x) = a_0 \cdot \exp(-\alpha \cdot x)$, where the damping coefficient α is assumed to be independent of frequency and equal to 1,1 m⁻¹. For drop-ball experiments, combining the geometrical and material attenuations, the comparison of acceleration

magnitudes for two different points is of the following form :

$$A_j = A_i \cdot \exp[-\alpha \cdot (r_j - r_i)] \cdot \frac{r_i}{r_j} \quad (1)$$

This expression allows the estimation of the acceleration signal at a point M_j from a measured signal at point M_i . The ratio r_i/r_j takes into account the geometrical attenuation (wave front expansion), whereas $\exp[-\alpha \cdot (r_j - r_i)]$ gives the material attenuation between M_i and M_j . For drop-ball experiments, this expression does not correctly describe the attenuation phenomena [13].

5.3 Estimation from experimental results

From the previous description of attenuation, the amplitude reduction factor, estimated from experimental results, can be compared with the geometrical attenuation. The amplitude reduction factor is drawn from acceleration magnitudes in the XZ plane, and geometrical attenuation (for infinite medium and spherical wave field) is determined by the distance ratio r_i/r_j (where i and j are the sensors numbers). From the amplitude reduction factor (related to total attenuation), it is then possible to compare geometrical and material attenuation. Table V gives values for the three amplitude reduction factors : the experimental (total), the

geometrical and the material one. Material attenuation is the ratio of total and geometrical attenuations (corresponding to the exponential term in expression (1)). Near the source, acceleration amplitude is the highest for mid-depth sensors whereas, further away from the source, surface amplitude is slightly higher.

Experimental and geometrical amplitude reduction factors are estimated from maximum acceleration values and sensors distances (see Table V). Experimental amplitude reduction factors values are fairly similar for surface and mid-depth sensors. However, geometrical attenuation is higher for surface sensors near the source (low reduction factor) and the related material attenuation is thus lower for surface sensors. Nevertheless, it should be noted that expression (1) is best adapted to mid-depth accelerations (and not really to surface acceleration waves) and non linear effects are possibly involved near the source.

For mid-depth sensors, the geometrical amplitude reduction factor increases with distance (geometrical attenuation accordingly decreases). From Table V, it appears that there is no material attenuation at all for the last two mid-depth sensors.

sensor number	sensor distance	acceleration magnitude	total amplitude reduction	geometrical attenuation	material attenuation
1	$r_1=0.11$ m	$a_1=757.11$ m.s ⁻²	$a_3/a_1=0.326$	$r_1/r_3=0.458$	0.712
3	$r_3=0.24$ m	$a_3=246.76$ m.s ⁻²	$a_5/a_3=0.556$	$r_3/r_5=0.649$	0.857
5	$r_5=0.37$ m	$a_5=137.15$ m.s ⁻²	$a_7/a_5=0.810$	$r_5/r_7=0.740$	1.09
7	$r_7=0.50$ m	$a_7=111.13$ m.s ⁻²	$a_9/a_7=0.767$	$r_7/r_9=0.794$	0.966
9	$r_9=0.63$ m	$a_9=85.24$ m.s ⁻²			
2	$r_2=0.19$ m	$a_2=877.96$ m.s ⁻²	$a_4/a_2=0.349$	$r_2/r_4=0.679$	0.514
4	$r_4=0.28$ m	$a_4=306.45$ m.s ⁻²	$a_6/a_4=0.544$	$r_4/r_6=0.700$	0.777
6	$r_6=0.40$ m	$a_6=166.62$ m.s ⁻²	$a_8/a_6=0.601$	$r_6/r_8=0.769$	0.782
8	$r_8=0.52$ m	$a_8=100.08$ m.s ⁻²	$a_{10}/a_8=0.744$	$r_8/r_{10}=0.800$	0.930
10	$r_{10}=0.65$ m	$a_{10}=74.50$ m.s ⁻²			

Table V : Comparison of amplitude reduction factors (total, geometrical, material).

6. SIMULATIONS WITH RHEOLOGICAL MODELS

6.1 Simplified solution for drop-ball tests

6.1.1 Equation of motion

The analysis of three-dimensional acceleration measurements shows that, for mid-depth sensors, acceleration waves are pressure waves (see Figure 5). We assume that the wave field is spherical and keep only the radial component of the field variables. The equation of motion, consequently, takes the following form :

$$\text{div}(\sigma(r,t)) = \frac{\partial \sigma_{rr}(r,t)}{\partial r} + \frac{2}{r} \cdot \sigma_{rr}(r,t) = \rho \cdot \frac{\partial^2 u_r(r,t)}{\partial t^2} \quad (2)$$

This differential equation can be rewritten in a simpler form using Fourier transforms of both radial stress and displacement (σ_{rr} and u_r). The Fourier transforms (or complex spectra) of $\sigma_{rr}(r,t)$ and $u_r(r,t)$ are denoted $\sigma_{rr}^*(r,\omega)$ and $u_r^*(r,\omega)$.

6.1.2 Viscoelastic models

To solve equation (2), a constitutive model for the medium is needed. Linear viscoelasticity gives a simple relation between stress and strain in the frequency domain. It is not realistic near the source but further away from the source it could be a good approximation of damping phenomena. The three viscoelastic models presented in Figure 14 lead to the following relation :

$$\sigma_{rr}^*(r,\omega) = E^*(\omega) \cdot \varepsilon_{rr}^*(r,\omega) \quad (3)$$

where $E^*(\omega)$ is the complex modulus. The different forms taken by this complex modulus are given in Figure (14). The corresponding variations of attenuation versus frequency are shown in Figure (15).

The equation of motion (2) combined with the constitutive equation (3) admits the following solution, in terms of radial displacement :

$$u^*(r,\omega) = \frac{U_0(\omega)}{r} \cdot \exp(i \cdot \xi(\omega) \cdot r) \quad (4)$$

where ξ is the complex function of ω defined by :

$$\xi^2(\omega) = \frac{\rho \cdot \omega^2}{E^*(\omega)} \quad (5)$$

in which $\xi(\omega)$ is the complex wave number. Its real part corresponds to the phase shift and its imaginary part to the damping coefficient [13]. In this expression, $U_0(\omega)$ is the source term independent of r .

Attenuation is expressed through two different parameters :

- geometrical attenuation due to spherical wave field propagation and inversely proportional to r (distance from the source) but independent of frequency ;
- viscous damping given by the imaginary part of $\xi(\omega)$ and frequency-dependent.

Expression (4) completely characterises the spherical (pressure) wave field in linear viscoelastic medium. It is then possible to simulate the propagation of these pressure waves for mid-depth sensors.

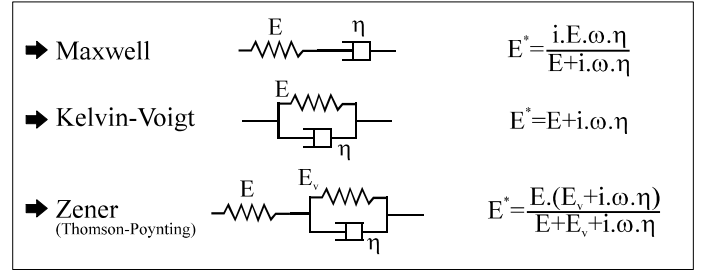


Figure 14 : Viscoelastic models used for simulations and the corresponding complex modulus.

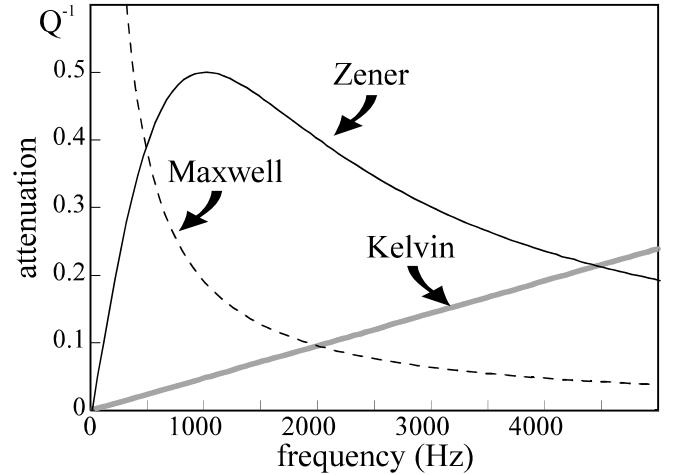


Figure 15 : Attenuation versus frequency for different viscoelastic models.

6.2 Complete solution (P-wave)

6.2.1 Stress and strain tensors

Previous assumption reduces strain tensor to only the first diagonal term ε_{rr} . Considering a purely radial displacement (as is verified experimentally), the complete strain tensor can be written :

$$\varepsilon = \begin{bmatrix} \frac{\partial u_r}{\partial r} & \frac{1}{2r} \frac{\partial u_r}{\partial \theta} & \frac{1}{2r \cdot \sin \theta} \frac{\partial u_r}{\partial \theta} \\ \frac{1}{2r} \frac{\partial u_r}{\partial \theta} & \frac{u_r}{r} & 0 \\ \frac{1}{2r \cdot \sin \theta} \frac{\partial u_r}{\partial \theta} & 0 & \frac{u_r}{r} \end{bmatrix}$$

Stress tensor is then determined assuming linear viscoelasticity :

$$\sigma_{rr} = (\lambda^* + 2\mu^*) \frac{\partial u_r}{\partial r} + \lambda^* \frac{2u_r}{r}$$

$$\sigma_{\theta\theta} = \lambda^* \left(\frac{\partial u_r}{\partial r} + \frac{2u_r}{r} \right) + 2\mu^* \frac{u_r}{r}$$

$$\sigma_{\varphi\varphi} = \lambda^* \left(\frac{\partial u_r}{\partial r} + \frac{2u_r}{r} \right) + 2\mu^* \frac{u_r}{r}$$

$$\sigma_{r\theta} = \frac{\mu^*}{r} \frac{\partial u_r}{\partial \theta} ; \quad \sigma_{r\varphi} = \frac{\mu^*}{r \cdot \sin \theta} \frac{\partial u_r}{\partial \varphi} \quad \text{and} \quad \sigma_{\theta\varphi} = 0$$

where λ^* and μ^* are complex Lamé constants.

6.2.2 Solution of the complete problem

No dependence in θ and φ (second and third spherical coordinates) is taken into account. Equation of motion leads to the following differential equation :

$$\frac{\partial^2 u_r}{\partial r^2} + \frac{2}{r} \frac{\partial u_r}{\partial r} - \frac{2}{r^2} u_r = \frac{\rho}{\lambda^* + 2\mu^*} \frac{\partial^2 u_r}{\partial t^2} \quad (6)$$

The solution of the differential equation (6) is then :

$$U^*(r, \omega) = \frac{U_0^*(\omega)}{r} \cdot \left(\xi(\omega) + \frac{i}{r} \right) \cdot \exp(i \cdot \xi(\omega) \cdot r) \quad (7)$$

It is slightly different from the simplified solution (4). It involves a term proportional to $1/r^2$ decreasing rapidly with increasing distance r . Differences between these two solutions are discussed in the next paragraph.

6.3 Simulations for drop-ball experiments

6.3.1 Other assumptions

Transient wave calculation

The measurements performed in centrifuge experiments concern acceleration parameters. Expression (4) is also verified by the Fourier transform of acceleration signals $a^*(r, \omega)$. This expression is verified for each frequency component. Using simplified solution (4)), acceleration $a(r, t)$ in the time domain is obtained by inverse Fourier transformation under the following form :

$$a(r, t) = \frac{1}{r} \cdot \int_{-\infty}^{+\infty} A_0(\omega) \cdot \exp(i \cdot \xi(\omega) \cdot r) \cdot \exp(i \cdot \omega \cdot t) \cdot d\omega$$

Behaviour parameters

For Maxwell and Kelvin-Voigt models, the behaviour parameters are Lamé constants λ and μ and viscosity coefficient η . λ and μ can be easily calculated from pressure waves velocity C using the relation :

$$C = \sqrt{\frac{\lambda + 2\mu}{\rho}} \quad \text{thus} \quad \lambda + 2\mu = \rho \cdot C^2$$

where ρ is the density of the sand ($\rho = 1650 \text{ kg/m}^3$) and $C = 423 \text{ m.s}^{-1}$ for drop-ball experiments (see §2.5).

6.3.2 Simulated acceleration

Simplified solution

Simulations were made for mid-depth sensors to take into account only the pressure waves. Considering the acceleration spectra for two sensors, i and j , the ratio of these two spectra can easily be written in frequency domain from expression (4). Denoting $a_m^*(r, \omega)$ as the Fourier transform of acceleration, this ratio takes the following form :

$$\frac{a_m^*(r_j, \omega)}{a_m^*(r_i, \omega)} = \frac{r_i}{r_j} \cdot \exp[i \cdot \xi(\omega) \cdot (r_j - r_i)] \quad (8)$$

If acceleration is known at distance r_i from the source, acceleration at distance r_j can be determined from viscoelastic simulations. From expression (8), the simulated acceleration $a_m^{sim}(r_j, t)$ can be expressed, after inverse Fourier transformation, as :

$$a_m^{sim}(r_j, t) = \int_{-\infty}^{+\infty} a_m^*(r_i, \omega) \cdot \frac{r_i}{r_j} \cdot \exp[i \cdot \xi(\omega) \cdot (r_j - r_i)] \cdot \exp(i \cdot \omega \cdot t) \cdot d\omega \quad (9)$$

Simulations for the complete solution

The complete solution defined by equation (7) gives the relation between the measured acceleration at distance r_i and the simulated acceleration at distance r_j . This solution can be written as follows :

$$\frac{a_m^*(r_j, \omega)}{a_m^*(r_i, \omega)} = \frac{r_i^2}{r_j^2} \cdot \frac{r_j \cdot \xi(\omega) + i}{r_i \cdot \xi(\omega) + i} \cdot \exp[i \cdot \xi(\omega) \cdot (r_j - r_i)]$$

This expression involves a term proportional to the ratio between squared distance. This term must decrease very quickly with increasing distance. It appears clearly in Figure 16 : near the source, the simplified solution and the complete solution lead to very different simulations. For an increasing source distance, the difference between both solutions becomes small (see Figure 16).

From the mechanical parameters determined, the difference between simplified and complete solutions (equations (4) and (7)) becomes small when the distance to the source is greater than 0.05m. As is the case for all accelerometers, the simplified solution is chosen.

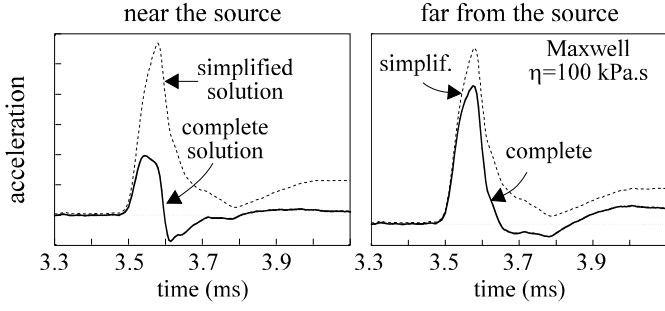


Figure 16 : viscoelastic modelling : comparison between simplified and complete solutions.

6.3.3 Simulations from experimental results

For the Maxwell and Kelvin-Voigt models, values of η have to be determined to agree with experimental results. This agreement is evaluated from the first acceleration peak. The best η values empirically determined by this procedure are the following :

- $\eta=150,000$ Pa.s for Maxwell's model
- $\eta=1000$ Pa.s for Kelvin's model

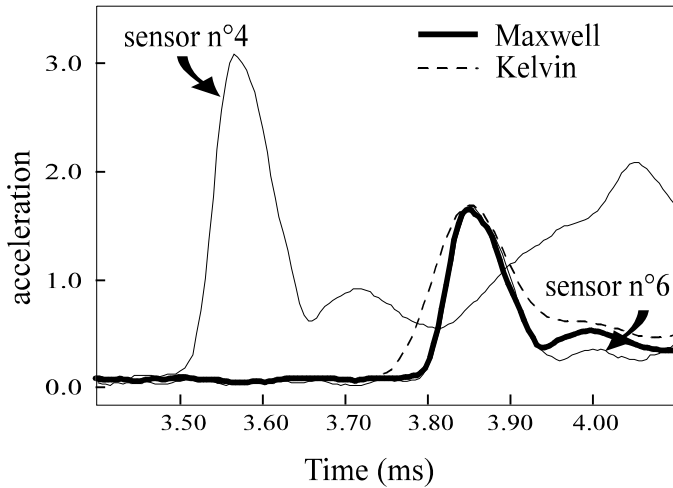


Figure 17 : Calculated acceleration signals for different viscoelastic simulations : magnitudes in the XZ plane (drop-ball experiments).

Acceleration signals obtained from these values of viscosity coefficient are presented on Figure 17. The acceleration given by sensor no.4 is the reference signal. Calculated accelerations at distance $r=r_6-r_4$ from reference sensor are compared with that given by sensor no.6. Judging by the curves of Figure 17, the Maxwell model seems to give simulated acceleration signals very close to the measured signals (for the chosen η value). The results are very poor for the Kelvin model.

6.3.4 Optimisation procedure (Zener)

To determine the best behaviour parameters (E_v and η) for the Zener model (see Figure 14), it is

necessary to quantify the agreement between measured and simulated accelerations. It is, for instance, possible to estimate the squared error between measured signals and calculated signals. Considering the time interval $[t_1; t_2]$ around first acceleration peak, the functional $J(E_v, \eta)$, defining the squared error between measured acceleration $a_m(r_j)$ and calculated acceleration $a_m^{sim}(r_j)$ can be written :

$$J(E_v, \eta) = \frac{1}{J_0} \cdot \int_{t_1}^{t_2} (a_m(r_j, t) - a_m^{sim}(r_j, t))^2 \cdot dt$$

A minimization of $J(E_v, \eta)$ gives the optimum parameters ($E_{v,opt}, \eta_{opt}$). The 3D curve of Figure 18 gives values of the functional $J(E_v, \eta)$ for $E_v \in [0; 50$ MPa] and $\eta \in [130; 160$ kPa.s] (for sensors 4 and 6).

The minimum error J_{min} is obtained for $E_v=25.5$ MPa and $\eta=146$ kPa.s. The optimum "viscous" modulus $E_{v,opt}$ is very low, and the optimum viscosity coefficient η_{opt} is very close to the viscosity value chosen for the Maxwell model. With these two optimum behaviour parameters, the simulations with the Zener and Maxwell models are very close. Both models have then to be compared from a quantitative point of view. Calculating the error $J(\eta)$ for the Maxwell model, the minimum value corresponds to an optimum viscosity $\eta_{opt}=144$ kPa.s. This value differs little from that determined empirically earlier ($\eta=150$ kPa.s). The minimum value of $J(\eta)$ for the Maxwell model is $J_{min}^{Max}=7.02 \cdot 10^{-3}$. For the Zener model, it is a little smaller $J_{min}^{Zen}=6.80 \cdot 10^{-3}$. The fit of the Zener model is slightly better than Maxwell's.

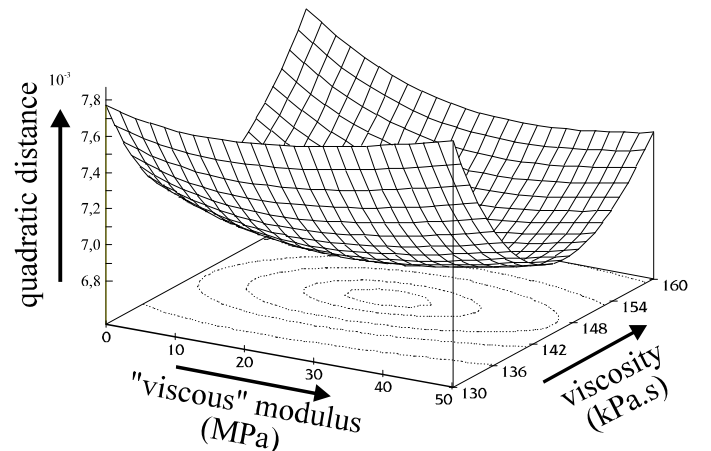


Figure 18 : Determination of optimal behaviour parameters from error minimization (Zener model).

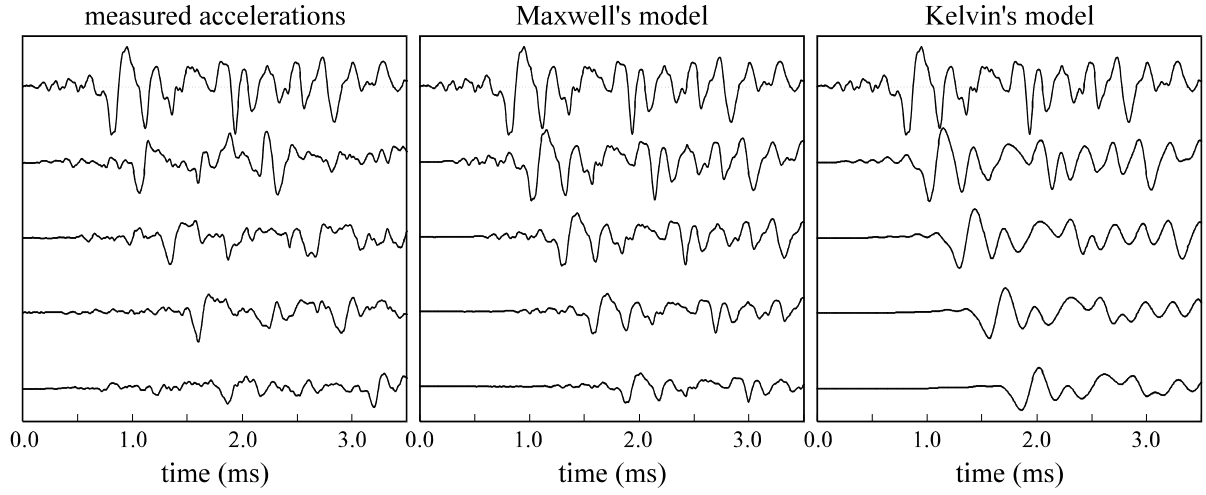


Figure 19 : Calculated acceleration signals for different viscoelastic simulations (explosive earthquake simulations).

6.4 Simulations for earthquake simulations

Using values of optimal viscosity estimated from drop-ball experiments ($\eta=150$ kPa.s for Maxwell's model, $\eta=1000$ Pa.s for Kelvin's model), we performed viscoelastic wave propagation simulations for explosive earthquake experiments. Figure 19 gives time acceleration curves for experimental measurements, Maxwell simulations and Kelvin simulations. For the left, center and right groups of curves, the first always corresponds to the same experimental acceleration measurement (sensor 2). From these curves, one can notice that damping phenomena are well estimated concerning acceleration amplitude. The form of the propagated waves is however more realistic for Maxwell's model, because there is a strong attenuation of high frequency components with Kelvin's model.

Since there is a strong soil-container interaction for explosive earthquake simulations, it is nevertheless difficult to compare directly experimental and numerical results. It is not really possible to compare the form and the amplitude of every acceleration peak because of the duration of the wave train (which implies wave reflections on medium boundaries) and the possible wave travel in the container itself.

6.5 Phase and group velocities

If $\xi(\omega)$ is the complex wave number, the classical expressions of phase and group velocities become complex. The real parts of these expressions correspond to phase shift terms and characterise the phase velocity V_ϕ and group velocity V_g . Their expressions then take the following form :

$$V_\phi = \text{Re} \left[\frac{\omega}{\xi(\omega)} \right] \quad \text{and} \quad V_g = \text{Re} \left[\frac{d\omega}{d\xi(\omega)} \right]$$

where Re denotes the real part of a complex number.

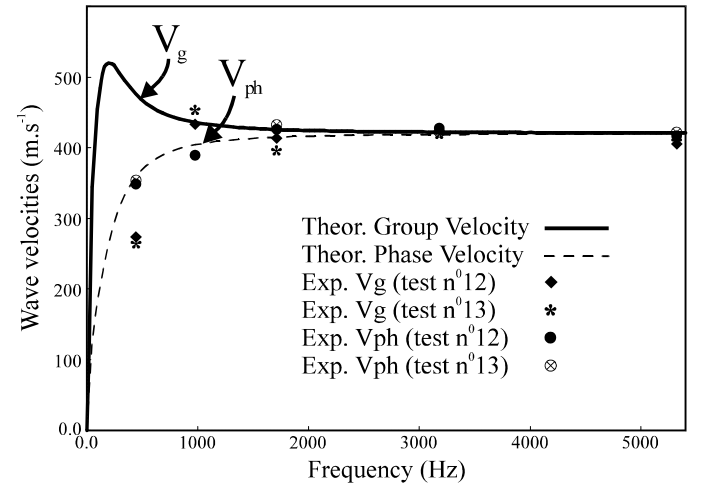


Figure 20 : Comparison between theoretical phase and group velocities (from Maxwell viscoelastic simulations) and experimental ones (estimated from filtered signals).

The determination of viscoelastic behaviour parameters allows the calculation of phase and group velocities for mid-depth sensors. The curves, given in Figure 20, indicate the values of V_ϕ and V_g for a Maxwell viscoelastic simulation. In Figure 20, these curves are compared with experimentally estimated values already given in Table IV. It allows a comparison in the frequency range [0 Hz; 5500 Hz]. Above this range, experimental and theoretical values coincide perfectly.

Between 0 and 5500 Hz, the comparison between experimental and theoretical values leads to the following conclusions :

- above 1500 Hz : phase and group velocities are almost identical; there is a good agreement between experimental and theoretical values ;
- at 975 Hz : group velocity is higher than phase velocity for theoretical values as well as for experimental values; the agreement between both results is not too bad ;
- at 440 Hz : theoretical group velocity is higher than theoretical phase velocity; experimental phase velocities are not far from theoretical ones, but for group velocities there is a large discrepancy. Actually, wavelengths for low frequencies ($f < 1000\text{Hz}$) are large and group velocity is greatly underestimated.

As indicated previously, there is a strong wave-guide effect at low frequencies. The underestimation of phase velocity is not significant, but for group velocity there is a strong influence of wave reflection (wave-guide effect). For low frequencies, geometrical dispersion has to be taken into account. This is the main drawback of dynamic experiments in a finite medium.

7. SIMULATIONS AT DIFFERENT SOURCE DISTANCES

Previous simulations were all made for the same pair of sensors (no.4 and no.6). It is necessary to make other simulations for different pairs of sensors to study the influence of sensor location and acceleration level. Determining the optimum parameters, as presented before, allows a comparison between all results.

For the Maxwell model, four pairs of "excitation-response" (in-out) signals are taken from the measured accelerations of sensors 2/4; 4/6; 6/8 and 8/10. The error term $J(\eta)$ is determined in each case to estimate the optimal viscosity value η_{opt} . As shown in Table VI, the results are very different from one case to another.

For the Maxwell model, the attenuation Q^{-1} is related to behaviour parameters by the relation :

$$Q^{-1} = \frac{E}{\eta \cdot \omega}$$

Using a linear viscoelastic model, it is possible to quantify damping phenomena. However, the proposed analytical description cannot describe the attenuation process in the whole medium. Some particular phenomena could have a strong influence

on damping : amplitude dependence of attenuation, mean stress dependence. From the results given in Table VI, material attenuation seems to be very strong near the source of excitation (low viscosity, high acceleration magnitude) and much lower than geometrical attenuation for the last sensors (8 and 10), which reduces the acceleration amplitude by approximately 20 % (see Table V).

sensor		behaviour parameters	
in	out	viscosity	$J(\eta)$
2	4	39 kPa.s	$1,63 \cdot 10^{-2}$
4	6	144 kPa.s	$7,02 \cdot 10^{-3}$
6	8	154 kPa.s	$2,22 \cdot 10^{-2}$
8	10	$+\infty$ no material attenuation	

Table VI : *Optimal (Maxwell) viscosity values for simulations at different distances from the source.*

As attenuation could depend on acceleration amplitude [12] or mean stress, the need for a more complete approach is clear. However, the constitutive models presented in this paper give a complete analytical description of a spherical wave field propagation related to drop-ball experiments. The quantification of attenuation is therefore made very easy.

8. CONCLUSION

Wave propagation phenomena have been investigated for a medium of finite extent (centrifuge soil mass) by considering three-dimensional acceleration components, wave reflections at the boundaries (homomorphic filtering), time-frequency processing and dispersive phenomena, attenuation and energy dissipation (linear viscoelastic simulations).

An original experimental arrangement (drop-ball) is proposed. During these experiments, acceleration waves generated at mid-depth are shown to be pure pressure waves. Wave reflections at boundaries are detected and filtered using a special signal-processing method. Dispersive phenomena are investigated and are shown to take two different forms : strong geometrical dispersion at low frequencies and weaker material dispersion at higher frequencies. For mid-depth sensors, pressure wave

propagation is completely described by spherical wave propagation in a viscoelastic medium. It allows the estimation of corresponding constitutive parameters and gives a quantification of dispersive and damping phenomena. We found a good numerical fitting when using rheological models but a realistic justification must be based on physical deformation mechanisms.

REFERENCES

- [1] S.Abe, Y.Kobayashi, T.Ikawa, "Seismic characteristics of the weight-dropping source", *Jnl of Physics Earth*, v.38, pp.189-212, 1990.
- [2] K.Arulanandan, J.Canclini, A.Anandarajah, "Simulation of earthquake motions in the centrifuge", *Jnl of Geotech. Eng.*, A.S.C.E, vol.108, No.GT5, pp.730-742, May 1982.
- [3] T.G.Barker, J.L.Stevens, "Array processing of Rayleigh waves for shear structure", Proc.2nd Int. Conf. on Recent Adv. in Geotech. Earthquake Eng. and Soil Dyn., St-Louis, pp.1393-1397, 1991.
- [4] J.A.Cheney, R.K.Brown, N.R.Dhat, O.Y.Z.Hor, "Modeling free-field conditions in centrifuge models", *Jal of Geotech. Eng.*, v.116, No.9, pp.1347-1367, 1990.
- [5] C.J.Coe, J.H.Prevost, R.H.Scanlan, "Dynamic stress wave reflections/attenuation: earthquake simulation in centrifuge soil models", *Earthquake Eng. and Struct. Dynamics*, v.13, pp.109-128, 1985.
- [6] J.S.Farnbach, "The complex envelope in seismic signal analysis", *Bull. of the Seism. Soc. of America*, v.65, No.4, pp.951-962, 1975.
- [7] P.Flandrin, J.P.Sessarego, "Time-frequency methods in acoustics" (in french), Colloque de Physique C2, Tome 51, pp.707-716, 1990.
- [8] M.J.Lighthill, "Group velocity", *Jnl of the Inst. of Maths and its Applic.*, v.1, pp.1-28, 1965.
- [9] M.P.Luong, "Exp. methods in earthquake eng.", "Recent adv. in earthquake eng. & struc. dynamics", ed. V.Davidovici, Presses de l'E.N.P.C, Paris, 1992.
- [10] M.P.Luong, "Centrifuge tests - A dynamic approach", in "Soil Dyn. and Geotech. Earthquake Eng.", pp.19-48, Balkema, Rotterdam, 1993.
- [11] M.P.Luong, "Centrif. simulation of Rayleigh waves in soils using a drop-ball arrangt", *Dyn. Geotech. Test. II*, ASTM STP 1213, ed. R.J.Ebelhar, V.P.Drnevich & B.L.Kutter, Philadelphia, 1995.
- [12] G.M.Mavko, "Frictional attenuation : an inherent amplitude dependence", *Jnl of Geophysical Research*, v.84, pp.4769-4775, 1979.
- [13] J.F.Semblat, "Soils under dynamic and transient loadings: dynamic response on S.H.P.B, wave propagation in centrifuge medium" (in French),

Rapport Etudes et Recherches des L.P.C, Laboratoire Central des Ponts et Chaussées, Paris, 206 p., 1995.

- [14] J.F.Semblat, M.P.Luong, J.J.Thomas, "Drop-ball arrangement for centrifuge experiments", *5th SECED Conference on European Seismic Design Practice*, Chester, UK, 26-27 October 1995, pp.567-575, Balkema, Rotterdam.
- [15] J-F.Semblat, G.Gary, M.P.Luong, "Dynamic response of sand using 3D-Hopkinson Bar", Proc. of *IS-Tokyo'95, 1st International Conf. on Earthquake Geotechnical Engineering*, Tokyo, 14-16 November 1995, pp.157-163, Balkema, Rotterdam, 1995.
- [16] J.P.Sessarego, J.Sageloli, P.Degoul, P.Flandrin, M.Zakharia, "Time-frequency analysis of signals in dispersive media. Application to Lamb waves" (in french), *Jnl d'Acoustique*, v.3, pp.273-280, 1990.
- [17] J.M.Tribolet, "A new phase unwrapping algorithm", *IEEE Trans. on Acoustics, Speech and Signal Proc.*, v.25, No.2, pp.170-177, 1977.
- [18] J.P.Tubach, "Spectral analysis of speach" (in french), in "La parole et son traitement automatique", Masson ed., pp.265-301, Paris, 1989.
- [19] T.J.Ulrych, "Application of homomorphic deconvolution to seismology", *Geophysics*, v.36, No.4, pp.650-660, 1971.